

TECHNICAL ADDENDUM
FOR
WEST FORK JIMMY CAMP CREEK
DRAINAGE BASIN PLANNING STUDY

Prepared for:

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Colorado Springs, CO 80911

Prepared by:

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KIOWA Project No. 98.93
wfc*.doc

June 1999
July 2000
November 2000
October 17, 2003

**HEC-1 HYDROGRAPH PACKAGE
HYDROLOGIC CALCULATIONS**

APPENDIX A

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1*****
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* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
CORPS OF ENGINEERS *
* SEPTEMBER 1990 *
ENGINEERING CENTER *
* VERSION 4.0 *
SECOND STREET *
*
CALIFORNIA 95616 *
* RUN DATE 10/19/1999 TIME 12:39:04 *
(916) 756-1104 *
*
*****
*****

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*
* U.S. ARMY
* HYDROLOGIC
* 609
* DAVIS,
*
*

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1
PAGE 1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	WEST JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY									
2	ID	KIOWA ENGINEERING - PROJECT NO. 98.93									
3	ID	2, 5, 10 & 100 YEAR STORMS FILENAME: WFJCEX.DAT EXISTING BASIN CONDITIONS									
4	ID	24HR STORM DURATION									
	*DIAGRAM										
5	IT	5	0	0	250						
6	IO	5									
7	JR	PREC	.47	.56	.70	1					
8	KK	E1010									
9	BA	.05									
10	IN	15									
11	PB	4.4									
12	PC	0.0	.0005	.0015	.0030	.0045	.0060	.0080	.0100	.0120	.0143
13	PC	.0165	.0188	.0210	.0233	.0255	.0278	.0320	.0390	.0460	.0530
14	PC	.0600	.0750	.1000	.4000	.7000	.7250	.7500	.7650	.7800	.7900
15	PC	.8000	.8100	.8200	.8250	.8300	.8350	.8400	.8450	.8500	.8550
16	PC	.8600	.8638	.8675	.8713	.8750	.8788	.8825	.8863	.8900	.8938
17	PC	.8975	.9013	.9050	.9083	.9115	.9148	.9180	.9210	.9240	.9270


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73      KK  E2045
74      KM      RUNOFF FROM BAS 2045
75      KO      0
76      BA      .061
77      LS      0      85
78      UD      .258

79      KK  R2040
80      KM      ROUTE FLOW FROM E2045 TO DP2040
81      RK      1200 .0314 .04      TRAP      20      8

82      KK  E2040
83      KM      RUNOFF FROM BAS 2040
84      KO      0
85      BA      .026
86      LS      0      69
87      UD      .268

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HEC-1 INPUT

1
PAGE 3

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88      KK  DP2040
89      KM      COMBINE FLOW FROM DP2020, E2030, E2045, E2040
90      HC      4

91      KK  R2050
92      KM      ROUTE FLOW FROM DP2040 TO DP2060
93      RK      600 .03 .03      TRAP      20      3

94      KK  E2050
95      KM      RUNOFF FROM BAS 2050
96      KO      0
97      BA      .020
98      LS      0      69
99      UD      .33

100     KK  E2060
101     KM      RUNOFF FROM BAS 2060
102     KO      0
103     BA      .024
104     LS      0      69
105     UD      .253

106     KK  DP2060
107     KM      COMBINE FLOW FROM DP2040, E2050, E2060
108     HC      3

109     KK  R2090
110     KM      ROUTE FLOW FROM DP2060 TO DP2090
111     RK      500 .023 .03      TRAP      30      3

112     KK  E2070
113     KM      RUNOFF FROM BAS 2070
114     KO      0
115     BA      .068
116     LS      0      68
117     UD      .463

118     KK  R2080
119     KM      ROUTE FLOW FROM E2070 TO DP2080
120     RK      1220 .035 .04      TRAP      10      5

121     KK  E2080
122     KM      RUNOFF FROM BAS 2080
123     KO      0
124     BA      .057
125     LS      0      69
126     UD      .247

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127 KK DP2080
 128 KM COMBINE FLOW FROM E2070, E2080
 129 HC 2 HEC-1 INPUT

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 PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

130 KK E2090
 131 KM RUNOFF FROM BAS 2090
 132 KO 0
 133 BA .019
 134 LS 0 69
 135 UD .414

136 KK DP2090
 137 KM COMBINE FLOW FROM DP2060, DP2080, E2090
 138 HC 3

139 KK R2100
 140 KM ROUTE FLOW FROM DP2090 TO DP2100
 141 RK 1800 .0169 .03 TRAP 60 4

142 KK E2110
 143 KM RUNOFF FROM BAS 2110
 144 KO 0
 145 BA .034
 146 LS 0 69
 147 UD .329

148 KK R2101
 149 KM ROUTE FLOW FROM E2110 TO DP2100
 150 RK 900 .025 .04 TRAP 40 5

151 KK E2100
 152 KM RUNOFF FROM BAS 2100
 153 KO 0
 154 BA .095
 155 LS 0 69
 156 UD .482

157 KK DP2100
 158 KM COMBINE FLOW FROM DP2090, E2110, E2100
 159 HC 3

160 KK E2120
 161 KM RUNOFF FROM BAS 2120
 162 KO 0
 163 BA .047
 164 LS 0 69
 165 UD .288

166 KK DP2120
 167 KM COMBINE FLOW FROM DP2100, E2120
 168 HC 2

169 KK R2120
 170 KM ROUTE FLOW FROM DP2120 THROUGH LAKE
 171 RK 1000 .025 .04 TRAP 40 5
 HEC-1 INPUT

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 PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

172 KK R2130
 173 KM ROUTE FLOW FROM DP2120 TO DP2130
 174 RK 550 .05 .04 TRAP 8 3

175 KK E2130

231	KK	E3010								
232	KM		RUNOFF FROM BAS 3010							
233	KO		0							
234	BA		.22							
235	LS		0	75.1						
236	UD		.28							
237	KK	R3012								
238	KM		ROUTE FLOW FROM E3010 TO DP 3020							
239	RK	2600	.03	.04		TRAP	15		3	
240	KK	E3012								
241	KM		RUNOFF FROM BASIN E3012							
242	KO		0							
243	BA		.21							
244	LS		0	75.1						
245	UD		.47							
246	KK	E3020								
247	KM		RUNOFF FROM BAS 3020							
248	KO		0							
249	BA		.188							
250	LS		0	77						
251	UD		.36							
252	KK	R3025								
253	KM		ROUTE FLOW FROM BASIN E3020 TO DP 3020							
254	RK	2600	.035	.04		TRAP	15		3	
						HEC-1 INPUT				

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LINE	ID	1	2	3	4	5	6	7	8	9	10
255	KK	E3025									
256	KM		RUNOFF FROM BASIN E3025								
257	KO		0								
258	BA		.26								
259	LS		0	72.2							
260	UD		.23								
261	KK	DP3020									
262	KM		COMBINE FLOW FROM E3015, E3012, E3025, R3015, R3012 AND R3025								
263	HC		6								
264	KK	R3030									
265	KM		ROUTE FLOW FROM DP3020 TO DP3030								
266	RK	3000	.05	.04		TRAP	20		10		
267	KK	E3030									
268	KM		RUNOFF FROM BAS 3030								
269	KO		0								
270	BA		.26								
271	LS		0	72.2							
272	UD		.34								
273	KK	E3035									
274	KM		RUNOFF FROM BASIN E3035								
275	KO		0								
276	BA		.16								
277	LS		0	72.2							
278	UD		.16								
279	KK	DP3030									
280	KM		COMBINE FLOW FROM R3030, E3030 AND E3035								
281	HC		3								
282	KK	R3040									
283	KM		ROUTE FLOW FROM DP 3030 TO DP 3040								
284	RK	1450	.03	.03		TRAP	30		5		

285	KK	E3040								
286	KM		RUNOFF FROM BAS 3040							
287	KO		0							
288	BA		.115							
289	LS		0	69.3						
290	UD		.294							
291	KK	DP3040								
292	KM		COMBINE FLOW FROM R3040, E3040							
293	HC		2							
294	KK	R3050								
295	KM		ROUTE FLOW FROM DP3040 TO DP3050							
296	RK	2850	.009	.03		TRAP	10		7	
						HEC-1 INPUT				

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

297	KK	E3050								
298	KM		RUNOFF FROM BAS 3050							
299	KO		0							
300	BA		.049							
301	LS		0	77.5						
302	UD		.371							
303	KK	DP3050								
304	KM		COMBINE FLOW FROM DP3040, E3050							
305	HC		2							
306	KK	E3060								
307	KM		RUNOFF FROM BAS 3060							
308	KO		0							
309	BA		.119							
310	LS		0	75.4						
311	UD		.257							
312	KK	R3070								
313	KM		ROUTE FLOW FROM E3060 TO DP3070							
314	RK	2000	.009	.04		TRAP	30		10	
315	KK	E3070								
316	KM		RUNOFF FROM BAS 3070							
317	KO		0							
318	BA		.077							
319	LS		0	77.1						
320	UD		.486							
321	KK	DP3070								
322	KM		COMBINE FLOW FROM R3070, E3070							
323	HC		2							
324	KK	R3080								
325	KM		ROUTE FLOW FROM DP3070 TO DP3080							
326	RK	350	.01	.04		TRAP	30		10	
327	KK	E3080								
328	KM		RUNOFF FROM BAS 3080							
329	KO		0							
330	BA		.050							
331	LS		0	75.4						
332	UD		.355							
333	KK	DP3080								
334	KM		COMBINE FLOW FROM R3080, E3080							
335	HC		2							
336	KK	R3090								
337	KM		ROUTE FLOW FROM DP3080 TO DP3090							

338	RK	1750	.01	.03	TRAP	30	10		
					HEC-1 INPUT				
LINE	ID12345678
	910						
339	KK	E3090							
340	KM		RUNOFF FROM BAS 3090						
341	KO	0							
342	BA	.082							
343	LS	0	76.85						
344	UD	.411							
345	KK	DP3090							
346	KM		COMBINE FLOW FROM DP3090, E3090						
347	HC	2							
348	KK	DP3091							
349	KM		COMBINE FLOW FROM DP3050, DP3090						
350	HC	2							
351	KK	R3100							
352	KM		ROUTE FLOW FROM DP3091 TO DP3100						
353	RK	350	.01	.03	TRAP	10	10		
354	KK	E3100							
355	KM		RUNOFF FROM BAS 3100						
356	KO	0							
357	BA	.095							
358	LS	0	75.75						
359	UD	.303							
360	KK	DP3100							
361	KM		COMBINE FLOW FROM DP3090, E3100						
362	KO	0							
363	HC	2							
364	KK	E3110							
365	KM		RUNOFF FROM BAS 3110						
366	KO	0							
367	BA	.018							
368	LS	0	75.15						
369	UD	.472							
370	KK	DP3110							
371	KM		COMBINE FLOW FROM E3110 AND DP3100						
372	HC	2							
373	KK	R5010							
374	KM		ROUTE FLOW FROM DP3110 TO DP5010						
375	RK	2900	.015	.04	TRAP	20	4		
376	KK	E5011							
377	KM		RUNOFF FROM BAS E5010						
378	KO	0							
379	BA	.156							
380	LS	0	74						
381	UD	.50							
					HEC-1 INPUT				

LINE	ID12345678
	910						
382	KK	DP5010							
383	KM		COMBINE FLOW FROM E5010, R5011, R5010						
384	HC	3							
385	KK	E5020							
386	KM		RUNOFF FROM BASIN 5020						

387	KO	0							
388	BA	.2							
389	LS	0	74						
390	UD	.4							
391	KK	E4010							
392	KM		RUNOFF FROM BAS 4010						
393	KO	0							
394	BA	.19							
395	LS	0	72.75						
396	UD	.497							
397	KK	R4020							
398	KM		ROUTE FLOW FROM E4010 TO DP4020						
399	RK	2400	.005	.05	TRAP	10	30		
400	KK	E4020							
401	KM		RUNOFF FROM BAS 4020						
402	KO	0							
403	BA	.135							
404	LS	0	76.65						
405	UD	.822							
406	KK	DP4020							
407	KM		COMBINE FLOW FROM R4020, E4020						
408	HC	2							
409	KK	E4030							
410	KM		RUNOFF FROM BAS 4030						
411	KO	0							
412	BA	.018							
413	LS	0	75.65						
414	UD	.251							
415	ZZ								

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT	(V) ROUTING	(--->)	DIVERSION OR PUMP FLOW
LINE	(.) CONNECTOR	(<---)	RETURN OF DIVERTED OR PUMPED FLOW
NO.			
8	E1010		
	.		
24	.	E1020	
	.	.	
30	.	.	E1030
	.	.	.
36	.	DP1030.....	
	.	.	
39	.	.	E1040
	.	.	.
45	.	.	E2010
	.	.	V
51	.	.	V
	.	.	R2020
	.	.	.
55	.	.	.
	.	.	E2020
	.	.	.
61	.	.	DP2020.....
	.	.	V
	.	.	V
64	.	.	R2030
	.	.	.
	.	.	.

67	E2030	.

73	E2045
	V
	V
79	R2040

82	E2040

88	DP2040.....	.
	V	.
	V	.
91	R2050	.

94	E2050	.

100	E2060

106	DP2060.....	.
	V	.
	V	.
109	R2090	.

112	E2070	.
	V	.
	V	.
118	R2080	.

121	E2080

127	DP2080.....	.

130	E2090

136	DP2090.....	.
	V	.
	V	.
139	R2100	.

142	E2110	.
	V	.
	V	.
148	R2101	.

151	E2100

157	DP2100.....	.

160	E2120	.

166	DP2120.....	.
	V	.
	V	.
169	R2120	.
	V	.

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172      .      .      .      .      V
      .      .      .      .      R2130
175      .      .      .      .      .
      .      .      .      .      E2130
181      .      .      .      .      .
      .      .      .      .      DP2130.....
184      .      .      .      .      .
      .      .      .      .      E2140
190      .      .      .      .      .
      .      .      .      .      E2150
196      .      .      .      .      .
      .      .      .      .      E2160
202      .      .      .      .      .
      .      .      .      .      DP2160.....
      .      .      .      .      V
204      .      .      .      .      V
      .      .      .      .      R5010
207      .      .      .      .      .
      .      .      .      .      E3000
213      .      .      .      .      .
      .      .      .      .      E3005
219      .      .      .      .      .
      .      .      .      .      DP3000.....
      .      .      .      .      V
222      .      .      .      .      V
      .      .      .      .      R3015
225      .      .      .      .      .
      .      .      .      .      E3015
231      .      .      .      .      .
      .      .      .      .      E3010
237      .      .      .      .      .
      .      .      .      .      V
      .      .      .      .      R3012
240      .      .      .      .      .
      .      .      .      .      E3012
246      .      .      .      .      .
E3020      .      .      .      .      .
V      .      .      .      .      .
V      .      .      .      .      .
252      .      .      .      .      .
R3025      .      .      .      .      .
*** HEC1 ERROR 5 *** TOO MANY HYDROGRAPHS. COMBINE MORE OFTEN.
255      .      .      .      .      .
E3025      .      .      .      .      .
.      .      .      .      .
.      .      .      .      .
261      .      .      .      .      .
DP3020.....
      .      .      .      .      V
264      .      .      .      .      V
      .      .      .      .      R3030

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267	E3030	.

273	E3035

279	.	.	.	DP3030
	.	.	.	V		.
	.	.	.	V		.
282	.	.	.	R3040		.

285	E3040	.

291	.	.	.	DP3040
	.	.	.	V		.
	.	.	.	V		.
294	.	.	.	R3050		.

297	E3050	.

303	.	.	.	DP3050

306	E3060	.
	V	.
	V	.
312	.	.	.	R3070		.

315	E3070

321	.	.	.	DP3070
	.	.	.	V		.
	.	.	.	V		.
324	.	.	.	R3080		.

327	E3080

333	.	.	.	DP3080
	.	.	.	V		.
	.	.	.	V		.
336	.	.	.	R3090		.

339	E3090

345	.	.	.	DP3090

348	.	.	.	DP3091
	.	.	.	V		.
	.	.	.	V		.
351	.	.	.	R3100		.

