B. HYDRAULICS

1. General: Hydraulic computations are included in this section for all storm sewers, major and minor greenbelts. Lined drainage swales are shown on the enclosed drainage plan as representative only and are not designed herein.

2. Greenbelt: The following is a summary of the major greenbelt design flows.

5 0	ζ,	Design	
Hydrograph Point	Design Control Point	Runoff CFS	Undeveloped Runoff CFS
1		158.4	212.5
	1	158.4	212.5
	2	166.2	234.5
2		167.4	256.0
	3	118.7	266.6
3		313.5	289.6
	4	315.4	343.7
	5	326.7	396.1
	6	349.8	471.1
	7	383.7	596.8
4		401.1	616.1
	8	411.6	651.5
	9	439.3	662.5
	10	471.8	677.2
	11	514.2	687.0
	12	544.3	718.6
5		640.4	735.6

The greenbelt design flows below the dam are controlled by the outlet works of the dam, since emergency releases will govern over spillway discharges.

Since the flows within the majority of this greenbelt are below those of the natural, undeveloped, state, no drainage facilities other than at road crossings are proposed. Special considerations must be given to control inlet flows to prevent bank erosion. Lined channels and gabion drop structures are specified where erosion within the primary greenbelt is considered possible.

The following are the principal minor greenbelt design flows:

Area	Location	Design Flow-CFS
IV	13	114.2
	14	114.2
V	рТ6	128.2/105.0
	15	105.0
	16	141.1
	17	155.5
	18	170.8

Area	Location	Design Flow-CFS
N. T. W.	19 20	178.3 192.4
XIII	C D Outfall	31.2 51.5 51.5

The following are the allowable spacings for 6-foot drop gabion units in the minor greenbelt, where erosion control is considered mandatory.

Point	Natural Slope	Allowable Slope	Spacing
From-To	<u>0</u>	%	Ft.
VI-15	2.81	1.00	331
15-16	2.95	1.00	308
16-17	2.79	1.00	335
17-18	2.34	1.00	448
18-19	2.22	1.00	492
19-20	1.82	1.00	732

The following are the capacities of the existing culverts near the outfall point of the primary basin. This analysis is based on detailed as-builts in previous basin studies, which conflict with information on other plans.

<u>Culvert Location</u>	Type	Capacity-CFS	Required head-FT
Cascade Avenue	10'x10' Conc.	Arch 900.45* Arch 812.85*	2.87
I - 25	10'x10' R.C.B	2544.1*	11.00

* Because of the interelation of these culverts, the capacity will be limited by the tailwater depths, which will limit the effective head. It may be concluded that a maximum of 1400 CFS will be contained by the three culverts without overtopping existing roadways.

The outlet works of the dam is limited in design so that emergency reservoir releases will not overtop the roadways below. Two 60-inch RCP culvert are tentatively provided with Howell-Bunger valves or other energy dissipator means. The reservoir can then be drained, under no inflow conditions, in about 21-1/2 hours.

The culvert on lower Rockrimmon Boulevard is designed for the 1400 CFS maximum release, a 10-foot by 7-foot reinforced concrete box being provided.

The above structures below the major dam are subject to redesign under detailed studies related to emergency release criteria for the dam. 3. Monument Creek: A detailed study of the Monument Creek channel has been made to determine its capability of withstanding the predicted 100 year storm of the Corns of Engineers. It has been requested that the channel be left in its existing state, and this request has been approved by the City Engineer and the City Drainage Board. This will require controlled drainage inlets to prevent bank erosion in susceptable areas, similar to that shown on the enclosed Plate Cl.

A portion of the area in the Southeast part of the flood plain is applicable to the proposed City Flood Plain ordinance pertaining to Floodway Fringe Zoning. This area is so designated on Plate Cl. Riprap to provide the prescribed flood protection elevation is shown and included in the cost estimate. It should be noted that the costs (\$ 34,705) attributed to reclaiming this 3.64 Acre parcel may prohibit its development.

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HYDRAULIC COMPUTATIONS BASIN AS PROPOSED (12 SHEETS)

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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
Delmonico Blvd.	XII A			21.2	21.2/30.0		
	XII B-halfway	580	6324 2.41%	30.0	0/30.0	30.0 30.0	2-10' CB 27"CMP @ 1% 30"CMP @ 1.83% min.
•	XII B		6310	41.2	11.2/30.0		
	XIII F	680	5.88% 6270	58.1	28.1/30.0	30.0	24"CMP @ 6.00% min. 2-10' CB 27"CMP @ 1%
	XIII G	560	2.14% 6258	76.1	18.0/30.0	58.1	42''CMP @ 1.14% min.
	Outfall	240	15.0% 6222		0/30.0	58.1	24"CMP @ 22.5% min. Dissipator
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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
I	${ t I}_{ t B}$ in.	1460'	6727.5	76.0	76.0/84.5		
	$I_{\overline{B}}$ cent.	1030'	6668			42	2-16'CB,w/30"CMP(60%)
	Ic cent1		6618		60.0/92.4	-86	1030'-30''CMP-3.57%min 2-12'CB,w/30''CMP(100%)
	Ic cent2	280'	.0071		22.0/34.9	86	280'-54"CMP-0.65%min.
	Ic final	460'	.0478		46/91.4	86	460'-42''CMP-2.92% min.
	I _D outfall	715'	0.0224		58/63.2	144.0	715'-48''CMP-1.22% min. 2-16', CB w/36'' CMP
	I _D greenbelt	225'	0.0533 6566	144.0	0.01 -		225'-48" CMP-3.43%min
		+ 100 m					



Project Checked by 0. Rockrimmon N. H. Patel

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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
II	II C		6650				
		1160'	.0517	90.3	90.3/95.1		
	II D		6590			35	2-16'CB,w/30"CMP(50%)
		690'	.0471	108.9	73.9/90.4		690'-30''CMP-2.48% min.
	II D cent.		6557.5			7.0	2-16'CB,w/30"CMP(50%)
		720 '	.0396	147	77.0/83.4		720'-36''CMP-3.75%min.
	II E cent.		6529			140	2-16'CB,w/36"CMP(100%)
		490'	.0469	162.2	22.2/90.4		490'-48"CMP-3.24% min
	II Ef		6506			C - 1 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2 - 2	2-10'CB,w/24"CMP(70%)
						+38.7	38.7cfs.direct fromII.
						=200.9	
	·	340'	.0471	200.9	0.0/-		340'-54"CMP-3.56% min
	III G cent.		6490				
	III L final		6501			3 8.7	2-12' CB,w/27" CMP
		380	0.0	38.7			385'-54"CMP-0.13% min
	II E final		6506				
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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
				25.1			
Rockrimmon Blvd	IV B in.		6501			22.0	2-12'CB,w/24"CMP(50%)
IV		5851	0.0547	45.4	23.4/30.0		585'-24"CMP-3.22% min
	IV B final		6469			34.0	2-8CB/21"CMP(50%)
		395	0.0456	61.6	27.6/30.0		395'-30''CMP-2.34% min
1	IV C final		6451			39.6	2-4'CB,w/18''CMP(50%)
		205	0.0536	66.5	26.9/30.0		205'-30''CMP-3.17% min
;	IV D final		6440			51.6	2-8'CB,w/21"CMP(50%)
		300	0.0267	71.5	19.9/30.0		300'-36''CMP-20.4% min
	IV E cent.		6432			118.8	67.2cfs direct from
	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1						36"CMP
		195	0.0615	143.7	24.9/30.0		195'-42"CMP-5.55% min
	IV E final		6420			124.4	2-4'CB,w/18''CMP(50%)
		280	0.0357	147.8	23.4/30.0		195'-42"CMP-5.55% min 2-4'CB,w/18"CMP(50%) 280'-48"CMP-2.56% min
	IV F final		6410			158.4	2-12'CB,w/30"CMP(78%)
	· · · · · · · · · · · · · · · · · · ·	100	0.085	158.4	0.0/-		100'-42''CMP-8.44% min.
	IV G final-		6401.5		× • × / · · · · · · · · · · · ·		
	ditch		0401.0	l			
	IV K final	·····	6460			67.2	the state of the s
	IV K IIIIGI	615	0.0455	67.2		07.2	2-16' w/36" CMP 615'-36"CMP-3.46% min
	IV E cent	013	6432	107.2	,	ł	013 50 CMF 3.40% IIIEII
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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
IV	IV M initial		6455	10.0			
	IV M cent.	215	0.0372 6447	17.4	17.4/31.6	17.4	2-8'CB,w/24"CMP(100%)
	IV o cent.	350	0.0400 6433	35.4	18.0/32.8		350'-24"CMP-2.02% min
ockrimmon Blvd		700'	0.0357	65.8	30.4/79.1	35.4	2-8'CB,w/24"CMP(100%) 700'-30"CMP-2.54% min
OCKITHIIIOII BIVQ	V c cent.	590'	6408	79.3	27.9/30.0	51.4	2-10'CB,w/21"CMP(50%) 590'-36"CMP-2.02% min
	V D-ditch		6370			79.3	2-10'CB,w/27"CMP(100%)
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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
	X A initial		6415	0			
el monico Blvd.		595	0.0319				
- X	X A final		6396	19.3		16.0	2-10'CB,w/21"CMP(50%)
		505	0.0218		28.5/30.0	* · · ·	505'-24"CMP-1.71% mir
	X B final		6385	44.5		38.0	2-12'CB,w/24"CMP(50%)
<u> </u>		390	0.0205		28.8/30.0	J. J. O. • U	390'-36"CMP-1.11% mir
	X C final		6377	66.8		46.4	2 6 CD /10 UG (5 0 0)
		240	0.0104		20 0 / 70 0	40.4	2-6'CB,w/18''CMP(50%)
	X D final		6374.5	75.2	28.8/30.0	CO 1	240'-42''CMP-0.85% mir
		670	0.0343	13.2	26.9/30.0	68.4	2-12'CB,w/24"CMP(50%)
	X F final		6351.5	95.3	20.9/30.0	95.3	670'-42"CMP-1.84% mir
İ	· · · · ·	345	0.0188	133.3	OF 7.1	95.5	2-10'CB,w/27" CMP
1	IX G-drainage	J 7 J	6345.0	95.3	95.3/-		345'-48''CMP-1.50% mir
i	ditch		0545.0	93.3			
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N. H. Patel
O. E. Watts

