

**UPPER SAND CREEK BASIN
DETENTION EVALUATION REPORT**

December, 2008

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Project # 08-600-400-00

Prepared For:

City of Colorado Springs

Prepared By:

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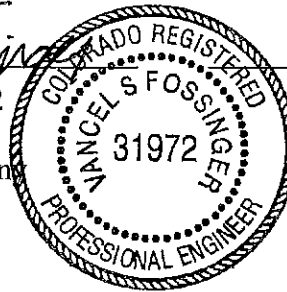
UPPER SAND CREEK BASIN
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ENGINEER'S STATEMENT:

The attached report was prepared under my direction and supervision and is correct to the best of my knowledge and belief. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Vancel Fossinger
Vancel Fossinger, Colorado PE # 31972

For and On Behalf of Wilson & Company



6-26-2009

Date

CITY OF COLORADO SPRINGS:

Filed in accordance with Section 7-7-906 of the Code of the City of Colorado Springs, 2001 as amended.

By: David E. Little
For The City Engineer

Date:

9/28/09

Conditions:

WILSON
& COMPANY

UPPER SAND CREEK BASIN DETENTION EVALUATION REPORT

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- C. HEC-1 Model Output
- D. Sand Creek Regional Pond No. 3 Concept Plan
- E. Watershed Map

INTRODUCTION

This report documents the results of a detention evaluation for the portion of Sand Creek Basin located north of Woodmen Road. This study was performed by Wilson and Company for the City of Colorado Springs Subdivision Engineering Review Team. The primary purpose of the study was to evaluate the potential impact of proposed developments within the basin which plan to deviate from the effective Drainage Basin Planning Study, and to identify potential alternative detention schemes for the watershed. A diversion of runoff from approximately 627 acres to Upper Sand Creek from a watershed located to the east as proposed in the draft Master Drainage Development Plan for Sterling Ranch was the most significant deviation evaluated in this study.

The existing comprehensive hydrologic models for the watershed were out of date and models for individual proposed development projects did not extend downstream beyond the boundaries of the individual developments. This study utilized data developed in existing studies to prepare a HEC-1 hydrologic model for all of the Upper Sand Creek Basin above Woodmen Road. The model was set up to export hydrographs at Woodmen Road that can be imported into a truncated, existing model that was prepared for the design of Sand Creek Detention Basin No. 2, by Kiowa Engineering, for the portion of the watershed located between Woodmen Road and Sand Creek Regional Pond No. 1. The new model and the truncated model were utilized in series to evaluate the potential effects of the proposed development and detention alternatives.

STUDY AREA LOCATION

The area evaluated in this study is the portion of Upper Sand Creek Basin located north of Woodmen Road, in El Paso County, Colorado. More specifically, the area lies between Shoup Road on the north and Woodmen Road on the south and bounds Vollmer Road. The lower portion of the area has been annexed into the City of Colorado Springs while the upper portion of the area remains in un-incorporated El Paso County.

HYDROLOGIC ANALYSIS

Using the HEC-1 model prepared for the "Design Analysis Report for Sand Creek Detention Basin No. 2", by Kiowa Engineering, (pond 2 model) as a base, a separate and revised HEC-1 model was developed for the portion of the Upper Sand Creek watershed located north of Woodmen Road. Generally, developed condition impervious percentages and restricted peak discharge rates as defined by approved or current draft studies, where applicable, were utilized in developing model input data. Spreadsheets used in the development of sub-basin input data are contained in Appendix 'B' of this report.

The Sterling Ranch detention pond stage/storage data presented in the draft MDDP for Sterling Ranch was input into the model. Stage/discharge data for the proposed Sterling Ranch ponds was revised slightly and was also input into the new model. The stage/storage and discharge data associated with Sand Creek Regional Pond 6 was copied from a Haestad Pond Pack model prepared and provided by M&S Civil Consultants. The stage/storage and discharge data for Sand Creek Regional Pond 3 is based on the grading and outlet concept contained in Appendix 'D' of

this report. Supporting data for the stage/storage and discharge data is contained in Appendix 'B' of this report.