354	E3100	.

360	.	.	.	DP3100


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364      .      .      .      .      .      E3110
      .      .      .      .      .      .
370      .      .      .      .      .      DP3110.....
      .      .      .      .      .      V
      .      .      .      .      .      V
373      .      .      .      .      .      R5010
      .      .      .      .      .      .
376      .      .      .      .      .      E5011
      .      .      .      .      .      .
382      .      .      .      .      .      DP5010.....
      .      .      .      .      .      .
385      .      .      .      .      .      E5020
      .      .      .      .      .      .
391      .      .      .      .      .      E4010
      .      .      .      .      .      V
      .      .      .      .      .      V
397      .      .      .      .      .      R4020
      .      .      .      .      .      .
400      .      .      .      .      .      .      E4020
      .      .      .      .      .      .      .
406      .      .      .      .      .      .      DP4020.....
      .      .      .      .      .      .      .
409      .      .      .      .      .      .      E4030

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(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION				
				RATIO 1	RATIO 2	RATIO 3	RATIO 4	
				.47	.56	.70	1.00	
HYDROGRAPH AT								
+	E1010	.05	1	FLOW	7.	12.	22.	49.
				TIME	6.33	6.33	6.25	6.25
HYDROGRAPH AT								
+	E1020	.04	1	FLOW	4.	7.	14.	32.
				TIME	6.42	6.42	6.33	6.33
HYDROGRAPH AT								
+	E1030	.02	1	FLOW	3.	5.	9.	19.
				TIME	6.33	6.33	6.25	6.25
2 COMBINED AT								
+	DP1030	.06	1	FLOW	6.	12.	22.	50.
				TIME	6.42	6.33	6.33	6.33
HYDROGRAPH AT								
+	E1040	.02	1	FLOW	4.	6.	11.	22.
				TIME	6.33	6.33	6.33	6.25
HYDROGRAPH AT								
+	E2010	.13	1	FLOW	24.	40.	68.	142.
				TIME	6.25	6.25	6.25	6.25
ROUTED TO								
+	R2020	.13	1	FLOW	23.	38.	67.	142.
				TIME	6.33	6.33	6.25	6.25

HYDROGRAPH AT									
+	E2020	.06	1	FLOW TIME	4. 6.33	9. 6.33	18. 6.25	47. 6.25	
2 COMBINED AT									
+	DP2020	.19	1	FLOW TIME	28. 6.33	47. 6.33	86. 6.25	189. 6.25	
ROUTED TO									
+	R2030	.19	1	FLOW TIME	27. 6.33	47. 6.33	84. 6.33	185. 6.25	
HYDROGRAPH AT									
+	E2030	.02	1	FLOW TIME	2. 6.17	5. 6.17	9. 6.17	22. 6.08	
HYDROGRAPH AT									
+	E2045	.06	1	FLOW TIME	35. 6.17	48. 6.17	70. 6.08	124. 6.08	
ROUTED TO									
+	R2040	.06	1	FLOW TIME	34. 6.17	48. 6.17	70. 6.17	121. 6.17	
HYDROGRAPH AT									
+	E2040	.03	1	FLOW TIME	3. 6.25	5. 6.17	11. 6.17	26. 6.17	
4 COMBINED AT									
+	DP2040	.30	1	FLOW TIME	61. 6.25	97. 6.25	164. 6.25	335. 6.17	
ROUTED TO									
+	R2050	.30	1	FLOW TIME	60. 6.25	96. 6.25	163. 6.25	333. 6.25	
HYDROGRAPH AT									
+	E2050	.02	1	FLOW TIME	2. 6.25	4. 6.25	7. 6.25	17. 6.25	
HYDROGRAPH AT									
+	E2060	.02	1	FLOW TIME	3. 6.17	5. 6.17	10. 6.17	24. 6.17	
3 COMBINED AT									
+	DP2060	.34	1	FLOW TIME	64. 6.25	105. 6.25	180. 6.25	372. 6.17	
ROUTED TO									
+	R2090	.34	1	FLOW TIME	63. 6.25	104. 6.25	179. 6.25	371. 6.25	
HYDROGRAPH AT									
+	E2070	.07	1	FLOW TIME	4. 6.42	8. 6.42	18. 6.42	44. 6.33	
ROUTED TO									
+	R2080	.07	1	FLOW TIME	4. 6.50	8. 6.50	17. 6.42	44. 6.42	
HYDROGRAPH AT									
+	E2080	.06	1	FLOW TIME	6. 6.17	12. 6.17	25. 6.17	58. 6.17	
2 COMBINED AT									
+	DP2080	.13	1	FLOW TIME	8. 6.25	17. 6.25	35. 6.25	88. 6.17	
HYDROGRAPH AT									
+	E2090	.02	1	FLOW	1.	3.	6.	14.	

+	E3000	.42	1	FLOW TIME	88. 6.33	140. 6.33	233. 6.33	474. 6.25
	HYDROGRAPH AT							
+	E3005	.24	1	FLOW TIME	68. 6.17	107. 6.17	176. 6.17	347. 6.17
	2 COMBINED AT							
+	DP3000	.66	1	FLOW TIME	147. 6.25	233. 6.25	388. 6.25	779. 6.17
	ROUTED TO							
+	R3015	.66	1	FLOW TIME	145. 6.33	228. 6.33	383. 6.25	777. 6.25
	HYDROGRAPH AT							
+	E3015	.11	1	FLOW TIME	35. 6.08	55. 6.08	91. 6.08	181. 6.08
	HYDROGRAPH AT							
+	E3010	.22	1	FLOW TIME	49. 6.17	81. 6.17	140. 6.17	288. 6.17
	ROUTED TO							
+	R3012	.22	1	FLOW TIME	48. 6.25	80. 6.25	136. 6.25	279. 6.17
	HYDROGRAPH AT							
+	E3012	.21	1	FLOW TIME	33. 6.42	54. 6.42	94. 6.33	199. 6.33
	HYDROGRAPH AT							
+	E3020	.19	1	FLOW TIME	44. 6.25	69. 6.25	115. 6.25	231. 6.25
	ROUTED TO							
+	R3025	.19	1	FLOW TIME	43. 6.33	68. 6.33	113. 6.33	228. 6.25
	HYDROGRAPH AT							
+	E3025	.26	1	FLOW TIME	47. 6.17	82. 6.17	147. 6.17	324. 6.08
	6 COMBINED AT							
+	DP3020	1.65	1	FLOW TIME	321. 6.25	528. 6.25	901. 6.25	1857. 6.17
	ROUTED TO							
+	R3030	1.65	1	FLOW TIME	319. 6.33	516. 6.33	885. 6.25	1846. 6.25
	HYDROGRAPH AT							
+	E3030	.26	1	FLOW TIME	36. 6.25	65. 6.25	119. 6.25	262. 6.25
	HYDROGRAPH AT							
+	E3035	.16	1	FLOW TIME	36. 6.08	63. 6.08	111. 6.08	234. 6.08
	3 COMBINED AT							
+	DP3030	2.07	1	FLOW TIME	367. 6.33	601. 6.25	1058. 6.25	2216. 6.25
	ROUTED TO							
+	R3040	2.07	1	FLOW TIME	362. 6.33	598. 6.33	1044. 6.25	2213. 6.25
	HYDROGRAPH AT							
+	E3040	.12	1	FLOW TIME	12. 6.25	23. 6.25	45. 6.17	110. 6.17

2 COMBINED AT									
+	DP3040	2.18	1	FLOW TIME	373. 6.33	618. 6.33	1089. 6.25	2316. 6.25	
ROUTED TO									
+	R3050	2.18	1	FLOW TIME	369. 6.42	609. 6.33	1075. 6.33	2290. 6.25	
HYDROGRAPH AT									
+	E3050	.05	1	FLOW TIME	12. 6.25	18. 6.25	31. 6.25	61. 6.25	
2 COMBINED AT									
+	DP3050	2.23	1	FLOW TIME	379. 6.42	627. 6.33	1104. 6.33	2351. 6.25	
HYDROGRAPH AT									
+	E3060	.12	1	FLOW TIME	29. 6.17	48. 6.17	81. 6.17	163. 6.17	
ROUTED TO									
+	R3070	.12	1	FLOW TIME	28. 6.33	45. 6.25	79. 6.25	161. 6.25	
HYDROGRAPH AT									
+	E3070	.08	1	FLOW TIME	15. 6.42	23. 6.42	38. 6.33	78. 6.33	
2 COMBINED AT									
+	DP3070	.20	1	FLOW TIME	42. 6.33	67. 6.33	114. 6.25	235. 6.25	
ROUTED TO									
+	R3080	.20	1	FLOW TIME	41. 6.33	67. 6.33	112. 6.25	233. 6.25	
HYDROGRAPH AT									
+	E3080	.05	1	FLOW TIME	10. 6.25	16. 6.25	28. 6.25	58. 6.25	
2 COMBINED AT									
+	DP3080	.25	1	FLOW TIME	50. 6.33	82. 6.33	139. 6.25	290. 6.25	
ROUTED TO									
+	R3090	.25	1	FLOW TIME	50. 6.42	81. 6.42	139. 6.33	283. 6.33	
HYDROGRAPH AT									
+	E3090	.08	1	FLOW TIME	17. 6.33	27. 6.33	45. 6.33	93. 6.25	
2 COMBINED AT									
+	DP3090	.33	1	FLOW TIME	66. 6.42	106. 6.42	184. 6.33	373. 6.33	
2 COMBINED AT									
+	DP3091	2.56	1	FLOW TIME	445. 6.42	732. 6.33	1288. 6.33	2722. 6.25	
ROUTED TO									
+	R3100	2.56	1	FLOW TIME	443. 6.42	727. 6.33	1285. 6.33	2714. 6.25	
HYDROGRAPH AT									
+	E3100	.09	1	FLOW TIME	21. 6.25	35. 6.17	60. 6.17	123. 6.17	
2 COMBINED AT									
+	DP3100	2.66	1	FLOW TIME	458. 6.42	757. 6.33	1334. 6.33	2828. 6.25	

2 COMBINED AT									
+	DP3110	2.67	1	FLOW TIME	461. 6.42	761. 6.33	1342. 6.33	2845. 6.25	
ROUTED TO									
+	R5010	2.67	1	FLOW TIME	454. 6.50	758. 6.42	1311. 6.33	2800. 6.33	
HYDROGRAPH AT									
+	E5011	.16	1	FLOW TIME	20. 6.42	35. 6.42	62. 6.42	133. 6.33	
3 COMBINED AT									
+	DP5010	3.53	1	FLOW TIME	561. 6.50	943. 6.42	1640. 6.33	3550. 6.33	
HYDROGRAPH AT									
+	E5020	.20	1	FLOW TIME	31. 6.33	52. 6.33	92. 6.33	200. 6.25	
2 COMBINED AT									
+	DP5011	3.73	1	FLOW TIME	587. 6.50	992. 6.42	1732. 6.33	3745. 6.33	
HYDROGRAPH AT									
+	E4010	.19	1	FLOW TIME	21. 6.42	38. 6.42	69. 6.42	153. 6.33	
ROUTED TO									
+	R4020	.19	1	FLOW TIME	21. 6.83	37. 6.75	68. 6.67	151. 6.58	
HYDROGRAPH AT									
+	E4020	.14	1	FLOW TIME	16. 6.83	26. 6.75	44. 6.75	90. 6.67	
2 COMBINED AT									
+	DP4020	.32	1	FLOW TIME	38. 6.83	63. 6.75	112. 6.67	238. 6.58	
HYDROGRAPH AT									
+	E4030	.02	1	FLOW TIME	5. 6.17	7. 6.17	12. 6.17	25. 6.17	

*** NORMAL END OF HEC-1 ***

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1*****
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*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
CORPS OF ENGINEERS *
* SEPTEMBER 1990 *
ENGINEERING CENTER *
* VERSION 4.0 *
SECOND STREET *
*
CALIFORNIA 95616 *
* RUN DATE 10/19/1999 TIME 12:41:18 *
756-1104 *
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X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10	
1	ID	WEST JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY										
2	ID	KIOWA ENGINEERING - PROJECT NO. 98.93										
3	ID	2, 5, 10 & 100 YEAR STORMS FILENAME: WFJCFUT.DAT DEV BASIN CONDITION										
4	ID	24HR STORM DURATION NO DETENTION										
		*DIAGRAM										
5	IT	5	0	0	250							
6	IO	5										
7	JR	PREC	.47	.56	.70	1						
8	KK	E1010										
9	BA	.05										
10	IN	15										
11	PB	4.4										
12	PC	0.0	.0005	.0015	.0030	.0045	.0060	.0080	.0100	.0120	.0143	
13	PC	.0165	.0188	.0210	.0233	.0255	.0278	.0320	.0390	.0460	.0530	
14	PC	.0600	.0750	.1000	.4000	.7000	.7250	.7500	.7650	.7800	.7900	
15	PC	.8000	.8100	.8200	.8250	.8300	.8350	.8400	.8450	.8500	.8550	
16	PC	.8600	.8638	.8675	.8713	.8750	.8788	.8825	.8863	.8900	.8938	
17	PC	.8975	.9013	.9050	.9083	.9115	.9148	.9180	.9210	.9240	.9270	

127 KK DP2080
 128 KM COMBINE FLOW FROM E2070, E2080
 129 HC 2
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

130 KK E2090
 131 KM RUNOFF FROM BAS 2090
 132 KO 0
 133 BA .019
 134 LS 0 75
 135 UD .414

136 KK DP2090
 137 KM COMBINE FLOW FROM DP2060, DP2080, E2090
 138 HC 3

139 KK R2100
 140 KM ROUTE FLOW FROM DP2090 TO DP2100
 141 RK 1800 .0169 .03 TRAP 60 4

142 KK E2110
 143 KM RUNOFF FROM BAS 2110
 144 KO 0
 145 BA .034
 146 LS 0 71
 147 UD .329

148 KK R2101
 149 KM ROUTE FLOW FROM E2110 TO DP2100
 150 RK 900 .025 .04 TRAP 40 5

151 KK E2100
 152 KM RUNOFF FROM BAS 2100
 153 KO 0
 154 BA .095
 155 LS 0 75.3
 156 UD .482

157 KK DP2100
 158 KM COMBINE FLOW FROM DP2090, E2110, E2100
 159 HC 3

160 KK E2120
 161 KM RUNOFF FROM BAS 2120
 162 KO 0
 163 BA .047
 164 LS 0 69
 165 UD .288

166 KK DP2120
 167 KM COMBINE FLOW FROM DP2100, E2120
 168 HC 2

169 KK R2120
 170 KM ROUTE FLOW FROM DP2120 THROUGH LAKE
 171 RK 1000 .025 .04 TRAP 40 5
 HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

172 KK R2130
 173 KM ROUTE FLOW FROM DP2120 TO DP2130
 174 RK 550 .05 .04 TRAP 8 3

175 KK E2130

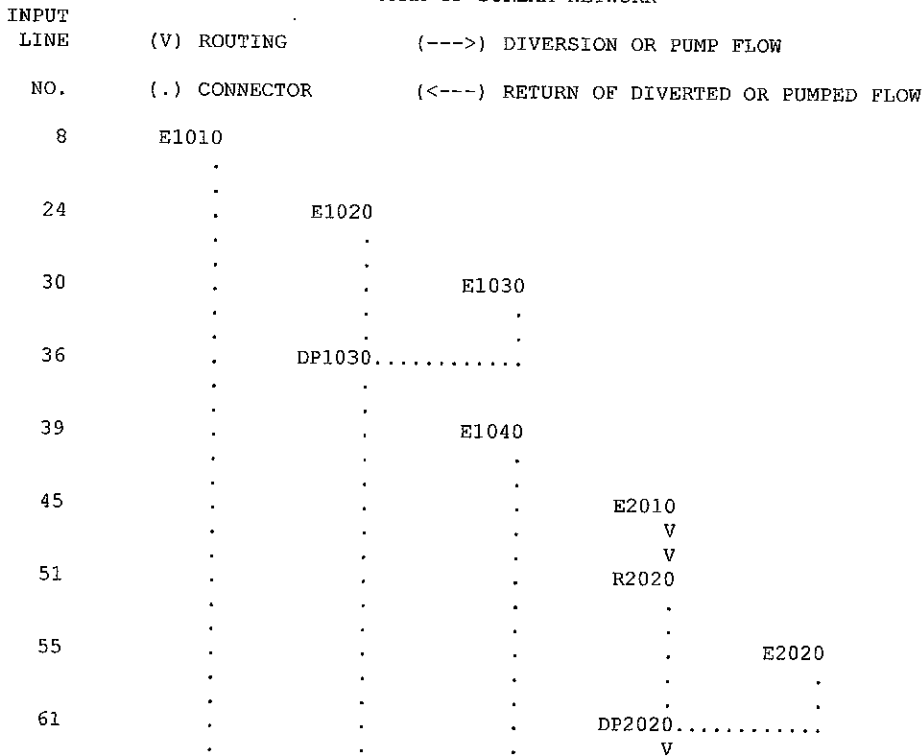
338	UD	.355								HEC-1 INPUT
339	KK	R3080								
340	KM	ROUTE FLOW FROM E3080 TO DP3090								
341	RK	1650	.01	.04		TRAP	30	10		
342	KK	E3090								
343	KM	RUNOFF FROM BAS 3090								
344	KO	0								
345	BA	.054								
346	LS	0	79							
347	UD	.411								
348	KK	DP3090								
349	KM	COMBINE FLOW FROM R3080, E3090								
350	HC	2								
351	KK	DP3091								
352	KM	COMBINE FLOW FROM DP3050, DP3090								
353	HC	2								
354	KK	R3100								
355	KM	ROUTE FLOW FROM DP3091 TO DP3100								
356	RK	350	.01	.03		TRAP	10	10		
357	KK	E3100								
358	KM	RUNOFF FROM BAS 3100								
359	KO	0								
360	BA	.095								
361	LS	0	82.9							
362	UD	.303								
363	KK	DP3100								
364	KM	COMBINE FLOW FROM R3100, E3100								
365	KO	0								
366	HC	2								
367	KK	E3110								
368	KM	RUNOFF FROM BAS 3110								
369	KO	0								
370	BA	.018								
371	LS	0	90							
372	UD	.472								
373	KK	DP3110								
374	KM	COMBINE FLOWS FROM E3110 AND DP3100								
375	HC	2								
376	KK	R5011								
377	KM	ROUTE FLOW FROM DP3110 TO DP5010								
378	RK	2900	.015	.04		TRAP	20	4		

379	KK	E5010								
380	KM	RUNOFF FROM BAS E5010								
381	KO	0								
382	BA	.156								
383	LS	0	88							
384	UD	.50								
385	KK	DP5010								
386	KM	COMBINE FLOW FROM E5010, R5011, AND R5010								

387	HC	3						
388	KK	E5020						
389	KM	RUNOFF FROM BASIN 5020						
390	KO	0						
391	BA	.2						
392	LS	0	88					
393	UD	.4						
394	KK	E4010						
395	KM	RUNOFF FROM BAS 4010						
396	KO	0						
397	BA	.19						
398	LS	0	86					
399	UD	.497						
400	KK	R4020						
401	KM	ROUTE FLOW FROM E4010 TO DP4020						
402	RK	2400	.005	.05	TRAP	10	30	
403	KK	E4020						
404	KM	RUNOFF FROM BAS 4020						
405	KO	0						
406	BA	.135						
407	LS	0	81.8					
408	UD	.822						
409	KK	DP4020						
410	KM	COMBINE FLOW FROM R4020, E4020						
411	HC	2						
412	KK	E4030						
413	KM	RUNOFF FROM BAS 4030						
414	KO	0						
415	BA	.018						
416	LS	0	90					
417	UD	.251						
418	ZZ							