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STREET	LOCATION	DIST	ELEVATION & SLOPE	TOTAL RUNOFF	STREET FLOW CAPACITY	PIPE FLOW	TYPE PIPE, CATCH BASIN & SLOPE %
Rockrimmon Blvd.	XIV A						
	XIV A final		6324	22.0		16.0	2-10'CB,w/21"CMP(50%)
	XIV B final	310	0.0565 6306.5	32.3	16.3/30.0	16.0	310'-21''CMP-3.48% min 465'-21''CMP-3.48% min
	XIV C final	465	0.0409 6287.5	41.6	25.6/30.0		2-10'CB,w/27"CMP(78%)
	XIII D - in	375	0.0987 6250	41.6	0.0/-		375'-30"CMP-3.5% min
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WESTERN ENGINEERS

Culvert 8 Channel Calculations

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AREA	LOCATION & DISTANCE	ELEV & S%	S 1/2	Q 5 O	b 8/3	b	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS
IVa- final	13 590 14 130 Pt. VI 178 15 305 16 520 17 530 18 135 19 220 20	6401.5 0.0466 6374 0.04615 6368 0.02809 6363 0.02951 6354 0.02788 6339.5 0.02358 6327 0.02222 6324 0.01818 6320	0.21589 0.21483 0.16760 0.17178 0.16699 0.15357 0.14907 0.13484	114.23 128.25 105.00 141.13 155.53 170.75 178.34	4.11 4.64 4.869 6.385 7.239 8.641 9.298 11.087	1.70 1.78 1.81 2.01 2.25 2.31 2.47	5.78 6.337 6.552 8.080 8.82 10.125 10.672 12.202	3' x 3' 3' x 3' 4' x 3' 4' x 3' 4' x 3' 4' x 3'	48"RCP A=12.560 ft ²	0.008 0.002 0.010 0.006

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Calc. by N. H. Patel Checked by O. E. Watts

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Culvert Channel Calculations

XII	AREA	LOCATION & DISTANCE	ELEV & S%	S 1/2	Q 50	b 8/3	b ;	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS
965' 0.01036	XII	C-begin 650'		0.10378	31.2	2.337	1.38	3.809	3 x 2		
RR B1vd 6238 200' 0.10500 0.32404 51.5 8.238 2.22 30''CMP RR B1vd 6217 220' 0.0500 0.22361 51.5 1.790 1.25 3.125 3 x 2				0.10178	51.5	3.933	1.67	5.578	3 x 2		
RR Blvd 6217 220' 0.0500 0.22361 51.5 1.790 1.25 3.125 3 x 2			5							7.011.01.05	
The state of the s		RR B1vd	6217							30"CMP	
				0.22361	51.5	1.790	1.25	3.125	3 x 2		
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Culvert & Channel Calculations

AREA	LOCATION & DISTANCE	ELEV & S%	S 1/2	Q 50	b 8/3	b	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS
XVI B	Hwy.R.O.W.	N 4.0%								The state of the s
	350'	use 3.5	0.1871	32.5	1.35	1.12	2.51	1.5'x2.0	127.8	12"
	Flood plain	the second of the second of			D-8/3= 4.88	D=1.81		24''RCP	42.3	
										ľ
XVI E	RR 18" pipe 320	N 3.4% use 3.0	0.1732	55.7	2.50	1.41	3.98	1.5'x2.5'	190.1	12"+
	Rd.culvert	min			D-8/3=	D=2.28		30''RCP	71.0	
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Culvert & Channel Calculations

AREA	LOCATION & DISTANCE	ELEV & S%	S 1/2	Q 50	b 8/3	ь	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS	
I - D	Point I	6551									
	490 1	4.2857 6530	0.2070	158.39	5.947	1.95	7.605	3'x3'*		0.007	WESTERN
	480	2.9167 6516	0.1708	166.21	7.563	2.14	9.159	3'x3'*	#	0.007	
	180	1.6667	0.1291	167.37	10.076	2.38	11.329	3'x3'*		0.003	A company
IIIG	Point II	6513								=0.0] used 0.042	. 7
	340	4.4118 6498	0.2100	118.72	4.394	1.74	6.055	4'x2'		0.005	Calc. Checked
	740	3.7838	0.1945	313.49	12.527	2.58	13.313	4'x3'*	8'x3'RCB	0.009	ked by
IIIG	Point III 310	6470								=0.01 used 0.012	4 y y y
	310 4 300	3.2258 6460 3.3333		315.41	13.649	2.66	14.151	4'x3.5'*		0.004	H.
	5 430	6450		326.74	13.907	2.68	14.365	4'x3.5'*	· · · · · · · · · · · · · · · · · · ·	0.004	Watts
	6 720	3.0233 6437		349.79	15.633	2.80	15.680	4'x3.5'*		0.005	
	720 7 110	2.5000 6419 1.8182		383.66	18.860	3.01	18.120	4'x4'*		0.009	
VIIIG	Point*IV		0.1346	401.07	23.124	3.25	21.125	4'x4'*	7'x4'RCB	$\frac{0.002}{=0.02}$	
	I OTHE IA	*Runoff i	s less tha							1sed 0.023	11-28-72
		channel	in natural	conditi	n, see	eave ext pag	e •				8-72

ENGINEERS

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Culvert 8 Channel Calculations

AREA	LOCATION & DISTANCE	ELEV 8	S 1/2	Q 50	b 8/3	b	S F AREA	USE DITCH	CULVERT ETC.	TIME
VIIIG	Point IV 1090	6417								
	8	3.3027 6381	0.1817	411.63	17.607	2.93	17.170	4'x4'*		0.013
	340	2.6471	0.1627	439.26	20.983	3.13	19.594	4'x4'*	· · · · · · · · · · · · · · · · · · ·	0 004
	9	6372	0.1027	439.40	20.903	J. LJ	19.594	4 X4		0.004
İ	450	2.0000	0.1414	471.82	25.933	3.39	22.984	4'x4'*		0.006
	10	6363								
	300	3.0000	0.1732	514.25	23.076	3.24	20.995	4'x4'*		0.003
	11	6354								
	980	1.9388	0.1392	544.34	30.392	3.60	25.920	5'x4'*		0.013
	12	6335								
TV C	520	3.2692	0.1808	640.35	27.527	3.47	24.082	5'x4'		0.005
IX G	Point V	6318			de la company de					=0.04
										used
					Ser			alicenspecies and in the second		0.032
		1				ar managan in ar mining an against an				
	•	*Runoff	s less th	an in na	tural sta	te-leave	channe	in natura	1	j
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HYDRAULIC COMPUTATIONS BASIN WITH NO STAGING CONSIDERED FOR COMPARITIVE PURPOSES ONLY GREENBELTS ONLY (3 sheets)

Culvert Channel Calculations

AREA	LOCATION & DISTANCE	ELEV 84 S%	S 1/2	Q 50	b 8/3	Ь	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS	1
Major Green-	490	6551									4
belt	1 490	4.29 6530	0.2070	225.9	8.482	2.23	9.95	3 x 3			
DOIL	480	2.92	0.1708	245 2						er er for ogsårer	
	2	6516	0.1/08	245.2	11.157	2.47	12.20	3×3.5			ļ
	180	1.67	0.1291	264.1	15 00			enance of the second			
	II	6513	0.1231		15.90	2.82	15.90	4 x 3.5			
	340	4.41	0.2100	8 346.2	70.01						
	3	6498	0.12100	340.2	12.81	2.60	13.52	3×3.5			
	740	3.78	0.1945	524.9	20.97	3.13	10 50				
	III 69	6470			20.97	3.13	19.59	4 x 4	0 = 5 0 5		
l	310(246)	3.23	0.1796	548.5	23.74	3.28	21.52		8x5RCB		ľ
	4	6460		0 (0 10 10 10 10 10 10 10 10 10 10 10 10 10	45.7.4	J. 20	41.54	4 x 4			
	300	3.33	0.1826	571.3	24.32	3.31	21.91	4 x 4			ĺ
	5	6450						7 2 4	e en en en en en en en en en en en en en	· } · · · · · · · · ·	
	430	3.02	0.1739	604.0	26.99	3.44	23.67	5 x 4			
	6	6437				tank'i managara ay ay ay ay ay ay ay ay ay ay ay ay ay		Y 17			
	720 7	2.50	0.1581	658.8	32.39	3.69	27.23	5 x 4.5			
ł	110	6419							* *************************************		
1	IV 116	1.82	0.1348	667.2	38.47	3.93	30.89	5 x 5			
1	1090(974)	6417							8x6RCB		
İ	8	3.30 6381	0.1817	686.5	29.36	3.55	25.21	5×4.5			
	340	2.65	0.1627	692.5	77 00						
1	9	6372	0.1027	092.5	33.08	3.75	28.12	5×4.5			
Ì	450	2.00	0.1414	700.5	38.50	7 07					
I	10	6363	~ ~ ~ ~ ~ ~	700.3	30.30	3.93	30.89	5 x 5			ı
]	300	3.00	0.1732	705.8	31.67	3.65	26 64	· j			
	11	6354	· · · · · · · · · · · · · · · · · · ·	/ J J • U	. 51 • 07	3.03	26.64	5×4.5			
1	980	1.94	0.1392	723.1	40.37	4.00	72 00				
	12	6335	·,- :.		T. U. • J I ;	4.00	32.00	5 x 5			