It was found that several relatively short times of concentration and routing segments were present in the portion of the model above Woodmen Road making a 4 minute time step computation interval desirable. In order to preserve the integrity of the portion of the pond 2 model downstream of Woodmen Road, which used a 5 minute time step, the model was bisected at Woodmen Road. The lower portion of the model was revised to import hydrographs generated and punched by the upstream model at the three drainage crossings of Woodmen Road.

Kiowa Engineering has made some minor revisions to the pond 2 model subsequent to the preparation of the Design Analysis Report for Sand Creek Detention Basin No. 2. These revisions include correction of a typo in the 24 hour storm distribution and a revision of the Pond 2 stage/discharge curve to match the as-constructed condition. These revisions are reflected in the model prepared in the scope of the current study as well as the peak flow rates presented in the "Analysis Point Comparison Summary" contained in Appendix 'A' of this report.

The current model utilizes the SCS Type IIa 24 hour storm distribution consistent with the pond 2 model and the Sand Creek DBPS. Rainfall depths of 2.1", 3.0", and 4.4" were utilized for simulation of the 2, 10 and 100 year storms respectively.

100 year Peak flow rates calculated by the model have been summarized for comparison with values from previous studies in the "Analysis Point Comparison Summary" contained in Appendix 'A' of this report. Model input and output is contained in Appendix 'C' of this report. A map of the modeled watershed is contained in Appendix 'E' of this report.

BASINS MODELED WITH DEVELOPED DISCHARGE RESTRICTIONS

Where discharge restrictions for drainage areas have been approved with previous studies, the restrictions were assumed in the current model. Basins SC6-1, SC6-3, and SC6-4 are assumed to require detention if they are developed under the current proposed land uses. The allowable developed condition peak 100 year discharge from Basin SC3-3 was recalculated in the current study at the direction of the City to reflect the existing development condition from the majority of the basin plus the future developed condition of the 22 acre multi-family site included in the Woodmen Heights Master Plan. The restricted 100 year discharge rates from the above mentioned basins as modeled are: Basin SC6-1, 84cfs; Basin SC6-3, 96cfs; Basin SC6-4, 53cfs; and Basin SC2-3, 280cfs.

DETENTION ALTERNATIVES

1. Configure Sand Creek Regional Detention Pond 3 to provide peak rate mitigation and extended detention of the difference between the 2 year developed and undeveloped condition runoff volumes from the watershed. The extended detention volume would be released over a period of approximately 72 hours. The HEC-1 model indicates that this alternative will generally maintain planned peak flow rates downstream of Woodmen Road in large storm events in addition to controlling frequent runoff to better mimic historical runoff than

conventional detention would. The model assumes that the three detention ponds proposed in the Sterling Ranch MDDP will also be constructed with slightly more restricted outlets than proposed in the MDDP. The presence of shallow bedrock in the bottom of Pond 3 will limit infiltration which is a normal benefit of this type of pond. However, controlling discharge from frequent events to a very small rate should still help to mitigate potential degradation of the downstream channel. This is the alternative that has been included in the HEC-1 model prepared in the scope of this study. A sketch showing the modeled pond concept is contained in Appendix 'D' of this report.

2. Implement off-line full spectrum detention for individual development projects in Pond 3 watershed and eliminate on-line ponds including Pond 3. Multiple conventional detention ponds constructed to serve small development projects are generally less efficient and effective at controlling downstream peak flow rates than regional ponds. However, research done by Urban Drainage and Flood Control District has indicated that the cumulative effect of multiple ponds is greatly enhanced when they are designed as full spectrum detention ponds providing extended detention of the difference between the undeveloped and developed condition runoff volumes from the watershed as well as 100 year peak flow rate mitigation.

In the case of the Pond 3 watershed, the peak 100 year flow rate at Woodmen Road as defined by the effective DBPS is less than the historic rate. Modeling performed for the current study found that due to changes in the hydrograph timing and volume, the peak 100 rate at Woodmen Road should be restricted even more to mitigate increases to peak rates downstream. It is very possible that on-site full spectrum ponds that control 100 year peak rates to pre-development rates would be adequate due to the significant lagging and narrowing of the hydrograph that would occur with implementation of such ponds. However additional modeling of this concept would be recommended prior to implementation.