1

SCHMATIC DIAGRAM OF STREAM NETWORK



64	.	.	.	V			
	.	.	.	R2030			
			
67	E2030		
		
			
73		E2045	
		V	
		V	
79		R2040	
	
	
82	E2040

88	DP2040
	V		
	V		
91	R2050		
		
			
94	E2050		
		
		
100	E2060	
	
	
106	DP2060
	V		
	V		
109	R2090		
		
			
112	E2070		
	V		
	V		
118	R2080		
		
		
121	E2080	
	
	
127	DP2080
		
		
130	E2090	
	
	
136	DP2090
	V		
	V		
139	R2100		
		
			
142	E2110		
	V		
	V		
148	R2101		
		
		
151	E2100	
	
	
157	DP2100
		
		
160	E2120		
		
		
166	DP2120

267	V		
	R3030		
270	E3030	
	
276	E3035

282	DP3030	
	V		
285	V		
	R3040		
		
288	E3040	
	
294	DP3040	
	V		
297	V		
	R3050		
		
300	E3050	
	
306	DP3050	
		
309	E3060	
	V	
315	V	
	R3070		
	
318	E3070

324	DP3070
	V	
327	V	
	R3071		
	
330	DP3050	
		
333	E3080	
	V	
339	V	
	R3080		
	
342	E3090

348	DP3090
	
351	DP3091	
	V		
354	V		
	R3100		
		
357	E3100	
	
363	DP3100	


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367 . . . . . E3110
. . . . .
373 . . . DP3110.....
. . . V
. . . V
376 . . . R5011
. . . .
. . . .
379 . . . E5010
. . . .
385 . . . DP5010.....
. . . .
388 . . . E5020
. . . .
394 . . . E4010
. . . V
400 . . . R4020
. . . V
403 . . . E4020
. . . .
409 . . . DP4020.....
. . . .
412 . . . E4030

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(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION			
				RATIO 1	RATIO 2	RATIO 3	RATIO 4
				.47	.56	.70	1.00
HYDROGRAPH AT							
+ E1010	.05	1	FLOW	7.	12.	22.	49.
			TIME	6.33	6.33	6.25	6.25
HYDROGRAPH AT							
+ E1020	.04	1	FLOW	22.	29.	42.	69.
			TIME	6.33	6.33	6.33	6.25
HYDROGRAPH AT							
+ E1030	.02	1	FLOW	6.	9.	14.	27.
			TIME	6.25	6.25	6.25	6.25
2 COMBINED AT							
+ DP1030	.06	1	FLOW	28.	38.	55.	96.
			TIME	6.33	6.33	6.25	6.25
HYDROGRAPH AT							
+ E1040	.02	1	FLOW	4.	6.	11.	22.
			TIME	6.33	6.33	6.33	6.25
HYDROGRAPH AT							
+ E2010	.13	1	FLOW	24.	40.	68.	142.
			TIME	6.25	6.25	6.25	6.25
ROUTED TO							
+ R2020	.13	1	FLOW	23.	38.	67.	142.

+	E2090	.02	1	FLOW TIME	3. 6.33	5. 6.33	9. 6.33	19. 6.25
	3 COMBINED AT							
+	DP2090	.48	1	FLOW TIME	93. 6.25	152. 6.25	260. 6.25	535. 6.25
	ROUTED TO							
+	R2100	.48	1	FLOW TIME	92. 6.33	149. 6.33	253. 6.25	534. 6.25
	HYDROGRAPH AT							
+	E2110	.03	1	FLOW TIME	4. 6.25	8. 6.25	14. 6.25	33. 6.17
	ROUTED TO							
+	R2101	.03	1	FLOW TIME	4. 6.33	7. 6.33	14. 6.25	33. 6.25
	HYDROGRAPH AT							
+	E2100	.09	1	FLOW TIME	15. 6.42	24. 6.42	42. 6.33	89. 6.33
	3 COMBINED AT							
+	DP2100	.61	1	FLOW TIME	111. 6.33	181. 6.33	308. 6.33	651. 6.25
	HYDROGRAPH AT							
+	E2120	.05	1	FLOW TIME	5. 6.25	9. 6.25	18. 6.17	45. 6.17
	2 COMBINED AT							
+	DP2120	.66	1	FLOW TIME	115. 6.33	189. 6.33	324. 6.25	692. 6.25
	ROUTED TO							
+	R2120	.66	1	FLOW TIME	113. 6.42	186. 6.33	323. 6.33	683. 6.25
	ROUTED TO							
+	R2130	.66	1	FLOW TIME	112. 6.42	185. 6.33	322. 6.33	679. 6.25
	HYDROGRAPH AT							
+	E2130	.01	1	FLOW TIME	1. 6.17	2. 6.17	5. 6.17	11. 6.08
	2 COMBINED AT							
+	DP2130	.67	1	FLOW TIME	113. 6.42	186. 6.33	325. 6.33	687. 6.25
	HYDROGRAPH AT							
+	E2140	.01	1	FLOW TIME	1. 6.08	2. 6.08	4. 6.08	9. 6.08
	HYDROGRAPH AT							
+	E2150	.01	1	FLOW TIME	4. 6.17	6. 6.17	11. 6.17	21. 6.17
	HYDROGRAPH AT							
+	E2160	.01	1	FLOW TIME	13. 6.00	17. 6.00	22. 6.00	35. 6.00
	4 COMBINED AT							
+	DP2160	.70	1	FLOW TIME	118. 6.42	196. 6.33	339. 6.33	723. 6.25
	ROUTED TO							
+	R5010	.70	1	FLOW TIME	117. 6.50	195. 6.42	334. 6.33	713. 6.33

HYDROGRAPH AT									
+	E3000	.42	1	FLOW	129.	190.	300.	568.	
				TIME	6.33	6.33	6.25	6.25	
HYDROGRAPH AT									
+	E3005	.24	1	FLOW	98.	144.	222.	407.	
				TIME	6.17	6.17	6.17	6.17	
2 COMBINED AT									
+	DP3000	.66	1	FLOW	215.	317.	493.	935.	
				TIME	6.25	6.25	6.25	6.17	
ROUTED TO									
+	R3015	.66	1	FLOW	210.	313.	493.	925.	
				TIME	6.33	6.25	6.25	6.25	
HYDROGRAPH AT									
+	E3015	.11	1	FLOW	51.	75.	115.	212.	
				TIME	6.08	6.08	6.08	6.08	
2 COMBINED AT									
+	DP3020	.77	1	FLOW	245.	365.	570.	1077.	
				TIME	6.25	6.25	6.25	6.17	
HYDROGRAPH AT									
+	E3010	.22	1	FLOW	96.	138.	211.	383.	
				TIME	6.17	6.17	6.17	6.17	
ROUTED TO									
+	R3012	.22	1	FLOW	93.	134.	204.	378.	
				TIME	6.25	6.25	6.17	6.17	
HYDROGRAPH AT									
+	E3012	.21	1	FLOW	64.	94.	147.	272.	
				TIME	6.33	6.33	6.33	6.33	
HYDROGRAPH AT									
+	E3020	.19	1	FLOW	159.	204.	275.	428.	
				TIME	6.17	6.17	6.17	6.17	
ROUTED TO									
+	R3025	.19	1	FLOW	158.	201.	270.	418.	
				TIME	6.25	6.25	6.25	6.25	
HYDROGRAPH AT									
+	E3025	.26	1	FLOW	273.	347.	463.	712.	
				TIME	6.08	6.08	6.08	6.08	
5 COMBINED AT									
+	DP3020	1.65	1	FLOW	761.	1059.	1562.	2737.	
				TIME	6.17	6.17	6.17	6.17	
ROUTED TO									
+	R3030	1.65	1	FLOW	754.	1043.	1525.	2680.	
				TIME	6.25	6.25	6.25	6.17	
HYDROGRAPH AT									
+	E3030	.26	1	FLOW	76.	116.	186.	361.	
				TIME	6.25	6.25	6.25	6.17	
HYDROGRAPH AT									
+	E3035	.16	1	FLOW	72.	106.	166.	306.	
				TIME	6.08	6.08	6.08	6.08	
3 COMBINED AT									
+	DP3030	2.07	1	FLOW	865.	1209.	1817.	3267.	
				TIME	6.25	6.25	6.17	6.17	
ROUTED TO									
+	R3040	2.07	1	FLOW	860.	1208.	1792.	3235.	
				TIME	6.25	6.25	6.25	6.17	

HYDROGRAPH AT									
+	E3040	.12	1	FLOW TIME	18. 6.25	31. 6.17	58. 6.17	129. 6.17	
2 COMBINED AT									
+	DP3040	2.18	1	FLOW TIME	878. 6.25	1239. 6.25	1848. 6.25	3364. 6.17	
ROUTED TO									
+	R3050	2.18	1	FLOW TIME	860. 6.33	1219. 6.25	1846. 6.25	3309. 6.25	
HYDROGRAPH AT									
+	E3050	.07	1	FLOW TIME	41. 6.25	56. 6.25	80. 6.25	136. 6.17	
2 COMBINED AT									
+	DP3050	2.26	1	FLOW TIME	898. 6.33	1275. 6.25	1926. 6.25	3444. 6.25	
HYDROGRAPH AT									
+	E3060	.12	1	FLOW TIME	42. 6.17	63. 6.17	100. 6.17	189. 6.08	
ROUTED TO									
+	R3070	.12	1	FLOW TIME	40. 6.25	61. 6.25	98. 6.25	189. 6.17	
HYDROGRAPH AT									
+	E3070	.08	1	FLOW TIME	18. 6.42	27. 6.42	44. 6.33	87. 6.33	
2 COMBINED AT									
+	DP3070	.20	1	FLOW TIME	57. 6.33	86. 6.25	140. 6.25	270. 6.25	
ROUTED TO									
+	R3071	.20	1	FLOW TIME	57. 6.42	85. 6.42	137. 6.33	264. 6.33	
2 COMBINED AT									
+	DP3050	2.45	1	FLOW TIME	946. 6.33	1336. 6.33	2049. 6.25	3705. 6.25	
HYDROGRAPH AT									
+	E3080	.05	1	FLOW TIME	15. 6.25	23. 6.25	38. 6.25	72. 6.25	
ROUTED TO									
+	R3080	.05	1	FLOW TIME	15. 6.42	23. 6.33	37. 6.33	71. 6.33	
HYDROGRAPH AT									
+	E3090	.05	1	FLOW TIME	14. 6.33	21. 6.33	34. 6.25	67. 6.25	
2 COMBINED AT									
+	DP3090	.11	1	FLOW TIME	29. 6.33	44. 6.33	71. 6.33	138. 6.25	
2 COMBINED AT									
+	DP3091	2.56	1	FLOW TIME	975. 6.33	1380. 6.33	2118. 6.25	3843. 6.25	
ROUTED TO									
+	R3100	2.56	1	FLOW TIME	973. 6.33	1380. 6.33	2111. 6.25	3839. 6.25	
HYDROGRAPH AT									
+	E3100	.09	1	FLOW	42.	61.	92.	166.	

				TIME	6.17	6.17	6.17	6.17
2 COMBINED AT								
+	DP3100	2.66	1	FLOW	1007.	1428.	2197.	3990.
				TIME	6.33	6.33	6.25	6.25
HYDROGRAPH AT								
+	E3110	.02	1	FLOW	10.	14.	19.	31.
				TIME	6.33	6.33	6.33	6.25
2 COMBINED AT								
+	DP3110	2.67	1	FLOW	1018.	1442.	2216.	4022.
				TIME	6.33	6.33	6.25	6.25
ROUTED TO								
+	R5011	2.67	1	FLOW	997.	1441.	2174.	3974.
				TIME	6.33	6.33	6.33	6.25
HYDROGRAPH AT								
+	E5010	.16	1	FLOW	74.	101.	146.	246.
				TIME	6.33	6.33	6.33	6.33
3 COMBINED AT								
+	DP5010	3.53	1	FLOW	1174.	1722.	2653.	4904.
				TIME	6.42	6.33	6.33	6.25
HYDROGRAPH AT								
+	E5020	.20	1	FLOW	112.	151.	216.	362.
				TIME	6.25	6.25	6.25	6.25
HYDROGRAPH AT								
+	E4010	.19	1	FLOW	78.	108.	160.	279.
				TIME	6.33	6.33	6.33	6.33
ROUTED TO								
+	R4020	.19	1	FLOW	76.	107.	158.	278.
				TIME	6.58	6.58	6.50	6.50
HYDROGRAPH AT								
+	E4020	.14	1	FLOW	27.	39.	61.	114.
				TIME	6.75	6.75	6.75	6.67
2 COMBINED AT								
+	DP4020	.32	1	FLOW	102.	145.	215.	383.
				TIME	6.67	6.58	6.58	6.50
HYDROGRAPH AT								
+	E4030	.02	1	FLOW	15.	20.	27.	44.
				TIME	6.08	6.08	6.08	6.08

*** NORMAL END OF HEC-1 ***

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1*****
*****
*
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
CORPS OF ENGINEERS
* JUN 1998
ENGINEERING CENTER
* VERSION 4.1
SECOND STREET
*
CALIFORNIA 95616
* RUN DATE 07JUL00 TIME 15:52:10
756-1104
*
*****
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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PAGE 1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	WEST JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY									
2	ID	KIOWA ENGINEERING - PROJECT NO. 98.93									
3	ID	2, 5, 10 & 100 YEAR STORMS FILENAME: WFJCDT.DAT DEV COND WITH DETENTION									
4	ID	24HR STORM DURATION									
	*DIAGRAM										
5	IT	5	0	0	250						
6	IO	5									
7	JR	PREC	.47	.56	.70	1					
8	KK	E1010									
9	BA	.05									
10	IN	15									
11	PB	4.4									
12	PC	0.0	.0005	.0015	.0030	.0045	.0060	.0080	.0100	.0120	.0143
13	PC	.0165	.0188	.0210	.0233	.0255	.0278	.0320	.0390	.0460	.0530
14	PC	.0600	.0750	.1000	.4000	.7000	.7250	.7500	.7650	.7800	.7900
15	PC	.8000	.8100	.8200	.8250	.8300	.8350	.8400	.8450	.8500	.8550
16	PC	.8600	.8638	.8675	.8713	.8750	.8788	.8825	.8863	.8900	.8938
17	PC	.8975	.9013	.9050	.9083	.9115	.9148	.9180	.9210	.9240	.9270

18	PC	.9300	.9325	.9350	.9375	.9400	.9425	.9450	.9475	.9500	.9525
19	PC	.9550	.9575	.9600	.9625	.9650	.9675	.9700	.9725	.9750	.9775
20	PC	.9800	.9813	.9825	.9838	.9850	.9863	.9875	.9888	.9900	.9913
21	PC	.9963	.9975	.9988	1.0000						
22	LS	0	73.2								
23	UD	.392									
24	KK	E1020									
25	KM		RUNOFF FROM BAS 1020								
26	KO	0									
27	BA	.041									
28	LS	0	88.8								
29	UD	.467									
30	KK	E1030									
31	KM		RUNOFF FROM BAS 1030								
32	KO	0									
33	BA	.020									
34	LS	0	80								
35	UD	.383									
36	KK	DP1030									
37	KM		COMBINE FLOW FROM E1020 & ROUTED FLOW FROM E1030								
38	HC	2									
39	KK	DB1031									
40	KM		ROUTE FLOW FROM DP 1030 THROUGH DETENTION BASIN 1031								
41	KO	0									
42	RS	1	ELEV	100							
43	SQ	0	15	30	45	60	90				
44	SV	0	.5	1	2.5	4	5				
45	SE	100	101	102	103	104	105				

HEC-1 INPUT

1
PAGE 2

LINE	ID	1	2	3	4	5	6	7	8	9	10
46	KK	E1040									
47	KM		RUNOFF FROM BAS 1040								
48	KO	0									
49	BA	.02171									
50	LS	0	75								
51	UD	.41									
52	KK	E2010									
53	KM		RUNOFF FROM BAS 2010								
54	KO	0									
55	BA	.125									
56	LS	0	75								
57	UD	.350									
58	KK	R2020									
59	KM		ROUTE FLOW FROM E2010 TO DP2020								
60	KO	0									
61	RK	800	.03	.05		TRAP	20	10			
62	KK	E2020									
63	KM		RUNOFF FROM BASIN 2020								
64	KO	0									
65	BA	.062									
66	LS	0	75								
67	UD	.376									
68	KK	DP2020									
69	KM		COMBINE FLOW FROM E2010 & ROUTED FLOW FROM E2020								
70	HC	2									
71	KK	R2030									
72	KM		ROUTE FLOW FROM DP2020 TO DP2040								
73	RK	800	.0275	.05		TRAP	20	8			

74	KK	E2030							
75	KM	RUNOFF FROM BAS 2030							
76	KO	0							
77	BA	.021							
78	LS	0	71						
79	UD	.233							
80	KK	E2045							
81	KM	RUNOFF FROM BAS 2045							
82	KO	0							
83	BA	.061							
84	LS	0	85						
85	UD	.258							
86	KK	R2040							
87	KM	ROUTE FLOW FROM E2045 TO DP2040							
88	RK	1200	.0314	.04		TRAP	20		8
						HEC-1 INPUT			