98

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Channel Calculations Culvert

AREA	LOCATION & DISTANCE	ELEV & S%	\$ 1/2	Q50 100	b 8/3	b	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS
Major	12	6335							r compression or some	
Green-	520	3.27	0.1808	732.3	31.48	3.65	26.64	4 x 4.5	la de la composición de la composición de la composición de la composición de la composición de la composición	
be1t	$\mathbf{V}_{_{\mathrm{c}}}$	6318								
	182		0.1754	899.5	39.86	3.99	31.84	5 x 5		
	Junct Pt.	6262								
	550	1.45	0.1206	1353.8	87.24	5.34	57.63	6 x 6		
	Χ	6254								
·	208		0.1315	1507.6	89.10	5.39	58.10	6 x 6		
	RR Blvd 14	1							10x9RCB	
	(594)740	1,.62	0.1273	1562.3	95.38	5.52	60.94	6 x 6.5		
	@track:XI	6206							10x9RCB	x160'
	culv.					i i				
IV H	14			Q50		<u> </u>				
			revious c	alc's, 12	48.2			e e e e e e e e e e e e e e e e	e e e e e e e e e e e e e e e e e e e	
	VI	6368								
	178	2.81		164.3		i			54"CMP	
V _, H	16	6354				0 01	0 88			
	520	2.79	0.1670	178.7	8.32	2.21	9.77	3 x 3		
	17	6339.5							i	
	530	1	0.1536	194.0	9.82	2.36	11.14	3 x 3		
	18	6327								
	135	2.22	0.1491	201.5	10.50	2.42	11.71	3 x 3.5	. 	
	19	6324	0 1740		10 40		17 21	1 7		
	220		0.1348	215.5	12.42	2.57	13.21	4 x 3.5	.	
		6320	0 1500	200 7	3 4 5 3	0 77	7, 07	1		
	470		0.1598	298.3	14.51	2.73	14.91	4 x 3.5	1	
V P	Junct. V P	6308	 	Q50					<u> </u>	<u> </u>
VP	VP	Come		****	2		A	<u>.</u>		
		1 *	revious c	alic's 2/	• Z					
	7 880	6366 6.59	0 2567	777	1 1 1	1 05	2.21	2 2 2		
L	Junct	6308	0.2567	37.7	1.14	1.05	7.21	2 x 2	<u> I</u>	<u> </u>

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WESTERN

ENGINEERS

North

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AREA	LOCATION & DISTANCE	ELEV & S%	S 1/2	Q 50	b 8/3	b	S F AREA	USE DITCH	CULVERT ETC.	TIME HRS
V S	Junct.	6308								
	820	1.71	0.1306	336.0	20.00	3.08	18.97	4 x 4		
	VIII	6294								
	1650	the contract of the state of the contract of t	0.1392	412.7	23.04	3.24	21.00	4 x 4		
XIIIA	Junct	6262		u de dimensi						
VII E	Junct	6320		ACT OF THE STATE O	A					
	340	5.88	0.2425	33.9	1.09	1.04	2.16	2 x 2		
	IX	6300			44					
	1000	3.80	0.1949	41.6	1.66	1.21	2.93	2 x 2		
VIIIA	Junct	6262				er i i i i i i i i i i i i i i i i i i i				to the to the total
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Rockrimmon North

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HYDROLOGIC ENGINEERING STUDY ROCKRIMMON NORTH DRAINAGE BASIN PART IV - COST ESTIMATE

A. UNIT PRICES AND ACREAGE

The following Unit prices are utilized for estimating the costs included in this section. Other unit prices. are as shown on the particular estimates. These costs are the current experience of various developers and contractors in the Colorado Springs region.

Item	<u>Unit</u>	<u>Unit Cost</u>
Excavation-Ditches Concrete in Minor Structures Concrete in Major Structures Concrete in Massive Structures Concrete Paving-Minor ditches Concrete Paving-Major ditches Structural Reinforcing Steel Gabion Velocity Control Units Riprap-Minor Riprap-Gabions Riprap-Major	Cubic Yard Cubic Yard Cubic Yard Cubic Yard Square Yard Square Yard Pound ea Cubic Yard Cubic Yard Cubic Yard	60.00 40.00 8.00 6.00 0.20 20.00-30.00 40.00
Riprap-Major	Cubic Yara	6.00

The acreage used in computed fees is the total area of the basin excluding that portion owned by the St. Francis Academy as follows:

Tota	al area		1401.20	acres
St.	Francis	Academy	15.75	
Net	Area	•	1385.45	acres

All of the above net area is within the City limits.

B. COLLECTION SYSTEM

Item	Quantity	Unit Price	Cost
Lined Drainage Swales	7830 LF	\$ 2.50	\$ 19575.00
Curb Outlets	19 ea	150.00	2850.00
Curb Inlets	28 ea	150.00	4200.00
Riprap	27 CY	40.00	1080.00
Four-Foot catch basin	5 ea	700.00	3500.00
Six-Foot catch basin	6 ea	900.00	5400.00
Eight-Foot catch basin	12 ea	1000.00	12000.00
Ten-Foot catch basin	18 ea	1100.00	19800.00
Twelve-Foot catch basin	11 ea	1200.00	13200.00
Sixtenn=Foot catch basin	18 ea	1400.00	25200.00
18 inch CMP culvert	874 LF	11.00	9614.00
21 inch CMP culvert	1260 LF	11.75	14805.00
24 inch CMP culvert	2941 LF	12.25	36027.25
27 inch CMP culvert	917 LF	13.00	11921.00
30 inch CMP culvert	5224 LF	13.75	71830.00
36 inch CMP culvert	3814 LF	15.00	57210.00
42 inch CMP culvert	2000 LF	17.00	34000.00
48 inch CMP culvert	2070 LF	19.00	39330.00
54 inch CMP culvert	1035 LF	21.00	21735.00

30 inch CMP Headwalls Energy Dissipators 2 foot x 1.5 foot lined channel	2 ea 6 ea 90 LF 870 LF 1835 LF 720 LF 20 LF res 5	2000.00 7.75 9.18 10.21 13.32 11.25	5429.00 600.00 12000.00 697.50 7986.60 18735.35 9590.40 225.00 10000.00 1087.30
Sub total 10% Engr. and Cont.			\$468628.40 46862.84
TOTAL			\$515491.24
Per acre cost			\$ 372.07

C. PRIMARY GREENBELT

<u>Item</u>	Quantity	Unit Price	Cost
4 foot x 2 foot lined drainage channel	340 LF	\$ 8.22	\$ 2794.80
4 foot x 4 foot lined drainage channel	450 LF	13.91	6259.50
5 foot x 4 foot lined drainage channel	520 LF	14.88	7737.60
Gabion Velocity Control Units	3 ea	2000.00	6000.00
7 foot x 4 foot RCB culvert	116 LF	60,00	6960.00
8 foot x 3 foot RCB culvert	64 LF	60.00	3840.00
10 foot x 7 foot RCB culvert	146 LF	90.00	13140.00
Dam No. 1 modifications	LS	3500.00	3500.00
Dam No. 2 modifications	LS	3250.00	3250.00
Subtota1			\$53481.90
10% Engr. & Cont.			5348.19
TOTAL			\$58830.09
Per acre cost			\$ 42.46

D. MONUMENT CREEK AREA FACILITIES

(As shown on Plate C1)

<u>Item</u>	Quantity U	Unit Price	Cost
Lined drainage swales	1260 LF	\$ 2.50	\$ 3150.00
Curb Outlets	5 ea	150.00	750.00
Curbe Inlets	1 ea	150.00	150.00
Ripran	601 CY	40.00	24040.00
12 foot catch basin	3 ea	1200.00	3600.00
24 inch RCP culvert	790 LF	10.50	8295.00
30 inch RCP culvert	50 LF	13.50	675.00

36 inch RCP culvert 48 inch RCP culvert 1.5'x2.0' concrete channel 1.5'x2.5' concrete channel 2.0'x2.0' concrete channel Energy Dissipators 12 foot x 10 foot RCB culvert	870 LF 100 LF 350 LF 320 LF 190 LF 1 ea 50 LF	\$ 19.00 30.50 8.73 10.18 10.95 2000.00 175.00	\$ 16530.00 3050.00 3055.50 3257.60 2080.50 2000.00 8750.00
Sub total 10% Engr. & Cont.			\$79383.60 7938.36
TOTAL			\$87321.96
Per acre cost			\$ 63.03