Potential upsides to this concept could include reduction of costs to build and maintain storm runoff conveyance facilities, ability to maintain Sand Creek in a more naturalistic condition, groundwater recharge, and reduction of overall runoff volumes with an associated benefit of improvement of downstream water quality.

Potential downsides could include deterioration of effectiveness over time if the multiple facilities are not maintained by property owners, districts, or owners associations. Implementing such a concept would also likely require recalculation of the basin drainage and detention fees. Overall, providing onsite detention generally requires more land and is more expensive to construct on a treated area basis than regional detention. This concept would need to be applied throughout the watershed to be effective.

3. Implement Alternative 2 with the addition of low impact development (LID) practices and facilities to supplement or take the place of the extended detention portion of the on-site ponds. The primary goal of LID is maintain natural hydrologic function on development sites. The presence of type A and B soils throughout the watershed make it favorable for construction of bioretention cells, infiltration swales, and pervious pavements to infiltrate excess

runoff volumes in frequent runoff events. Decreasing and disconnecting impervious areas could also reduce peak runoff rates and volumes.

Potential upsides to this concept include the benefits noted under Alternative 2 with the added benefit of less overall runoff volume contributed to the downstream channel system.

Potential downsides to this concept include the negative issues mentioned under Alternative 2 with potential concerns about water rights issues. Care would need to be taken in the implementation of LID to mimic the predevelopment runoff condition and not infiltrate historic runoff volumes.

4. Move Sand Creek Regional Detention Pond 3 to the south boundary of the proposed Sterling Ranch Development. This alternative seemed to have merit in initial modeling. The alternative showed promise of being able to match previously defined peak flow rates in flows crossing Woodmen Road with less overall detention storage volume and associated required land. However, it was found that the alternative had negative affects in the control of peak flow rates downstream of Woodmen Road. In addition, much of the excavation for the original Pond 3 was completed in the summer of 2007 to provide embankment for an expansion of Woodmen Road and thus the original site became less viable for future commercial development. These two issues make this alternative less viable.

SUMMARY

This report was prepared to provide the results of the hydrologic modeling prepared in the scope of the study and to provide ideas for drainage treatment alternatives in the Pond 3 watershed. Due to the very limited scope of the current study, all of the concepts presented including the revised design concept for Sand Creek Regional Pond 3 (Pond 3) should be considered very preliminary in nature and require more study and development prior to implementation.

In general, the study determined that with some minor changes to the proposed outlets of the detention ponds in the proposed Sterling Ranch Development, that downstream peak flow rate increases and at least a portion of impacts from increased runoff volume due to the drainage diversion proposed with the development can be mitigated by reconfiguring Pond 3 to provide a significant amount of extended detention.

The largest complication in the study area appears to be funding the construction of Pond 3. Pond 3 is a regional facility identified by the Sand Creek DBPS to be funded through the collection of Drainage and Detention Fees. It is our understanding that the portion of the watershed that extends into un-incorporated El Paso County was included in the distribution of fees per acre. El-Paso County currently utilizes a separate drainage fee structure to fund regional drainage improvements as land development occurs. Currently, there does not appear to be a process for transfer of drainage fees collected by El Paso County to the City of Colorado Springs to fund downstream improvements, such as Pond 3, that are at least partially required due to development in the un-incorporated areas of the County. In order for construction of Pond 3 to occur at the time that it is needed, a solution to the funding issue will need to be found.

REFERENCES

1. City of Colorado Springs/County of El Paso Drainage Criteria Manual, dated November 1991.
2. Draft Master Development Drainage Plan for Sterling Ranch, October 2007, By M&S Civil Consultants.
3. Master Development Drainage Plan Update for Woodmen Heights and Final Drainage Report for Forrest Meadows Filings No. 1 and No. 4, February 2006, By ESI Engineering and Surveying Inc.
4. Master Development Drainage Plan for Woodmen Heights Master Plan, June 2004, By Classic Consulting Engineers and Surveyors.
5. Design Analysis Report , sand Creek Detention Basin No. 2, May 22, 2006, By Kiowa Engineering Corporation.
6. Sand Creek Drainage Basin Planning Study, By Kiowa Engineering Corporation.

APPENDIX

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APPENDIX A.

