

176	KM	RUNOFF FROM BAS 2130			
177	KO	0			
178	BA	.010			
179	LS	0		69	
180	UD	.218			
181	KK	DP2130			
182	KM	COMBINE FLOW FROM DP2120, E2130			
183	HC	2			
184	KK	E2140			
185	KM	RUNOFF FROM BAS 2140			
186	KO	0			
187	BA	.007			
188	LS	0		69	
189	UD	.16			
190	KK	E2150			
191	KM	RUNOFF FROM BAS 2150			
192	KO	0			
193	BA	.015			
194	LS	0		76.3	
195	UD	.259			
196	KK	E2160			
197	KM	RUNOFF FROM BAS 2160			
198	KO	0			
199	BA	.012			
200	LS	0		92	
201	UD	.162			
202	KK	DP2160			
203	HC	4			
204	KK	R5010			
205	KM	ROUTE FLOW FROM DP2160 TO DP5010			
206	RK	2900	.015	.04	TRAP 20 4
207	KK	E3000			
208	KM	RUNOFF FROM BAS 3000			
209	KO	0			
210	BA	.42			
211	LS	0		81	
212	UD	.414			

HEC-1 INPUT

1
PAGE 6

LINE	ID	1	2	3	4	5	6	7	8	9	10
213	KK	E3005									
214	KM	RUNOFF FROM BASIN 3005									
215	KO	0									
216	BA	.24									
217	LS	0		81							
218	UD	.27									
219	KK	DP3000									
220	KM	COMBINE FLOW FROM E3000 AND E3005									
221	HC	2									
222	KK	R3015									
223	KM	ROUTE FLOW FROM DP3000 TO DP3020									
224	RK	3200	.031	.04	TRAP	15	3				
225	KK	E3015									
226	KM	RUNOFF FROM BASIN 3015									
227	KO	0									
228	BA	.11									
229	LS	0		81							
230	UD	.21									

231	KK	DP3020							
232	KM		COMBINE FLOW FROM BASIN 3015 AND R3015						
233	HC		2						
234	KK	E3010							
235	KM		RUNOFF FROM BAS 3010						
236	KO		0						
237	BA		.22						
238	LS		0	82					
239	UD		.28						
240	KK	R3012							
241	KM		ROUTE FLOW FROM E3010 TO DP 3020						
242	RK		2600	.03	.04	TRAP	15	3	
243	KK	E3012							
244	KM		RUNOFF FROM BASIN E3012						
245	KO		0						
246	BA		.21						
247	LS		0	82					
248	UD		.47						
249	KK	E3020							
250	KM		RUNOFF FROM BAS 3020						
251	KO		0						
252	BA		.188						
253	LS		0	93					
254	UD		.36						

HEC-1 INPUT

LINE	ID1.....2.....3.....4.....5.....6.....7.....8.....9.....10							
255	KK	R3025							
256	KM		ROUTE FLOW FROM BASIN E3020 TO DP 3020						
257	RK		2600	.035	.04	TRAP	15	3	
258	KK	E3025							
259	KM		RUNOFF FROM BASIN E3025						
260	KO		0						
261	BA		.26						
262	LS		0	93					
263	UD		.23						
264	KK	DP3020							
265	KM		COMBINE FLOW FROM DP3020, E3012, E3025, R3012 AND R3025						
266	HC		5						
267	KK	R3030							
268	KM		ROUTE FLOW FROM DP3020 TO DP3030						
269	RK		3000	.05	.04	TRAP	20	10	
270	KK	E3030							
271	KM		RUNOFF FROM BAS 3030						
272	KO		0						
273	BA		.26						
274	LS		0	79					
275	UD		.34						
276	KK	E3035							
277	KM		RUNOFF FROM BASIN E3035						
278	KO		0						
279	BA		.16						
280	LS		0	79					
281	UD		.16						
282	KK	DP3030							
283	KM		COMBINE FLOW FROM R3030, E3030 AND E3035						
284	HC		3						

338	UD	.355								HEC-1 INPUT
LINE	ID1.....2.....3.....4.....5.....6.....7.....8.....9.....10								
339	KK	R3080								
340	KM	ROUTE FLOW FROM E3080 TO DP3090								
341	RK	1650 .01 .04 TRAP	30	10						
342	KK	E3090								
343	KM	RUNOFF FROM BAS 3090								
344	KO	0								
345	BA	.054								
346	LS	0 79								
347	UD	.411								
348	KK	DP3090								
349	KM	COMBINE FLOW FROM R3080, E3090								
350	HC	2								
351	KK	DP3091								
352	KM	COMBINE FLOW FROM DP3050, DP3090								
353	HC	2								
354	KK	R3100								
355	KM	ROUTE FLOW FROM DP3091 TO DP3100								
356	RK	350 .01 .03 TRAP	10	10						
357	KK	E3100								
358	KM	RUNOFF FROM BAS 3100								
359	KO	0								
360	BA	.095								
361	LS	0 82.9								
362	UD	.303								
363	KK	DP3100								
364	KM	COMBINE FLOW FROM R3100, E3100								
365	KO	0								
366	HC	2								
367	KK	E3110								
368	KM	RUNOFF FROM BAS 3110								
369	KO	0								
370	BA	.018								
371	LS	0 90								
372	UD	.472								
373	KK	DP3110								
374	KM	COMBINE FLOWS FROM E3110 AND DP3100								
375	HC	2								
376	KK	R5011								
377	KM	ROUTE FLOW FROM DP3110 TO DP5010								
378	RK	2900 .015 .04 TRAP	20	4						
										HEC-1 INPUT

LINE	ID1.....2.....3.....4.....5.....6.....7.....8.....9.....10								
379	KK	E5010								
380	KM	RUNOFF FROM BAS E5010								
381	KO	0								
382	BA	.156								
383	LS	0 88								
384	UD	.50								
385	KK	DP5010								
386	KM	COMBINE FLOW FROM E5010, R5011, AND R5010								

387	HC	3						
388	KK	E5020						
389	KM	RUNOFF FROM BASIN 5020						
390	KO	0						
391	BA	.2						
392	LS	0	88					
393	UD	.4						
394	KK	E4010						
395	KM	RUNOFF FROM BAS 4010						
396	KO	0						
397	BA	.19						
398	LS	0	86					
399	UD	.497						
400	KK	R4020						
401	KM	ROUTE FLOW FROM E4010 TO DP4020						
402	RK	2400	.005	.05	TRAP	10	30	
403	KK	E4020						
404	KM	RUNOFF FROM BAS 4020						
405	KO	0						
406	BA	.135						
407	LS	0	81.8					
408	UD	.822						
409	KK	DP4020						
410	KM	COMBINE FLOW FROM R4020, E4020						
411	HC	2						
412	KK	E4030						
413	KM	RUNOFF FROM BAS 4030						
414	KO	0						
415	BA	.018						
416	LS	0	90					
417	UD	.251						
418	ZZ							

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
LINE		
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
8	E1010	
	.	
24	.	E1020
	.	.
30	.	E1030
	.	.
36	.	DP1030.....
	.	.
39	.	E1040
	.	.
45	.	E2010
	.	V
	.	V
51	.	R2020
	.	.
55	.	E2020
	.	.
	.	.
61	.	DP2020.....
	.	V

64	.	.	.	V			
	.	.	.	R2030			
67	E2030		
73	E2045	
	V	
79	V	
	R2040	
82	E2040

88	.	.	.	DP2040		
	.	.	.	V			
	.	.	.	V			
91	.	.	.	R2050			
94	E2050		
100	E2060	
106	.	.	.	DP2060		
	.	.	.	V			
	.	.	.	V			
109	.	.	.	R2090			
112	E2070		
	V		
	V		
118	R2080		
121	E2080	
	
127	DP2080	
130	E2090	
	
136	.	.	.	DP2090		
	.	.	.	V			
	.	.	.	V			
139	.	.	.	R2100			
142	E2110		
	V		
	V		
148	R2101		
151	E2100	
	
157	.	.	.	DP2100		
160	E2120		
		
166	.	.	.	DP2120		

267	V		
	R3030		
270	E3030	
	
276	E3035

282	DP3030	
	V		
285	V		
	R3040		
		
288	E3040	
	
294	DP3040	
	V		
297	V		
	R3050		
		
300	E3050	
	
306	DP3050	
		
309	E3060	
	V	
315	V	
	R3070	
	
318	E3070

324	DP3070
	V	
327	V	
	R3071	
	
330	DP3050	
		
333	E3080	
	V	
339	V	
	R3080	
	
342	E3090

348	DP3090
	
351	DP3091	
	V		
354	V		
	R3100		
		
357	E3100	
	
363	DP3100	

+	E2090	.02	1	FLOW TIME	3. 6.33	5. 6.33	9. 6.33	19. 6.25
	3 COMBINED AT							
+	DP2090	.48	1	FLOW TIME	93. 6.25	152. 6.25	260. 6.25	535. 6.25
	ROUTED TO							
+	R2100	.48	1	FLOW TIME	92. 6.33	149. 6.33	253. 6.25	534. 6.25
	HYDROGRAPH AT							
+	E2110	.03	1	FLOW TIME	4. 6.25	8. 6.25	14. 6.25	33. 6.17
	ROUTED TO							
+	R2101	.03	1	FLOW TIME	4. 6.33	7. 6.33	14. 6.25	33. 6.25
	HYDROGRAPH AT							
+	E2100	.09	1	FLOW TIME	15. 6.42	24. 6.42	42. 6.33	89. 6.33
	3 COMBINED AT							
+	DP2100	.61	1	FLOW TIME	111. 6.33	181. 6.33	308. 6.33	651. 6.25
	HYDROGRAPH AT							
+	E2120	.05	1	FLOW TIME	5. 6.25	9. 6.25	18. 6.17	45. 6.17
	2 COMBINED AT							
+	DP2120	.66	1	FLOW TIME	115. 6.33	189. 6.33	324. 6.25	692. 6.25
	ROUTED TO							
+	R2120	.66	1	FLOW TIME	113. 6.42	186. 6.33	323. 6.33	683. 6.25
	ROUTED TO							
+	R2130	.66	1	FLOW TIME	112. 6.42	185. 6.33	322. 6.33	679. 6.25
	HYDROGRAPH AT							
+	E2130	.01	1	FLOW TIME	1. 6.17	2. 6.17	5. 6.17	11. 6.08
	2 COMBINED AT							
+	DP2130	.67	1	FLOW TIME	113. 6.42	186. 6.33	325. 6.33	687. 6.25
	HYDROGRAPH AT							
+	E2140	.01	1	FLOW TIME	1. 6.08	2. 6.08	4. 6.08	9. 6.08
	HYDROGRAPH AT							
+	E2150	.01	1	FLOW TIME	4. 6.17	6. 6.17	11. 6.17	21. 6.17
	HYDROGRAPH AT							
+	E2160	.01	1	FLOW TIME	13. 6.00	17. 6.00	22. 6.00	35. 6.00
	4 COMBINED AT							
+	DP2160	.70	1	FLOW TIME	118. 6.42	196. 6.33	339. 6.33	723. 6.25
	ROUTED TO							
+	R5010	.70	1	FLOW TIME	117. 6.50	195. 6.42	334. 6.33	713. 6.33

HYDROGRAPH AT +	E3000	.42	1	FLOW TIME	129. 6.33	190. 6.33	300. 6.25	568. 6.25
HYDROGRAPH AT +	E3005	.24	1	FLOW TIME	98. 6.17	144. 6.17	222. 6.17	407. 6.17
2 COMBINED AT +	DP3000	.66	1	FLOW TIME	215. 6.25	317. 6.25	493. 6.25	935. 6.17
ROUTED TO +	R3015	.66	1	FLOW TIME	210. 6.33	313. 6.25	493. 6.25	925. 6.25
HYDROGRAPH AT +	E3015	.11	1	FLOW TIME	51. 6.08	75. 6.08	115. 6.08	212. 6.08
2 COMBINED AT +	DP3020	.77	1	FLOW TIME	245. 6.25	365. 6.25	570. 6.25	1077. 6.17
HYDROGRAPH AT +	E3010	.22	1	FLOW TIME	96. 6.17	138. 6.17	211. 6.17	383. 6.17
ROUTED TO +	R3012	.22	1	FLOW TIME	93. 6.25	134. 6.25	204. 6.17	378. 6.17
HYDROGRAPH AT +	E3012	.21	1	FLOW TIME	64. 6.33	94. 6.33	147. 6.33	272. 6.33
HYDROGRAPH AT +	E3020	.19	1	FLOW TIME	159. 6.17	204. 6.17	275. 6.17	428. 6.17
ROUTED TO +	R3025	.19	1	FLOW TIME	158. 6.25	201. 6.25	270. 6.25	418. 6.25
HYDROGRAPH AT +	E3025	.26	1	FLOW TIME	273. 6.08	347. 6.08	463. 6.08	712. 6.08
5 COMBINED AT +	DP3020	1.65	1	FLOW TIME	761. 6.17	1059. 6.17	1562. 6.17	2737. 6.17
ROUTED TO +	R3030	1.65	1	FLOW TIME	754. 6.25	1043. 6.25	1525. 6.25	2680. 6.17
HYDROGRAPH AT +	E3030	.26	1	FLOW TIME	76. 6.25	116. 6.25	186. 6.25	361. 6.17
HYDROGRAPH AT +	E3035	.16	1	FLOW TIME	72. 6.08	106. 6.08	166. 6.08	306. 6.08
3 COMBINED AT +	DP3030	2.07	1	FLOW TIME	865. 6.25	1209. 6.25	1817. 6.17	3267. 6.17
ROUTED TO +	R3040	2.07	1	FLOW TIME	860. 6.25	1208. 6.25	1792. 6.25	3235. 6.17

HYDROGRAPH AT +	E3040	.12	1	FLOW TIME	18. 6.25	31. 6.17	58. 6.17	129. 6.17
2 COMBINED AT +	DP3040	2.18	1	FLOW TIME	878. 6.25	1239. 6.25	1848. 6.25	3364. 6.17
ROUTED TO +	R3050	2.18	1	FLOW TIME	860. 6.33	1219. 6.25	1846. 6.25	3309. 6.25
HYDROGRAPH AT +	E3050	.07	1	FLOW TIME	41. 6.25	56. 6.25	80. 6.25	136. 6.17
2 COMBINED AT +	DP3050	2.26	1	FLOW TIME	898. 6.33	1275. 6.25	1926. 6.25	3444. 6.25
HYDROGRAPH AT +	E3060	.12	1	FLOW TIME	42. 6.17	63. 6.17	100. 6.17	189. 6.08
ROUTED TO +	R3070	.12	1	FLOW TIME	40. 6.25	61. 6.25	98. 6.25	189. 6.17
HYDROGRAPH AT +	E3070	.08	1	FLOW TIME	18. 6.42	27. 6.42	44. 6.33	87. 6.33
2 COMBINED AT +	DP3070	.20	1	FLOW TIME	57. 6.33	86. 6.25	140. 6.25	270. 6.25
ROUTED TO +	R3071	.20	1	FLOW TIME	57. 6.42	85. 6.42	137. 6.33	264. 6.33
2 COMBINED AT +	DP3050	2.45	1	FLOW TIME	946. 6.33	1336. 6.33	2049. 6.25	3705. 6.25
HYDROGRAPH AT +	E3080	.05	1	FLOW TIME	15. 6.25	23. 6.25	38. 6.25	72. 6.25
ROUTED TO +	R3080	.05	1	FLOW TIME	15. 6.42	23. 6.33	37. 6.33	71. 6.33
HYDROGRAPH AT +	E3090	.05	1	FLOW TIME	14. 6.33	21. 6.33	34. 6.25	67. 6.25
2 COMBINED AT +	DP3090	.11	1	FLOW TIME	29. 6.33	44. 6.33	71. 6.33	138. 6.25
2 COMBINED AT +	DP3091	2.56	1	FLOW TIME	975. 6.33	1380. 6.33	2118. 6.25	3843. 6.25
ROUTED TO +	R3100	2.56	1	FLOW TIME	973. 6.33	1380. 6.33	2111. 6.25	3839. 6.25
HYDROGRAPH AT +	E3100	.09	1	FLOW	42.	61.	92.	166.