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PAGE 3

LINE	ID	1	2	3	4	5	6	7	8	9	10
89	KK	E2040									
90	KM	RUNOFF FROM BAS 2040									
91	KO	0									
92	BA	.026									
93	LS	0	71								
94	UD	.268									
95	KK	DP2040									
96	KM	COMBINE FLOW FROM DP2020, E2030, E2045, E2040									
97	HC	4									
98	KK	R2050									
99	KM	ROUTE FLOW FROM DP2040 TO DP2060									
100	RK	600	.03	.03		TRAP	20			3	
101	KK	E2050									
102	KM	RUNOFF FROM BAS 2050									
103	KO	0									
104	BA	.020									
105	LS	0	71								
106	UD	.33									
107	KK	E2060									
108	KM	RUNOFF FROM BAS 2060									
109	KO	0									
110	BA	.024									
111	LS	0	73.1								
112	UD	.253									
113	KK	DP2060									
114	KM	COMBINE FLOW FROM DP2040, E2050, E2060									
115	HC	3									
116	KK	R2090									
117	KM	ROUTE FLOW FROM DP2060 TO DP2090									
118	RK	500	.023	.03		TRAP	30			3	
119	KK	E2070									
120	KM	RUNOFF FROM BAS 2070									
121	KO	0									
122	BA	.068									
123	LS	0	75								
124	UD	.463									
125	KK	R2080									
126	KM	ROUTE FLOW FROM E2070 TO DP2080									
127	RK	1220	.035	.04		TRAP	10			5	

128 KK E2080
 129 KM RUNOFF FROM BAS 2080
 130 KO 0
 131 BA .057
 132 LS 0 71
 133 UD .247

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

134 KK DP2080
 135 KM COMBINE FLOW FROM E2070, E2080
 136 HC 2

 137 KK E2090
 138 KM RUNOFF FROM BAS 2090
 139 KO 0
 140 BA .019
 141 LS 0 75
 142 UD .414

 143 KK DP2090
 144 KM COMBINE FLOW FROM DP2060, DP2080, E2090
 145 HC 3

 146 KK DB2091
 147 KM ROUTE FLOW FROM DP 2090 THROUGH DET BASIN 2091
 148 KO 0
 149 RS 1 ELEV 100
 150 SQ 0 100 200 300 400 500
 151 SV 0 .5 1 2 3 4.5
 152 SE 100 101 102 103 104 105

 153 KK R2100
 154 KM ROUTE FLOW FROM DB2091 TO DP2100
 155 RK 1800 .0169 .03 TRAP 60 4

 156 KK E2110
 157 KM RUNOFF FROM BAS 2110
 158 KO 0
 159 BA .034
 160 LS 0 71
 161 UD .329

 162 KK R2101
 163 KM ROUTE FLOW FROM E2110 TO DP2100
 164 RK 900 .025 .04 TRAP 40 5

 165 KK E2100
 166 KM RUNOFF FROM BAS 2100
 167 KO 0
 168 BA .095
 169 LS 0 75.3
 170 UD .482

 171 KK DP2100
 172 KM COMBINE FLOW FROM DP2090, DB2111, E2100
 173 HC 3

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

174 KK E2120
 175 KM RUNOFF FROM BAS 2120
 176 KO 0
 177 BA .047

232	KM	COMBINE FLOW FROM E3000 AND E3005					
233	HC	2					
234	KK	R3015					
235	KM	ROUTE FLOW FROM DP3000 TO DP3020					
236	RK	3200	.031	.04	TRAP	15	3
237	KK	E3015					
238	KM	RUNOFF FROM BASIN 3015					
239	KO	0					
240	BA	.11					
241	LS	0 81					
242	UD	.21					
243	KK	DP3020					
244	KM	COMBINE FLOW FROM BASIN 3015 AND R3015					
245	HC	2					
246	KK	E3010					
247	KM	RUNOFF FROM BAS 3010					
248	KO	0					
249	BA	.22					
250	LS	0 82					
251	UD	.28					
252	KK	R3012					
253	KM	ROUTE FLOW FROM E3010 TO DP 3020					
254	RK	2600	.03	.04	TRAP	15	3
255	KK	E3012					
256	KM	RUNOFF FROM BASIN E3012					
257	KO	0					
258	BA	.21					
259	LS	0 82					
260	UD	.47					

HEC-1 INPUT

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10
261	KK	E3020									
262	KM	RUNOFF FROM BAS 3020									
263	KO	0									
264	BA	.188									
265	LS	0 93									
266	UD	.36									
267	KK	R3025									
268	KM	ROUTE FLOW FROM BASIN E3020 TO DP 3020									
269	RK	2600	.035	.04	TRAP	15	3				
270	KK	E3025									
271	KM	RUNOFF FROM BASIN E3025									
272	KO	0									
273	BA	.26									
274	LS	0 93									
275	UD	.23									
276	KK	DP3020									
277	KM	COMBINE FLOW FROM DP3020, E3012, E3025, R3012 AND R3025									
278	HC	5									
279	KK	DB3021									
280	KM	ROUTE FLOW FROM DP3020 TO DETENTION BASIN 3021									
281	KO	0									
282	RS	1	ELEV	100							
283	SQ	0	150	400	600	1200	3000				
284	SV	0	15	45	60	75	90				
285	SE	100	102	104	106	108	110				

286	KK	R3030							
287	KM		ROUTE FLOW FROM DB3021 TO DP3030						
288	RK	3000	.05	.04		TRAP	20	10	
289	KK	E3030							
290	KM		RUNOFF FROM BAS 3030						
291	KO		0						
292	BA		.260						
293	LS		0	79					
294	UD		.34						
295	KK	E3035							
296	KM		RUNOFF FROM BASIN 3035						
297	KO		0						
298	BA		.16						
299	LS		0	79					
300	UD		.16						
301	KK	DP3030							
302	KM		COMBINE FLOW FROM DP3031, R3030 AND E3035						
303	HC		3						

HEC-1 INPUT

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LINE	ID12345678910	
304	KK	DB3031										
305	KM		ROUTE FLOW FROM DP3030 TO DB3031									
306	KO		0									
307	RS		1	ELEV	100							
308	SQ		0	100	400	1000	2400	3000				
309	SV		0	2	4	6	10	12				
310	SE		100	102	104	106	108	110				
311	KK	R3040										
312	KM		ROUTE FLOW FROM DB3031 TO DP3040									
313	RK	1450	.03	.03				TRAP	30	5		
314	KK	E3040										
315	KM		RUNOFF FROM BAS 3040									
316	KO		0									
317	BA		.115									
318	LS		0	72.3								
319	UD		.294									
320	KK	DP3040										
321	KM		COMBINE FLOW FROM R3040, E3040									
322	HC		2									
323	KK	R3050										
324	KM		ROUTE FLOW FROM DP3040 TO DP3050									
325	RK	2850	.009	.03				TRAP	10	7		
326	KK	E3050										
327	KM		RUNOFF FROM BAS 3050									
328	KO		0									
329	BA		.074									
330	LS		0	87.3								
331	UD		.371									
332	KK	DP3050										
333	KM		COMBINE FLOW FROM DP3050, E3050									
334	HC		2									
335	KK	E3060										
336	KM		RUNOFF FROM BAS 3060									
337	KO		0									
338	BA		.119									
339	LS		0	79								
340	UD		.257									

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341      KK  DB3061
342      KM          ROUTE FLOW FROM E3060 THROUGH DET BASIN DB3061
343      KO          0
344      RS          1      ELEV      100
345      SQ          0          30      60      90      120      160      200
346      SV          0          .4      .8      1.2      1.6      2.0      2.5
347      SE          100      101      102      103      104      105      106
                                     HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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348      KK  R3070
349      KM          ROUTE FLOW FROM DB3061 TO DP3070
350      RK          2000      .009      .04          TRAP          30      10

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351      KK  E3070
352      KM          RUNOFF FROM BAS 3070
353      KO          0
354      BA          .077
355      LS          0          79.3
356      UD          .486

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357      KK  DP3070
358      KM          COMBINE FLOW FROM R3070 AND E3070
359      HC          2

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360      KK  R3071
361      KM          ROUTE FLOW FROM DP3070 TO DP3050
362      RK          2250      .015      .04          TRAP          30      10

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363      KK  DP3050
364      KM          COMBINE FLOW FROM R3071 AND DP3050
365      HC          2

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366      KK  E3080
367      KM          RUNOFF FROM BAS 3080
368      KO          0
369      BA          .053
370      LS          0          79.2
371      UD          .355

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372      KK  R3080
373      KM          ROUTE FLOW FROM E3080 TO DP 3090
374      RK          1650      .01      .04          TRAP          30      10

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375      KK  E3090
376      KM          RUNOFF FROM BAS 3090
377      KO          0
378      BA          .054
379      LS          0          79
380      UD          .411

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381      KK  DP3090
382      KM          COMBINE FLOW FROM R3080 AND E3090
383      HC          2

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384      KK  DB3091
385      KM          ROUTE FLOW FROM E3090 TO DETENTION BASIN 3091
386      KO          0
387      RS          1      ELEV      100
388      SQ          0          5      15      30      100      120
389      SV          0          .25      .5      1.0      1.5      3.0
390      SE          100      101      102      103      104      105
                                     HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10


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445      KK DP5010
446      KM          COMBINE FLOW FROM DB5021 AND R5010
447      HC          2

448      KK R5011
449      KM          ROUTE FLOW FROM DP5010 TO DP5011
450      RK          1300      .004      .04          TRAP          90          4

451      KK E5010
452      KM          RUNOFF FROM BAS E5010
453      KO          0
454      BA          .156
455      LS          0          88
456      UD          .50

457      KK DB5011
458      KM          ROUTE FLOW FROM E5010 THROUGH DET BASIN DB5011
459      KO          0
460      RS          1      ELEV      100
461      SQ          0      20      40      60      120      150
462      SV          0      2      4      6      8      12
463      SE          100      102      104      106      108      110

464      KK DP5011
465      KM          COMBINE FLOW FROM R5011, DB5011
466      HC          2

467      KK E4010
468      KM          RUNOFF FROM BAS 4010
469      KO          0
470      BA          .19
471      LS          0          86
472      UD          .497

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HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

473      KK DB4011
474      KM          ROUTE FLOW FROM E4010 THROUGH DET BASIN DB4011
475      KO          0
476      RS          1      ELEV      100
477      SQ          0      40      80      120      180      220
478      SV          0      2      4      6      10      13
479      SE          100      102      104      106      108      110

480      KK R4020
481      KM          ROUTE FLOW FROM DB4011 TO DP4020
482      RK          2400      .005      .05          TRAP          10          30

483      KK E4020
484      KM          RUNOFF FROM BAS 4020
485      KO          0
486      BA          .135
487      LS          0      81.8
488      UD          .822

489      KK DP4020
490      KM          COMBINE FLOW FROM R4020, E4020
491      HC          2

492      KK DB4021
493      KM          ROUTE FLOW FROM 4020 THROUGH DET BASIN DB4021
494      KO          0
495      RS          1      ELEV      100
496      SQ          0      10      20      40      80      150      250
497      SV          0      1.0      2      3.0      4      6      10
498      SE          100      101      102      103      104      105      106

```



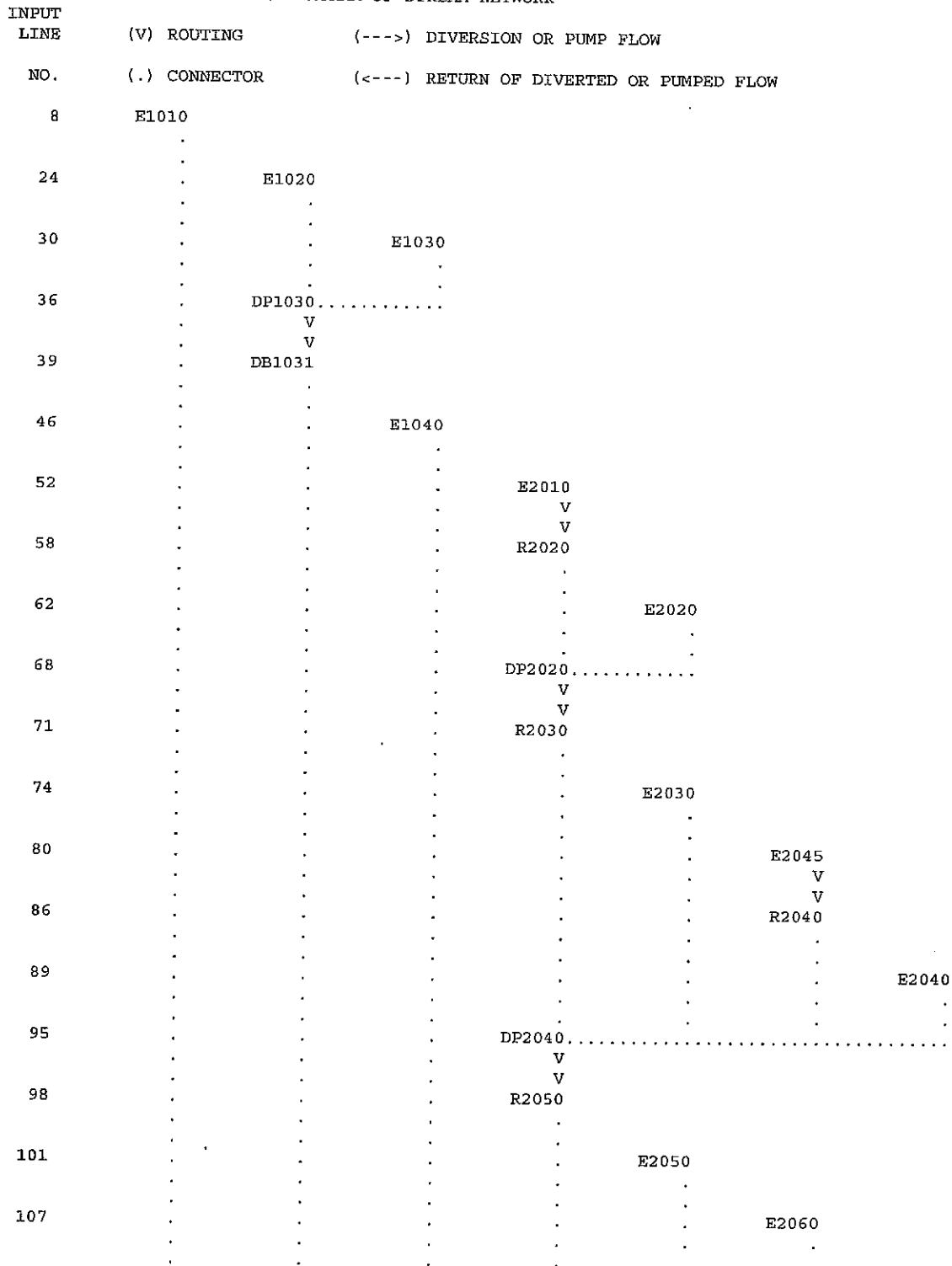
```