E. DAM

Item	Quantity Unit Price	Cost
Foundation Stripping Core Trench Excavation Spillway Excavation Dam Enbankment Spillway Concrete Spillway Steel Seeding and Mulching Riprap Powerline Relocation 60" RCP class D outlet conduits Valve Structure Energy Dissipator	22,133 CY \$ 0.40 6,356 CY 0.60 34,820 CY 0.20 236,344 CY 0.45 1542.30 CY 40.00 169,650 Lbs 0.20 5.44 ac 850.00 7099 CY 6.00 LS 25150.00 900 LF 50.00 LS 10000.00 LS 5000.00	\$ 8853.20 3813.60 6964.00 106354.80 61692.00 33930.00 4624.00 42594.00 25150.00 45000.00 10000.00 5000.00
Sub total 5.32% Design and 3.00% Inspection Geological Investigation	on	\$353975.60 29450.77 12000.00
TOTAL		\$395426.37
Per acre cost		\$ 285.41

F. RECOMMENDED FEES

Item	Cost per acre
Collection System Greenbelt Monument Creek Facilities Dam Hydrology Study	\$372.07 42.46 63.03 285.41 9.38
TOTAL	\$772.35

The recommended per acre drainage fee is \$772.35.

G. UNSTAGED FACILITIES

The following is an estimate of the facilities required if no staging were performed - that is, if no flood control dams were used to limit the basin runoff.

1. Collection System:

<u>Item</u>	Quantity	Unit Price	Cost
Lined Drainage Swales Curb Outlets Curb Inlets Riprap Four foot catch basin Six foot catch basin Eight foot catch basin Ten foot catch basin Twelve foot catch basin Sixteen foot catch basin Sixteen foot catch basin 18 inch CMP culvert 21 inch CMP culvert 24 inch CMP culvert 27 inch CMP culvert 30 inch CMP culvert 48 inch CMP culvert 48 inch CMP culvert 49 inch CMP culvert 40 inch CMP culvert 41 inch CMP culvert 42 inch CMP culvert 43 inch CMP culvert 44 inch CMP culvert 54 inch CMP culvert 554 inch CMP Headwalls 554 inch CMP Headwalls Energy Dissipators	7830 LF 19 ea 28 ea 6 CY 5 ea 6 ea 12 ea 18 ea 11 ea 18 ea 874 LF 1260 LF 2941 LF 917 LF 5224 LF 3814 LF 2000 LF 2070 LF 1213 LF 2 ea 2 ea 6 ea	\$ 2.50 150.00 150.00 40.00 700.00 900.00 1000.00 1100.00 1200.00 14.00 11.75 12.25 13.00 13.75 15.00 17.00 19.00 21.00 300.00 696.00 2000.00	\$19575.00 2850.00 4200.00 240.00 3500.00 5400.00 12000.00 13200.00 25200.00 9614.00 14805.00 36027.25 11921.00 71830.00 57210.00 34000.00 39330.00 25473.00 600.00 1392.00 12000.00
2 foot by 1.5 foot lined channel	90 LF	7.75	697.50
2 foot by 2 foot lined channel	3090 LF	9.18	28366.20
3 foot by 2 foot lined channel	1835 LF	10.21	18735.35
3 foot by 3 foot lined channel	1770 LF	13.32	23576.40
3 foot by 3.5 foot lined channel	135 LF	15.00	2025.00
4 foot by 2 foot lined channel	20 LF	11.25	225.00
4 foot by 3.5 foot lined	690 LF	15.27	10536.30
<pre>channel 4 foot by 4 foot lined channel</pre>	2470 LF	13.91	34357.70
Sub total 10% Engr. & Cont.			\$538686.70 53868.67
TOTAL			\$592555.37
PER ACRE COST			\$ 427.70

2. Primary Greenbelt System

Item	Quantity	Unit Price	Cost
3 foot x 3 foot lined	490 LF	\$ 13.32	\$ 6526.80
	820 LF	15.00	12300.00
channel 4 foot x 3.5 foot lined	340 LF	15.27	5191.80
channel 4 foot x 4 foot lined	1286 LF	13,91	17888.26
channel 4 foot x 4.5 foot lined	520 LF	15.42	8018.40
channel 5 foot x 4 foot lined	430 LF	14.88	6398.40
channel 5 foot x 4.5 foot lined	2334 LF	16.42	38324.28
channel 5 foot x 5 foot lined	3250 LF	17.80	57850.00
channel 6 foot x 6 foot lined	2630 LF	21.98	57807.40
<pre>channe1 6 foot x 6.5 foot lined</pre>	740 LF	23.75	17575.00
channel 8 foot x 5 foot RCB culvert 8 foot x 6 foot RCB culvert 10 foot x 9 foot RCB culvert Remove Existing Culverts	116 LF 740 LF	65.00 70.00 100.00 5000.00	15990.00 8120.00 74000.00 5000.00
	Lump sum	35000.00	35000.00
Sub total 10% Engr. & Cont.			\$365990.34 36599.03
TOTAL			\$402589.37
Per acre cost			\$ 290.58

3. Recommended Fees:

Item	Cost Per Acre
Collection System	\$427.70
Primary Greenbelt Monument Creek Facilities	290.58 63.03
(See Para. F) Drainage Study	9.38
TOTAL	\$790.69

The fees would be recommended at \$790.69 per acre.

HYDROLOGIC ENGINEERING STUDY ROCKRIMMON NORTH DRAINAGE BASIN SECTION V - CONCLUSIONS AND RECOMMENDATIONS

A. CONCLUSIONS

This report is prepared for the Golden Cycle Land Corporation as required by the City Engineer and the City Drainage Board due to the revised Master Land Use Plan for the Rockrimmon Area and requested revisions to existing master basin studies. This study replaces that portion previously studied in March of 1967 by Karcich and Weber and a portion of the Dry Creek Drainage Basin Report prepared by R. Keith Hook and Associates in November, 1966. It also encompasses a portion of Monument Creek in accordance with the proposed flood plain ordinance.

This study is prepared in accordance with all current drainage criteria and regulations of the City. In addition, it complies with the decision of the City Attorney that the outflow under fully developed conditions remain below that of the basin in its natural state. It will comply with these requirements in Senate Bill #116 now before the State Legislature.

The basin lies West of Interstate 25, being bounded on the West by the Mount Saint Francis Academy, on the South by the Rockrimmon South Basin and on the North by Woodmen Road, encompassing all "unstudied" portions of current and future filings within Rockrimmon. The total area of the basin is 1,401.20 acres, of which 15.75 acres is within the Mount St. Francis Academy and outside the City Limits. The balance of 1,385.45 acres is used for computing proposed drainage fees.

This study incorporates all proposed developments within the basin, which comprise about 95% of the total area. Proposed facilities include all items approved on Master Development Plans by City Council and project items which have not yet been presented to Council.

Existing drainage and flood control facilities are utilized wherever possible and predicted runoff is well within their capacities. It is intended that alternatives to structures proposed herein may be approved at the time of platting of areas within the basin, however these alternatives are to be designed in accordance with criteria included in this report. The runoff upon full development from a loo year storm under the proposed facilities would be 34.5 percent of the runoff from the natural basin.

The drainage facilities proposed within this report have an estimated current construction cost of approximately \$1,070,000 or a net cost per acre of \$772.35, all of which are considered applicable toward drainage fee assessment. The proposed fee is \$772.35 as compared with the current fee of \$774.00, which is assessed on outdated criteria. Structures required for storm drainage without the proposed reservoirs would create a fee of \$790.69 per acre, however, the runoff from the basin would be 150 percent of the natural

runoff and would increase the flows in Monument Creek. This would be in violation of current interpretaions of Colorado Law and would be in clear violation of current proposed legislation.

B. RECOMMENDATIONS

It is recommended that this report be accepted as proposed, and that a drainage fee on the Rockrimmon North Basin, as defined herein, be set in the amount of \$ 772.35 per acre.