				TIME	6.17	6.17	6.17	6.17
+	2 COMBINED AT							
	DP3100	2.66	1	FLOW	1007.	1428.	2197.	3990.
				TIME	6.33	6.33	6.25	6.25
+	HYDROGRAPH AT							
	E3110	.02	1	FLOW	10.	14.	19.	31.
				TIME	6.33	6.33	6.33	6.25
+	2 COMBINED AT							
	DP3110	2.67	1	FLOW	1018.	1442.	2216.	4022.
				TIME	6.33	6.33	6.25	6.25
+	ROUTED TO							
	R5011	2.67	1	FLOW	997.	1441.	2174.	3974.
				TIME	6.33	6.33	6.33	6.25
+	HYDROGRAPH AT							
	E5010	.16	1	FLOW	74.	101.	146.	246.
				TIME	6.33	6.33	6.33	6.33
+	3 COMBINED AT							
	DP5010	3.53	1	FLOW	1174.	1722.	2653.	4904.
				TIME	6.42	6.33	6.33	6.25
+	HYDROGRAPH AT							
	E5020	.20	1	FLOW	112.	151.	216.	362.
				TIME	6.25	6.25	6.25	6.25
+	HYDROGRAPH AT							
	E4010	.19	1	FLOW	78.	108.	160.	279.
				TIME	6.33	6.33	6.33	6.33
+	ROUTED TO							
	R4020	.19	1	FLOW	76.	107.	158.	278.
				TIME	6.58	6.58	6.50	6.50
+	HYDROGRAPH AT							
	E4020	.14	1	FLOW	27.	39.	61.	114.
				TIME	6.75	6.75	6.75	6.67
+	2 COMBINED AT							
	DP4020	.32	1	FLOW	102.	145.	215.	383.
				TIME	6.67	6.58	6.58	6.50
+	HYDROGRAPH AT							
	E4030	.02	1	FLOW	15.	20.	27.	44.
				TIME	6.08	6.08	6.08	6.08

*** NORMAL END OF HEC-1 ***

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*
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
CORPS OF ENGINEERS *
* JUN 1998 *
ENGINEERING CENTER *
* VERSION 4.1 *
SECOND STREET *
*
CALIFORNIA 95616 *
* RUN DATE 07JUL00 TIME 15:52:10 *
756-1104 *
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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PAGE 1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	WEST JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY									
2	ID	KIOWA ENGINEERING - PROJECT NO. 98.93									
3	ID	2, 5, 10 & 100 YEAR STORMS FILENAME: WFJCDDET.DAT DEV COND WITH DETENTION									
4	ID	24HR STORM DURATION									
	*DIAGRAM										
5	IT	5	0	0	250						
6	IO	5									
7	JR	PREC	.47	.56	.70	1					
8	KK	E1010									
9	BA	.05									
10	IN	.15									
11	PB	4.4									
12	PC	0.0	.0005	.0015	.0030	.0045	.0060	.0080	.0100	.0120	.0143
13	PC	.0165	.0188	.0210	.0233	.0255	.0278	.0320	.0390	.0460	.0530
14	PC	.0600	.0750	.1000	.4000	.7000	.7250	.7500	.7650	.7800	.7900
15	PC	.8000	.8100	.8200	.8250	.8300	.8350	.8400	.8450	.8500	.8550
16	PC	.8600	.8638	.8675	.8713	.8750	.8788	.8825	.8863	.8900	.8938
17	PC	.8975	.9013	.9050	.9083	.9115	.9148	.9180	.9210	.9240	.9270

18	PC	.9300	.9325	.9350	.9375	.9400	.9425	.9450	.9475	.9500	.9525
19	PC	.9550	.9575	.9600	.9625	.9650	.9675	.9700	.9725	.9750	.9775
20	PC	.9800	.9813	.9825	.9838	.9850	.9863	.9875	.9888	.9900	.9913
21	PC	.9963	.9975	.9988	1.0000						
22	LS	0	73.2								
23	UD	.392									
24	KK	E1020									
25	KM	RUNOFF FROM BAS 1020									
26	KO	0									
27	BA	.041									
28	LS	0	88.8								
29	UD	.467									
30	KK	E1030									
31	KM	RUNOFF FROM BAS 1030									
32	KO	0									
33	BA	.020									
34	LS	0	80								
35	UD	.383									
36	KK	DP1030									
37	KM	COMBINE FLOW FROM E1020 & ROUTED FLOW FROM E1030									
38	HC	2									
39	KK	DB1031									
40	KM	ROUTE FLOW FROM DP 1030 THROUGH DETENTION BASIN 1031									
41	KO	0									
42	RS	1	ELEV	100							
43	SQ	0	15	30	45	60	90				
44	SV	0	.5	1	2.5	4	5				
45	SE	100	101	102	103	104	105				
											HEC-1 INPUT

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PAGE 2

LINE	ID12345678910
46	KK	E1040									
47	KM	RUNOFF FROM BAS 1040									
48	KO	0									
49	BA	.02171									
50	LS	0	75								
51	UD	.41									
52	KK	E2010									
53	KM	RUNOFF FROM BAS 2010									
54	KO	0									
55	BA	.125									
56	LS	0	75								
57	UD	.350									
58	KK	R2020									
59	KM	ROUTE FLOW FROM E2010 TO DP2020									
60	KO	0									
61	RK	800	.03	.05		TRAP	20	10			
62	KK	E2020									
63	KM	RUNOFF FROM BASIN 2020									
64	KO	0									
65	BA	.062									
66	LS	0	75								
67	UD	.376									
68	KK	DP2020									
69	KM	COMBINE FLOW FROM E2010 & ROUTED FLOW FROM E2020									
70	HC	2									
71	KK	R2030									
72	KM	ROUTE FLOW FROM DP2020 TO DP2040									
73	RK	800	.0275	.05		TRAP	20	8			

74	KK	E2030							
75	KM		RUNOFF FROM BAS 2030						
76	KO		0						
77	BA		.021						
78	LS		0	71					
79	UD		.233						
80	KK	E2045							
81	KM		RUNOFF FROM BAS 2045						
82	KO		0						
83	BA		.061						
84	LS		0	85					
85	UD		.258						
86	KK	R2040							
87	KM		ROUTE FLOW FROM E2045 TO DP2040						
88	RK		1200	.0314	.04		TRAP	20	8
							HEC-1 INPUT		

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PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
89	KK	E2040									
90	KM		RUNOFF FROM BAS 2040								
91	KO		0								
92	BA		.026								
93	LS		0	71							
94	UD		.268								
95	KK	DP2040									
96	KM		COMBINE FLOW FROM DP2020, E2030, E2045, E2040								
97	HC		4								
98	KK	R2050									
99	KM		ROUTE FLOW FROM DP2040 TO DP2060								
100	RK		600	.03	.03		TRAP	20		3	
101	KK	E2050									
102	KM		RUNOFF FROM BAS 2050								
103	KO		0								
104	BA		.020								
105	LS		0	71							
106	UD		.33								
107	KK	E2060									
108	KM		RUNOFF FROM BAS 2060								
109	KO		0								
110	BA		.024								
111	LS		0	73.1							
112	UD		.253								
113	KK	DP2060									
114	KM		COMBINE FLOW FROM DP2040, E2050, E2060								
115	HC		3								
116	KK	R2090									
117	KM		ROUTE FLOW FROM DP2060 TO DP2090								
118	RK		500	.023	.03		TRAP	30		3	
119	KK	E2070									
120	KM		RUNOFF FROM BAS 2070								
121	KO		0								
122	BA		.068								
123	LS		0	75							
124	UD		.463								
125	KK	R2080									
126	KM		ROUTE FLOW FROM E2070 TO DP2080								
127	RK		1220	.035	.04		TRAP	10		5	

128 KK E2080
 129 KM RUNOFF FROM BAS 2080
 130 KO 0
 131 BA .057
 132 LS 0 71
 133 UD .247

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

134 KK DP2080
 135 KM COMBINE FLOW FROM E2070, E2080
 136 HC 2

137 KK E2090
 138 KM RUNOFF FROM BAS 2090
 139 KO 0
 140 BA .019
 141 LS 0 75
 142 UD .414

143 KK DP2090
 144 KM COMBINE FLOW FROM DP2060, DP2080, E2090
 145 HC 3

146 KK DB2091
 147 KM ROUTE FLOW FROM DP 2090 THROUGH DET BASIN 2091
 148 KO 0
 149 RS 1 ELEV 100
 150 SQ 0 100 200 300 400 500
 151 SV 0 .5 1 2 3 4.5
 152 SE 100 101 102 103 104 105

153 KK R2100
 154 KM ROUTE FLOW FROM DB2091 TO DP2100
 155 RK 1800 .0169 .03 TRAP 60 4

156 KK E2110
 157 KM RUNOFF FROM BAS 2110
 158 KO 0
 159 BA .034
 160 LS 0 71
 161 UD .329

162 KK R2101
 163 KM ROUTE FLOW FROM E2110 TO DP2100
 164 RK 900 .025 .04 TRAP 40 5

165 KK E2100
 166 KM RUNOFF FROM BAS 2100
 167 KO 0
 168 BA .095
 169 LS 0 75.3
 170 UD .482

171 KK DP2100
 172 KM COMBINE FLOW FROM DP2090, DB2111, E2100
 173 HC 3

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

174 KK E2120
 175 KM RUNOFF FROM BAS 2120
 176 KO 0
 177 BA .047

232	KM	COMBINE FLOW FROM E3000 AND E3005									
233	HC	2									
234	KK	R3015									
235	KM	ROUTE FLOW FROM DP3000 TO DP3020									
236	RK	3200	.031	.04	TRAP	15	3				
237	KK	E3015									
238	KM	RUNOFF FROM BASIN 3015									
239	KO	0									
240	BA	.11									
241	LS	0	81								
242	UD	.21									
243	KK	DP3020									
244	KM	COMBINE FLOW FROM BASIN 3015 AND R3015									
245	HC	2									
246	KK	E3010									
247	KM	RUNOFF FROM BAS 3010									
248	KO	0									
249	BA	.22									
250	LS	0	82								
251	UD	.28									
252	KK	R3012									
253	KM	ROUTE FLOW FROM E3010 TO DP 3020									
254	RK	2600	.03	.04	TRAP	15	3				
255	KK	E3012									
256	KM	RUNOFF FROM BASIN E3012									
257	KO	0									
258	BA	.21									
259	LS	0	82								
260	UD	.47									
		HEC-1 INPUT									
1											
PAGE	7										
LINE	ID	1	2	3	4	5	6	7	8	9	10
261	KK	E3020									
262	KM	RUNOFF FROM BAS 3020									
263	KO	0									
264	BA	.188									
265	LS	0	93								
266	UD	.36									
267	KK	R3025									
268	KM	ROUTE FLOW FROM BASIN E3020 TO DP 3020									
269	RK	2600	.035	.04	TRAP	15	3				
270	KK	E3025									
271	KM	RUNOFF FROM BASIN E3025									
272	KO	0									
273	BA	.26									
274	LS	0	93								
275	UD	.23									
276	KK	DP3020									
277	KM	COMBINE FLOW FROM DP3020, E3012, E3025, R3012 AND R3025									
278	HC	5									
279	KK	DB3021									
280	KM	ROUTE FLOW FROM DP3020 TO DETENTION BASIN 3021									
281	KO	0									
282	RS	1	ELEV	100							
283	SQ	0	150	400	600	1200	3000				
284	SV	0	15	45	60	75	90				
285	SE	100	102	104	106	108	110				

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286      KK  R3030
287      KM          ROUTE FLOW FROM DB3021 TO DP3030
288      RK    3000    .05    .04          TRAP        20        10

289      KK  E3030
290      KM          RUNOFF FROM BAS 3030
291      KO          0
292      BA    .260
293      LS          0        79
294      UD    .34

295      KK  E3035
296      KM          RUNOFF FROM BASIN 3035
297      KO          0
298      BA    .16
299      LS          0        79
300      UD    .16

301      KK  DP3030
302      KM          COMBINE FLOW FROM DP3031, R3030 AND E3035
303      HC          3

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

304      KK  DB3031
305      KM          ROUTE FLOW FROM DP3030 TO DB3031
306      KO          0
307      RS          1      ELEV    100
308      SQ          0      100    400    1000    2400    3000
309      SV          0        2      4      6      10      12
310      SE    100    102    104    106    108    110

311      KK  R3040
312      KM          ROUTE FLOW FROM DB3031 TO DP3040
313      RK    1450    .03    .03          TRAP        30        5

314      KK  E3040
315      KM          RUNOFF FROM BAS 3040
316      KO          0
317      BA    .115
318      LS          0      72.3
319      UD    .294

320      KK  DP3040
321      KM          COMBINE FLOW FROM R3040, E3040
322      HC          2

323      KK  R3050
324      KM          ROUTE FLOW FROM DP3040 TO DP3050
325      RK    2850    .009    .03          TRAP        10        7

326      KK  E3050
327      KM          RUNOFF FROM BAS 3050
328      KO          0
329      BA    .074
330      LS          0      87.3
331      UD    .371

332      KK  DP3050
333      KM          COMBINE FLOW FROM DP3050, E3050
334      HC          2

335      KK  E3060
336      KM          RUNOFF FROM BAS 3060
337      KO          0
338      BA    .119
339      LS          0      79
340      UD    .257

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341 KK DB3061
 342 KM ROUTE FLOW FROM E3060 THROUGH DET BASIN DB3061
 343 KO 0
 344 RS 1 ELEV 100
 345 SQ 0 30 60 90 120 160 200
 346 SV 0 .4 .8 1.2 1.6 2.0 2.5
 347 SE 100 101 102 103 104 105 106
 HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

348 KK R3070
 349 KM ROUTE FLOW FROM DB3061 TO DP3070
 350 RK 2000 .009 .04 TRAP 30 10

 351 KK E3070
 352 KM RUNOFF FROM BAS 3070
 353 KO 0
 354 BA .077
 355 LS 0 79.3
 356 UD .486

 357 KK DP3070
 358 KM COMBINE FLOW FROM R3070 AND E3070
 359 HC 2

 360 KK R3071
 361 KM ROUTE FLOW FROM DP3070 TO DP3050
 362 RK 2250 .015 .04 TRAP 30 10

 363 KK DP3050
 364 KM COMBINE FLOW FROM R3071 AND DP3050
 365 HC 2

 366 KK E3080
 367 KM RUNOFF FROM BAS 3080
 368 KO 0
 369 BA .053
 370 LS 0 79.2
 371 UD .355

 372 KK R3080
 373 KM ROUTE FLOW FROM E3080 TO DP 3090
 374 RK 1650 .01 .04 TRAP 30 10

 375 KK E3090
 376 KM RUNOFF FROM BAS 3090
 377 KO 0
 378 BA .054
 379 LS 0 79
 380 UD .411

 381 KK DP3090
 382 KM COMBINE FLOW FROM R3080 AND E3090
 383 HC 2

 384 KK DB3091
 385 KM ROUTE FLOW FROM E3090 TO DETENTION BASIN 3091
 386 KO 0
 387 RS 1 ELEV 100
 388 SQ 0 5 15 30 100 120
 389 SV 0 .25 .5 1.0 1.5 3.0
 390 SE 100 101 102 103 104 105
 HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

391	KK	DP3092							
392	KM	COMBINE FLOW FROM DP3050 AND DB3091							
393	HC	2							
394	KK	R3100							
395	KM	ROUTE FLOW FROM DP3091 TO DP3100							
396	RK	350 .01 .03 TRAP	10	10					
397	KK	E3100							
398	KM	RUNOFF FROM BAS 3100							
399	KO	0							
400	BA	.095							
401	LS	0 82.9							
402	UD	.303							
403	KK	DB3101							
404	KM	ROUTE FLOW FROM E3100 TO DB3101							
405	KO	0							
406	RS	1 ELEV 100							
407	SQ	0 30 60 90 130 140							
408	SV	0 0.5 1.0 1.5 3.0 4.0							
409	SE	100 101 102 103 104 105							
410	KK	DP3102							
411	KM	COMBINE FLOW FROM E3100 AND DB3101							
412	KO	0							
413	HC	2							
414	KK	E3110							
415	KM	RUNOFF FROM BAS 3110							
416	KO	0							
417	BA	.018							
418	LS	0 90							
419	UD	.472							
420	KK	DP3110							
421	KM	COMBINE FLOW FROM E3100 AND DP3102							
422	HC	2							
423	KK	R3110							
424	KM	ROUTE FLOW FROM DP3110 TO DP3111							
425	RK	900 .004 .04 TRAP	70	4					
426	KK	DP3111							
427	KM	COMBINE FLOW FROM DP2160 AND R3110							
428	HC	2							
429	KK	R5010							
430	KM	ROUTE FLOW FROM DP3111 TO DP5010							
431	RK	2100 .004 .04 TRAP	90	4					
		HEC-1 INPUT							

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

432	KK	E5020							
433	KM	RUNOFF FROM BASIN 5020							
434	KO	0							
435	BA	.2							
436	LS	0 88							
437	UD	.4							
438	KK	DB5021							
439	KM	ROUTE FLOW FROM E5020 THROUGH DET BASIN DB 5021							
440	KO	0							
441	RS	1 ELEV 100							
442	SQ	0 20 40 80 120 250							
443	SV	0 2 4 6 10 13							
444	SE	100 102 104 106 108 110							

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445      KK DP5010
446      KM          COMBINE FLOW FROM DB5021 AND R5010
447      HC          2

448      KK R5011
449      KM          ROUTE FLOW FROM DP5010 TO DP5011
450      RK 1300    .004    .04          TRAP      90      4

451      KK E5010
452      KM          RUNOFF FROM BAS E5010
453      KO          0
454      BA    .156
455      LS          0      88
456      UD    .50

457      KK DB5011
458      KM          ROUTE FLOW FROM E5010 THROUGH DET BASIN DB5011
459      KO          0
460      RS          1      ELEV      100
461      SQ          0      20      40      60      120      150
462      SV          0      2      4      6      8      12
463      SE 100     102     104     106     108     110

464      KK DP5011
465      KM          COMBINE FLOW FROM R5011, DB5011
466      HC          2

467      KK E4010
468      KM          RUNOFF FROM BAS 4010
469      KO          0
470      BA    .19
471      LS          0      86
472      UD    .497

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HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