499      KK      E4030
500      KM      RUNOFF FROM BAS 4030
501      KO      0
502      BA      .018
503      LS      0      90
504      UD      .251
505      ZZ

```

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



113	.	.	.	DP2060.....	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
116	.	.	.	R2090	.	.	.

119	E2070	.	.
	V	.	.
	V	.	.
125	R2080	.	.

128	E2080	.

134	DP2080.....	.	.

137	E2090	.

143	.	.	.	DP2090.....	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
146	.	.	.	DB2091	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
153	.	.	.	R2100	.	.	.

156	E2110	.	.
	V	.	.
	V	.	.
162	R2101	.	.

165	E2100	.

171	.	.	.	DP2100.....	.	.	.

174	E2120	.	.

180	.	.	.	DP2120.....	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
183	.	.	.	R2120	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
186	.	.	.	R2130	.	.	.

189	E2130	.	.

195	.	.	.	DP2130.....	.	.	.

198	E2140	.	.

204	E2150	.

210	E2160

216	.	.	.	DP2160.....	.	.	.

219	E3000		
		
225		E3005	
	
231	DP3000.....		
	V		
	V		
234	R3015		
		
237		E3015	
	
243	DP3020.....		
		
246		E3010	
	V	
	V	
252		R3012	
	
255			E3012

261			E3020
	V
	V
267			R3025

270
E3025

276
DP3020.....
	V		.
	V		.
279	DB3021		.
	V		.
	V		.
286	R3030		.

289		E3030	.

295			E3035

301	DP3030.....		.
	V		.
	V		.
304	DB3031		.
	V		.
	V		.
311	R3040		.

314		E3040	.

320	DP3040.....		.
	V		.
	V		.

323	R3050	.	.	.

326	E3050	.	.

332	DP3050

335	E3060	.	.
	V	.	.
	V	.	.
341	DB3061	.	.
	V	.	.
	V	.	.
348	R3070	.	.

351	E3070	.

357	DP3070
	V	.	.
	V	.	.
360	R3071	.	.

363	DP3050

366	E3080	.	.
	V	.	.
	V	.	.
372	R3080	.	.

375	E3090	.

381	DP3090
	V	.	.
	V	.	.
384	DB3091	.	.

391	DP3092
	V	.	.
	V	.	.
394	R3100	.	.	.

397	E3100	.	.
	V	.	.
	V	.	.
403	DB3101	.	.

410	DP3102

414	E3110	.	.

420	DP3110
	V	.	.
	V	.	.
423	R3110	.	.	.

426	DP3111
	V	.	.

429	.	.	.	V				
	.	.	.	R5010				
432		E5020		
		V		
438		V		
		DB5021		
445		
	DP5010.....			
	V			
448	V			
	R5011			
451		E5010		
		V		
457		V		
		DB5011		
464		
	DP5011.....			
467		E4010		
		V		
473		V		
		DB4011		
480		V		
		V		
483		R4020		
		E4020
489
		DP4020.....		.
		V		.
492		V		.
		DB4021		.
499		E4030

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION				
				RATIO 1	RATIO 2	RATIO 3	RATIO 4	
				.47	.56	.70	1.00	
HYDROGRAPH AT								
+	E1010	.05	1	FLOW	7.	12.	22.	49.
				TIME	6.33	6.33	6.25	6.25
HYDROGRAPH AT								
+	E1020	.04	1	FLOW	22.	29.	42.	69.
				TIME	6.33	6.33	6.33	6.25
HYDROGRAPH AT								
+	E1030	.02	1	FLOW	6.	9.	14.	27.
				TIME	6.25	6.25	6.25	6.25
2 COMBINED AT								
+	DP1030	.06	1	FLOW	28.	38.	55.	96.

+	R2090	.34	1	FLOW TIME	74. 6.25	119. 6.25	200. 6.25	403. 6.25
HYDROGRAPH AT								
+	E2070	.07	1	FLOW TIME	11. 6.42	17. 6.33	31. 6.33	65. 6.33
ROUTED TO								
+	R2080	.07	1	FLOW TIME	10. 6.42	17. 6.42	30. 6.42	64. 6.33
HYDROGRAPH AT								
+	E2080	.06	1	FLOW TIME	8. 6.17	15. 6.17	29. 6.17	64. 6.17
2 COMBINED AT								
+	DP2080	.13	1	FLOW TIME	16. 6.33	28. 6.25	51. 6.25	113. 6.25
HYDROGRAPH AT								
+	E2090	.02	1	FLOW TIME	3. 6.33	5. 6.33	9. 6.33	19. 6.25
3 COMBINED AT								
+	DP2090	.48	1	FLOW TIME	93. 6.25	152. 6.25	260. 6.25	535. 6.25
ROUTED TO								
+	DB2091	.48	1	FLOW TIME	90. 6.33	147. 6.33	241. 6.33	473. 6.33
** PEAK STAGES IN FEET **								
	1	STAGE			100.90	101.47	102.41	104.73
		TIME			6.33	6.33	6.33	6.33
ROUTED TO								
+	R2100	.48	1	FLOW TIME	90. 6.42	144. 6.42	237. 6.42	468. 6.42
HYDROGRAPH AT								
+	E2110	.03	1	FLOW TIME	4. 6.25	8. 6.25	14. 6.25	33. 6.17
ROUTED TO								
+	R2101	.03	1	FLOW TIME	4. 6.33	7. 6.33	14. 6.25	33. 6.25
HYDROGRAPH AT								
+	E2100	.09	1	FLOW TIME	15. 6.42	24. 6.42	42. 6.33	89. 6.33
3 COMBINED AT								
+	DP2100	.61	1	FLOW TIME	108. 6.42	175. 6.42	292. 6.42	584. 6.33
HYDROGRAPH AT								
+	E2120	.05	1	FLOW TIME	5. 6.25	9. 6.25	18. 6.17	45. 6.17
2 COMBINED AT								
+	DP2120	.66	1	FLOW TIME	112. 6.42	181. 6.42	306. 6.33	618. 6.33
ROUTED TO								
+	R2120	.66	1	FLOW TIME	110. 6.42	181. 6.42	304. 6.42	612. 6.33
ROUTED TO								
+	R2130	.66	1	FLOW TIME	108. 6.42	181. 6.42	304. 6.42	609. 6.42

HYDROGRAPH AT									
+	E2130	.01	1	FLOW TIME	1. 6.17	2. 6.17	5. 6.17	11. 6.08	
2 COMBINED AT									
+	DP2130	.67	1	FLOW TIME	109. 6.42	182. 6.42	306. 6.42	615. 6.33	
HYDROGRAPH AT									
+	E2140	.01	1	FLOW TIME	1. 6.08	2. 6.08	4. 6.08	9. 6.08	
HYDROGRAPH AT									
+	E2150	.01	1	FLOW TIME	4. 6.17	6. 6.17	11. 6.17	21. 6.17	
HYDROGRAPH AT									
+	E2160	.01	1	FLOW TIME	13. 6.00	17. 6.00	22. 6.00	35. 6.00	
4 COMBINED AT									
+	DP2160	.70	1	FLOW TIME	114. 6.42	188. 6.42	316. 6.42	640. 6.33	
HYDROGRAPH AT									
+	E3000	.42	1	FLOW TIME	129. 6.33	190. 6.33	300. 6.25	568. 6.25	
HYDROGRAPH AT									
+	E3005	.24	1	FLOW TIME	98. 6.17	144. 6.17	222. 6.17	407. 6.17	
2 COMBINED AT									
+	DP3000	.66	1	FLOW TIME	215. 6.25	317. 6.25	493. 6.25	935. 6.17	
ROUTED TO									
+	R3015	.66	1	FLOW TIME	210. 6.33	313. 6.25	493. 6.25	925. 6.25	
HYDROGRAPH AT									
+	E3015	.11	1	FLOW TIME	51. 6.08	75. 6.08	115. 6.08	212. 6.08	
2 COMBINED AT									
+	DP3020	.77	1	FLOW TIME	245. 6.25	365. 6.25	570. 6.25	1077. 6.17	
HYDROGRAPH AT									
+	E3010	.22	1	FLOW TIME	96. 6.17	138. 6.17	211. 6.17	383. 6.17	
ROUTED TO									
+	R3012	.22	1	FLOW TIME	93. 6.25	134. 6.25	204. 6.17	378. 6.17	
HYDROGRAPH AT									
+	E3012	.21	1	FLOW TIME	64. 6.33	94. 6.33	147. 6.33	272. 6.33	
HYDROGRAPH AT									
+	E3020	.19	1	FLOW TIME	159. 6.17	204. 6.17	275. 6.17	428. 6.17	
ROUTED TO									
+	R3025	.19	1	FLOW TIME	158. 6.25	201. 6.25	270. 6.25	418. 6.25	
HYDROGRAPH AT									
+	E3025	.26	1	FLOW TIME	273. 6.08	347. 6.08	463. 6.08	712. 6.08	

5 COMBINED AT								
+	DP3020	1.65	1	FLOW TIME	761. 6.17	1059. 6.17	1562. 6.17	2737. 6.17
ROUTED TO								
+	DB3021	1.65	1	FLOW TIME	255. 6.75	348. 6.75	554. 6.67	1808. 6.42
** PEAK STAGES IN FEET **								
	1	STAGE		TIME	102.84 6.75	103.58 6.75	105.54 6.67	108.68 6.42
ROUTED TO								
+	R3030	1.65	1	FLOW TIME	255. 6.83	347. 6.83	553. 6.75	1761. 6.50
HYDROGRAPH AT								
+	E3030	.26	1	FLOW TIME	76. 6.25	116. 6.25	186. 6.25	361. 6.17
HYDROGRAPH AT								
+	E3035	.16	1	FLOW TIME	72. 6.08	106. 6.08	166. 6.08	306. 6.08
3 COMBINED AT								
+	DP3030	2.07	1	FLOW TIME	290. 6.67	401. 6.58	637. 6.58	2007. 6.50
ROUTED TO								
+	DB3031	2.07	1	FLOW TIME	289. 6.75	399. 6.67	636. 6.67	2012. 6.50
** PEAK STAGES IN FEET **								
	1	STAGE		TIME	103.26 6.75	104.00 6.67	104.79 6.67	107.45 6.50
ROUTED TO								
+	R3040	2.07	1	FLOW TIME	289. 6.75	399. 6.67	635. 6.67	1987. 6.50
HYDROGRAPH AT								
+	E3040	.12	1	FLOW TIME	18. 6.25	31. 6.17	58. 6.17	129. 6.17
2 COMBINED AT								
+	DP3040	2.18	1	FLOW TIME	294. 6.75	410. 6.67	653. 6.67	2043. 6.50
ROUTED TO								
+	R3050	2.18	1	FLOW TIME	294. 6.75	409. 6.75	651. 6.67	1998. 6.58
HYDROGRAPH AT								
+	E3050	.07	1	FLOW TIME	41. 6.25	56. 6.25	80. 6.25	136. 6.17
2 COMBINED AT								
+	DP3050	2.26	1	FLOW TIME	308. 6.75	432. 6.58	682. 6.67	2061. 6.58
HYDROGRAPH AT								
+	E3060	.12	1	FLOW TIME	42. 6.17	63. 6.17	100. 6.17	189. 6.08
ROUTED TO								
+	DB3061	.12	1	FLOW TIME	33. 6.25	50. 6.25	82. 6.25	165. 6.25
** PEAK STAGES IN FEET **								
	1	STAGE		TIME	101.09	101.68	102.73	105.11

HYDROGRAI	+	E3110	.02	1	FLOW	10.	14.	19.	31.
					TIME	6.33	6.33	6.33	6.25
2 COMBIN	+	2 COMBINED AT							
		DP3110	2.67	1	FLOW	405.	585.	943.	2500.
					TIME	6.58	6.50	6.42	6.50
ROUTED TO	+	ROUTED TO							
		R3110	2.67	1	FLOW	403.	581.	940.	2493.
					TIME	6.58	6.58	6.50	6.58
		2 COMBINED AT							
		DP3111	3.38	1	FLOW	506.	759.	1246.	3043.
					TIME	6.58	6.50	6.42	6.50
HYDROGRAPI	+	ROUTED TO							
		R5010	3.38	1	FLOW	505.	756.	1243.	3035.
					TIME	6.67	6.58	6.50	6.58
1		HYDROGRAPH AT							
		E5020	.20	1	FLOW	112.	151.	216.	362.
					TIME	6.25	6.25	6.25	6.25
*** NORMAI	+	ROUTED TO							
		DB5021	.20	1	FLOW	41.	65.	93.	197.
					TIME	6.75	6.67	6.67	6.58
					** PEAK STAGES IN FEET **				
				1	STAGE	104.04	105.26	106.67	109.19
					TIME	6.75	6.67	6.67	6.58
		2 COMBINED AT							
		DP5010	3.58	1	FLOW	545.	819.	1333.	3232.
					TIME	6.67	6.58	6.50	6.58
		ROUTED TO							
		R5011	3.58	1	FLOW	540.	810.	1323.	3200.
					TIME	6.75	6.67	6.58	6.58
		HYDROGRAPH AT							
		E5010	.16	1	FLOW	74.	101.	146.	246.
					TIME	6.33	6.33	6.33	6.33
		ROUTED TO							
		DB5011	.16	1	FLOW	30.	41.	59.	127.
					TIME	6.92	6.92	6.92	6.75
					** PEAK STAGES IN FEET **				
				1	STAGE	103.02	104.10	105.90	108.47
					TIME	6.92	6.92	6.92	6.75
		2 COMBINED AT							
		DP5011	3.73	1	FLOW	570.	848.	1375.	3324.
					TIME	6.75	6.67	6.58	6.58
		HYDROGRAPH AT							
		E4010	.19	1	FLOW	78.	108.	160.	279.
					TIME	6.33	6.33	6.33	6.33
		ROUTED TO							
		DB4011	.19	1	FLOW	46.	64.	94.	157.
					TIME	6.75	6.75	6.75	6.75
					** PEAK STAGES IN FEET **				
				1	STAGE	102.30	103.20	104.72	107.23
					TIME	6.75	6.75	6.75	6.75
		ROUTED TO							
		R4020	.19	1	FLOW	46.	64.	94.	157.
					TIME	7.00	7.00	7.00	6.92

HYDROGRAPH AT								
+	E4020	.14	1	FLOW	27.	39.	61.	114.
				TIME	6.75	6.75	6.75	6.67
2 COMBINED AT								
+	DP4020	.32	1	FLOW	70.	100.	150.	265.
				TIME	6.92	6.92	6.92	6.83
ROUTED TO								
+	DB4021	.32	1	FLOW	48.	77.	121.	211.
				TIME	7.58	7.33	7.25	7.25
				** PEAK STAGES IN FEET **				
			1	STAGE	103.21	103.92	104.58	105.61
				TIME	7.58	7.33	7.25	7.25
HYDROGRAPH AT								
+	E4030	.02	1	FLOW	15.	20.	27.	44.
				TIME	6.08	6.08	6.08	6.08
1								

*** NORMAL END OF HEC-1 ***

HYDRAULIC CALCULATIONS

APPENDIX B

WEST FORK JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY

SUMMARY OF CHANNEL IMPROVEMENTS RIPRAP LINED

"n"= 0.04 SS 3 H TO 1 V.
DEPTH = 3 FT AVERAGE VELOCITY = 7 FPS MAX VEL 9 FPS

CHANNEL NUMBER	DRAINAGE NAME	CHANNEL LENGTH (FT)	100-YEAR FLOW (CFS)	REQD. AREA (S.F.)	CHANNEL DEPTH (FT)	EX. SLOPE (FT/FT)	FUTURE SLOPE (FT/FT)	CHANNEL BOTTOM WIDTH	CHANNEL TOP WIDTH	HYDR. RADIUS (FEET)	R.O.W. REQD. (FEET)	DROP IN SEGMENT (FT)	NUMBER OF CHECK STRUC.
5012	WFJC	1400	3320	474	5.0	0.003	0.003	100	130	4.43	160	0.0	0
5011	WFJC	1270	3190	456	5.0	0.006	0.004	90	120	4.38	150	2.5	2
5010	WFJC	2050	2640	377	4.0	0.006	0.004	90	114	3.59	144	4.1	3
3110	WFJC	870	2500	357	4.0	0.006	0.004	90	114	3.59	144	1.7	1
3040	WFJC	2350	360	51	3.0	0.025	0.010	20	38	2.28	68	35.3	11
3030-1	WFJC	1060	1850	264	4.0	0.030	0.010	65	89	3.47	119	21.2	5
3030-2	WFJC	900	1760	251	4.0	0.030	0.010	60	84	3.43	114	18.0	8
3000	WFJC	3230	570	81	3.0	0.020	0.010	25	43	2.37	73	32.3	10
3005	WFJC	3000	410	59	3.0	0.020	0.010	20	38	2.28	68	30.0	10
3012	WFJC	2000	380	54	3.0	0.010	0.005	20	38	2.28	68	10.0	4
3015	WFJC	1550	935	134	4.0	0.015	0.010	35	59	3.18	89	7.8	3
3021	WFJC	1750	420	60	3.0	0.020	0.010	20	38	2.28	68	17.5	6
3025	WFJC	1380	910	130	4.0	0.018	0.010	35	59	3.18	89	11.0	4
3060	WFJC	2000	190	27	2.0	0.015	0.010	20	32	1.62	62	10.0	3
3070	WFJC	800	190	27	4.0	0.015	0.010	10	34	2.54	64	4.0	1
2160	DRWY A	1030	620	89	3.0	0.026	0.013	30	48	2.44	78	13.4	11
4020 (1)	DFA	2500	265	53	3.0	0.005	0.005	20	38	2.28	68	0.0	4
4010 (1)	DFA	900	280	56	3.0	0.005	0.005	20	38	2.28	68	0.0	0

(1) Grasslined channel section

WEST FORK JIMMY CAMP CREEK DRAINAGE BASIN PLANNING STUDY

UNIT COSTS

SUMMARY OF CHANNEL IMPROVEMENTS
RIPRAP LINED

DEPTH	MAINT. RD (\$/FT)	CHAN EX. (\$/FT)	REVEG (\$/FT^2)	RIPRAP (\$/FT.)
1	\$15	\$10	\$0.25	\$72
2	\$15	\$15	\$0.25	\$99
3	\$15	\$18	\$0.25	\$126
4	\$15	\$19	\$0.25	\$144
5	\$15	\$19	\$0.25	\$171

NUMBER OF DROP STRUC.	CHANNEL NUMBER	MAINT. RD (\$/FT.)	CHAN EX (\$/YD^3)	REVEG (\$/FT^2)	RIPRAP (\$/FT.)	COST PER FT.	CHECK & DROP STRUCTURES	TOTAL CONST. COST, CHANNEL & DROPS	ENGR. & CONT	O&M
0	5012	\$15	\$19	\$5	\$171	\$210	\$0	\$293,300	\$43,995	\$5,600
0	5011	\$15	\$19	\$5	\$171	\$210	\$84,000	\$350,065	\$52,510	\$5,080
0	5010	\$15	\$19	\$3	\$144	\$181	\$120,600	\$491,650	\$73,748	\$8,200
0	3110	\$15	\$19	\$3	\$144	\$181	\$40,200	\$197,670	\$29,651	\$3,480
0	3040	\$15	\$18	\$2	\$126	\$160	\$191,400	\$567,400	\$85,110	\$9,400
0	3030-1	\$15	\$19	\$3	\$144	\$181	\$163,500	\$355,360	\$53,304	\$4,240
0	3030-2	\$15	\$19	\$3	\$144	\$181	\$249,600	\$412,500	\$61,875	\$3,600
0	3000	\$15	\$18	\$2	\$126	\$160	\$189,000	\$705,800	\$105,870	\$12,920
0	3005	\$15	\$18	\$2	\$126	\$160	\$174,000	\$654,000	\$98,100	\$12,000
0	3012	\$15	\$18	\$2	\$126	\$160	\$69,600	\$389,600	\$58,440	\$8,000
0	3015	\$15	\$19	\$3	\$144	\$181	\$71,100	\$351,650	\$52,748	\$6,200
0	3021	\$15	\$18	\$2	\$126	\$160	\$104,400	\$384,400	\$57,660	\$7,000
0	3025	\$15	\$19	\$3	\$144	\$181	\$94,800	\$344,580	\$51,687	\$5,520
0	3060	\$15	\$15	\$0	\$99	\$129	\$46,800	\$304,800	\$45,720	\$8,000
0	3070	\$15	\$19	\$3	\$144	\$181	\$16,200	\$161,000	\$24,150	\$3,200
0	2160	\$15	\$18	\$2	\$126	\$160	\$224,400	\$389,200	\$58,380	\$4,120
0	4020 (1)	\$15	\$18	\$10	\$0	\$43	\$69,600	\$175,850	\$26,378	\$10,000
0	4010 (1)	\$15	\$18	\$10	\$0	\$43	\$0	\$38,250	\$5,738	\$3,600
								\$6,352,975	\$952,946	\$106,560

TOTAL COST OF IMPROVEMENTS \$7,305,921

CURRENT DATE: 07-07-2000
CURRENT TIME: 12:17:35

FILE DATE: 07-07-2000
FILE NAME: 5010

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.20	80.00	5 RCB	15.00	6.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 5010 DATE: 07-07-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0		0 1
102.66	350	350	0	0	0	0	0		0 1
102.96	700	700	0	0	0	0	0		0 1
103.25	1050	1050	0	0	0	0	0		0 1
103.74	1400	1400	0	0	0	0	0		0 1
104.32	1750	1750	0	0	0	0	0		0 1
104.88	2100	2100	0	0	0	0	0		0 1
105.41	2450	2450	0	0	0	0	0		0 1
105.93	2800	2800	0	0	0	0	0		0 1
106.45	3150	3150	0	0	0	0	0		0 1
106.71	3320	3320	0	0	0	0	0		0 1
110.00	5186	5186	0	0	0	0	0	0 OVERTOPPING	

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 5010 DATE: 07-07-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
102.66	0.00	350	0	0.00
102.96	0.00	700	0	0.00
103.25	0.00	1050	0	0.00
103.74	0.00	1400	0	0.00
104.32	0.00	1750	0	0.00
104.88	0.00	2100	0	0.00
105.41	0.00	2450	0	0.00
105.93	0.00	2800	0	0.00
106.45	0.00	3150	0	0.00
106.71	0.00	3320	0	0.00

<1> TOLERANCE (FT) = 0.010

<2> TOLERANCE (%) = 1.000

CURRENT DATE: 07-07-2000
CURRENT TIME: 12:17:35

FILE DATE: 07-07-2000
FILE NAME: 5010

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 5 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.20	0.00	-0.80	0-NF	0.00	100.00	0.00	0.00
350	102.66	100.41	1.50	2.66	6-FF	0.00	0.00	0.00	3.85
700	102.96	101.03	2.38	2.96	6-FF	0.00	0.00	0.00	5.10
1050	103.25	101.52	3.10	3.25	6-FF	0.00	0.00	0.00	6.02
1400	103.74	101.95	3.74	3.55	6-FF	0.00	0.00	0.00	11.72
1750	104.32	102.34	4.32	3.87	6-FF	0.00	0.00	0.00	12.47
2100	104.88	102.69	4.88	4.20	6-FF	0.00	0.00	0.00	13.05
2450	105.41	103.02	5.41	4.56	6-FF	0.00	0.00	0.00	13.60
2800	105.93	103.33	5.93	4.93	6-FF	0.00	0.00	0.00	14.07
3150	106.45	103.62	6.45	5.34	6-FF	0.00	0.00	0.00	14.52
3320	106.71	103.75	6.71	5.54	6-FF	0.00	0.00	0.00	14.71

El. inlet face invert 100.00 ft El. outlet invert 99.20 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

*** SITE DATA ***** CULVERT INVERT *****

INLET STATION (FT)	100.00
INLET ELEVATION (FT)	100.00
OUTLET STATION (FT)	180.00
OUTLET ELEVATION (FT)	99.20
NUMBER OF BARRELS	5.00
SLOPE (V-FT/H-FT)	0.0100
CULVERT LENGTH ALONG SLOPE (FT)	80.00

***** CULVERT DATA SUMMARY *****

BARREL SHAPE	BOX
BARREL SPAN	15.00 FT
BARREL RISE	6.00 FT
BARREL MATERIAL	CONCRETE
BARREL MANNING'S N	0.012
INLET TYPE	CONVENTIONAL
INLET EDGE AND WALL	SQUARE EDGE (90-45 DEG.)
INLET DEPRESSION	NONE

CURRENT DATE: 07-07-2000
CURRENT TIME: 12:17:35