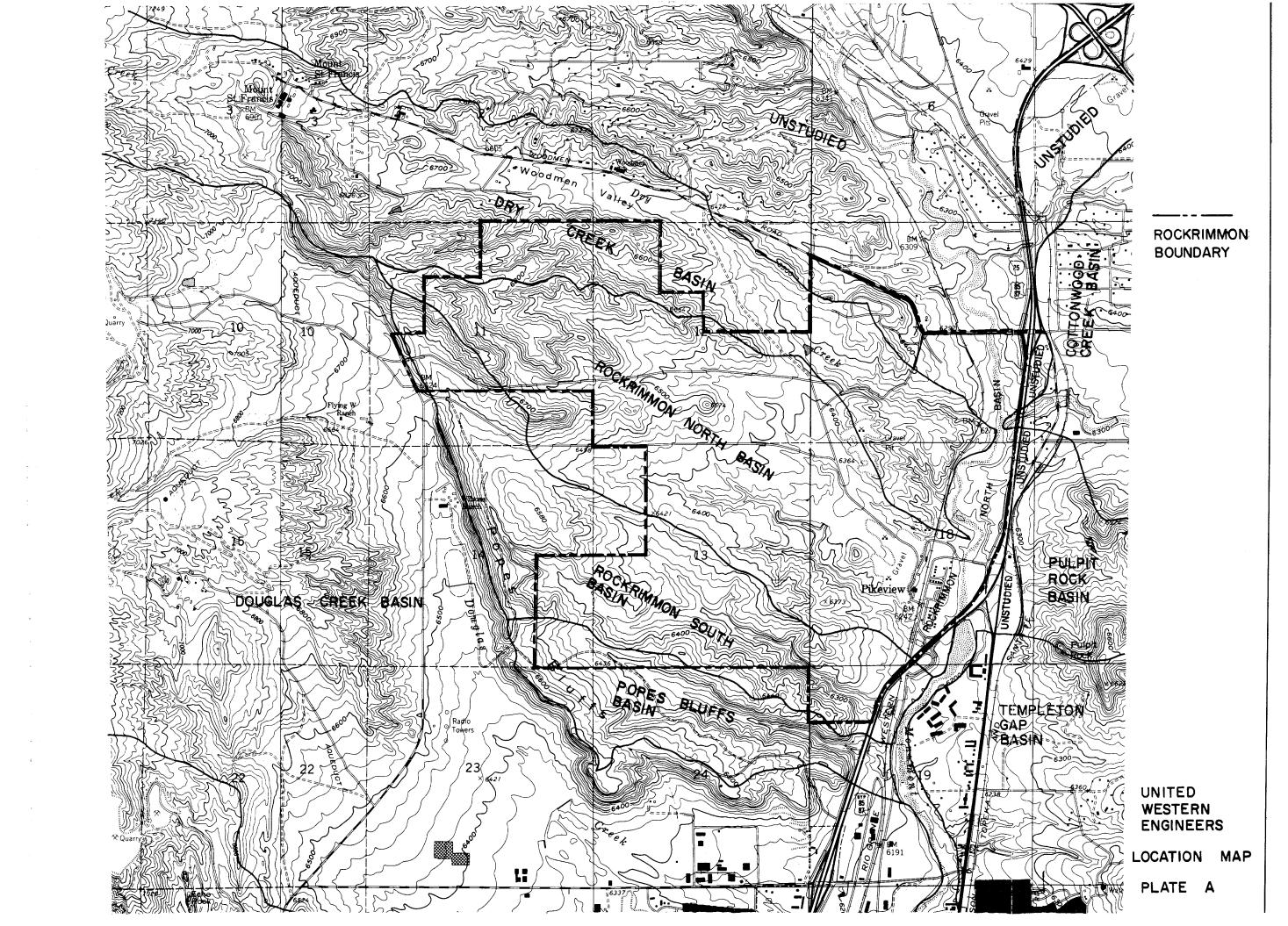
ENGINEERING HYDROLOGIC STUDY

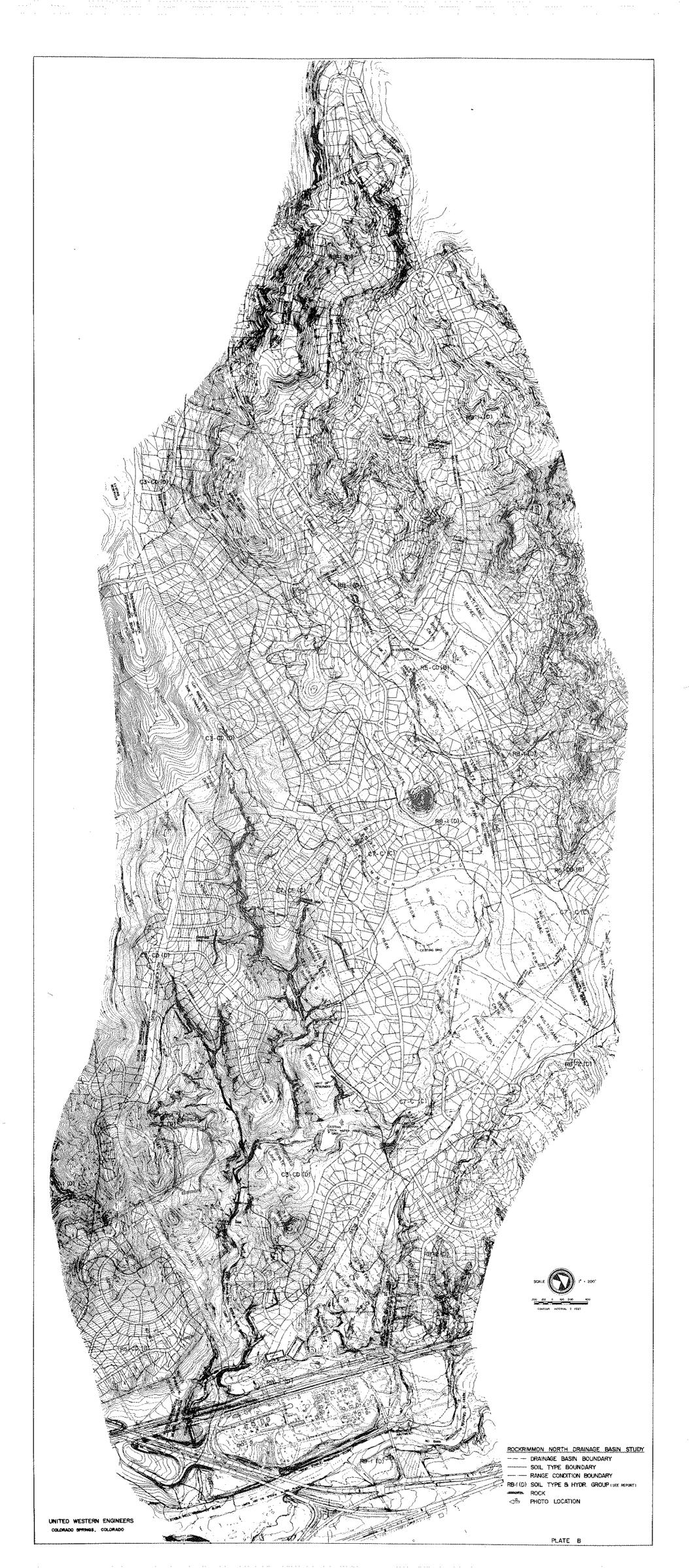
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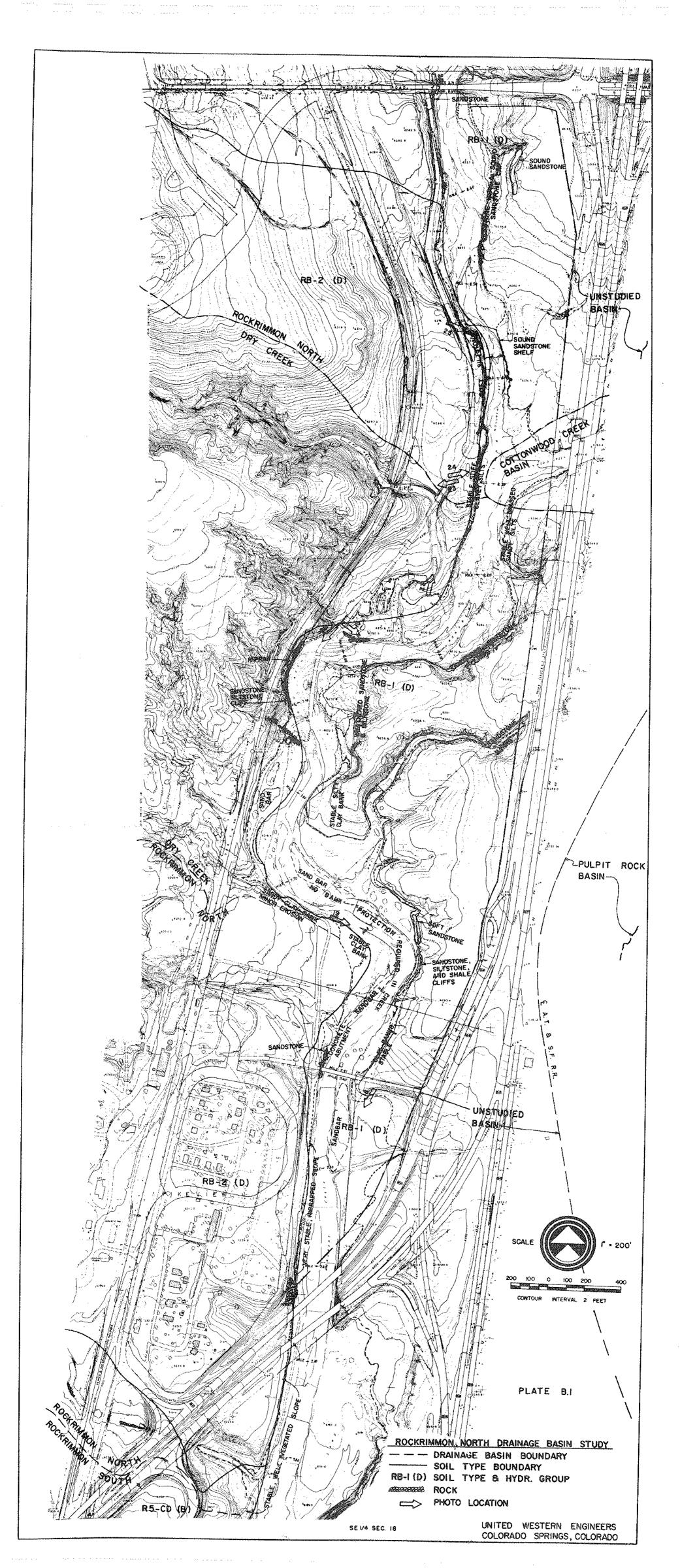
ROCKRIMMON NORTH DRAINAGE BASIN