473      KK DB4011
474      KM          ROUTE FLOW FROM E4010 THROUGH DET BASIN DB4011
475      KO          0
476      RS          1      ELEV      100
477      SQ          0      40      80      120      180      220
478      SV          0      2      4      6      10      13
479      SE 100     102     104     106     108     110

480      KK R4020
481      KM          ROUTE FLOW FROM DB4011 TO DP4020
482      RK 2400    .005    .05          TRAP      10      30

483      KK E4020
484      KM          RUNOFF FROM BAS 4020
485      KO          0
486      BA    .135
487      LS          0      81.8
488      UD    .822

489      KK DP4020
490      KM          COMBINE FLOW FROM R4020, E4020
491      HC          2

492      KK DB4021
493      KM          ROUTE FLOW FROM 4020 THROUGH DET BASIN DB4021
494      KO          0
495      RS          1      ELEV      100
496      SQ          0      10      20      40      80      150      250
497      SV          0      1.0    2      3.0    4      6      10
498      SE 100     101     102     103     104     105     106

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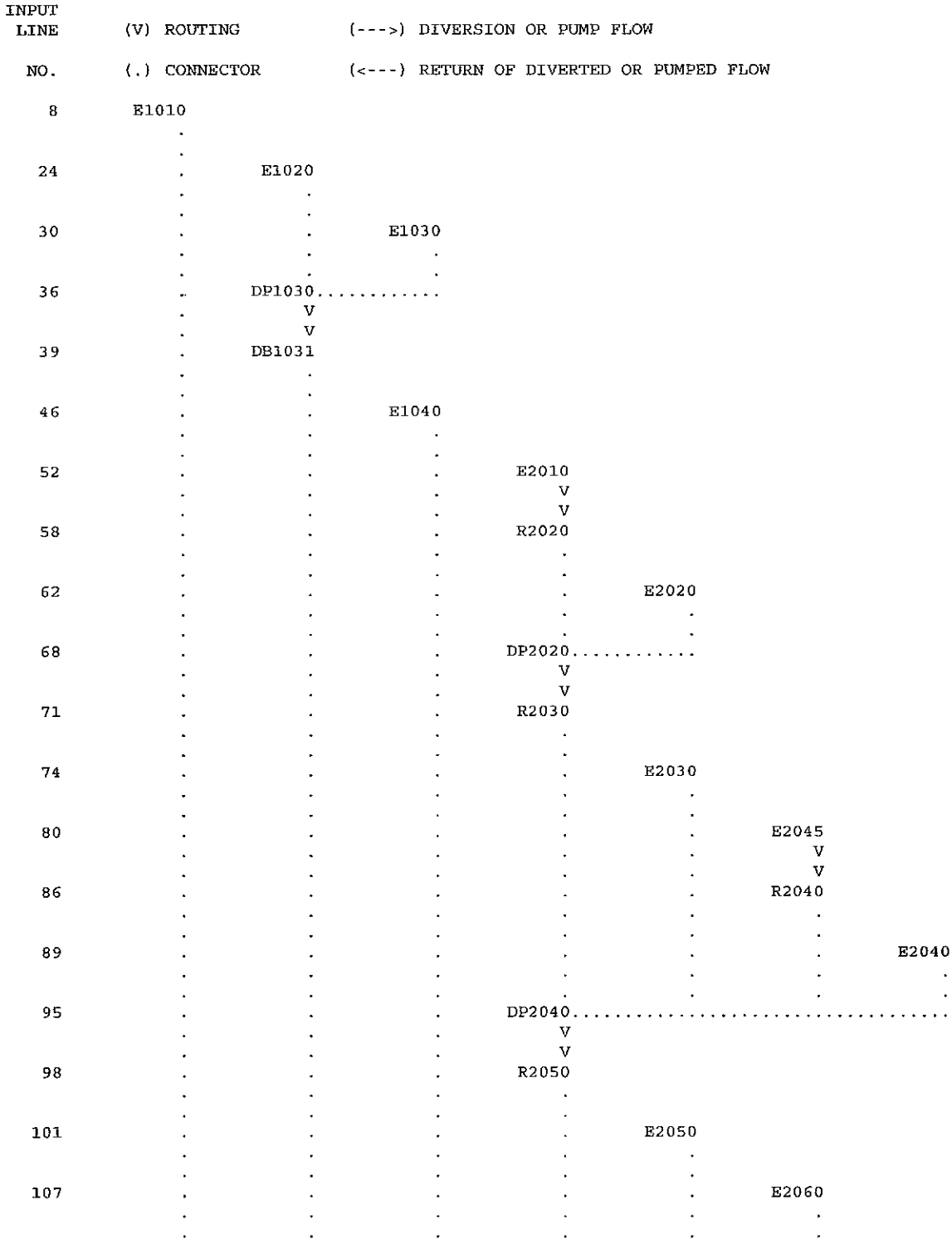
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499      KK      E4030
500      KM      RUNOFF FROM BAS 4030
501      KO      0
502      BA      .018
503      LS      0      90
504      UD      .251
505      ZZ

```

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



113	.	.	.	DP2060.....		
	.	.	.	V		
	.	.	.	V		
116	.	.	.	R2090		
		
119	.	.	.		E2070	
	.	.	.		V	
	.	.	.		V	
125	.	.	.		R2080	
	
128	.	.	.			E2080
	
134	.	.	.		DP2080.....	
	
137	.	.	.			E2090
	
	
143	.	.	.	DP2090.....		
	.	.	.	V		
	.	.	.	V		
146	.	.	.	DB2091		
	.	.	.	V		
	.	.	.	V		
153	.	.	.	R2100		
		
156	.	.	.		E2110	
	.	.	.		V	
	.	.	.		V	
162	.	.	.		R2101	
	
165	.	.	.			E2100
	
171	.	.	.	DP2100.....		
		
174	.	.	.		E2120	
	
180	.	.	.	DP2120.....		
	.	.	.	V		
	.	.	.	V		
183	.	.	.	R2120		
	.	.	.	V		
	.	.	.	V		
186	.	.	.	R2130		
		
189	.	.	.		E2130	
	
195	.	.	.	DP2130.....		
		
198	.	.	.		E2140	
	
204	.	.	.			E2150
	
210	.	.	.			E2160
	
216	.	.	.	DP2160.....		

219	E3000			
			
225		E3005		
			
231	DP3000		
	V			
	V			
234	R3015			
			
237		E3015		
			
243	DP3020		
			
246		E3010		
	V		
	V		
252		R3012		
			
255			E3012	
			
261				E3020
			V
			V
267				R3025

270
E3025

276
DP3020							
	V			
	V			
279	DB3021			
	V			
	V			
286	R3030			
			
289		E3030		
			
295			E3035	
			
301	DP3030		
	V			
	V			
304	DB3031			
	V			
	V			
311	R3040			
			
314		E3040		
			
320	DP3040		
	V			
	V			

323	R3050	.	.	.

326	E3050	.	.

332	DP3050.....	.	.	.

335	E3060	.	.
	V	.	.
	V	.	.
341	DB3061	.	.
	V	.	.
	V	.	.
348	R3070	.	.

351	E3070	.

357	DP3070.....	.	.
	V	.	.
	V	.	.
360	R3071	.	.

363	DP3050.....	.	.	.

366	E3080	.	.
	V	.	.
	V	.	.
372	R3080	.	.

375	E3090	.

381	DP3090.....	.	.
	V	.	.
	V	.	.
384	DB3091	.	.

391	DP3092.....	.	.	.
	V	.	.
	V	.	.
394	R3100	.	.	.

397	E3100	.	.
	V	.	.
	V	.	.
403	DB3101	.	.

410	DP3102.....	.	.	.

414	E3110	.	.

420	DP3110.....	.	.	.
	V	.	.
	V	.	.
423	R3110	.	.	.

426	DP3111.....	.	.	.
	V	.	.

+	R2090	.34	1	FLOW TIME	74. 6.25	119. 6.25	200. 6.25	403. 6.25
HYDROGRAPH AT								
+	E2070	.07	1	FLOW TIME	11. 6.42	17. 6.33	31. 6.33	65. 6.33
ROUTED TO								
+	R2080	.07	1	FLOW TIME	10. 6.42	17. 6.42	30. 6.42	64. 6.33
HYDROGRAPH AT								
+	E2080	.06	1	FLOW TIME	8. 6.17	15. 6.17	29. 6.17	64. 6.17
2 COMBINED AT								
+	DP2080	.13	1	FLOW TIME	16. 6.33	28. 6.25	51. 6.25	113. 6.25
HYDROGRAPH AT								
+	E2090	.02	1	FLOW TIME	3. 6.33	5. 6.33	9. 6.33	19. 6.25
3 COMBINED AT								
+	DP2090	.48	1	FLOW TIME	93. 6.25	152. 6.25	260. 6.25	535. 6.25
ROUTED TO								
+	DB2091	.48	1	FLOW TIME	90. 6.33	147. 6.33	241. 6.33	473. 6.33
** PEAK STAGES IN FEET **								
	1	STAGE			100.90	101.47	102.41	104.73
		TIME			6.33	6.33	6.33	6.33
ROUTED TO								
+	R2100	.48	1	FLOW TIME	90. 6.42	144. 6.42	237. 6.42	468. 6.42
HYDROGRAPH AT								
+	E2110	.03	1	FLOW TIME	4. 6.25	8. 6.25	14. 6.25	33. 6.17
ROUTED TO								
+	R2101	.03	1	FLOW TIME	4. 6.33	7. 6.33	14. 6.25	33. 6.25
HYDROGRAPH AT								
+	E2100	.09	1	FLOW TIME	15. 6.42	24. 6.42	42. 6.33	89. 6.33
3 COMBINED AT								
+	DP2100	.61	1	FLOW TIME	108. 6.42	175. 6.42	292. 6.42	584. 6.33
HYDROGRAPH AT								
+	E2120	.05	1	FLOW TIME	5. 6.25	9. 6.25	18. 6.17	45. 6.17
2 COMBINED AT								
+	DP2120	.66	1	FLOW TIME	112. 6.42	181. 6.42	306. 6.33	618. 6.33
ROUTED TO								
+	R2120	.66	1	FLOW TIME	110. 6.42	181. 6.42	304. 6.42	612. 6.33
ROUTED TO								
+	R2130	.66	1	FLOW TIME	108. 6.42	181. 6.42	304. 6.42	609. 6.42

HYDROGRAPH AT +	E2130	.01	1	FLOW TIME	1. 6.17	2. 6.17	5. 6.17	11. 6.08
2 COMBINED AT +	DP2130	.67	1	FLOW TIME	109. 6.42	182. 6.42	306. 6.42	615. 6.33
HYDROGRAPH AT +	E2140	.01	1	FLOW TIME	1. 6.08	2. 6.08	4. 6.08	9. 6.08
HYDROGRAPH AT +	E2150	.01	1	FLOW TIME	4. 6.17	6. 6.17	11. 6.17	21. 6.17
HYDROGRAPH AT +	E2160	.01	1	FLOW TIME	13. 6.00	17. 6.00	22. 6.00	35. 6.00
4 COMBINED AT +	DP2160	.70	1	FLOW TIME	114. 6.42	188. 6.42	316. 6.42	640. 6.33
HYDROGRAPH AT +	E3000	.42	1	FLOW TIME	129. 6.33	190. 6.33	300. 6.25	568. 6.25
HYDROGRAPH AT +	E3005	.24	1	FLOW TIME	98. 6.17	144. 6.17	222. 6.17	407. 6.17
2 COMBINED AT +	DP3000	.66	1	FLOW TIME	215. 6.25	317. 6.25	493. 6.25	935. 6.17
ROUTED TO +	R3015	.66	1	FLOW TIME	210. 6.33	313. 6.25	493. 6.25	925. 6.25
HYDROGRAPH AT +	E3015	.11	1	FLOW TIME	51. 6.08	75. 6.08	115. 6.08	212. 6.08
2 COMBINED AT +	DP3020	.77	1	FLOW TIME	245. 6.25	365. 6.25	570. 6.25	1077. 6.17
HYDROGRAPH AT +	E3010	.22	1	FLOW TIME	96. 6.17	138. 6.17	211. 6.17	383. 6.17
ROUTED TO +	R3012	.22	1	FLOW TIME	93. 6.25	134. 6.25	204. 6.17	378. 6.17
HYDROGRAPH AT +	E3012	.21	1	FLOW TIME	64. 6.33	94. 6.33	147. 6.33	272. 6.33
HYDROGRAPH AT +	E3020	.19	1	FLOW TIME	159. 6.17	204. 6.17	275. 6.17	428. 6.17
ROUTED TO +	R3025	.19	1	FLOW TIME	158. 6.25	201. 6.25	270. 6.25	418. 6.25
HYDROGRAPH AT +	E3025	.26	1	FLOW TIME	273. 6.08	347. 6.08	463. 6.08	712. 6.08

5 COMBINED AT								
+	DP3020	1.65	1	FLOW	761.	1059.	1562.	2737.
				TIME	6.17	6.17	6.17	6.17
ROUTED TO								
+	DB3021	1.65	1	FLOW	255.	348.	554.	1808.
				TIME	6.75	6.75	6.67	6.42
				** PEAK STAGES IN FEET **				
			1	STAGE	102.84	103.58	105.54	108.68
				TIME	6.75	6.75	6.67	6.42
ROUTED TO								
+	R3030	1.65	1	FLOW	255.	347.	553.	1761.
				TIME	6.83	6.83	6.75	6.50
HYDROGRAPH AT								
+	E3030	.26	1	FLOW	76.	116.	186.	361.
				TIME	6.25	6.25	6.25	6.17
HYDROGRAPH AT								
+	E3035	.16	1	FLOW	72.	106.	166.	306.
				TIME	6.08	6.08	6.08	6.08
3 COMBINED AT								
+	DP3030	2.07	1	FLOW	290.	401.	637.	2007.
				TIME	6.67	6.58	6.58	6.50
ROUTED TO								
+	DB3031	2.07	1	FLOW	289.	399.	636.	2012.
				TIME	6.75	6.67	6.67	6.50
				** PEAK STAGES IN FEET **				
			1	STAGE	103.26	104.00	104.79	107.45
				TIME	6.75	6.67	6.67	6.50
ROUTED TO								
+	R3040	2.07	1	FLOW	289.	399.	635.	1987.
				TIME	6.75	6.67	6.67	6.50
HYDROGRAPH AT								
+	E3040	.12	1	FLOW	18.	31.	58.	129.
				TIME	6.25	6.17	6.17	6.17
2 COMBINED AT								
+	DP3040	2.18	1	FLOW	294.	410.	653.	2043.
				TIME	6.75	6.67	6.67	6.50
ROUTED TO								
+	R3050	2.18	1	FLOW	294.	409.	651.	1998.
				TIME	6.75	6.75	6.67	6.58
HYDROGRAPH AT								
+	E3050	.07	1	FLOW	41.	56.	80.	136.
				TIME	6.25	6.25	6.25	6.17
2 COMBINED AT								
+	DP3050	2.26	1	FLOW	308.	432.	682.	2061.
				TIME	6.75	6.58	6.67	6.58
HYDROGRAPH AT								
+	E3060	.12	1	FLOW	42.	63.	100.	189.
				TIME	6.17	6.17	6.17	6.08
ROUTED TO								
+	DB3061	.12	1	FLOW	33.	50.	82.	165.
				TIME	6.25	6.25	6.25	6.25
				** PEAK STAGES IN FEET **				
			1	STAGE	101.09	101.68	102.73	105.11

+	E3110	.02	1	FLOW TIME	10. 6.33	14. 6.33	19. 6.33	31. 6.25
2 COMBINED AT								
+	DP3110	2.67	1	FLOW TIME	405. 6.58	585. 6.50	943. 6.42	2500. 6.50
ROUTED TO								
+	R3110	2.67	1	FLOW TIME	403. 6.58	581. 6.58	940. 6.50	2493. 6.58
2 COMBINED AT								
+	DP3111	3.38	1	FLOW TIME	506. 6.58	759. 6.50	1246. 6.42	3043. 6.50
ROUTED TO								
+	R5010	3.38	1	FLOW TIME	505. 6.67	756. 6.58	1243. 6.50	3035. 6.58