FILE DATE: 07-07-2000
FILE NAME: 5010

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 80.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.004
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.20
CULVERT NO.1 OUTLET INVERT ELEVATION 99.20 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.20	0.000	0.00	0.00
350.00	100.41	0.552	3.45	0.30
700.00	101.03	0.583	4.48	0.46
1050.00	101.52	0.600	5.19	0.58
1400.00	101.95	0.612	5.76	0.69
1750.00	102.34	0.621	6.24	0.78
2100.00	102.69	0.627	6.65	0.87
2450.00	103.02	0.633	7.02	0.95
2800.00	103.33	0.637	7.35	1.03
3150.00	103.62	0.641	7.65	1.10
3320.00	103.75	0.643	7.79	1.14

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 80.00
CREST LENGTH (FT) 200.00
OVERTOPPING CREST ELEVATION (FT) 110.00

442 E SWEETELS RD
 MESA PUNGE ROADWAY, DRAW 1

CURRENT DATE: 03-21-2000
 CURRENT TIME: 13:56:48

FILE DATE: 03-21-2000
 FILE NAME: 2160-1

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	2 RCB	8.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 2160-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.94	70	70	0	0	0	0	0	0	1
102.25	140	140	0	0	0	0	0	0	1
102.67	210	210	0	0	0	0	0	0	1
103.25	280	280	0	0	0	0	0	0	1
103.78	350	350	0	0	0	0	0	0	1
104.27	420	420	0	0	0	0	0	0	1
104.76	490	490	0	0	0	0	0	0	1
105.26	560	560	0	0	0	0	0	0	1
105.70	620	620	0	0	0	0	0	0	1
106.33	700	700	0	0	0	0	0	0	1
108.00	882	882	0	0	0	0	0	0	1

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 2160-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.94	0.00	70	0	0.00
102.25	0.00	140	0	0.00
102.67	0.00	210	0	0.00
103.25	0.00	280	0	0.00
103.78	0.00	350	0	0.00
104.27	0.00	420	0	0.00
104.76	0.00	490	0	0.00
105.26	0.00	560	0	0.00
105.70	0.00	620	0	0.00
106.33	0.00	700	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

PRINT DATE: 03-21-2000
PRINT TIME: 13:56:48

FILE DATE: 03-21-2000
FILE NAME: 2160-1

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 2 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
70	101.94	99.65	1.28	1.94	6-FF	0.00	0.00	0.00	5.19
140	102.25	99.99	2.03	2.25	6-FF	0.00	0.00	0.00	6.54
210	102.67	100.25	2.67	2.55	6-FF	0.00	0.00	0.00	10.59
280	103.25	100.48	3.25	2.87	6-FF	0.00	0.00	0.00	11.52
350	103.78	100.69	3.78	3.22	6-FF	0.00	0.00	0.00	12.18
420	104.27	100.88	4.27	3.59	6-FF	0.00	0.00	0.00	12.77
490	104.76	101.06	4.76	3.99	6-FF	0.00	0.00	0.00	13.30
560	105.26	101.22	5.26	4.43	6-FF	0.00	0.00	0.00	13.75
620	105.70	101.36	5.70	4.83	6-FF	0.00	0.00	0.00	14.11
700	106.33	101.53	6.33	5.40	6-FF	0.00	0.00	0.00	14.56

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

* ** SITE DATA ***** CULVERT INVERT *****
INLET STATION (FT) 100.00
INLET ELEVATION (FT) 100.00
OUTLET STATION (FT) 200.00
OUTLET ELEVATION (FT) 99.00
NUMBER OF BARRELS 2.00
SLOPE (V-FT/H-FT) 0.0100
CULVERT LENGTH ALONG SLOPE (FT) 100.01

***** CULVERT DATA SUMMARY *****
BARREL SHAPE BOX
BARREL SPAN 8.00 FT
BARREL RISE 5.00 FT
BARREL MATERIAL CONCRETE
BARREL MANNING'S N 0.012
INLET TYPE CONVENTIONAL
INLET EDGE AND WALL SQUARE EDGE (30-75 DEG. FLARE)
INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
 CURRENT TIME: 13:56:48

FILE DATE: 03-21-2000
 FILE NAME: 2160-1

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
 BOTTOM WIDTH (FT) 40.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.005
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 99.00
 CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
70.00	99.65	0.556	2.55	0.20
140.00	99.99	0.586	3.30	0.31
210.00	100.25	0.603	3.83	0.39
280.00	100.48	0.614	4.25	0.46
350.00	100.69	0.623	4.59	0.53
420.00	100.88	0.629	4.90	0.59
490.00	101.06	0.635	5.16	0.64
560.00	101.22	0.639	5.40	0.69
620.00	101.36	0.642	5.59	0.74
700.00	101.53	0.646	5.82	0.79

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE	PAVED
EMBANKMENT TOP WIDTH (FT)	45.00
CREST LENGTH (FT)	100.00
OVERTOPPING CREST ELEVATION (FT)	108.00

UNDER MATERIAL: DP 3000

CULVERT @ 3025

CURRENT DATE: 03-21-2000
CURRENT TIME: 12:44:45

FILE DATE: 03-21-2000
FILE NAME: 3000

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	98.80	120.01	2 RCB	12.00	6.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3000 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
102.22	100	100	0	0	0	0	0	0	1
102.50	200	200	0	0	0	0	0	0	1
102.76	300	300	0	0	0	0	0	0	1
103.13	400	400	0	0	0	0	0	0	1
103.65	500	500	0	0	0	0	0	0	1
104.12	600	600	0	0	0	0	0	0	1
104.58	700	700	0	0	0	0	0	0	1
105.01	800	800	0	0	0	0	0	0	1
105.43	900	900	0	0	0	0	0	0	1
105.71	965	965	0	0	0	0	0	0	1
108.00	1458	1458	0	0	0	0	0	0	OVERTOPPING

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3000 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
102.22	0.00	100	0	0.00
102.50	0.00	200	0	0.00
102.76	0.00	300	0	0.00
103.13	0.00	400	0	0.00
103.65	0.00	500	0	0.00
104.12	0.00	600	0	0.00
104.58	0.00	700	0	0.00
105.01	0.00	800	0	0.00
105.43	0.00	900	0	0.00
105.71	0.00	965	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
START TIME: 12:44:45

FILE DATE: 03-21-2000
FILE NAME: 3000

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 2 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	98.80	0.00	-1.20	0-NF	0.00	100.00	0.00	0.00
100	102.22	99.86	1.24	2.22	6-FF	0.00	0.00	0.00	3.94
200	102.50	100.38	1.97	2.50	6-FF	0.00	0.00	0.00	5.28
300	102.76	100.79	2.58	2.76	6-FF	0.00	0.00	0.00	6.28
400	103.13	101.14	3.13	3.02	6-FF	0.00	0.00	0.00	11.74
500	103.65	101.45	3.65	3.29	6-FF	0.00	0.00	0.00	12.51
600	104.12	101.73	4.12	3.58	6-FF	0.00	0.00	0.00	13.16
700	104.58	101.99	4.58	3.88	6-FF	0.00	0.00	0.00	13.67
800	105.01	102.23	5.01	4.20	6-FF	0.00	0.00	0.00	14.17
900	105.43	102.45	5.43	4.53	6-FF	0.00	0.00	0.00	14.59
965	105.71	102.59	5.71	4.76	6-FF	0.00	0.00	0.00	14.87

El. inlet face invert 100.00 ft El. outlet invert 98.80 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

** SITE DATA ***** CULVERT INVERT *****

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INLET STATION (FT)                                100.00
INLET ELEVATION (FT)                            100.00
OUTLET STATION (FT)                             220.00
OUTLET ELEVATION (FT)                          98.80
NUMBER OF BARRELS                               2.00
SLOPE (V-FT/H-FT)                             0.0100
CULVERT LENGTH ALONG SLOPE (FT)              120.01

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***** CULVERT DATA SUMMARY *****

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BARREL SHAPE                                    BOX
BARREL SPAN                                    12.00 FT
BARREL RISE                                     6.00 FT
BARREL MATERIAL                                CONCRETE
BARREL MANNING'S N                            0.012
INLET TYPE                                      CONVENTIONAL
INLET EDGE AND WALL                           SQUARE EDGE (30-75 DEG. FLARE)
INLET DEPRESSION                               NONE

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CURRENT DATE: 03-21-2000
 RENT TIME: 12:44:45

FILE DATE: 03-21-2000
 FILE NAME: 3000

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
 BOTTOM WIDTH (FT) 25.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.005
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 98.80
 CULVERT NO.1 OUTLET INVERT ELEVATION 98.80 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	98.80	0.000	0.00	0.00
100.00	99.86	0.576	3.36	0.33
200.00	100.38	0.597	4.26	0.49
300.00	100.79	0.609	4.87	0.62
400.00	101.14	0.615	5.34	0.73
500.00	101.45	0.620	5.73	0.83
600.00	101.73	0.624	6.06	0.91
700.00	101.99	0.627	6.35	0.99
800.00	102.23	0.629	6.61	1.07
900.00	102.45	0.631	6.85	1.14
965.00	102.59	0.632	6.99	1.18

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE	PAVED
EMBANKMENT TOP WIDTH (FT)	45.00
CREST LENGTH (FT)	100.00
OVERTOPPING CREST ELEVATION (FT)	108.00

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:21:29

FILE DATE: 03-21-2000
 FILE NAME: 3000-2

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	18.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 3000-2 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.88	65	65	0	0	0	0	0	0	1
102.14	130	130	0	0	0	0	0	0	1
102.39	195	195	0	0	0	0	0	0	1
102.85	260	260	0	0	0	0	0	0	1
103.32	325	325	0	0	0	0	0	0	1
103.75	390	390	0	0	0	0	0	0	1
104.17	455	455	0	0	0	0	0	0	1
104.57	520	520	0	0	0	0	0	0	1
104.88	570	570	0	0	0	0	0	0	1
105.39	650	650	0	0	0	0	0	0	1
108.00	992	992	0	0	0	0	0	0	1

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3000-2 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.88	0.00	65	0	0.00
102.14	0.00	130	0	0.00
102.39	0.00	195	0	0.00
102.85	0.00	260	0	0.00
103.32	0.00	325	0	0.00
103.75	0.00	390	0	0.00
104.17	0.00	455	0	0.00
104.57	0.00	520	0	0.00
104.88	0.00	570	0	0.00
105.39	0.00	650	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:21:29

FILE DATE: 03-21-2000
FILE NAME: 3000-2

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 1 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
65	101.88	99.93	1.13	1.88	6-FF	0.00	0.00	0.00	3.88
130	102.14	100.39	1.79	2.14	6-FF	0.00	0.00	0.00	5.21
195	102.39	100.75	2.34	2.39	6-FF	0.00	0.00	0.00	6.20
260	102.85	101.05	2.85	2.64	6-FF	0.00	0.00	0.00	11.29
325	103.32	101.32	3.32	2.90	6-FF	0.00	0.00	0.00	11.95
390	103.75	101.57	3.75	3.18	6-FF	0.00	0.00	0.00	12.57
455	104.17	101.79	4.17	3.48	6-FF	0.00	0.00	0.00	13.11
520	104.57	102.00	4.57	3.80	6-FF	0.00	0.00	0.00	13.56
570	104.88	102.15	4.88	4.05	6-FF	0.00	0.00	0.00	13.90
650	105.39	102.38	5.39	4.49	6-FF	0.00	0.00	0.00	14.38

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

*** SITE DATA ***** CULVERT INVERT *****

INLET STATION (FT)	100.00
INLET ELEVATION (FT)	100.00
OUTLET STATION (FT)	200.00
OUTLET ELEVATION (FT)	99.00
NUMBER OF BARRELS	1.00
SLOPE (V-FT/H-FT)	0.0100
CULVERT LENGTH ALONG SLOPE (FT)	100.01

***** CULVERT DATA SUMMARY *****

BARREL SHAPE	BOX
BARREL SPAN	18.00 FT
BARREL RISE	5.00 FT
BARREL MATERIAL	CONCRETE
BARREL MANNING'S N	0.012
INLET TYPE	CONVENTIONAL
INLET EDGE AND WALL	SQUARE EDGE (30-75 DEG. FLARE)
INLET DEPRESSION	NONE

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:23:44

FILE DATE: 03-21-2000
FILE NAME: 3000-2

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 20.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.005
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
65.00	99.93	0.560	3.07	0.29
130.00	100.39	0.580	3.88	0.43
195.00	100.75	0.590	4.42	0.54
260.00	101.05	0.596	4.85	0.64
325.00	101.32	0.601	5.19	0.72
390.00	101.57	0.604	5.49	0.80
455.00	101.79	0.607	5.75	0.87
520.00	102.00	0.609	5.98	0.94
570.00	102.15	0.610	6.15	0.98
650.00	102.38	0.612	6.39	1.05

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 108.00

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:31:25

FILE DATE: 03-21-2000
FILE NAME: 3005-1

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	9.00	4.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3005-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.28	21	21	0	0	0	0	0	0	1
101.47	42	42	0	0	0	0	0	0	1
101.82	63	63	0	0	0	0	0	0	1
102.19	84	84	0	0	0	0	0	0	1
102.52	105	105	0	0	0	0	0	0	1
102.83	126	126	0	0	0	0	0	0	1
103.14	147	147	0	0	0	0	0	0	1
103.43	168	168	0	0	0	0	0	0	1
103.73	189	189	0	0	0	0	0	0	1
103.95	205	205	0	0	0	0	0	0	1
108.00	444	444	0	0	0	0	0	0	1

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3005-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.28	0.00	21	0	0.00
101.47	0.00	42	0	0.00
101.82	0.00	63	0	0.00
102.19	0.00	84	0	0.00
102.52	0.00	105	0	0.00
102.83	0.00	126	0	0.00
103.14	0.00	147	0	0.00
103.43	0.00	168	0	0.00
103.73	0.00	189	0	0.00
103.95	0.00	205	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:31:25

FILE DATE: 03-21-2000
 FILE NAME: 3005-1

 ***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 1 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
21	101.28	99.58	0.88	1.28	6-FF	0.00	0.00	0.00	4.04
42	101.47	99.86	1.39	1.47	6-FF	0.00	0.00	0.00	5.31
63	101.82	100.07	1.82	1.65	6-FF	0.00	0.00	0.00	8.83
84	102.19	100.26	2.19	1.82	6-FF	0.00	0.00	0.00	9.58
105	102.52	100.42	2.52	2.00	6-FF	0.00	0.00	0.00	10.42
126	102.83	100.57	2.83	2.19	6-FF	0.00	0.00	0.00	10.83
147	103.14	100.70	3.14	2.39	6-FF	0.00	0.00	0.00	11.39
168	103.43	100.83	3.43	2.61	6-FF	0.00	0.00	0.00	11.74
189	103.73	100.94	3.73	2.83	6-FF	0.00	0.00	0.00	12.13
205	103.95	101.03	3.95	3.01	6-FF	0.00	0.00	0.00	12.44

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
 El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

*** SITE DATA ***** CULVERT INVERT *****

INLET STATION (FT) 100.00
 INLET ELEVATION (FT) 100.00
 OUTLET STATION (FT) 200.00
 OUTLET ELEVATION (FT) 99.00
 NUMBER OF BARRELS 1.00
 SLOPE (V-FT/H-FT) 0.0100
 CULVERT LENGTH ALONG SLOPE (FT) 100.01

***** CULVERT DATA SUMMARY *****

BARREL SHAPE BOX
 BARREL SPAN 9.00 FT
 BARREL RISE 4.00 FT
 BARREL MATERIAL CONCRETE
 BARREL MANNING'S N 0.012
 INLET TYPE CONVENTIONAL
 INLET EDGE AND WALL 1:1 BEVEL
 INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:31:25

FILE DATE: 03-21-2000
 FILE NAME: 3005-1

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
 BOTTOM WIDTH (FT) 10.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.010
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 99.00
 CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
21.00	99.58	0.719	3.10	0.36
42.00	99.86	0.742	3.90	0.53
63.00	100.07	0.753	4.43	0.67
84.00	100.26	0.760	4.84	0.79
105.00	100.42	0.765	5.18	0.89
126.00	100.57	0.769	5.46	0.98
147.00	100.70	0.772	5.71	1.06
168.00	100.83	0.774	5.94	1.14
189.00	100.94	0.776	6.14	1.21
205.00	101.03	0.778	6.28	1.27

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE	PAVED
EMBANKMENT TOP WIDTH (FT)	45.00
CREST LENGTH (FT)	100.00
OVERTOPPING CREST ELEVATION (FT)	108.00

DP 3020 e Proposed N/S Collection
 3000-2, 3005-1 & 3010 ALSO

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:05:34

FILE DATE: 03-21-2000
 FILE NAME: 3020-1

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	1 RCB	12.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 3020-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.89	45	45	0	0	0	0	0	0	1
102.15	90	90	0	0	0	0	0	0	1
102.50	135	135	0	0	0	0	0	0	1
102.98	180	180	0	0	0	0	0	0	1
103.44	225	225	0	0	0	0	0	0	1
103.88	270	270	0	0	0	0	0	0	1
104.31	315	315	0	0	0	0	0	0	1
104.73	360	360	0	0	0	0	0	0	1
105.15	405	405	0	0	0	0	0	0	1
105.29	420	420	0	0	0	0	0	0	1
108.00	676	676	0	0	0	0	0	0	OVERTOPPING

Handwritten note: H₂O = 11

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3020-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.89	0.00	45	0	0.00
102.15	0.00	90	0	0.00
102.50	0.00	135	0	0.00
102.98	0.00	180	0	0.00
103.44	0.00	225	0	0.00
103.88	0.00	270	0	0.00
104.31	0.00	315	0	0.00
104.73	0.00	360	0	0.00
105.15	0.00	405	0	0.00
105.29	0.00	420	0	0.00

 <1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

REPORT DATE: 03-21-2000
 REPORT TIME: 14:05:34

FILE DATE: 03-21-2000
 FILE NAME: 3020-1

 ***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 1 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
45	101.89	99.72	1.20	1.89	6-FF	0.00	0.00	0.00	4.93
90	102.15	100.07	1.91	2.15	6-FF	0.00	0.00	0.00	6.21
135	102.50	100.35	2.50	2.40	6-FF	0.00	0.00	0.00	10.42
180	102.98	100.58	2.98	2.65	6-FF	0.00	0.00	0.00	11.25
225	103.44	100.79	3.44	2.92	6-FF	0.00	0.00	0.00	11.92
270	103.88	100.98	3.88	3.19	6-FF	0.00	0.00	0.00	12.53
315	104.31	101.15	4.31	3.49	6-FF	0.00	0.00	0.00	13.05
360	104.73	101.31	4.73	3.80	6-FF	0.00	0.00	0.00	13.51