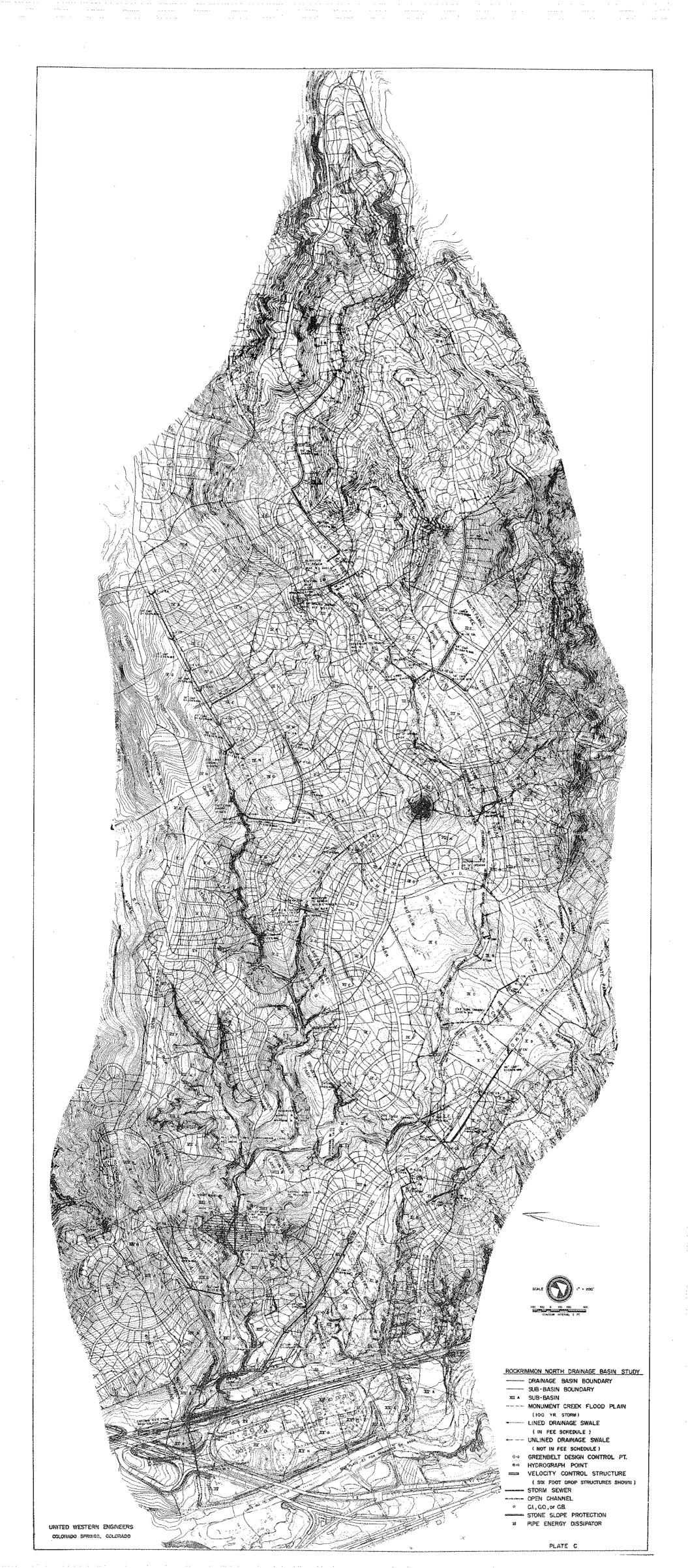
SECTION VI - APPENDIX

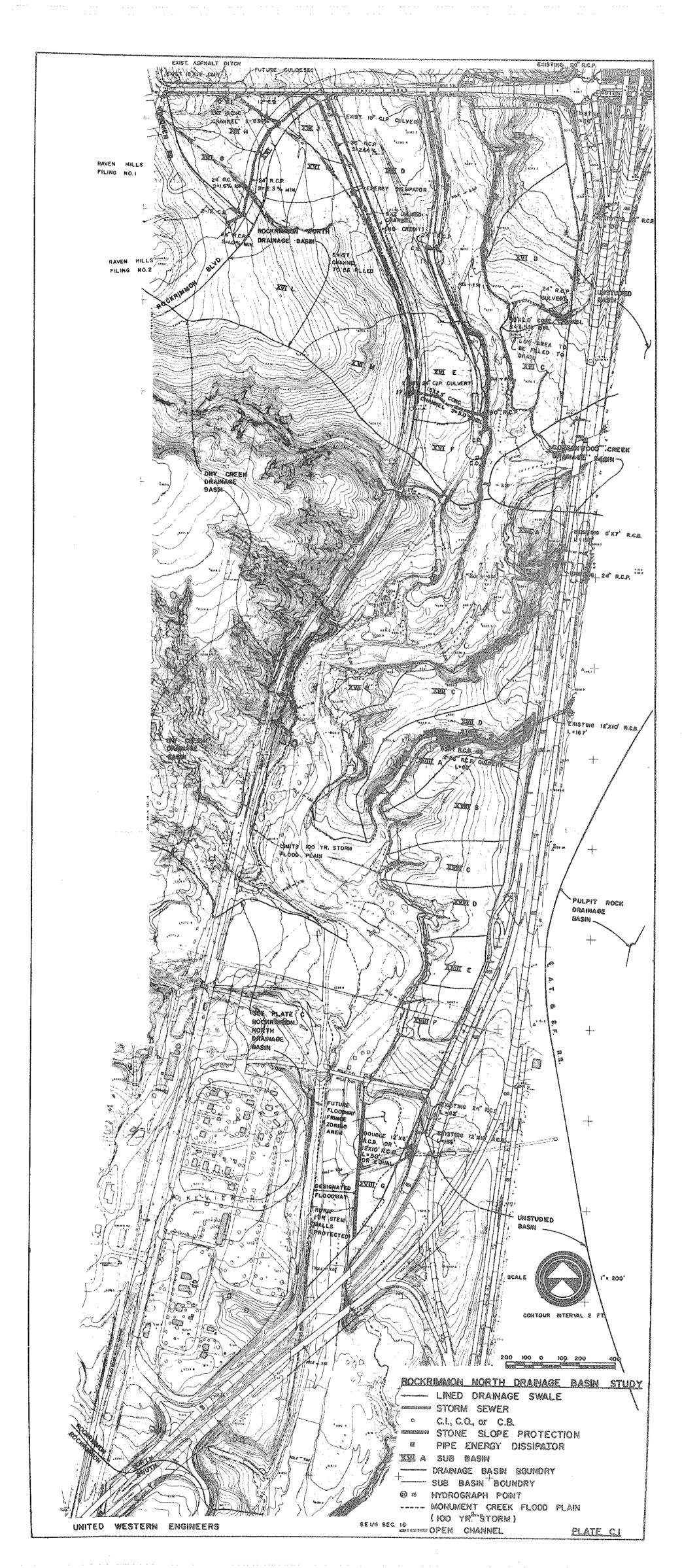
Plate	Description
A B B1	Location Map Soils, Range and Existing Channels map. Soils, Range and Existing Channels map, Monument Creek portion.
C C1 E F G H I J K L M N	Drainage Plan Drainage Plan, Monument Creek portion Typical Channel Section-Type 1 Typical Channel Section-Type 2 Typical Channel Section-Type 3 Typical Velocity Check Dam Typical Flood Control Dam Spillway Standard City Catch Basin Curb Inlet Curb Outlet Culvert Energy Dissipator Photographs - Basin in Natural State, 7 sheets



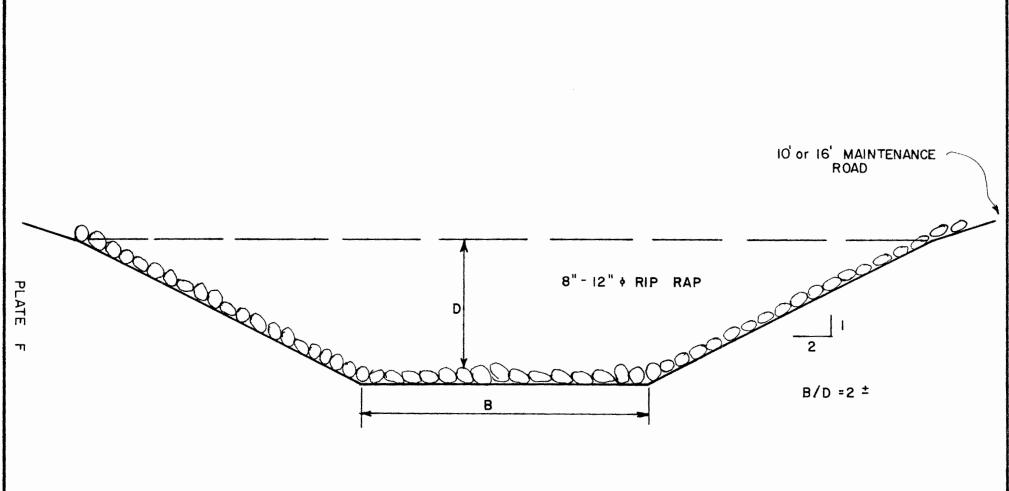






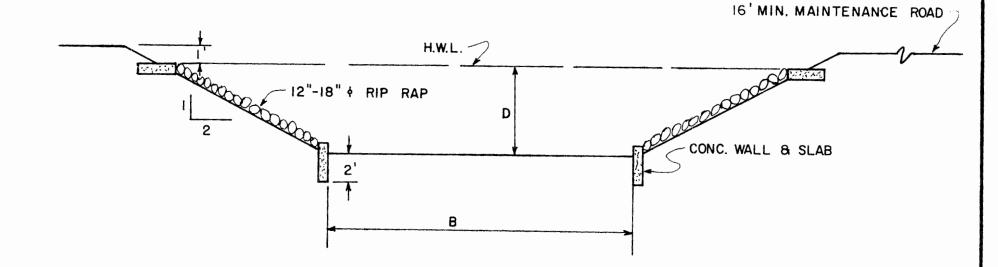


SECTION OF PROPOSED CONCRETE DITCH LINING TYPE I DITCH



SECTION OF PROPOSED CONCRETE DITCH LINING

TYPE 2 DITCH MAX. VELOCITY IO F.P.S.