HYDROGRAPH AT								
+	E5020	.20	1	FLOW TIME	112. 6.25	151. 6.25	216. 6.25	362. 6.25
ROUTED TO								
+	DB5021	.20	1	FLOW TIME	41. 6.75	65. 6.67	93. 6.67	197. 6.58
** PEAK STAGES IN FEET **								
	1	STAGE			104.04	105.26	106.67	109.19
		TIME			6.75	6.67	6.67	6.58
2 COMBINED AT								
+	DP5010	3.58	1	FLOW TIME	545. 6.67	819. 6.58	1333. 6.50	3232. 6.58
ROUTED TO								
+	R5011	3.58	1	FLOW TIME	540. 6.75	810. 6.67	1323. 6.58	3200. 6.58
HYDROGRAPH AT								
+	E5010	.16	1	FLOW TIME	74. 6.33	101. 6.33	146. 6.33	246. 6.33
ROUTED TO								
+	DB5011	.16	1	FLOW TIME	30. 6.92	41. 6.92	59. 6.92	127. 6.75
** PEAK STAGES IN FEET **								
	1	STAGE			103.02	104.10	105.90	108.47
		TIME			6.92	6.92	6.92	6.75
2 COMBINED AT								
+	DP5011	3.73	1	FLOW TIME	570. 6.75	848. 6.67	1375. 6.58	3324. 6.58
HYDROGRAPH AT								
+	E4010	.19	1	FLOW TIME	78. 6.33	108. 6.33	160. 6.33	279. 6.33
ROUTED TO								
+	DB4011	.19	1	FLOW TIME	46. 6.75	64. 6.75	94. 6.75	157. 6.75
** PEAK STAGES IN FEET **								
	1	STAGE			102.30	103.20	104.72	107.23
		TIME			6.75	6.75	6.75	6.75
ROUTED TO								
+	R4020	.19	1	FLOW TIME	46. 7.00	64. 7.00	94. 7.00	157. 6.92

HYDROGRAPH AT								
+	E4020	.14	1	FLOW	27.	39.	61.	114.
				TIME	6.75	6.75	6.75	6.67
2 COMBINED AT								
+	DP4020	.32	1	FLOW	70.	100.	150.	265.
				TIME	6.92	6.92	6.92	6.83
ROUTED TO								
+	DB4021	.32	1	FLOW	48.	77.	121.	211.
				TIME	7.58	7.33	7.25	7.25
				** PEAK STAGES IN FEET **				
			1	STAGE	103.21	103.92	104.58	105.61
				TIME	7.58	7.33	7.25	7.25
HYDROGRAPH AT								
+	E4030	.02	1	FLOW	15.	20.	27.	44.
				TIME	6.08	6.08	6.08	6.08
1								

*** NORMAL END OF HEC-1 ***

HYDRAULIC CALCULATIONS

APPENDIX B

Evaluation of Conceptual Alternatives

Parameter	Flood Control		Disturbance	Environmental		Operations and Maintenance	
	Reduced Hazard	No change		Limited or No Impact	Mitigation required	Reduced effort	Increased effort
Alternative Concept: Floodplain Preservation (do nothing)							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative		Limited area of wetland or riparian area exist in these segments			Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Residential and commercial property not impacted by the 100-year event at this time.		This concept works well for the segments that have significant wetland and riparian resources	Limited mitigation with this concept		Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
WFJCC NO. OF FONTAINE BLVD		Residential and commercial property not impacted by the 100-year event at this time.		Limited area of wetland or riparian area exist in these segments			Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
Alternative Concept: Channelization							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Channelization of 100-year flow in these segments would benefit developed properties along Marksheffel Road, south of the basin			Limited habitat now exists along drainageway.			More maintenance of linings would result with this alternative
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Little change in floodplain width would result from channelizing in these segments Residential and commercial property not impacted by the 100-year event at this time.	Construction of channel linings will disturb existing wetland and riparian resource		Mitigation would be required wherever the channelization altered or eliminated wetland habitat		More maintenance of linings would result with this alternative
WFJCC NO OF FONTAINE BLVD		Residential and commercial property not impacted by the 100-year event at this time.		Limited impacts due to absence of wetland areas in these segments.	Mitigation would be required wherever the channelization altered or eliminated wetland habitat		More maintenance of linings would result with this alternative

Parameter	Open Space/Aesthetics		Implementation			Comments	Relative Advantages/Disadvantages of Alternative Concept
	Enhancement or Degradation	No change	Low Feasibility	Moderate Feasibility	High Feasibility		
Alternative Concept: Floodplain Preservation (do nothing)							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Poorly defined drainageway in these segments the enhancement opportunities in these segments		X		Floodplain preservation in these segments does not address the significant areas within the existing floodplain especially south of the basin	Ad: Low initial cost of improvements DAd: Large area within 100-year floodplain not addressed by this concept.
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Floodplain preservation has the opportunity if implemented to preserve the natural floodplain that can enhance the open space			X		When combined with preservation of the floodplain as open space, this concept has positive benefits to the existing wetland and riparian resources in these segments.	Ad: Reduces the need for mitigation along these segments with regard to permitting of the proposed stormwater facilities
WFJCC NO. OF FONTAINE BLVD	Opportunities for vegetative and visual enhancement of drainageway is limited by the naturally narrow drainageways			X			Ad: Low initial cost
Alternative Concept: Channelization							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Planting along new channel could enhance the drainageway as open space. No vegetation now exists.				X	Development of land adjacent to these segments will require that a channel be constructed	Ad: Greatly reduces extent of 100-year floodplain within these segments and offsite from the basin DAd: Land acquisition will be required through area downstream of Marksheffel Road
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Naturalness of existing drainageway could be viewed as a degradation			X			Ad: Reduces width of drainageway compared to the 100-year floodplain widths DAd: Moderate to high initial costs and would cause disturbance to wetland and riparian areas that would not have to be mitigated for.
WFJCC NO OF FONTAINE BLVD	Naturalness of existing drainageway could be viewed as a degradation			X			Ad: Reduces width of drainageway compared to 100-year floodplain widths DAd: Moderate to high initial costs

Evaluation of Conceptual Alternatives

Parameter	Flood Control		Environmental			Operations and Maintenance	
	Reduced Hazard	No change	Disturbance	Limited or No Impact	Mitigation required	Reduced effort	Increased effort
Reach							
Alternative Concept: Onsite Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative w/o channelization		Limited area of wetland or riparian area exist in these segments			Increase maintenance compared to regional detention
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.	Disturbance wetland & riparian areas could result from the construction of embankments for the on-stream detention basins		Mitigation would be required within detention basin bottom for onstream sites		Increase maintenance compared to regional detention
WFJCC NO. OF FONTAINE BLVD	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.		Limited area of wetland or riparian area exist in these segments	Mitigation would be required within detention basin bottom for onstream sites		Increase maintenance compared to regional detention
Alternative Concept: Regional Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative w/o channelization	Disturbance of wetland & riparian areas could result from the construction of embankments for the on-stream detention basins			Lower maintenance compared to onsite detention concept	
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.	Disturbance of wetland and riparian areas could result from the construction of embankments for the on-stream detention basins		Mitigation would be required wherever the construction altered or eliminated wetland habitat	Lower maintenance compared to onsite detention concept	
WFJCC NO OF FONTAINE BLVD	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.		Limited impacts due to absence of wetland areas in these segments.		Lower maintenance compared to onsite detention concept	