405	105.15	101.46	5.15	4.13	6-FF	0.00	0.00	0.00	13.93
420	105.29	101.51	5.29	4.25	6-FF	0.00	0.00	0.00	14.07

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
 El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

** SITE DATA ***** CULVERT INVERT *****

INLET STATION (FT) 100.00
 INLET ELEVATION (FT) 100.00
 OUTLET STATION (FT) 200.00
 OUTLET ELEVATION (FT) 99.00
 NUMBER OF BARRELS 1.00
 SLOPE (V-FT/H-FT) 0.0100
 CULVERT LENGTH ALONG SLOPE (FT) 100.01

***** CULVERT DATA SUMMARY *****

BARREL SHAPE BOX
 BARREL SPAN 12.00 FT
 BARREL RISE 5.00 FT
 BARREL MATERIAL CONCRETE
 BARREL MANNING'S N 0.012
 INLET TYPE CONVENTIONAL
 INLET EDGE AND WALL 1:1 BEVEL
 INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
 CURRENT TIME: 14:05:34

FILE DATE: 03-21-2000
 FILE NAME: 3020-1

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
 BOTTOM WIDTH (FT) 15.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.010
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 99.00
 CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
45.00	99.72	0.757	3.64	0.45
90.00	100.07	0.784	4.61	0.67
135.00	100.35	0.797	5.25	0.84
180.00	100.58	0.805	5.75	0.99
225.00	100.79	0.811	6.16	1.12
270.00	100.98	0.815	6.51	1.24
315.00	101.15	0.819	6.82	1.34
360.00	101.31	0.822	7.09	1.44
405.00	101.46	0.824	7.34	1.54
420.00	101.51	0.825	7.42	1.57

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
 EMBANKMENT TOP WIDTH (FT) 45.00
 CREST LENGTH (FT) 100.00
 OVERTOPPING CREST ELEVATION (FT) 108.00

ROADWAY 3020-2
 WEAT COLLECTOR ~~3020-2~~
 1
 FOR RET BASIN OUTLET, 3030

CURRENT DATE: 03-21-2000
 CURRENT TIME: 13:59:27

FILE DATE: 03-21-2000
 FILE NAME: 3020-2

 ***** FHWA CULVERT ANALYSIS *****
 ***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	2 RCB	15.00	8.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

 SUMMARY OF CULVERT FLOWS (CFS) FILE: 3020-2 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
103.59	210	210	0	0	0	0	0	0	1
103.98	420	420	0	0	0	0	0	0	1
104.34	630	630	0	0	0	0	0	0	1
104.70	840	840	0	0	0	0	0	0	1
105.23	1050	1050	0	0	0	0	0	0	1
105.89	1260	1260	0	0	0	0	0	0	1
106.52	1470	1470	0	0	0	0	0	0	1
107.15	1680	1680	0	0	0	0	0	0	1
107.77	1890	1890	0	0	0	0	0	0	1
108.10	2000	2000	0	0	0	0	0	0	1
112.00	3203	3203	0	0	0	0	0	0	1

3020-2 DB 3020-2

 SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3020-2 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
103.59	0.00	210	0	0.00
103.98	0.00	420	0	0.00
104.34	0.00	630	0	0.00
104.70	0.00	840	0	0.00
105.23	0.00	1050	0	0.00
105.89	0.00	1260	0	0.00
106.52	0.00	1470	0	0.00
107.15	0.00	1680	0	0.00
107.77	0.00	1890	0	0.00
108.10	0.00	2000	0	0.00

 <1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
PRINT TIME: 13:59:27

FILE DATE: 03-21-2000
FILE NAME: 3020-2

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 2 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	99.00	0.00	-1.00	0-NF	0.00	100.00	0.00	0.00
210	103.59	99.87	1.83	3.59	6-FF	0.00	0.00	0.00	6.07
420	103.98	100.31	2.89	3.98	6-FF	0.00	0.00	0.00	7.65
630	104.34	100.67	3.78	4.34	6-FF	0.00	0.00	0.00	8.76
840	104.70	100.98	4.54	4.70	6-FF	0.00	0.00	0.00	9.64
1050	105.23	101.26	5.23	5.08	6-FF	0.00	0.00	0.00	14.20
1260	105.89	101.51	5.89	5.47	6-FF	0.00	0.00	0.00	14.87
1470	106.52	101.75	6.52	5.88	6-FF	0.00	0.00	0.00	15.47
1680	107.15	101.98	7.15	6.31	6-FF	0.00	0.00	0.00	16.01
1890	107.77	102.19	7.77	6.76	6-FF	0.00	0.00	0.00	16.50
2000	108.10	102.29	8.10	7.01	6-FF	0.00	0.00	0.00	16.75

El. inlet face invert 100.00 ft El. outlet invert 99.00 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

** SITE DATA ***** CULVERT INVERT *****
INLET STATION (FT) 100.00
INLET ELEVATION (FT) 100.00
OUTLET STATION (FT) 200.00
OUTLET ELEVATION (FT) 99.00
NUMBER OF BARRELS 2.00
SLOPE (V-FT/H-FT) 0.0100
CULVERT LENGTH ALONG SLOPE (FT) 100.01

***** CULVERT DATA SUMMARY *****
BARREL SHAPE BOX
BARREL SPAN 15.00 FT
BARREL RISE 8.00 FT
BARREL MATERIAL CONCRETE
BARREL MANNING'S N 0.012
INLET TYPE CONVENTIONAL
INLET EDGE AND WALL 1:1 BEVEL
INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
 CURRENT TIME: 13:59:27

FILE DATE: 03-21-2000
 FILE NAME: 3020-2

 ***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
 BOTTOM WIDTH (FT) 75.00
 SIDE SLOPE H/V (X:1) 3.0
 CHANNEL SLOPE V/H (FT/FT) 0.005
 MANNING'S N (.01-0.1) 0.030
 CHANNEL INVERT ELEVATION (FT) 99.00
 CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
210.00	99.87	0.588	3.11	0.27
420.00	100.31	0.623	4.05	0.41
630.00	100.67	0.643	4.72	0.52
840.00	100.98	0.657	5.24	0.62
1050.00	101.26	0.667	5.69	0.70
1260.00	101.51	0.675	6.07	0.78
1470.00	101.75	0.682	6.42	0.86
1680.00	101.98	0.687	6.73	0.93
1890.00	102.19	0.692	7.01	1.00
2000.00	102.29	0.694	7.15	1.03

 ***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
 EMBANKMENT TOP WIDTH (FT) 45.00
 CREST LENGTH (FT) 112.00
 OVERTOPPING CREST ELEVATION (FT) 112.00

DETENTION BASIN 30601
OUTLET PIPE

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:01:36

FILE DATE: 03-21-2000
FILE NAME: 3060-1

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	98.80	120.01	1 RCP	5.00	5.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3060-1 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.89	18	18	0	0	0	0	0	0	1
102.25	35	35	0	0	0	0	0	0	1
102.86	53	53	0	0	0	0	0	0	1
103.43	70	70	0	0	0	0	0	0	1
103.93	88	88	0	0	0	0	0	0	1
104.41	105	105	0	0	0	0	0	0	1
104.88	123	123	0	0	0	0	0	0	1
105.38	140	140	0	0	0	0	0	0	1
105.91	158	158	0	0	0	0	0	0	1
106.15	165	165	0	0	0	0	0	0	1
110.00	255	255	0	0	0	0	0	0	OVERTOPPING

Handwritten note: 110.00 @ 1.2 cfs for Det Basin Outlet

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3060-1 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.89	0.00	18	0	0.00
102.25	0.00	35	0	0.00
102.86	0.00	53	0	0.00
103.43	0.00	70	0	0.00
103.93	0.00	88	0	0.00
104.41	0.00	105	0	0.00
104.88	0.00	123	0	0.00
105.38	0.00	140	0	0.00
105.91	0.00	158	0	0.00
106.15	0.00	165	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:01:36

FILE DATE: 03-21-2000
FILE NAME: 3060-1

***** CULVERT # 1 *****

PERFORMANCE CURVE FOR 1 BARREL(S)

Q (cfs)	HWE (ft)	TWE (ft)	ICH (ft)	OCH (ft)	FLOW TYPE	CCE (ft)	FCE (ft)	TCE (ft)	VO (fps)
0	100.00	98.80	0.00	-1.20	0-NF	0.00	100.00	0.00	0.00
18	101.89	99.32	1.56	1.89	6-FF	0.00	0.00	0.00	5.20
35	102.25	99.57	2.25	2.21	6-FF	0.00	0.00	0.00	9.21
53	102.86	99.77	2.86	2.52	6-FF	0.00	0.00	0.00	10.36
70	103.43	99.94	3.43	2.84	6-FF	0.00	0.00	0.00	10.99
88	103.93	100.09	3.93	3.20	6-FF	0.00	0.00	0.00	11.67
105	104.41	100.22	4.41	3.59	6-FF	0.00	0.00	0.00	12.12
123	104.88	100.35	4.88	4.01	6-FF	0.00	0.00	0.00	12.62
140	105.38	100.46	5.38	4.47	6-FF	0.00	0.00	0.00	13.06
158	105.91	100.57	5.91	4.97	6-FF	0.00	0.00	0.00	13.44
165	106.15	100.61	6.15	5.19	6-FF	0.00	0.00	0.00	13.62

El. inlet face invert 100.00 ft El. outlet invert 98.80 ft
El. inlet throat invert 0.00 ft El. inlet crest 0.00 ft

* ** SITE DATA ***** CULVERT INVERT *****

INLET STATION (FT) 100.00
INLET ELEVATION (FT) 100.00
OUTLET STATION (FT) 220.00
OUTLET ELEVATION (FT) 98.80
NUMBER OF BARRELS 1.00
SLOPE (V-FT/H-FT) 0.0100
CULVERT LENGTH ALONG SLOPE (FT) 120.01

***** CULVERT DATA SUMMARY *****

BARREL SHAPE CIRCULAR
BARREL DIAMETER 5.00 FT
BARREL MATERIAL CONCRETE
BARREL MANNING'S N 0.012
INLET TYPE CONVENTIONAL
INLET EDGE AND WALL SQUARE EDGE WITH HEADWALL
INLET DEPRESSION NONE

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:01:36

FILE DATE: 03-21-2000
FILE NAME: 3060-1

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 10.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.010
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 98.80
CULVERT NO.1 OUTLET INVERT ELEVATION 98.80 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	98.80	0.000	0.00	0.00
17.50	99.32	0.713	2.91	0.32
35.00	99.57	0.737	3.68	0.48
52.50	99.77	0.749	4.19	0.61
70.00	99.94	0.756	4.58	0.71
87.50	100.09	0.761	4.90	0.80
105.00	100.22	0.765	5.18	0.89
122.50	100.35	0.768	5.42	0.96
140.00	100.46	0.771	5.63	1.03
157.50	100.57	0.773	5.83	1.10
165.00	100.61	0.774	5.91	1.13

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 30.00
CREST LENGTH (FT) 110.00
OVERTOPPING CREST ELEVATION (FT) 110.00

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:03:30

FILE DATE: 03-21-2000
FILE NAME: 3060-2

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
	1	100.00	99.00	100.01	1 RCB	8.00	4.00	.012
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (CFS) FILE: 3060-2 DATE: 03-21-2000

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
101.30	20	20	0	0	0	0	0	0	1
101.50	40	40	0	0	0	0	0	0	1
101.90	60	60	0	0	0	0	0	0	1
102.28	80	80	0	0	0	0	0	0	1
102.63	100	100	0	0	0	0	0	0	1
102.96	120	120	0	0	0	0	0	0	1
103.28	140	140	0	0	0	0	0	0	1
103.60	160	160	0	0	0	0	0	0	1
103.91	180	180	0	0	0	0	0	0	1
104.15	195	195	0	0	0	0	0	0	1
108.00	395	395	0	0	0	0	0	0	1

SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3060-2 DATE: 03-21-2000

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
101.30	0.00	20	0	0.00
101.50	0.00	40	0	0.00
101.90	0.00	60	0	0.00
102.28	0.00	80	0	0.00
102.63	0.00	100	0	0.00
102.96	0.00	120	0	0.00
103.28	0.00	140	0	0.00
103.60	0.00	160	0	0.00
103.91	0.00	180	0	0.00
104.15	0.00	195	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
CURRENT TIME: 14:03:30

FILE DATE: 03-21-2000
FILE NAME: 3060-2

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****

BOTTOM WIDTH (FT) 10.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.010
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
20.00	99.56	0.718	3.05	0.35
40.00	99.83	0.741	3.84	0.52
60.00	100.05	0.752	4.36	0.65
80.00	100.23	0.759	4.77	0.76
100.00	100.39	0.764	5.10	0.86
120.00	100.53	0.768	5.38	0.95
140.00	100.66	0.771	5.63	1.03
160.00	100.78	0.773	5.86	1.11
180.00	100.89	0.775	6.06	1.18
195.00	100.98	0.777	6.20	1.23

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 108.00

Proposed Culvert in
The Crossings

CURRENT DATE: 03-21-2000
CURRENT TIME: 13:53:01

FILE DATE: 03-21-2000
FILE NAME: 3110-1

***** FHWA CULVERT ANALYSIS *****
***** HY-8, VERSION 3.2 *****

C U L V #	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (FT)	OUTLET ELEV. (FT)	CULVERT LENGTH (FT)	BARRELS SHAPE MATERIAL	SPAN (FT)	RISE (FT)	MANNING n	INLET TYPE
1	100.00	99.00	100.01	5 RCB	15.00	6.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

***** SUMMARY OF CULVERT FLOWS (CFS) FILE: 3110-1 DATE: 03-21-2000 *****

ELEV (FT)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
100.00	0	0	0	0	0	0	0	0	1
102.45	340	340	0	0	0	0	0	0	1
102.74	680	680	0	0	0	0	0	0	1
103.02	1020	1020	0	0	0	0	0	0	1
103.32	1360	1360	0	0	0	0	0	0	1
103.86	1700	1700	0	0	0	0	0	0	1
104.36	2040	2040	0	0	0	0	0	0	1
104.84	2380	2380	0	0	0	0	0	0	1
105.31	2720	2720	0	0	0	0	0	0	1
105.77	3060	3060	0	0	0	0	0	0	1
106.13	3320	3320	0	0	0	0	0	0	1
115.00	7658	7658	0	0	0	0	0	0	1

***** 0 OVERTOPPING *****

***** SUMMARY OF ITERATIVE SOLUTION ERRORS FILE: 3110-1 DATE: 03-21-2000 *****

HEAD ELEV (FT)	HEAD ERROR (FT)	TOTAL FLOW (CFS)	FLOW ERROR (CFS)	% FLOW ERROR
100.00	0.00	0	0	0.00
102.45	0.00	340	0	0.00
102.74	0.00	680	0	0.00
103.02	0.00	1020	0	0.00
103.32	0.00	1360	0	0.00
103.86	0.00	1700	0	0.00
104.36	0.00	2040	0	0.00
104.84	0.00	2380	0	0.00
105.31	0.00	2720	0	0.00
105.77	0.00	3060	0	0.00
106.13	0.00	3320	0	0.00

<1> TOLERANCE (FT) = 0.010 <2> TOLERANCE (%) = 1.000

CURRENT DATE: 03-21-2000
PRINT TIME: 13:53:01

FILE DATE: 03-21-2000
FILE NAME: 3110-1

***** TAILWATER *****

***** REGULAR CHANNEL CROSS SECTION *****
BOTTOM WIDTH (FT) 90.00
SIDE SLOPE H/V (X:1) 3.0
CHANNEL SLOPE V/H (FT/FT) 0.005
MANNING'S N (.01-0.1) 0.030
CHANNEL INVERT ELEVATION (FT) 99.00
CULVERT NO.1 OUTLET INVERT ELEVATION 99.00 FT

***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (CFS)	W.S.E. (FT)	FROUDE NUMBER	VEL. (FPS)	SHEAR (PSF)
0.00	99.00	0.000	0.00	0.00
340.00	100.04	0.606	3.51	0.32
680.00	100.57	0.642	4.57	0.49
1020.00	101.00	0.663	5.32	0.62
1360.00	101.37	0.677	5.91	0.74
1700.00	101.70	0.687	6.41	0.84
2040.00	102.01	0.696	6.85	0.94
2380.00	102.29	0.703	7.24	1.03
2720.00	102.56	0.708	7.59	1.11
3060.00	102.82	0.713	7.91	1.19
3320.00	103.00	0.717	8.14	1.25

***** ROADWAY OVERTOPPING DATA *****

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH (FT) 45.00
CREST LENGTH (FT) 100.00
OVERTOPPING CREST ELEVATION (FT) 115.00

EVALUATION OF ALTERNATIVES

APPENDIX C

Evaluation of Conceptual Alternatives

Parameter	Flood Control		Disturbance	Environmental		Operations and Maintenance	
	Reduced Hazard	No change		Limited or No Impact	Mitigation required	Reduced effort	Increased effort
Alternative Concept: Floodplain Preservation (do nothing)							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative		Limited area of wetland or riparian area exist in these segments			Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Residential and commercial property not impacted by the 100-year event at this time.		This concept works well for the segments that have significant wetland and riparian resources	Limited mitigation with this concept		Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
WFJCC NO. OF FONTAINE BLVD		Residential and commercial property not impacted by the 100-year event at this time.		Limited area of wetland or riparian area exist in these segments			Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
Alternative Concept: Channelization							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Channelization of 100-year flow in these segments would benefit developed properties along Marksheffel Road, south of the basin			Limited habitat now exists along drainageway.			More maintenance of linings would result with this alternative
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Little change in floodplain widths would result from channelizing in these segments. Residential and commercial property not impacted by the 100-year event at this time.	Construction of channel linings will disturb existing wetland and riparian resource		Mitigation would be required wherever the channelization altered or eliminated wetland habitat		More maintenance of linings would result with this alternative
WFJCC NO OF FONTAINE BLVD		Residential and commercial property not impacted by the 100-year event at this time.		Limited impacts due to absence of wetland areas in these segments.	Mitigation would be required wherever the channelization altered or eliminated wetland habitat		More maintenance of linings would result with this alternative

Parameter	Open Space/Aesthetics		Implementation			Comments	Relative Advantages/Disadvantages of Alternative Concept
	Enhancement or Degradation	No change	Low Feasibility	Moderate Feasibility	High Feasibility		
Alternative Concept: Floodplain Preservation (do nothing)							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Poorly defined drainageway in these segments the enhancement opportunities in these segments	X			Floodplain preservation in these segments does not address the significant areas within the existing floodplain especially south of the basin	Ad: Low initial cost of improvements DAd: Large area within 100-year floodplain not addressed by this concept.
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Floodplain preservation has the opportunity if implemented to preserve the natural floodplain that can enhance the open space			X		When combined with preservation of the floodplain as open space, this concept has positive benefits to the existing wetland and riparian resources in these segments.	Ad: Reduces the need for mitigation along these segments with regard to permitting of the proposed stormwater facilities