SECTION OF PROPOSED CONCRETE DITCH LINING

TYPE 3 DITCH
MAX. VELOCITY GOVERNED
BY BOTTOM SOILS,
NOT TO EXCEED 10 F.P.S.

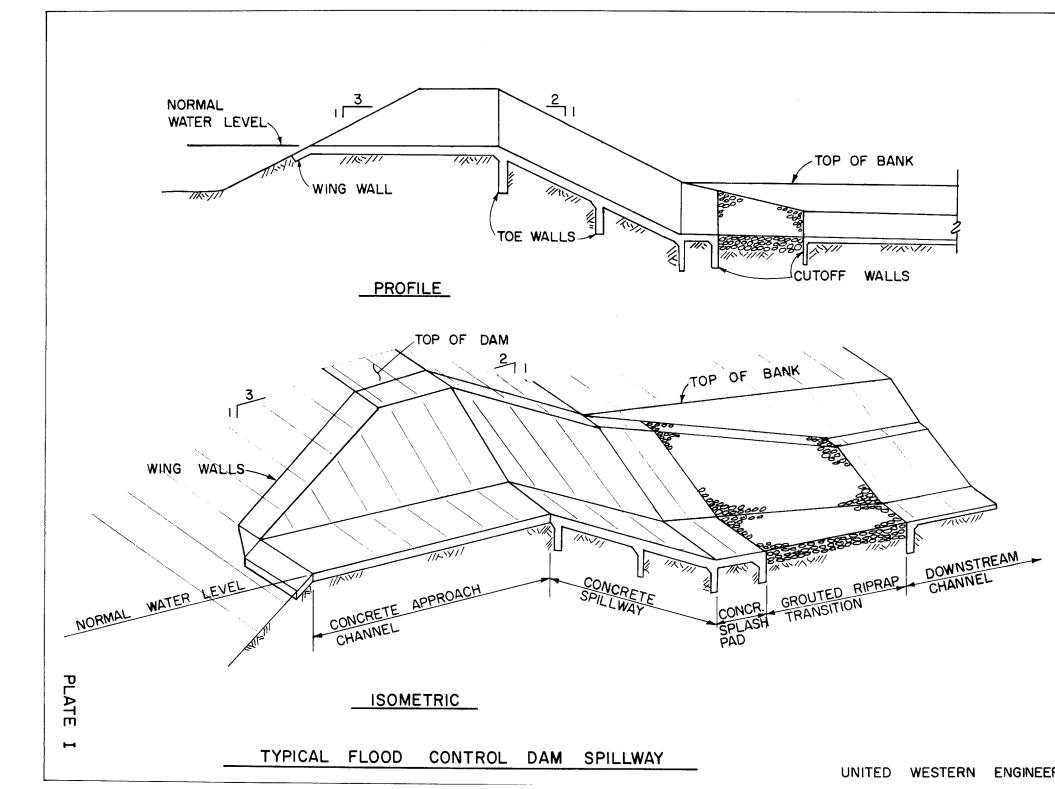
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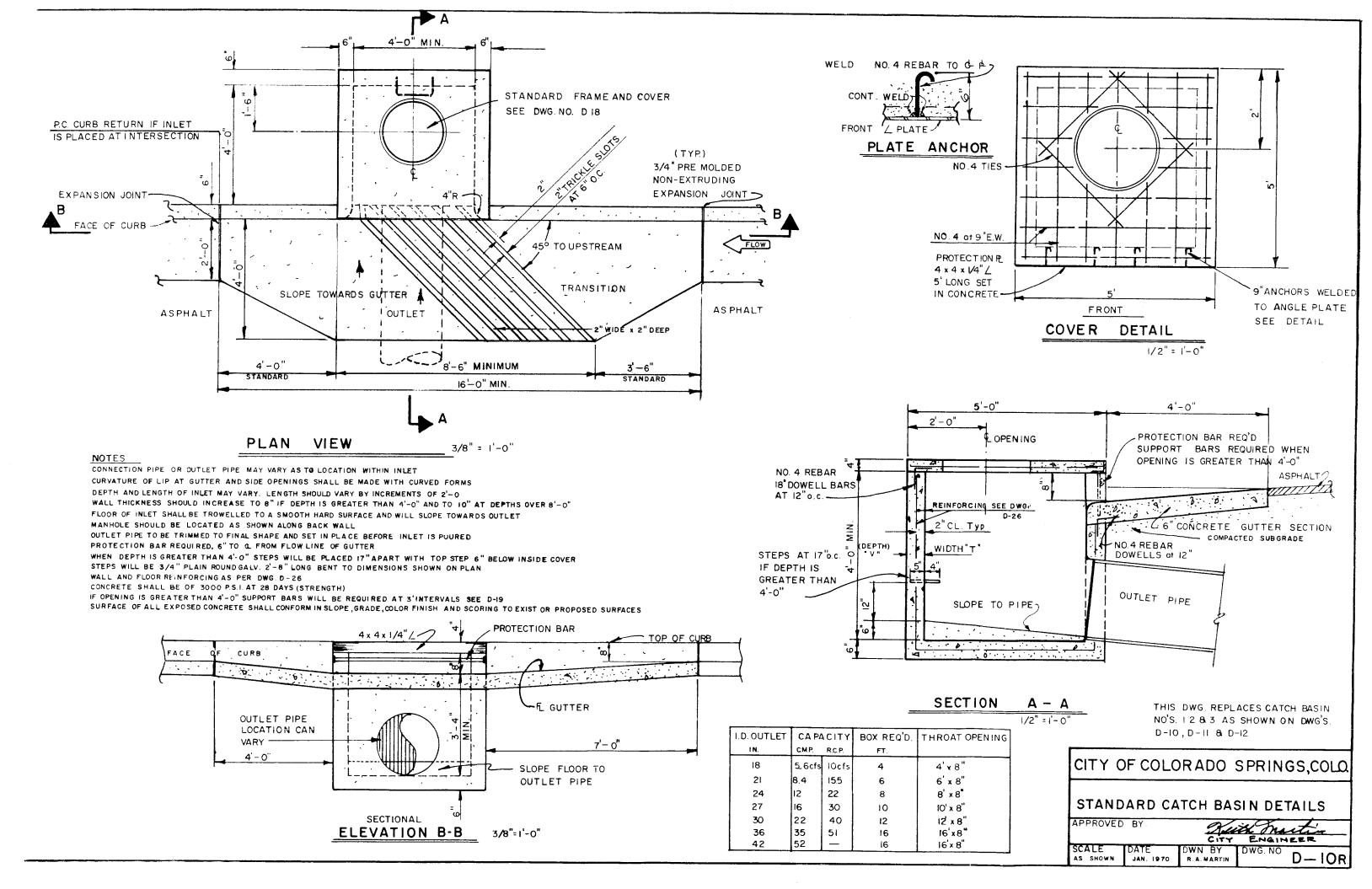
6' DROP SHOWN (UNITS OF VARIABLE SIZE TO FIT REQUIREMENTS)

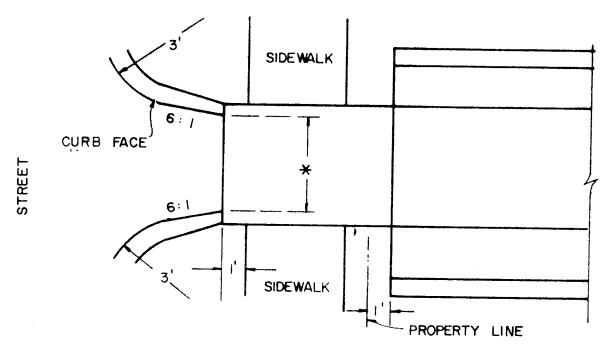
TYPICAL GABION VELOCITY CHECK DAM

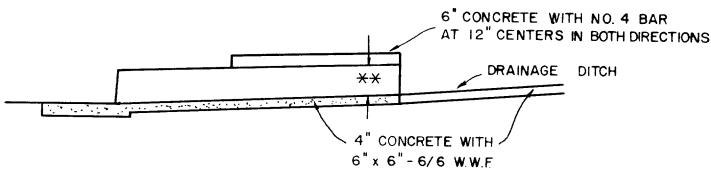
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TOP OF BANK -12' X3' X1' GABION UNITS PROTECTION 12' X 3' X 3' ABUTMENT UNITS SLOPE .12'X3'X1' SLOPE UNITS LINING CHANNEL FLOW **BOTTOM** SLOPE 12' X 3' X 3' LINING WEIR UNITS 6'x3'x3' UNIT ~12' X 3' X 3' 12' X 3' X 3' GABION UNIT COUNTER WEIR 12'X3'X1' FLOOR SLAB COUNTER WEIR 12' X3' X3' GABION UNITS 12' X 3' X I' GABION UNITS SECTION WEIR BLOCKS FLOOR SLAB