Parameter	Open Space/Aesthetics		Implementation			Comments	Relative Advantages/Disadvantages of Alternative Concept
	Enhancement or Degradation	No change	Low Feasibility	Moderate Feasibility	High Feasibility		
Reach							
Alternative Concept: Onsite Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Limited enhancement of existing wetlands would result from this concept	X			Onsite detention areas could be used to create new wetland areas if site conditions are there to create a wetland/riparian resource	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: Hydrologic impact of onsite detention is potentially not as effective as regional detention concept Private maintenance agreements would be required
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Limited enhancement of existing wetlands would result from this concept		X		Onsite detention areas could be used to as mitigation sites for wetland or riparian resources disturbed by channelization of drainageway	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: Hydrologic impact of onsite detention is potentially not as effective as regional detention concept Private maintenance agreements would be required
WFJCC NO. OF FONTAINE BLVD		Limited enhancement of existing wetlands would result from this concept	X			Numerous small drainageways in this portion of the basin would create the need for numerous onsite detention basins	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: This concept conflicts with the Colorado Centre master drainage plan approved by the City of Colorado Springs
Alternative Concept: Regional Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Offstream basins could be used to expand the wetland resources to areas off of the main drainageway			X		Dedication of land could be achieved through development process	Ad: Fewer sites, less maintenance compared to onsite detention concept. Conforms with Cross Creek MDDP DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Offstream basins could be used to expand the wetland resources to areas off of the main drainageway			X		Dedication of land could be achieved through development process	Ad: Fewer sites, less maintenance compared to onsite detention concept DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments
WFJCC NO OF FONTAINE BLVD					X	Natural detention area now exists north of Fontaine Boulevard	Ad: Fewer sites, less maintenance compared to onsite detention concept. Conforms with Colorado Centre MDDP DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments

WEST FORK JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY

SUMMARY OF CHANNEL IMPROVEMENTS RIPRAP LINED

"n"= 0.04 SS 3 H TO 1 V.
DEPTH = 3 FT AVERAGE VELOCITY = 7 FPS MAX VEL 9 FPS

CHANNEL NUMBER	DRAINAGE NAME	CHANNEL LENGTH (FT)	100-YEAR FLOW (CFS)	REQD. AREA (S.F.)	CHANNEL DEPTH (FT)	EX. SLOPE (FT/FT)	FUTURE SLOPE (FT/FT)	CHANNEL BOTTOM WIDTH	CHANNEL TOP WIDTH	HYDR. RADIUS (FEET)	R.O.W. REQD. (FEET)	DROP IN SEGMENT (FT)	NUMBER OF CHECK STRUC.
5012	WFJC	1400	3320	474	5.0	0.003	0.003	100	130	4.43	160	0.0	0
5011	WFJC	1270	3190	456	5.0	0.006	0.004	90	120	4.38	150	2.5	2
5010	WFJC	2050	2640	377	4.0	0.006	0.004	90	114	3.59	144	4.1	3
3110	WFJC	870	2500	357	4.0	0.006	0.004	90	114	3.59	144	1.7	1
3040	WFJC	2350	360	51	3.0	0.025	0.010	20	38	2.28	68	35.3	11
3030-1	WFJC	1060	1850	264	4.0	0.030	0.010	65	89	3.47	119	21.2	5
3030-2	WFJC	900	1760	251	4.0	0.030	0.010	60	84	3.43	114	18.0	8
3000	WFJC	3230	570	81	3.0	0.020	0.010	25	43	2.37	73	32.3	10
3005	WFJC	3000	410	59	3.0	0.020	0.010	20	38	2.28	68	30.0	10
3012	WFJC	2000	380	54	3.0	0.010	0.005	20	38	2.28	68	10.0	4
3015	WFJC	1550	935	134	4.0	0.015	0.010	35	59	3.18	89	7.8	3
3021	WFJC	1750	420	60	3.0	0.020	0.010	20	38	2.28	68	17.5	6
3025	WFJC	1380	910	130	4.0	0.018	0.010	35	59	3.18	89	11.0	4
3060	WFJC	2000	190	27	2.0	0.015	0.010	20	32	1.62	62	10.0	3
3070	WFJC	800	190	27	4.0	0.015	0.010	10	34	2.54	64	4.0	1
2160	DRWY A	1030	620	89	3.0	0.026	0.013	30	48	2.44	78	13.4	11
4020 (1)	DFA	2500	265	53	3.0	0.005	0.005	20	38	2.28	68	0.0	4
4010 (1)	DFA	900	280	56	3.0	0.005	0.005	20	38	2.28	68	0.0	0

(1) Grasslined channel section

WEST FORK JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY

UNIT COSTS

SUMMARY OF CHANNEL IMPROVEMENTS

RIPRAP LINED

DEPTH	MAINT. RD (\$/FT)	CHAN EX. (\$/FT)	REVEG (\$/FT^2)	RIPRAP (\$/FT.)
1	\$15	\$10	\$0.25	\$72
2	\$15	\$15	\$0.25	\$99
3	\$15	\$18	\$0.25	\$126
4	\$15	\$19	\$0.25	\$144
5	\$15	\$19	\$0.25	\$171

NUMBER OF DROP STRUC.	CHANNEL NUMBER	MAINT. RD (\$/FT.) \$15.00	CHAN EX (\$/YD^3) \$7.50	REVEG (\$/FT^2) \$0.25	RIPRAP (\$/FT.)	COST PER FT.	CHECK & DROP STRUCTURES	TOTAL CONST. COST, CHANNEL & DROPS	ENGR. & CONT	O&M
0	5012	\$15	\$19	\$5	\$171	\$210	\$0	\$293,300	\$43,995	\$5,600
0	5011	\$15	\$19	\$5	\$171	\$210	\$84,000	\$350,065	\$52,510	\$5,080
0	5010	\$15	\$19	\$3	\$144	\$181	\$120,600	\$491,650	\$73,748	\$8,200
0	3110	\$15	\$19	\$3	\$144	\$181	\$40,200	\$197,670	\$29,651	\$3,480
0	3040	\$15	\$18	\$2	\$126	\$160	\$191,400	\$567,400	\$85,110	\$9,400
0	3030-1	\$15	\$19	\$3	\$144	\$181	\$163,500	\$355,360	\$53,304	\$4,240
0	3030-2	\$15	\$19	\$3	\$144	\$181	\$249,600	\$412,500	\$61,875	\$3,600
0	3000	\$15	\$18	\$2	\$126	\$160	\$189,000	\$705,800	\$105,870	\$12,920
0	3005	\$15	\$18	\$2	\$126	\$160	\$174,000	\$654,000	\$98,100	\$12,000
0	3012	\$15	\$18	\$2	\$126	\$160	\$69,600	\$389,600	\$58,440	\$8,000
0	3015	\$15	\$19	\$3	\$144	\$181	\$71,100	\$351,650	\$52,748	\$6,200
0	3021	\$15	\$18	\$2	\$126	\$160	\$104,400	\$384,400	\$57,660	\$7,000
0	3025	\$15	\$19	\$3	\$144	\$181	\$94,800	\$344,580	\$51,687	\$5,520
0	3060	\$15	\$15	\$0	\$99	\$129	\$46,800	\$304,800	\$45,720	\$8,000
0	3070	\$15	\$19	\$3	\$144	\$181	\$16,200	\$161,000	\$24,150	\$3,200
0	2160	\$15	\$18	\$2	\$126	\$160	\$224,400	\$389,200	\$58,380	\$4,120
0	4020 (1)	\$15	\$18	\$10	\$0	\$43	\$69,600	\$175,850	\$26,378	\$10,000
0	4010 (1)	\$15	\$18	\$10	\$0	\$43	\$0	\$38,250	\$5,738	\$3,600

\$6,352,975 \$952,946 \$106,560

TOTAL COST OF IMPROVEMENTS \$7,305,921

CURRENT DATE: 07-07-2000
 CURRENT TIME: 12:17:35

FILE DATE: 07-07-2000
 FILE NAME: 5010

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.20	80.00	5 RCB	15.00	6.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 5010 DATE: 07-07-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
102.66	350	350	0	0	0	0	0	0	1
102.96	700	700	0	0	0	0	0	0	1
103.25	1050	1050	0	0	0	0	0	0	1
103.74	1400	1400	0	0	0	0	0	0	1
104.32	1750	1750	0	0	0	0	0	0	1
104.88	2100	2100	0	0	0	0	0	0	1
105.41	2450	2450	0	0	0	0	0	0	1
105.93	2800	2800	0	0	0	0	0	0	1
106.45	3150	3150	0	0	0	0	0	0	1
106.71	3320	3320	0	0	0	0	0	0	1
110.00	5186	5186	0	0	0	0	0	0	OVERTOPPING

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 5010 DATE: 07-07-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
102.66	0.00	350	0	0.00
102.96	0.00	700	0	0.00
103.25	0.00	1050	0	0.00
103.74	0.00	1400	0	0.00
104.32	0.00	1750	0	0.00
104.88	0.00	2100	0	0.00
105.41	0.00	2450	0	0.00
105.93	0.00	2800	0	0.00
106.45	0.00	3150	0	0.00
106.71	0.00	3320	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 07-07-2000
CURRENT TIME: 12:17:35

FILE DATE: 07-07-2000
FILE NAME: 5010

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 80.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.004
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.20
CULVERT NO.1 OUTLET INVERT ELEVATION 99.20 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.20	0.000	0.00	0.00
350.00	100.41	0.552	3.45	0.30
700.00	101.03	0.583	4.48	0.46
1050.00	101.52	0.600	5.19	0.58
1400.00	101.95	0.612	5.76	0.69
1750.00	102.34	0.621	6.24	0.78
2100.00	102.69	0.627	6.65	0.87
2450.00	103.02	0.633	7.02	0.95
2800.00	103.33	0.637	7.35	1.03
3150.00	103.62	0.641	7.65	1.10
3320.00	103.75	0.643	7.79	1.14

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 80.00
CREST LENGTH (FT) 200.00
OVERTOPPING CREST ELEVATION (FT) 110.00

W E SWEFFELS RD
 MESA RUNGE ROADWAY, DRAW #

CURRENT DATE: 03-21-2000