WFJCC NO. OF FONTAINE BLVD	Opportunities for vegetative and visual enhancement of drainageway is limited by the naturally narrow drainageways			X			Ad: Low initial cost
Alternative Concept: Channelization							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Planting along new channel could enhance the drainageway as open space. No vegetation now exists.				X	Development of land adjacent to these segments will require that a channel be constructed	Ad: Greatly reduces extent of 100-year floodplain within these segments and offsite from the basin DAd: Land acquisition will be required through area downstream of Marksheffel Road
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Naturalness of existing drainageway could be viewed as a degradation			X			Ad: Reduces width of drainageway compared to the 100-year floodplain widths DAd: Moderate to high initial costs and would cause disturbance to wetland and riparian areas that would not have to be mitigated for.
WFJCC NO OF FONTAINE BLVD	Naturalness of existing drainageway could be viewed as a degradation			X			Ad: Reduces width of drainageway compared to 100-year floodplain widths DAd: Moderate to high initial costs

Evaluation of Conceptual Alternatives

Parameter	Flood Control		Environmental			Operations and Maintenance	
	Reduced Hazard	No change	Disturbance	Limited or No impact	Mitigation required	Reduced effort	Increased effort
Alternative Concept: Onsite Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property impacted by the 100-year floodplain are not benefitted by this alternative w/o channelization		Limited area of wetland or riparian area exist in these segments			Increase maintenance compared to regional detention
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.	Disturbance wetland & riparian areas could result from the construction of embankments for the on-stream detention basins		Mitigation would be required within detention basin bottom for onstream sites		Increase maintenance compared to regional detention
WFJCC NO. OF FONTAINE BLVD	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.		Limited area of wetland or riparian area exist in these segments	Mitigation would be required within detention basin bottom for onstream sites		Increase maintenance compared to regional detention
Alternative Concept: Regional Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property impacted by the 100-year floodplain are not benefitted by this alternative w/o channelization	Disturbance of wetland & riparian areas could result from the construction of embankments for the on-stream detention basins			Lower maintenance compared to onsite detention concept	
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.	Disturbance of wetland and riparian areas could result from the construction of embankments for the on-stream detention basins		Mitigation would be required wherever the construction altered or eliminated wetland habitat	Lower maintenance compared to onsite detention concept	
WFJCC NO OF FONTAINE BLVD	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.		Limited impacts due to absence of wetland areas in these segments.		Lower maintenance compared to onsite detention concept	

Parameter	Open Space/Aesthetics		Implementation			Comments	Relative Advantages/Disadvantages of Alternative Concept
	Enhancement or Degradation	No change	Low Feasibility	Moderate Feasibility	High Feasibility		
Alternative Concept: Onsite Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Limited enhancement of existing wetlands would result from this concept	X			Onsite detention areas could be used to create new wetland areas if site conditions are there to create a wetland/riparian resource	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: Hydrologic impact of onsite detention is potentially not as effective as regional detention concept Private maintenance agreements would be required
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Limited enhancement of existing wetlands would result from this concept		X		Onsite detention areas could be used to as mitigation sites for wetland or riparian resources disturbed by channelization of drainageway	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: Hydrologic impact of onsite detention is potentially not as effective as regional detention concept Private maintenance agreements would be required
WFJCC NO. OF FONTAINE BLVD		Limited enhancement of existing wetlands would result from this concept	X			Numerous small drainageways in this portion of the basin would create the need for numerous onsite detention basins	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: This concept conflicts with the Colorado Centre master drainage plan approved by the City of Colorado Springs
Alternative Concept: Regional Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Offstream basins could be used to expand the wetland resources to areas off of the main drainageway			X		Dedication of land could be achieved through development process	Ad: Fewer sites, less maintenance compared to onsite detention concept. Conforms with Cross Creek MDDP DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Offstream basins could be used to expand the wetland resources to areas off of the main drainageway			X		Dedication of land could be achieved through development process	Ad: Fewer sites, less maintenance compared to onsite detention concept DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments
WFJCC NO OF FONTAINE BLVD					X	Natural detention area now exists north of Fontaine Boulevard	Ad: Fewer sites, less maintenance compared to onsite detention concept. Conforms with Colorado Centre MDDP DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments

Evaluation of Conceptual Alternatives

Parameter	Flood Control		Environmental			Operations and Maintenance	
	Reduced Hazard	No change	Disturbance	Limited or No impact	Mitigation required	Reduced effort	Increased effort
Alternative Concept: Onsite Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative w/o channelization		Limited area of wetland or riparian area exist in these segments			Increase maintenance compared to regional detention
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.	Disturbance wetland & riparian areas could result from the construction of embankments for the on-stream detention basins		Mitigation would be required within detention basin bottom for onstream sites		Increase maintenance compared to regional detention
WFJCC NO. OF FONTAINE BLVD	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.		Limited area of wetland or riparian area exist in these segments	Mitigation would be required within detention basin bottom for onstream sites		Increase maintenance compared to regional detention
Alternative Concept: Regional Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative w/o channelization	Disturbance of wetland & riparian areas could result from the construction of embankments for the on-stream detention basins			Lower maintenance compared to onsite detention concept	
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.	Disturbance of wetland and riparian areas could result from the construction of embankments for the on-stream detention basins		Mitigation would be required wherever the construction altered or eliminated wetland habitat	Lower maintenance compared to onsite detention concept	
WFJCC NO OF FONTAINE BLVD	Implementation of onsite detention would have no effect upon reducing existing 100-year floodplain widths	Residential and commercial property not impacted by the 100-year event at this time.		Limited impacts due to absence of wetland areas in these segments.		Lower maintenance compared to onsite detention concept	

Parameter	Open Space/Aesthetics		Implementation			Comments	Relative Advantages/Disadvantages of Alternative Concept
	Enhancement or Degradation	No change	Low Feasibility	Moderate Feasibility	High Feasibility		
Alternative Concept: Onsite Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Limited enhancement of existing wetlands would result from this concept	X			Onsite detention areas could be used to create new wetland areas if site conditions are there to create a wetland/riparian resource	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: Hydrologic impact of onsite detention is potentially not as effective as regional detention concept Private maintenance agreements would be required
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY		Limited enhancement of existing wetlands would result from this concept		X		Onsite detention areas could be used to as mitigation sites for wetland or riparian resources disturbed by channelization of drainageway	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: Hydrologic impact of onsite detention is potentially not as effective as regional detention concept Private maintenance agreements would be required
WFJCC NO. OF FONTAINE BLVD		Limited enhancement of existing wetlands would result from this concept	X			Numerous small drainageways in this portion of the basin would create the need for numerous onsite detention basins	Ad: May work best in a phased development scenario compared to regional concept Lower initial costs DAd: This concept conflicts with the Colorado Centre master drainage plan approved by the City of Colorado Springs
Alternative Concept: Regional Detention							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Offstream basins could be used to expand the wetland resources to areas off of the main drainageway			X		Dedication of land could be achieved through development process	Ad: Fewer sites, less maintenance compared to onsite detention concept. Conforms with Cross Creek MDDP DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments
WFJCC & DRWY A SEGMENTS NO. OF MESA RIDGE PRKWY	Offstream basins could be used to expand the wetland resources to areas off of the main drainageway			X		Dedication of land could be achieved through development process	Ad: Fewer sites, less maintenance compared to onsite detention concept DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments
WFJCC NO OF FONTAINE BLVD					X	Natural detention area now exists north of Fontaine Boulevard	Ad: Fewer sites, less maintenance compared to onsite detention concept. Conforms with Colorado Centre MDDP DAd: Phasing of detention basin construction could hinder implementation and administration of system by local governments

Evaluation of Conceptual Alternatives

Parameter	Flood Control		Disturbance	Environmental		Operations and Maintenance	
	Reduced Hazard	No change		Limited or No impact	Mitigation required	Reduced effort	Increased effort
Alternative Concept: Floodplain Preservation (do nothing)							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Residential and commercial property impacted by the 100-year floodplain are not benefited by this alternative		Limited area of wetland or riparian area exist in these segments			Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
WFJCC & DRWY A SEGMENTS NO, OF MESA RIDGE PRKWY		Residential and commercial property not impacted by the 100-year event at this time.		This concept works well for the segments that have significant wetland and riparian resources	Limited mitigation with this concept		Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
WFJCC NO, OF FONTAINE BLVD		Residential and commercial property not impacted by the 100-year event at this time.		Limited area of wetland or riparian area exist in these segments			Unlined banks increase annual annual maintenance responsibility as flows increase in duration and frequency
Alternative Concept: Channelization							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Channelization of 100-year flow in these segments would benefit developed properties along Marksheffel Road, south of the basin			Limited habitat now exists along drainageway.			More maintenance of linings would result with this alternative
WFJCC & DRWY A SEGMENTS NO, OF MESA RIDGE PRKWY		Little change in floodplain width would result from channelizing in these segments Residential and commercial property not impacted by the 100-year event at this time.	Construction of channel linings will disturb existing wetland and riparian resource		Mitigation would be required wherever the channelization altered or eliminated wetland habitat		More maintenance of linings would result with this alternative
WFJCC NO OF FONTAINE BLVD		Residential and commercial property not impacted by the 100-year event at this time.		Limited impacts due to absence of wetland areas in these segments.	Mitigation would be required wherever the channelization altered or eliminated wetland habitat		More maintenance of linings would result with this alternative

Parameter	Open Space/Aesthetics		Implementation			Comments	Relative Advantages/Disadvantages of Alternative Concept
	Enhancement or Degradation	No change	Low Feasibility	Moderate Feasibility	High Feasibility		
Alternative Concept: Floodplain Preservation (do nothing)							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160		Poorly defined drainageway in these segments the enhancement opportunities in these segments	X			Floodplain preservation in these segments does not address the significant areas within the existing floodplain especially south of the basin	Ad: Low initial cost of improvements DAd: Large area within 100-year floodplain not addressed by this concept.
WFJCC & DRWY A SEGMENTS NO, OF MESA RIDGE PRKWY	Floodplain preservation has the opportunity if implemented to preserve the natural floodplain that can enhance the open space			X		When combined with preservation of the floodplain as open space, this concept has positive benefits to the existing wetland and riparian resources in these segments.	Ad: Reduces the need for mitigation along these segments with regard to permitting of the proposed stormwater facilities
WFJCC NO, OF FONTAINE BLVD	Opportunities for vegetative and visual enhancement of drainageway is limited by the naturally narrow drainageways			X			Ad: Low initial cost
Alternative Concept: Channelization							
WFJCC & DRWY A SEGMENTS 5010, 3110 2160	Planting along new channel could enhance the drainageway as open space. No vegetation now exists.				X	Development of land adjacent to these segments will require that a channel be constructed	Ad: Greatly reduces extent of 100-year floodplain within these segments and offsite from the basin DAd: Land acquisition will be required through area downstream of Marksheffel Road
WFJCC & DRWY A SEGMENTS NO, OF MESA RIDGE PRKWY	Naturalness of existing drainageway could be viewed as a degradation			X			Ad: Reduces width of drainageway compared to the 100-year floodplain widths DAd: Moderate to high initial costs and would cause disturbance to wetland and riparian areas that would not have to be mitigated for.
WFJCC NO OF FONTAINE BLVD	Naturalness of existing drainageway could be viewed as a degradation			X			Ad: Reduces width of drainageway compared to the 100-year floodplain widths DAd: Moderate to high initial costs