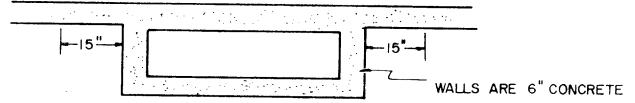






EXTEND EVERY OTHER BAR 15"
INTO SIDEWALK ON BOTH SIDES,
BEND EVERY OTHER BAR INTO WALLS

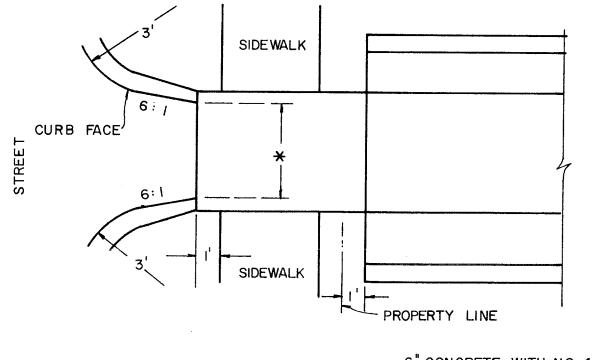
SIDEWALK

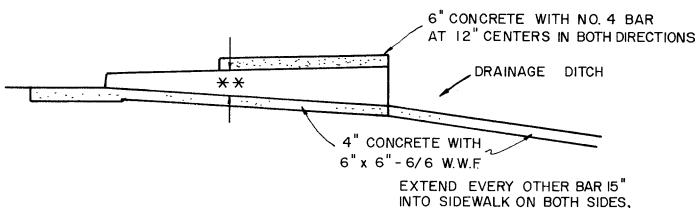


NOTES:

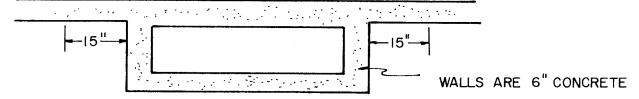
- I. SIDEWALK MAY BE PLACED ANYWHERE FROM CURB TO PROPERTY LINE BY EXTENDING THE TOP SLAB TO MATCH THE POSITION OF THE SIDEWALK
- 2. * VARIABLE DEPENDING ON QUANTITY OF WATER, MINIMUM 4' * * VARIABLE DEPENDING ON QUANTITY OF WATER, MINIMUM 1'

CURB INLET





SIDEWALK

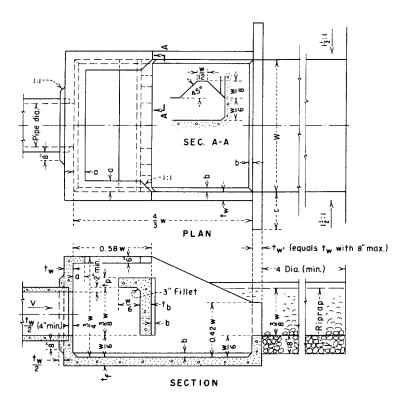


NOTES:

- I. SIDEWALK MAY BE PLACED ANYWHERE FROM CURB TO PROPERTY LINE BY EXTENDING THE TOP SLAB TO MATCH THE POSITION OF THE SIDEWALK
- 2. * VARIABLE DEPENDING ON QUANTITY OF WATER. MINIMUM 4' * VARIABLE DEPENDING ON QUANTITY OF WATER. MINIMUM 1'

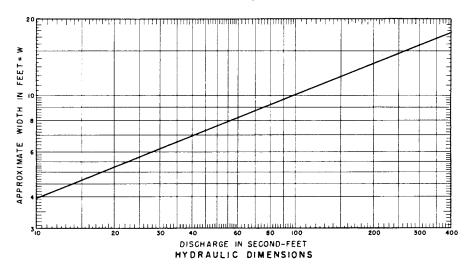
CURB OUTLET

BEND EVERY OTHER BAR INTO WALLS



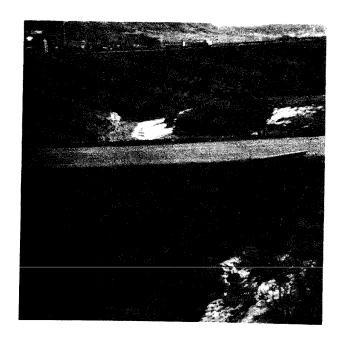
Q	a	b	С	tw	†f	† _b	tp
100	9"	3"	3'-0"	8"	8"	9"	8"
200	12"	4"	3'-0"	10"	11"	10"	8"
300	14"	6"	3'-0"	12"	12"	12"	8"
400	16"	6"	3'- 0"	12"	13"	12"	8"

SUGGESTED CONCRETE DIMENSIONS



CULVERT ENERGY DISSIPATOR

PLATE M



1. Existing culverts near outfall 2. Future Rockrimmon Boulevard point of primary greenbelt, crossing of primary greenbelt. point of primary greenbelt, Cascade Avenue and I-25.





3. Primary greenbelt near toe of proposed high dam.



4. Minor greenbelt Shopping Center and School site - fully lined channel proposed.

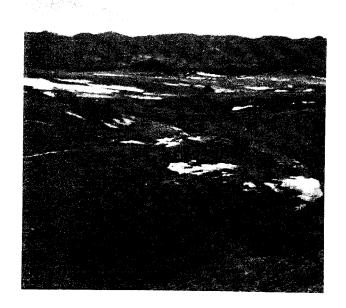


5. Future alignment of Arterial connection to the West, to have storm sewer.



6. View from existing erosion control dam in minor greenbelt, (to remain).





7. Single Family Residential area, 8. Primary greenbelt-to remain in existing dike to be removed. and natural condition with controlled inlets; part of lineal park system.



with inlet control.



9. Primary Greenbelt below 10. Primary greenbelt (lineal Park) Rockrimmon Boulevard crossing - to remain in natural condition controlled inlets.

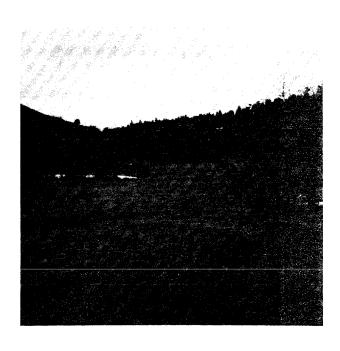


11. Primary greenbelt at minor 12. Primary greenbelt between dams road crossing-to remain in natural #1 and #2 - to remain natural. condition with inlet control.

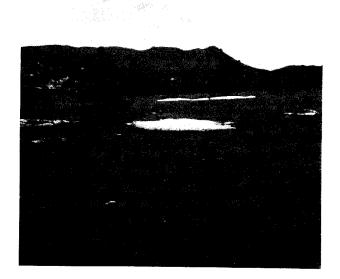




13. Primary greenbelt above dam #1, to remain natural-controlled inlet with energy dissipator proposed.



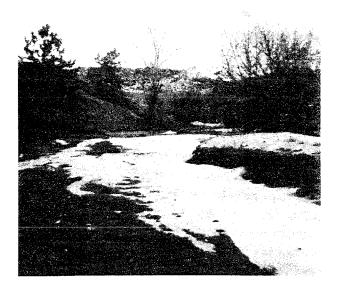
14. Approximate location of future street with storm sewer.



street with storm sewer, and park street near ton of proposed storm site.



15. Approximate location of future 16. Approximate location of future sewer.



17. Minor greenbelt to remain 18. Monument Creek, proposed natural, riprapping of encroaching Floodway Fringe Zoning Area. fills required. Pulpit Rock in background. This greenbelt formally in Dry Creek Basin.



18. Monument Creek, proposed



19. Monument Creek Channel to remain in natural state.



20. Monument Creek Channel to remain in existing state, City sewer main manhole in foreground.



21. Monument Creek Channel to
remain in existing state, riprap
Outfall. protects City sewer crossing.

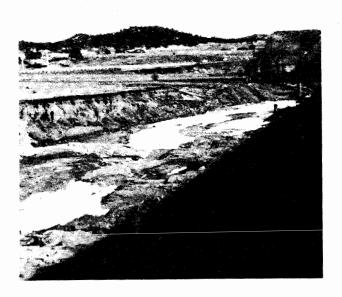




23. Existing Railroad Bridge across 24. Cottonwood Creek Primary primary Dry Creek Greenbelt.



greenbelt outfall.



25. Monument Creek Channel to 25. Monument Creek Channel to remain in natural state-sandstone shelf in foreground.



26. Monument Creek Channel from Woodmen Road, to remain in natural state.