 CURRENT TIME: 13:56:48

FILE DATE: 03-21-2000
 FILE NAME: 2160-1

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	2 RCB	8.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 2160-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.94	70	70	0	0	0	0	0	0	1
102.25	140	140	0	0	0	0	0	0	1
102.67	210	210	0	0	0	0	0	0	1
103.25	280	280	0	0	0	0	0	0	1
103.78	350	350	0	0	0	0	0	0	1
104.27	420	420	0	0	0	0	0	0	1
104.76	490	490	0	0	0	0	0	0	1
105.26	560	560	0	0	0	0	0	0	1
105.70	620	620	0	0	0	0	0	0	1
106.33	700	700	0	0	0	0	0	0	1
108.00	882	882	0	0	0	0	0	0	OVERTOPPING

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 2160-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.94	0.00	70	0	0.00
102.25	0.00	140	0	0.00
102.67	0.00	210	0	0.00
103.25	0.00	280	0	0.00
103.78	0.00	350	0	0.00
104.27	0.00	420	0	0.00
104.76	0.00	490	0	0.00
105.26	0.00	560	0	0.00
105.70	0.00	620	0	0.00
106.33	0.00	700	0	0.00

 <1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000

FILE DATE: 03-21-2000

CURRENT TIME: 13:56:48

FILE NAME: 2160-1

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****

BOTTOM WIDTH (FT) 40.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.005
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 99.00
 CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
70.00	99.65	0.556	2.55	0.20
140.00	99.99	0.586	3.30	0.31
210.00	100.25	0.603	3.83	0.39
280.00	100.48	0.614	4.25	0.46
350.00	100.69	0.623	4.59	0.53
420.00	100.88	0.629	4.90	0.59
490.00	101.06	0.635	5.16	0.64
560.00	101.22	0.639	5.40	0.69
620.00	101.36	0.642	5.59	0.74
700.00	101.53	0.646	5.82	0.79

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
 EMBANKMENT TOP WIDTH (FT) 45.00
 CREST LENGTH (FT) 100.00
 OVERTOPPING CREST ELEVATION (FT) 108.00

UNDER ARTERIAL: DP 3000

CULVERT @ 3025

CURRENT DATE: 03-21-2000
CURRENT TIME: 12:44:45

FILE DATE: 03-21-2000
FILE NAME: 3000

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	98.80	120.01	2 RCB	12.00	6.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3000 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
102.22	100	100	0	0	0	0	0	0	1
102.50	200	200	0	0	0	0	0	0	1
102.76	300	300	0	0	0	0	0	0	1
103.13	400	400	0	0	0	0	0	0	1
103.65	500	500	0	0	0	0	0	0	1
104.12	600	600	0	0	0	0	0	0	1
104.58	700	700	0	0	0	0	0	0	1
105.01	800	800	0	0	0	0	0	0	1
105.43	900	900	0	0	0	0	0	0	1
105.71	965	965	0	0	0	0	0	0	1
108.00	1458	1458	0	0	0	0	0	0	OVERTOPPING

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3000 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
102.22	0.00	100	0	0.00
102.50	0.00	200	0	0.00
102.76	0.00	300	0	0.00
103.13	0.00	400	0	0.00
103.65	0.00	500	0	0.00
104.12	0.00	600	0	0.00
104.58	0.00	700	0	0.00
105.01	0.00	800	0	0.00
105.43	0.00	900	0	0.00
105.71	0.00	965	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
 (RENT TIME: 12:44:45

FILE DATE: 03-21-2000
 FILE NAME: 3000

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
 BOTTOM WIDTH (FT) 25.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.005
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 98.80
 CULVERT NO.1 OUTLET INVERT ELEVATION 98.80 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	98.80	0.000	0.00	0.00
100.00	99.86	0.576	3.36	0.33
200.00	100.38	0.597	4.26	0.49
300.00	100.79	0.609	4.87	0.62
400.00	101.14	0.615	5.34	0.73
500.00	101.45	0.620	5.73	0.83
600.00	101.73	0.624	6.06	0.91
700.00	101.99	0.627	6.35	0.99
800.00	102.23	0.629	6.61	1.07
900.00	102.45	0.631	6.85	1.14
965.00	102.59	0.632	6.99	1.18

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
 EMBANKMENT TOP WIDTH (FT) 45.00
 CREST LENGTH (FT) 100.00
 OVERTOPPING CREST ELEVATION (FT) 108.00

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:21:29

FILE DATE: 03-21-2000
 FILE NAME: 3000-2

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	18.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 3000-2 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.88	65	65	0	0	0	0	0	0	1
102.14	130	130	0	0	0	0	0	0	1
102.39	195	195	0	0	0	0	0	0	1
102.85	260	260	0	0	0	0	0	0	1
103.32	325	325	0	0	0	0	0	0	1
103.75	390	390	0	0	0	0	0	0	1
104.17	455	455	0	0	0	0	0	0	1
104.57	520	520	0	0	0	0	0	0	1
104.88	570	570	0	0	0	0	0	0	1
105.39	650	650	0	0	0	0	0	0	1
108.00	992	992	0	0	0	0	0	0	OVERTOPPING

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3000-2 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.88	0.00	65	0	0.00
102.14	0.00	130	0	0.00
102.39	0.00	195	0	0.00
102.85	0.00	260	0	0.00
103.32	0.00	325	0	0.00
103.75	0.00	390	0	0.00
104.17	0.00	455	0	0.00
104.57	0.00	520	0	0.00
104.88	0.00	570	0	0.00
105.39	0.00	650	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:23:44

FILE DATE: 03-21-2000
FILE NAME: 3000-2

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 20.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.005
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
65.00	99.93	0.560	3.07	0.29
130.00	100.39	0.580	3.88	0.43
195.00	100.75	0.590	4.42	0.54
260.00	101.05	0.596	4.85	0.64
325.00	101.32	0.601	5.19	0.72
390.00	101.57	0.604	5.49	0.80
455.00	101.79	0.607	5.75	0.87
520.00	102.00	0.609	5.98	0.94
570.00	102.15	0.610	6.15	0.98
650.00	102.38	0.612	6.39	1.05

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 108.00

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:31:25

FILE DATE: 03-21-2000
FILE NAME: 3005-1

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	9.00	4.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3005-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.28	21	21	0	0	0	0	0	0	1
101.47	42	42	0	0	0	0	0	0	1
101.82	63	63	0	0	0	0	0	0	1
102.19	84	84	0	0	0	0	0	0	1
102.52	105	105	0	0	0	0	0	0	1
102.83	126	126	0	0	0	0	0	0	1
103.14	147	147	0	0	0	0	0	0	1
103.43	168	168	0	0	0	0	0	0	1
103.73	189	189	0	0	0	0	0	0	1
103.95	205	205	0	0	0	0	0	0	1
108.00	444	444	0	0	0	0	0	0	1

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3005-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.28	0.00	21	0	0.00
101.47	0.00	42	0	0.00
101.82	0.00	63	0	0.00
102.19	0.00	84	0	0.00
102.52	0.00	105	0	0.00
102.83	0.00	126	0	0.00
103.14	0.00	147	0	0.00
103.43	0.00	168	0	0.00
103.73	0.00	189	0	0.00
103.95	0.00	205	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:31:25

FILE DATE: 03-21-2000
FILE NAME: 3005-1

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 1 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
21	101.28	99.58	0.88	1.28	6-FF	0.00	0.00	0.00	4.04
42	101.47	99.86	1.39	1.47	6-FF	0.00	0.00	0.00	5.31
63	101.82	100.07	1.82	1.65	6-FF	0.00	0.00	0.00	8.83
84	102.19	100.26	2.19	1.82	6-FF	0.00	0.00	0.00	9.58
105	102.52	100.42	2.52	2.00	6-FF	0.00	0.00	0.00	10.42
126	102.83	100.57	2.83	2.19	6-FF	0.00	0.00	0.00	10.83
147	103.14	100.70	3.14	2.39	6-FF	0.00	0.00	0.00	11.39
168	103.43	100.83	3.43	2.61	6-FF	0.00	0.00	0.00	11.74
189	103.73	100.94	3.73	2.83	6-FF	0.00	0.00	0.00	12.13
205	103.95	101.03	3.95	3.01	6-FF	0.00	0.00	0.00	12.44

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

*** SITE DATA ***** CULVERT INVERT *****

INLET STATION (FT) 100.00
INLET ELEVATION (FT) 100.00
OUTLET STATION (FT) 200.00
OUTLET ELEVATION (FT) 99.00
NUMBER OF BARRELS 1.00
SLOPE (V-FT/H-FT) 0.0100
CULVERT LENGTH ALONG SLOPE (FT) 100.01

***** CULVERT DATA SUMMARY *****

BARREL SHAPE BOX
BARREL SPAN 9.00 FT
BARREL RISE 4.00 FT
BARREL MATERIAL CONCRETE
BARREL MANNING'S N 0.012
INLET TYPE CONVENTIONAL
INLET EDGE AND WALL 1:1 BEVEL
INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:31:25

FILE DATE: 03-21-2000
FILE NAME: 3005-1

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 10.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.010
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
21.00	99.58	0.719	3.10	0.36
42.00	99.86	0.742	3.90	0.53
63.00	100.07	0.753	4.43	0.67
84.00	100.26	0.760	4.84	0.79
105.00	100.42	0.765	5.18	0.89
126.00	100.57	0.769	5.46	0.98
147.00	100.70	0.772	5.71	1.06
168.00	100.83	0.774	5.94	1.14
189.00	100.94	0.776	6.14	1.21
205.00	101.03	0.778	6.28	1.27

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 108.00

DP 3020 e Proposed N/S Collector,
 3000-2, 3005-1 & 3010 ALSO 1

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:05:34

FILE DATE: 03-21-2000
 FILE NAME: 3020-1

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	12.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 ***** SUMMARY OF CULVERT FLOWS (CFS) FILE: 3020-1 DATE: 03-21-2000 *****

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.89	45	45	0	0	0	0	0	0	1
102.15	90	90	0	0	0	0	0	0	1
102.50	135	135	0	0	0	0	0	0	1
102.98	180	180	0	0	0	0	0	0	1
103.44	225	225	0	0	0	0	0	0	1
103.88	270	270	0	0	0	0	0	0	1
104.31	315	315	0	0	0	0	0	0	1
104.73	360	360	0	0	0	0	0	0	1
105.15	405	405	0	0	0	0	0	0	1
105.29	420	420	0	0	0	0	0	0	1
108.00	676	676	0	0	0	0	0	0	OVERTOPPING

Handwritten note: Head = H

 ***** SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3020-1 DATE: 03-21-2000 *****

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.89	0.00	45	0	0.00
102.15	0.00	90	0	0.00
102.50	0.00	135	0	0.00
102.98	0.00	180	0	0.00
103.44	0.00	225	0	0.00
103.88	0.00	270	0	0.00
104.31	0.00	315	0	0.00
104.73	0.00	360	0	0.00
105.15	0.00	405	0	0.00
105.29	0.00	420	0	0.00

 <1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:05:34

FILE DATE: 03-21-2000
FILE NAME: 3020-1

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 1 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
45	101.89	99.72	1.20	1.89	6-FF	0.00	0.00	0.00	4.93
90	102.15	100.07	1.91	2.15	6-FF	0.00	0.00	0.00	6.21
135	102.50	100.35	2.50	2.40	6-FF	0.00	0.00	0.00	10.42
180	102.98	100.58	2.98	2.65	6-FF	0.00	0.00	0.00	11.25
225	103.44	100.79	3.44	2.92	6-FF	0.00	0.00	0.00	11.92
270	103.88	100.98	3.88	3.19	6-FF	0.00	0.00	0.00	12.53
315	104.31	101.15	4.31	3.49	6-FF	0.00	0.00	0.00	13.05
360	104.73	101.31	4.73	3.80	6-FF	0.00	0.00	0.00	13.51
405	105.15	101.46	5.15	4.13	6-FF	0.00	0.00	0.00	13.93
420	105.29	101.51	5.29	4.25	6-FF	0.00	0.00	0.00	14.07

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

* ** SITE DATA ***** CULVERT INVERT *****
INLET STATION (FT) 100.00
INLET ELEVATION (FT) 100.00
OUTLET STATION (FT) 200.00
OUTLET ELEVATION (FT) 99.00
NUMBER OF BARRELS 1.00
SLOPE (V-FT/H-FT) 0.0100
CULVERT LENGTH ALONG SLOPE (FT) 100.01

***** CULVERT DATA SUMMARY *****
BARREL SHAPE BOX
BARREL SPAN 12.00 FT
BARREL RISE 5.00 FT
BARREL MATERIAL CONCRETE
BARREL MANNING'S N 0.012
INLET TYPE CONVENTIONAL
INLET EDGE AND WALL 1:1 BEVEL
INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:05:34

FILE DATE: 03-21-2000
FILE NAME: 3020-1

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 15.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.010
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
45.00	99.72	0.757	3.64	0.45
90.00	100.07	0.784	4.61	0.67
135.00	100.35	0.797	5.25	0.84
180.00	100.58	0.805	5.75	0.99
225.00	100.79	0.811	6.16	1.12
270.00	100.98	0.815	6.51	1.24
315.00	101.15	0.819	6.82	1.34
360.00	101.31	0.822	7.09	1.44
405.00	101.46	0.824	7.34	1.54
420.00	101.51	0.825	7.42	1.57

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 108.00

ROADWAY 3020-2
 USE AT COLLECTOR ~~INLET~~ 1
 FIRST BASIN OUTLET, 3030

CURRENT DATE: 03-21-2000
 CURRENT TIME: 13:59:27

FILE DATE: 03-21-2000
 FILE NAME: 3020-2

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	2 RCB	15.00	8.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 3020-2 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
103.59	210	210	0	0	0	0	0	0	1
103.98	420	420	0	0	0	0	0	0	1
104.34	630	630	0	0	0	0	0	0	1
104.70	840	840	0	0	0	0	0	0	1
105.23	1050	1050	0	0	0	0	0	0	1
105.89	1260	1260	0	0	0	0	0	0	1
106.52	1470	1470	0	0	0	0	0	0	1
107.15	1680	1680	0	0	0	0	0	0	1
107.77	1890	1890	0	0	0	0	0	0	1
108.10	2000	2000	0	0	0	0	0	0	1
112.00	3203	3203	0	0	0	0	0	0	OVERTOPPING

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3020-2 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
103.59	0.00	210	0	0.00
103.98	0.00	420	0	0.00
104.34	0.00	630	0	0.00
104.70	0.00	840	0	0.00
105.23	0.00	1050	0	0.00
105.89	0.00	1260	0	0.00
106.52	0.00	1470	0	0.00
107.15	0.00	1680	0	0.00
107.77	0.00	1890	0	0.00
108.10	0.00	2000	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 13:59:27

FILE DATE: 03-21-2000
FILE NAME: 3020-2

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 75.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.005
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
210.00	99.87	0.588	3.11	0.27
420.00	100.31	0.623	4.05	0.41
630.00	100.67	0.643	4.72	0.52
840.00	100.98	0.657	5.24	0.62
1050.00	101.26	0.667	5.69	0.70
1260.00	101.51	0.675	6.07	0.78
1470.00	101.75	0.682	6.42	0.86
1680.00	101.98	0.687	6.73	0.93
1890.00	102.19	0.692	7.01	1.00
2000.00	102.29	0.694	7.15	1.03

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 112.00
OVERTOPPING CREST ELEVATION (FT) 112.00

DETENTION BASIN 3060
OUTLET PIPE

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:01:36

FILE DATE: 03-21-2000
FILE NAME: 3060-1

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	98.80	120.01	1 RCP	5.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3060-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.89	18	18	0	0	0	0	0	0	1
102.25	35	35	0	0	0	0	0	0	1
102.86	53	53	0	0	0	0	0	0	1
103.43	70	70	0	0	0	0	0	0	1
103.93	88	88	0	0	0	0	0	0	1
104.41	105	105	0	0	0	0	0	0	1
104.88	123	123	0	0	0	0	0	0	1
105.38	140	140	0	0	0	0	0	0	1
105.91	158	158	0	0	0	0	0	0	1
106.15	165	165	0	0	0	0	0	0	1
110.00	255	255	0	0	0	0	0	0	OVERTOPPING

*Handwritten note: H₀ = 2.12
0.012 for Det
Basin Outlet*

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3060-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.89	0.00	18	0	0.00
102.25	0.00	35	0	0.00
102.86	0.00	53	0	0.00
103.43	0.00	70	0	0.00
103.93	0.00	88	0	0.00
104.41	0.00	105	0	0.00
104.88	0.00	123	0	0.00
105.38	0.00	140	0	0.00
105.91	0.00	158	0	0.00
106.15	0.00	165	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:01:36

FILE DATE: 03-21-2000
FILE NAME: 3060-1

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 10.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.010
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 98.80
CULVERT NO.1 OUTLET INVERT ELEVATION 98.80 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	98.80	0.000	0.00	0.00
17.50	99.32	0.713	2.91	0.32
35.00	99.57	0.737	3.68	0.48
52.50	99.77	0.749	4.19	0.61
70.00	99.94	0.756	4.58	0.71
87.50	100.09	0.761	4.90	0.80
105.00	100.22	0.765	5.18	0.89
122.50	100.35	0.768	5.42	0.96
140.00	100.46	0.771	5.63	1.03
157.50	100.57	0.773	5.83	1.10
165.00	100.61	0.774	5.91	1.13

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 30.00
CREST LENGTH (FT) 110.00
OVERTOPPING CREST ELEVATION (FT) 110.00

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:03:30

FILE DATE: 03-21-2000
FILE NAME: 3060-2

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	8.00	4.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3060-2 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0		0 1
101.30	20	20	0	0	0	0	0		0 1
101.50	40	40	0	0	0	0	0		0 1
101.90	60	60	0	0	0	0	0		0 1
102.28	80	80	0	0	0	0	0		0 1
102.63	100	100	0	0	0	0	0		0 1
102.96	120	120	0	0	0	0	0		0 1
103.28	140	140	0	0	0	0	0		0 1
103.60	160	160	0	0	0	0	0		0 1
103.91	180	180	0	0	0	0	0		0 1
104.15	195	195	0	0	0	0	0		0 1
108.00	395	395	0	0	0	0	0	OVERTOPPING	

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3060-2 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.30	0.00	20	0	0.00
101.50	0.00	40	0	0.00
101.90	0.00	60	0	0.00
102.28	0.00	80	0	0.00
102.63	0.00	100	0	0.00
102.96	0.00	120	0	0.00
103.28	0.00	140	0	0.00
103.60	0.00	160	0	0.00
103.91	0.00	180	0	0.00
104.15	0.00	195	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:03:30

FILE DATE: 03-21-2000
FILE NAME: 3060-2

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 10.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.010
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
20.00	99.56	0.718	3.05	0.35
40.00	99.83	0.741	3.84	0.52
60.00	100.05	0.752	4.36	0.65
80.00	100.23	0.759	4.77	0.76
100.00	100.39	0.764	5.10	0.86
120.00	100.53	0.768	5.38	0.95
140.00	100.66	0.771	5.63	1.03
160.00	100.78	0.773	5.86	1.11
180.00	100.89	0.775	6.06	1.18
195.00	100.98	0.777	6.20	1.23

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 108.00

CURRENT DATE: 03-21-2000
CURRENT TIME: 13:53:01

FILE DATE: 03-21-2000
FILE NAME: 3110-1

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	5 RCB	15.00	6.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

***** SUMMARY OF CULVERT FLOWS (CFS) FILE: 3110-1 DATE: 03-21-2000 *****

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
102.45	340	340	0	0	0	0	0	0	1
102.74	680	680	0	0	0	0	0	0	1
103.02	1020	1020	0	0	0	0	0	0	1
103.32	1360	1360	0	0	0	0	0	0	1
103.86	1700	1700	0	0	0	0	0	0	1
104.36	2040	2040	0	0	0	0	0	0	1
104.84	2380	2380	0	0	0	0	0	0	1
105.31	2720	2720	0	0	0	0	0	0	1
105.77	3060	3060	0	0	0	0	0	0	1
106.13	3320	3320	0	0	0	0	0	0	1
115.00	7658	7658	0	0	0	0	0	0	1

***** SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3110-1 DATE: 03-21-2000 *****

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
102.45	0.00	340	0	0.00
102.74	0.00	680	0	0.00
103.02	0.00	1020	0	0.00
103.32	0.00	1360	0	0.00
103.86	0.00	1700	0	0.00
104.36	0.00	2040	0	0.00
104.84	0.00	2380	0	0.00
105.31	0.00	2720	0	0.00
105.77	0.00	3060	0	0.00
106.13	0.00	3320	0	0.00

<1> TOLERANCE (FT) = 0.010
<2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
PRINT TIME: 13:53:01

FILE DATE: 03-21-2000
FILE NAME: 3110-1

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 90.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.005
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

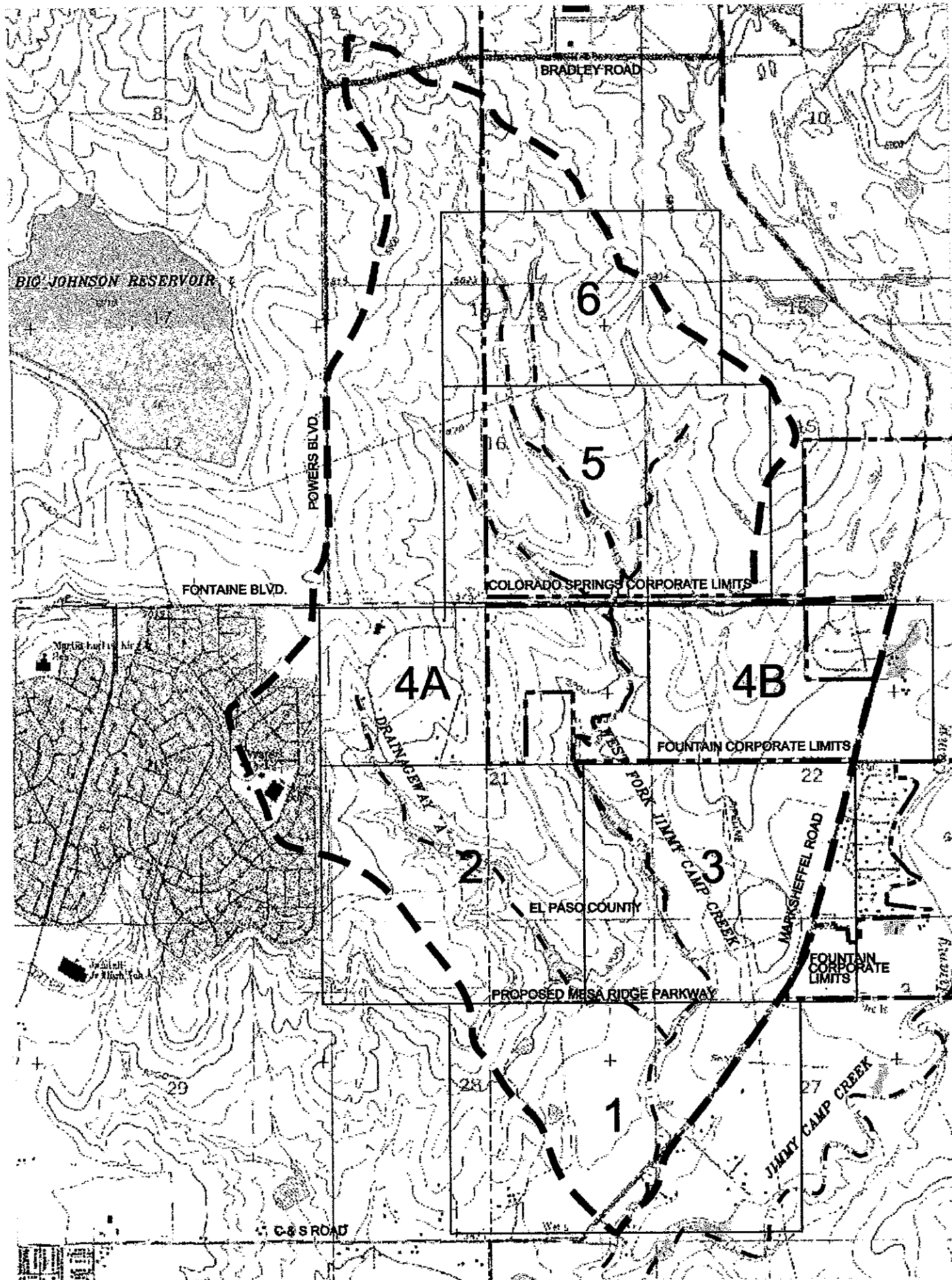
FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
340.00	100.04	0.606	3.51	0.32
680.00	100.57	0.642	4.57	0.49
1020.00	101.00	0.663	5.32	0.62
1360.00	101.37	0.677	5.91	0.74
1700.00	101.70	0.687	6.41	0.84
2040.00	102.01	0.696	6.85	0.94
2380.00	102.29	0.703	7.24	1.03
2720.00	102.56	0.708	7.59	1.11
3060.00	102.82	0.713	7.91	1.19
3320.00	103.00	0.717	8.14	1.25

***** ROADWAY OVERTOPPING DATA *****

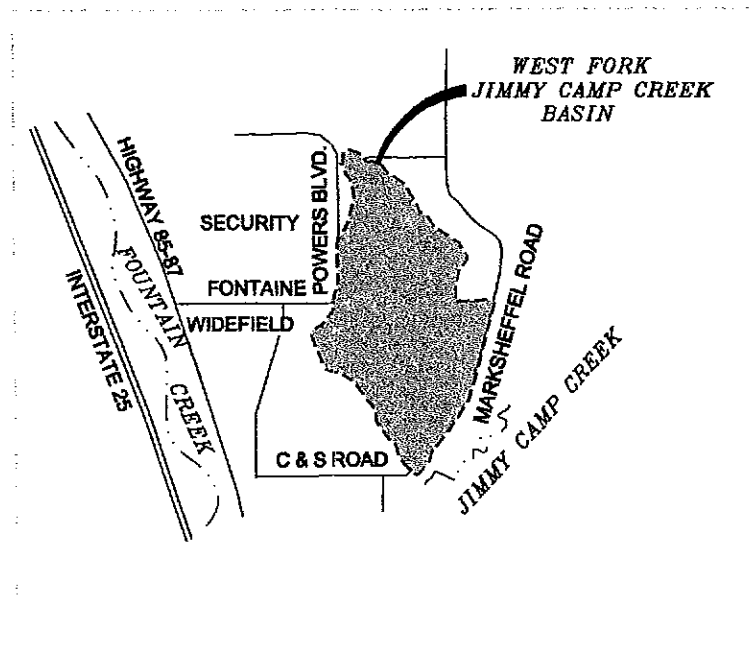
ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 115.00

**SELECTED PLAN DRAWINGS
SHEETS 1 - 7**

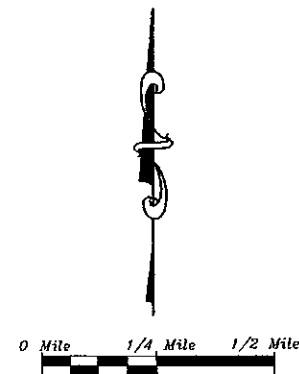
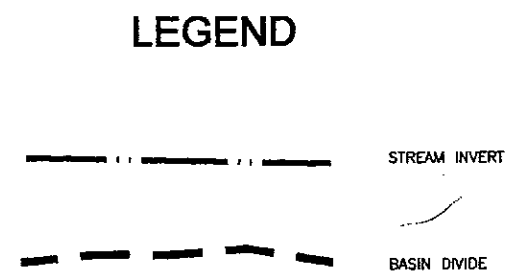
APPENDIX C



INDEX MAP
NTS



VICINITY MAP
NTS

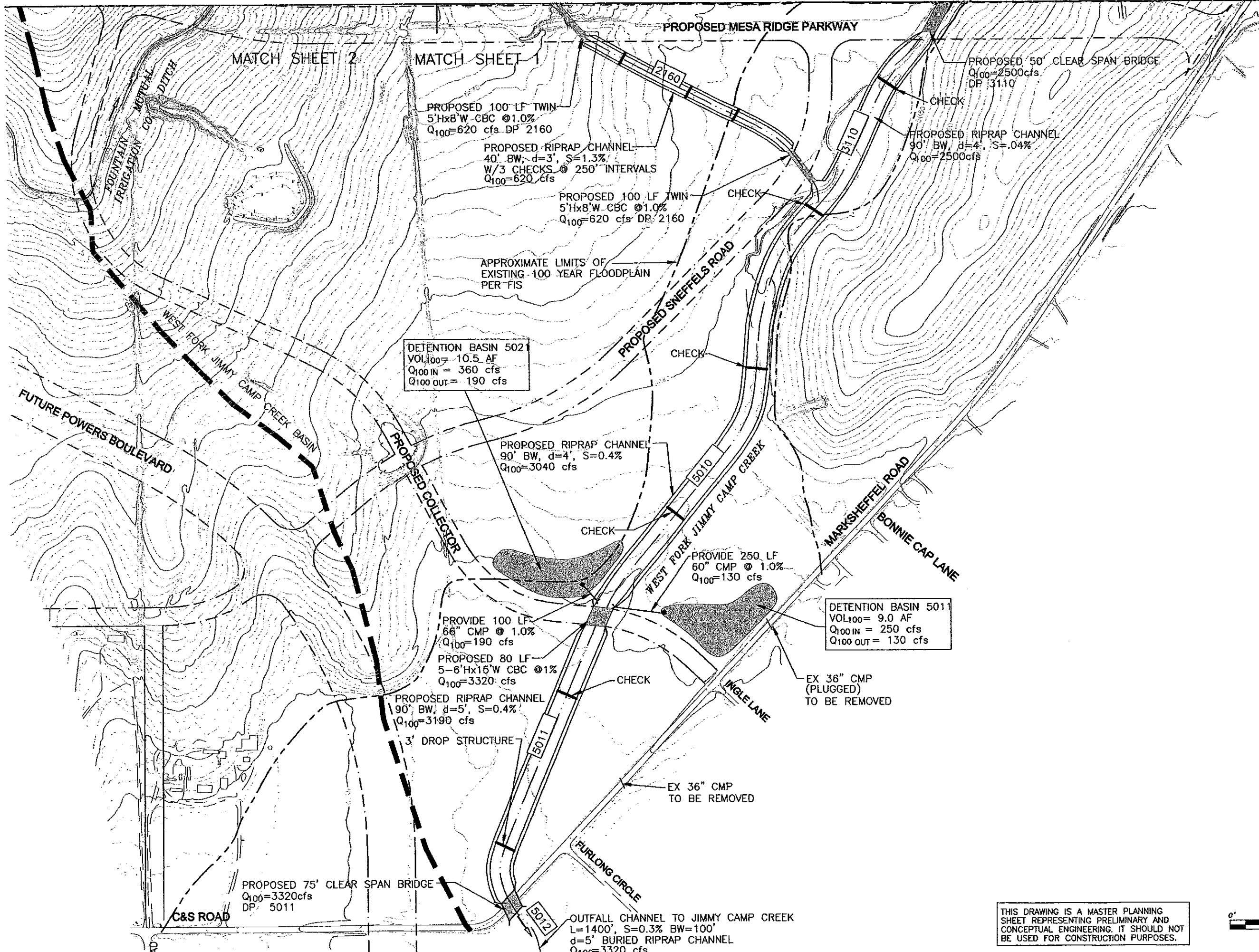


Kiowa Engineering Corporation
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 Colorado Springs, Colorado
 80910-3127
 (719) 630-7342

**WEST FORK JIMMY CAMP CREEK
 DRAINAGE BASIN PLANNING STUDY**
 PRELIMINARY PLAN
 EL PASO COUNTY, COLORADO

Project No.: 9893
Date: 2/00
Design: RNW
Drawn: CAD
Check: RNW
Revisions:

INDEX

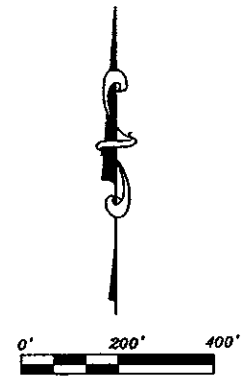


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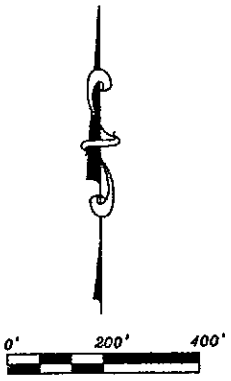
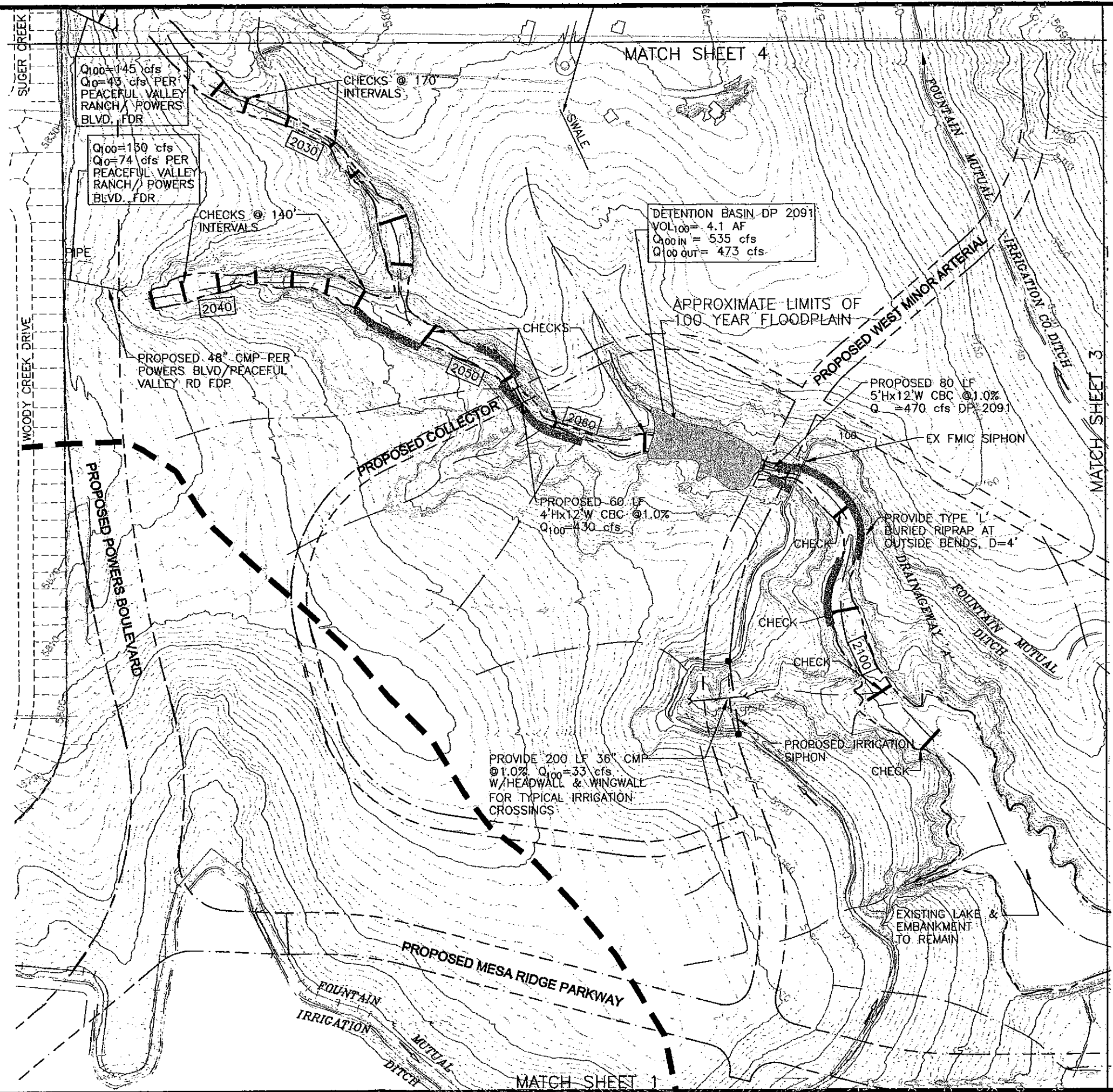
**WEST FORK JIMMY CAMP CREEK
 DRAINAGE BASIN PLANNING STUDY
 PRELIMINARY PLAN
 EL PASO COUNTY, COLORADO**

Project No.:	8893
Date:	7/00
Design:	RNW
Drawn:	CAD
Check:	RNW
Revisions:	

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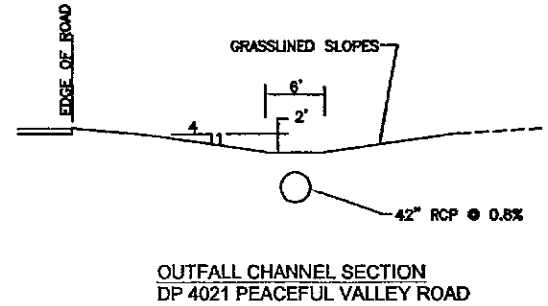
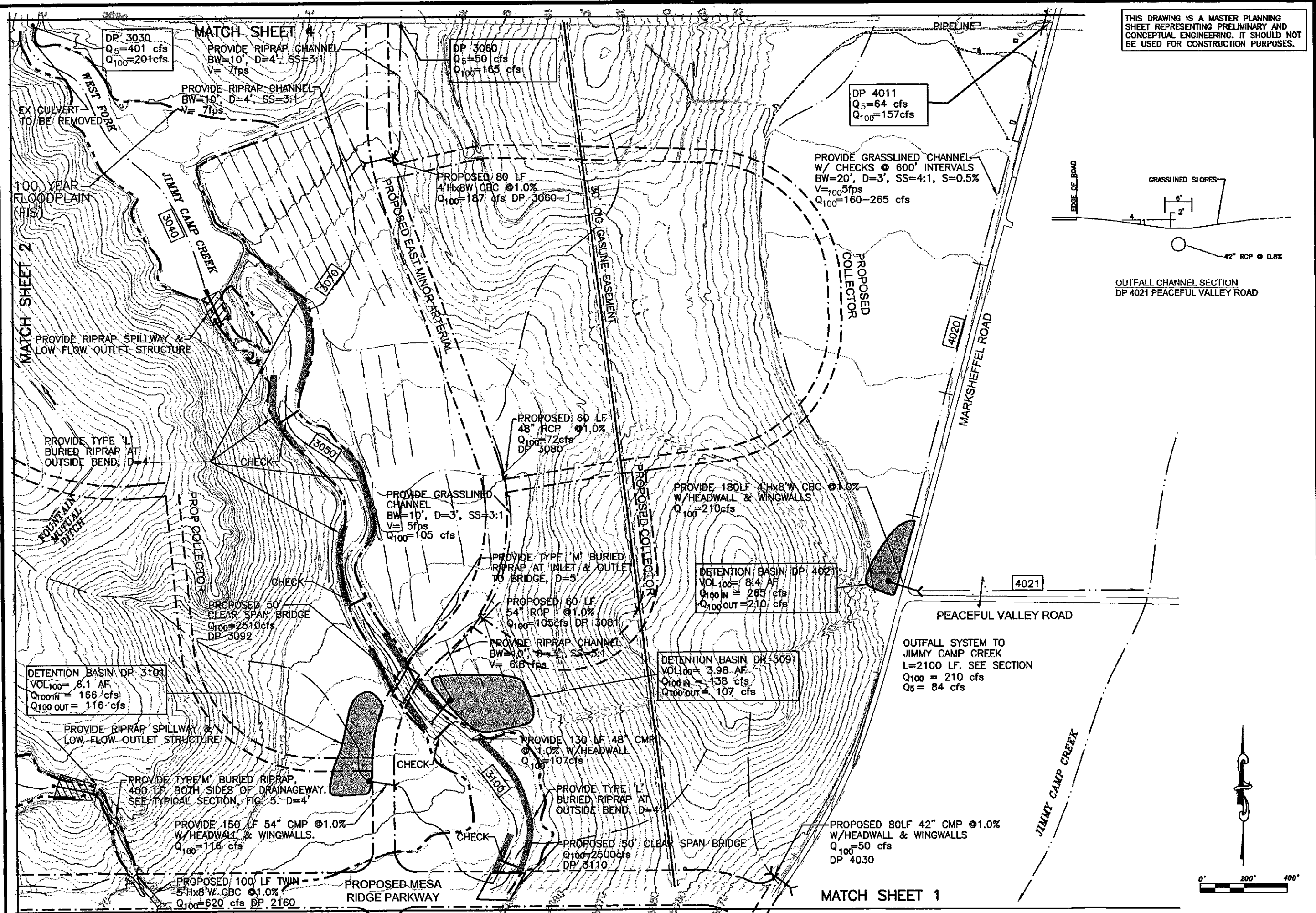


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WEST FORK JIMMY CAMP CREEK
DRAINAGE BASIN PLANNING STUDY
 PRELIMINARY PLAN
 EL PASO COUNTY, COLORADO

Project No.:	9893
Date:	7/00
Design:	RNW
Drawn:	CAD
Check:	RNW
Revisions:	

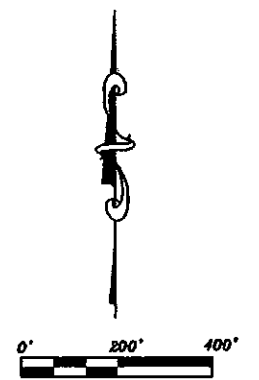
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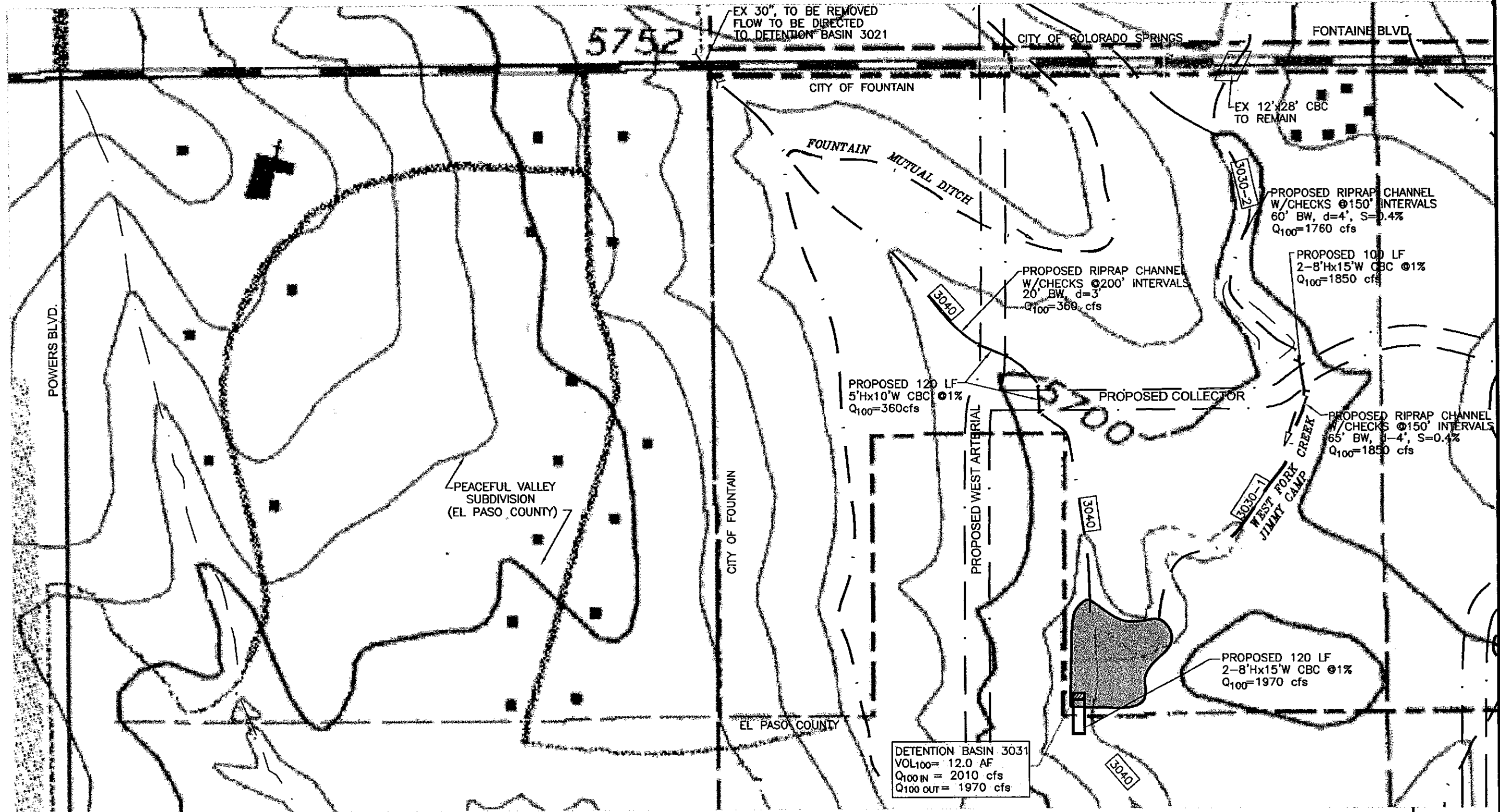


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**WEST FORK JIMMY CAMP CREEK
 DRAINAGE BASIN PLANNING STUDY
 PRELIMINARY PLAN
 EL PASO COUNTY, COLORADO**

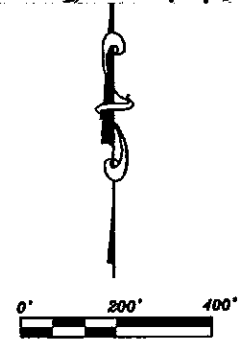
Project No.:	9893
Date:	7/00
Designer:	RNW
Drawer:	CAD
Checker:	RNW
Reviewer:	





DETENTION BASIN 3031
 VOL₁₀₀ = 12.0 AF
 Q_{100 IN} = 2010 cfs
 Q_{100 OUT} = 1970 cfs

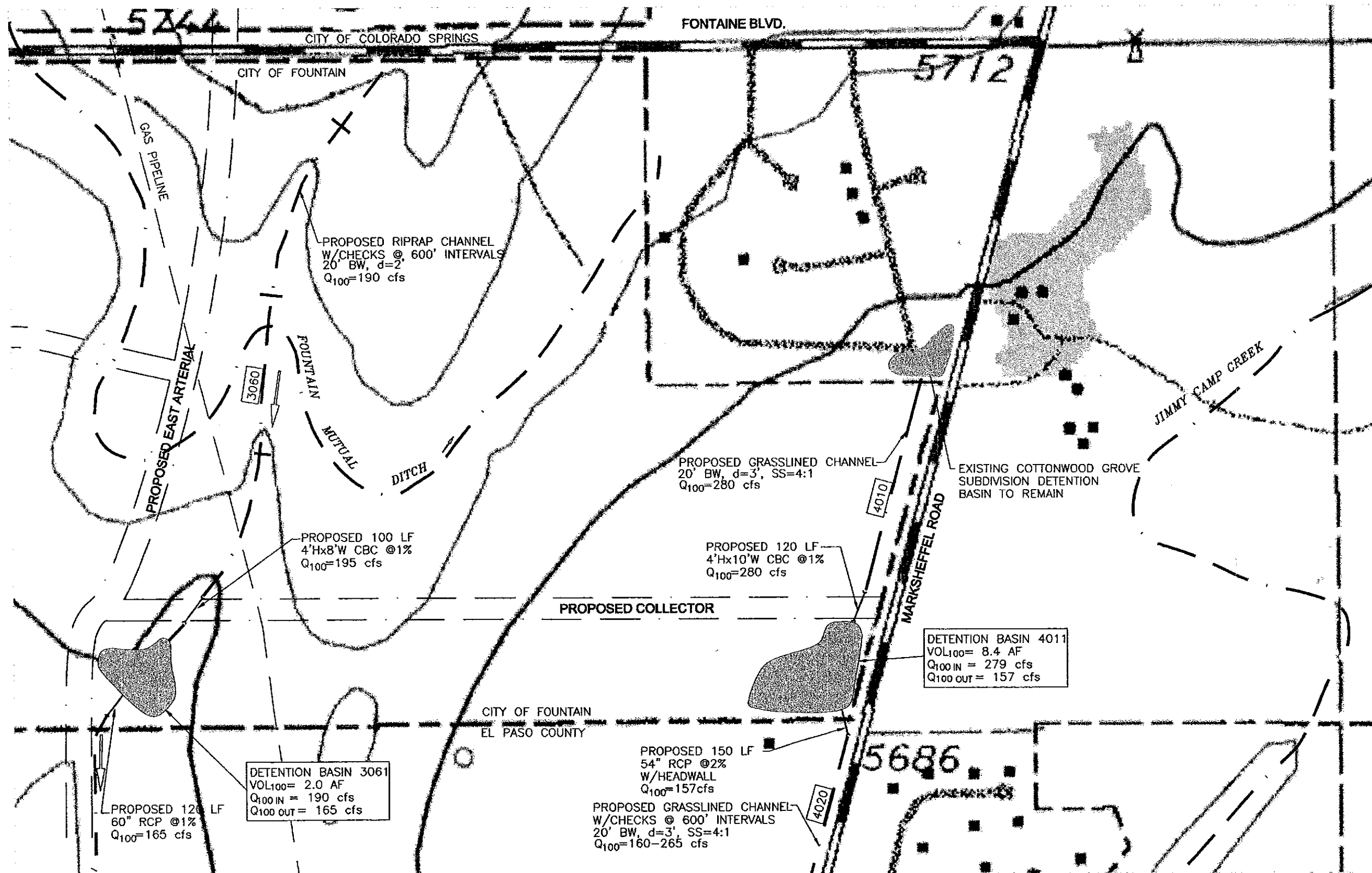
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**WEST FORK JIMMY CAMP CREEK
 DRAINAGE BASIN PLANNING STUDY**
 PRELIMINARY PLAN
 EL PASO COUNTY, COLORADO

Project No.:	9893
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Drawer:	CAD
Checker:	RNW
Revisions:	

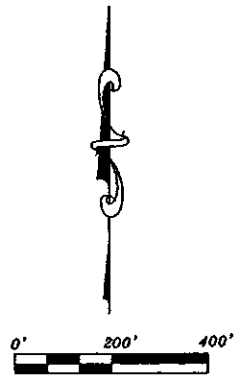


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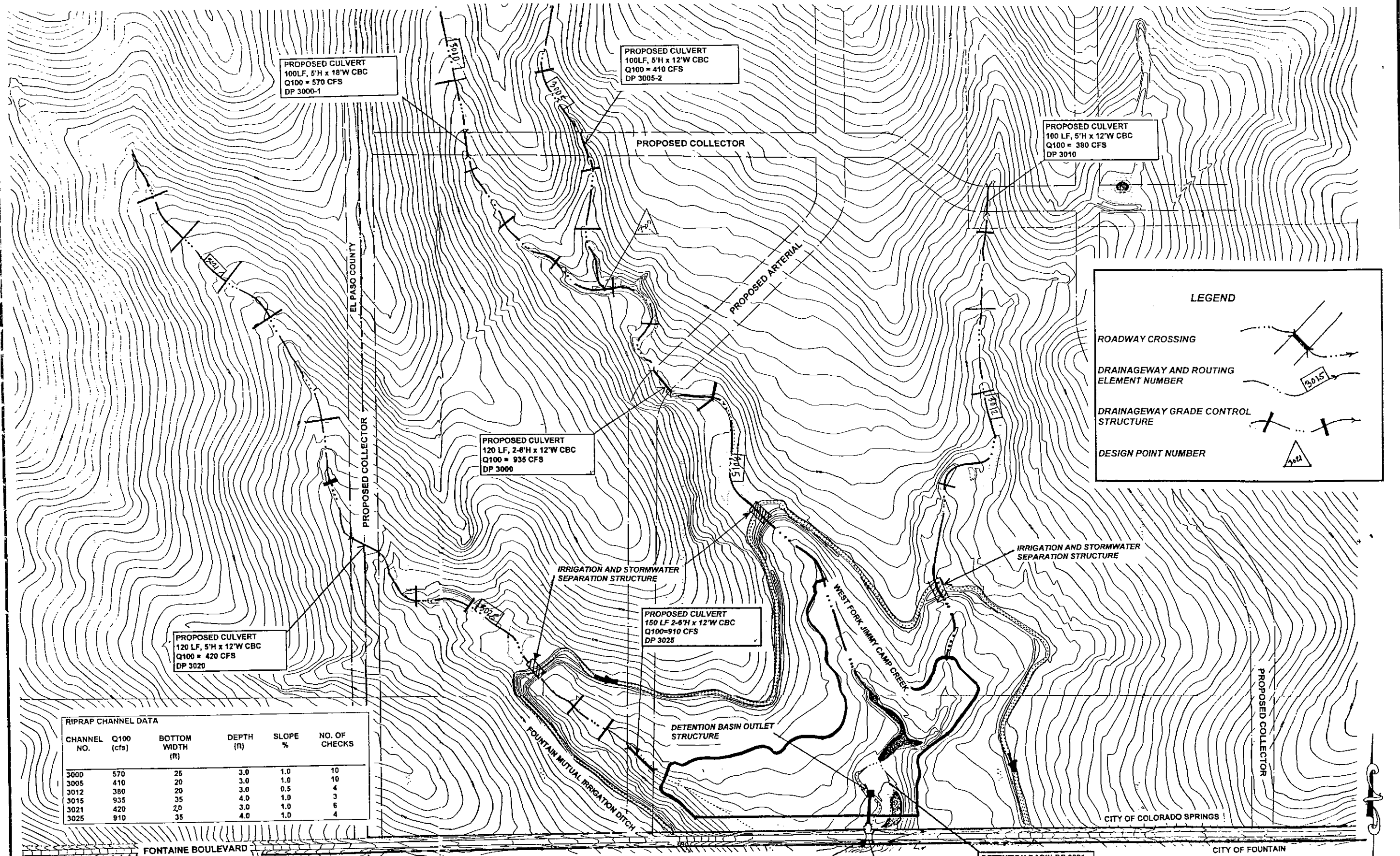
**WEST FORK JIMMY CAMP CREEK
DRAINAGE BASIN PLANNING STUDY**
PRELIMINARY PLAN
EL PASO COUNTY, COLORADO

Project No.: 9893
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PROPOSED CULVERT
100LF, 5'H x 18'W CBC
Q100 = 570 CFS
DP 3000-1

PROPOSED CULVERT
100LF, 5'H x 12'W CBC
Q100 = 410 CFS
DP 3005-2

PROPOSED CULVERT
100 LF, 5'H x 12'W CBC
Q100 = 380 CFS
DP 3010

PROPOSED CULVERT
120 LF, 2-6'H x 12'W CBC
Q100 = 935 CFS
DP 3000

PROPOSED CULVERT
120 LF, 5'H x 12'W CBC
Q100 = 420 CFS
DP 3020

PROPOSED CULVERT
150 LF 2-6'H x 12'W CBC
Q100=910 CFS
DP 3025

DETENTION BASIN DP 3021
VOL100 = 80 AF
Q100 IN = 2,740 CFS
Q100 OUT = 1,810 CFS

LEGEND

- ROADWAY CROSSING
- DRAINAGEWAY AND ROUTING ELEMENT NUMBER
- DRAINAGEWAY GRADE CONTROL STRUCTURE
- DESIGN POINT NUMBER

RIPRAP CHANNEL DATA

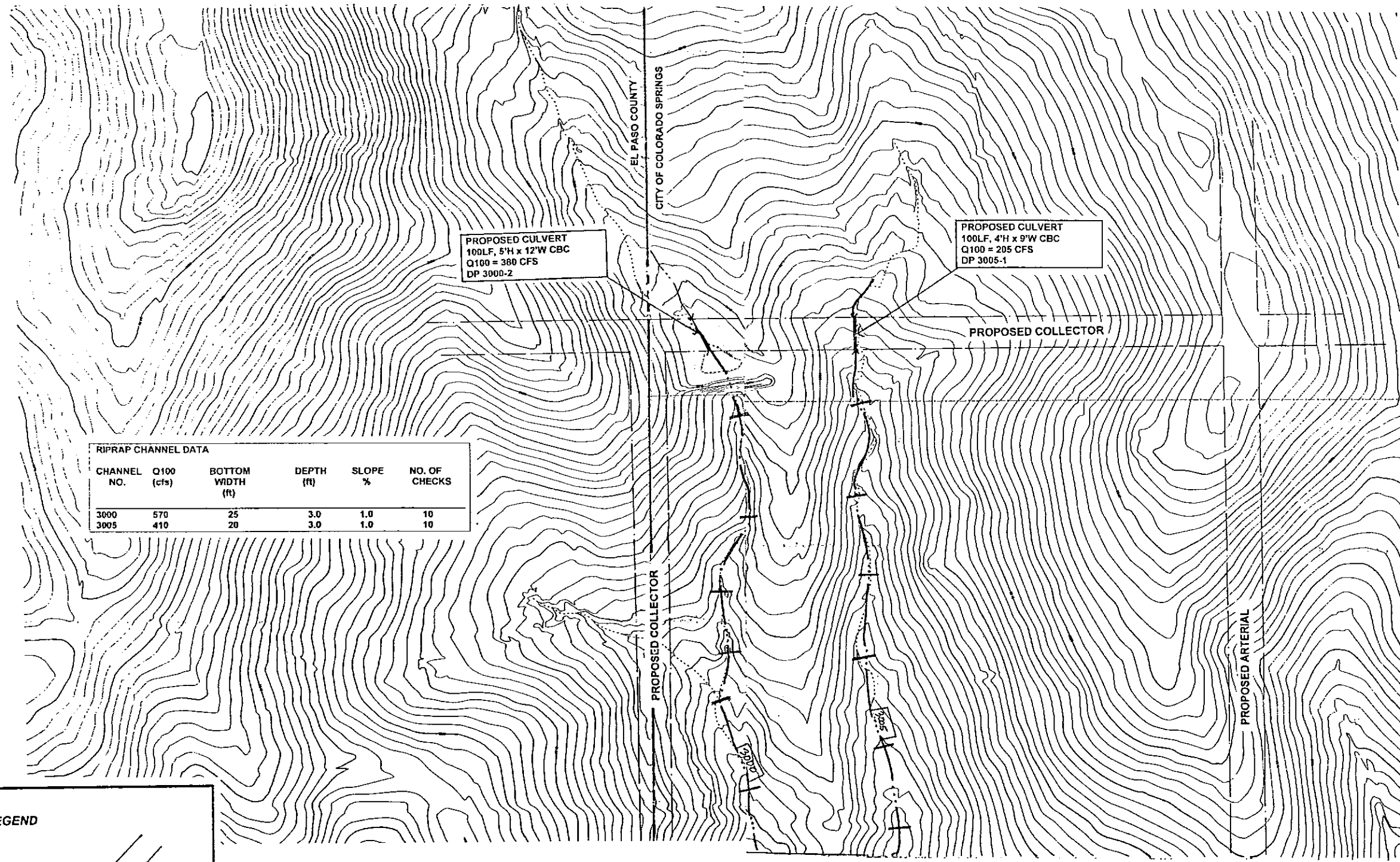
CHANNEL NO.	Q100 (cfs)	BOTTOM WIDTH (ft)	DEPTH (ft)	SLOPE %	NO. OF CHECKS
3000	570	25	3.0	1.0	10
3005	410	20	3.0	1.0	10
3012	380	20	3.0	0.5	4
3015	935	35	4.0	1.0	3
3021	420	20	3.0	1.0	6
3025	910	35	4.0	1.0	4

Kiowa Engineering Corporation
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Colorado Springs, Colorado
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**WEST FORK JIMMY CAMP CREEK
DRAINAGE BASIN PLANNING STUDY**
PRELIMINARY PLAN
EL PASO COUNTY, COLORADO

Project No.: 9893
Date: 2/00
Design: RNW
Drawn: CAD
Check: RNW
Revisions:

THIS DRAWING IS A MASTER PLANNING SHEET REPRESENTING PRELIMINARY AND CONCEPTUAL ENGINEERING. IT SHOULD NOT BE USED FOR CONSTRUCTION PURPOSES.



RIPRAP CHANNEL DATA

CHANNEL NO.	Q100 (cfs)	BOTTOM WIDTH (ft)	DEPTH (ft)	SLOPE %	NO. OF CHECKS
3000	570	25	3.0	1.0	10
3005	410	20	3.0	1.0	10

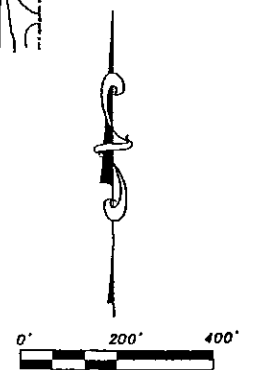
LEGEND

ROADWAY CROSSING

DRAINAGEWAY AND ROUTING ELEMENT NUMBER

DRAINAGEWAY GRADE CONTROL STRUCTURE

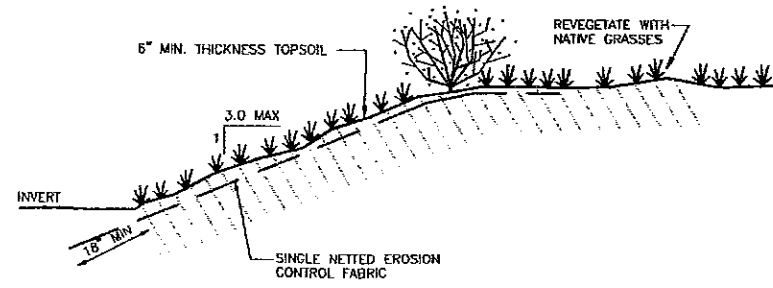
DESIGN POINT NUMBER



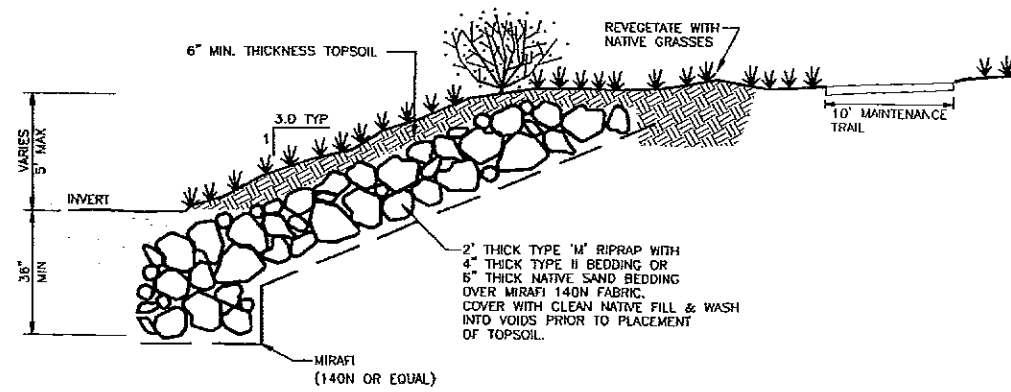
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**WEST FORK JIMMY CAMP CREEK
 DRAINAGE BASIN PLANNING STUDY**
 PRELIMINARY PLAN
 EL PASO COUNTY, COLORADO

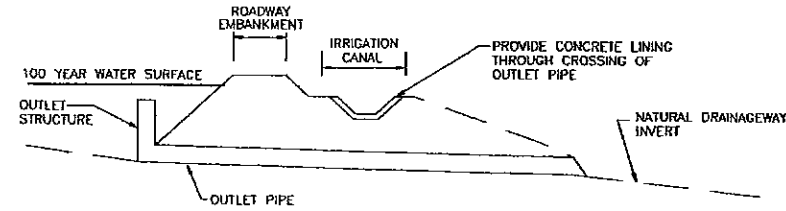
Project No.:	9893
Date:	2/00
Design:	RNW
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Check:	RNW
Revisions:	



GRASSLINED BANK DETAIL

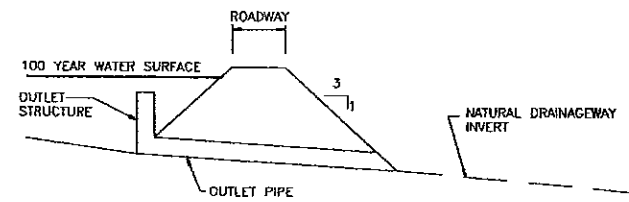


RIPRAP BANK LINING DETAIL
NTS



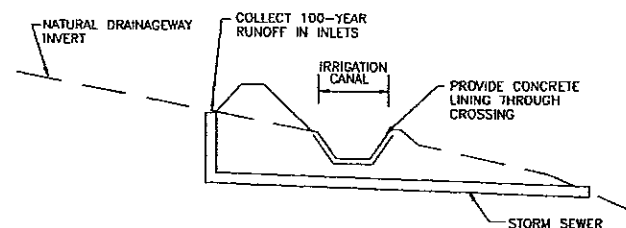
IRRIGATION CANAL CROSSING
AT DETENTION BASIN

(NTS)



IRRIGATION CROSSING WITH SIPHON
AT DETENTION BASIN

(NTS)



IRRIGATION CROSSING
AT SEWER CROSSING

(NTS)

RIPRAP GRADATIONS

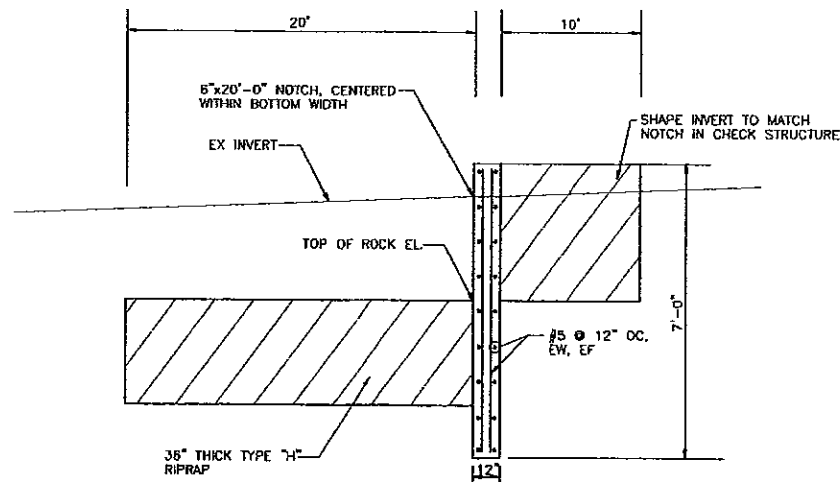
TYPE H RIPRAP INTERMEDIATE ROCK DIMENSION IN INCHES	% SMALLER THAN GIVEN SIZE BY WEIGHT	D 50 INCHES
30	100	18
24	50-70	
18	35-50	
6	2-10	

TYPE M RIPRAP INTERMEDIATE ROCK DIMENSION IN INCHES	% SMALLER THAN GIVEN SIZE BY WEIGHT	D 50 INCHES
21	100	12
18	50-70	
12	35-50	
4	2-10	

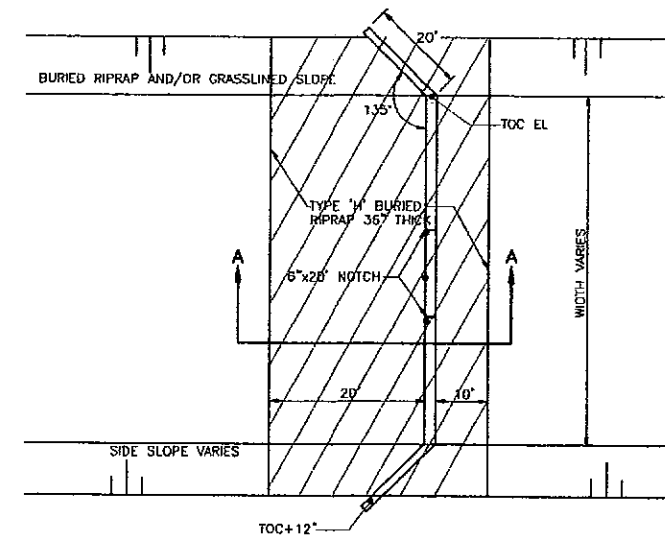
SEED MIX

AREAS DISTURBED BY THE EARTHWORK SHALL BE PERMANENTLY REVEGETATED WITH NATIVE GRASSES. NATIVE SEED MIX FOR THIS PROJECT SHALL BE AS FOLLOWS:

NATIVE SEED MIX		pls/acre
BLUE GRAMA	<i>Chondrosium hirsutum</i>	2.0
SIDEONIS GRAMA	<i>Bouteloua curtipendula</i>	3.0
SLENDER WHEATGRASS	<i>Agropyron trachypodium</i>	2.0
WESTERN WHEATGRASS	<i>Agropyron smithii</i>	4.0
		11.0 lbs



SECTION A-A



TYPICAL CHECK STRUCTURE PLAN

NO SCALE

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West Fork Jimmy Camp Creek
Drainage Basin Planning Study
TYPICAL DRAINAGEWAY DETAILS
EL PASO COUNTY, COLORADO

Project No.: 9893
Date: 7/00
Design: RNW
Drawn: CAD
Check: RNW
Revisions:

SHEET 7