PEAK FLOW RATE AND VOLUME SUMMARIES

Analysis Point Comparison Summary

6/26/2009

			Peak 100 Year Flow Rates			
Design Point			Developed Condition			Existing Condition
WCI **	Pond 2 *	DBPS ***	WCI	Pond 2	DBPS	DBPS
Pond 1 (in) 4	Pond 1 (in) 4	Pond 1 (in) 4	5537	5499	5929	5420
Pond 1 (out)	Pond 1 (out)	97	3815	3935	3194	
Pond 2 (in) 20	Pond 2 (in) 20	Pond 2 (in) 20	7484	7331	8429	4033
Pond 2 (out)	Pond 2 (out)	98	4063	4083	4849	
21	21		6586	6412		
28	28		6184	6011		
29	29		5840	5668		
35	35		5718	5542		
36	36		3857	3663		
37	37	37	3344	3153	3087	3230
85	85		2885	2976		
42	42		2247	2789		
48	48	48	2222	2714	2403	2750
49	49	51	1836	2264	2396	2766
51	51	58	104	443	655	340
Pond 6 (in) 54	Pond 6 (in) 54	54	1382	1428	2285	376
Pond 6 (out) 54	Pond 6 (out) 54	92	286	85	94	
Pond 3 (in) 53	Pond 3 (in) 53	60	2967	4376	3225	2629
Pond 3 (out) 53	Pond 3 (out) 53	94	1819	2152	2157	
60	60	60	2618	4266	3225	2629
61			2506			
63	63	63	2292	3340	2514	2508
68			2242			
87	87	87	1569	3194	2510	2545
69			2372			
70	70	70	2078	2770	2081	2251
71	71	71	1922	2580	1946	2164

* Pond 2 refers to the HEC-1 Analysis done for the design of Sand Creek Pond 2 By Kiowa Engineering (Sediment Pools Empty) as updated by Kiowa on 12/17/2008

** WCI refers to the revised HEC-1 analysis done by Wilson & Co to update the Kiowa Pond 2 model per current development plans and change the concept design of Pond -3 to include extended detention of the difference of the 2 year undeveloped and developed condition runoff volumes.

***DBPS refers to the TR-20 modeling done for the Sand Creek Drainage Basin Planning Study, prepared by Kiowa Engineering

Runoff Volume From Proposed Sterling Ranch Diversion To Upper Sand Creek

P_{10} (in) = 3

P_{100} (in) = 4.4

Watershed Area	Area (ac)	CN	S	la	Q10 (in)	V10 (ac-ft)	Q100 (in)	V100 (ac-ft)
To DB-SRMN*	69.6	76	3.16	0.63	1.02	5.89	2.05	11.89
To DB-SRMS**	99.7	85	1.76	0.35	1.59	13.20	2.82	23.41
To Sand Creek***	458.1	77	2.99	0.60	1.07	40.89	2.13	81.30
Total	627.4					60.0		116.6

* Proposed Sterling Ranch 'Middle' North Detention Basin

** Proposed Sterling Ranch ' Middle South' Detention Basin

***Proposed direct discharge to Upper Sand Creek near the southern Sterling Ranch boundary

Note: Diversion is as shown on the draft Master Development Drainage Plan (MDDP) for Sterling Ranch, dated October 2007, prepared by M&S Civil Consultants, Inc. Runoff volumes were calculated per SCS Runoff Curve Number Method. CN values were selected to be consistent with impervious area percentages presented in the above noted MDDP.

APPENDIX B

HEC -1 INPUT DATA CALCULATIONS & SUMMARIES

BASIN DATA

6/26/2008

	Basin	Basin	Basin	Sub-Area 1		Sub-Area 2		Sub-Area 3			Sub-Area 4		Sub-Area 5		Sub-Area 6		Sub-area	Sub-Area	Sub-Area
Basin	Area SF	Area (ac)	Area (sm)	Area (ac)	% Imp	Area (ac)	% Imp	Area (ac)	% Imp		Area (ac)	% Imp	Area (ac)	% Imp	Area (ac)	% Imp	Total Area (ac)	Wt % Imp.	CN
SC3-1	4739789	109	0.170	23	60	7	5	16	80		24	85	13	5	26	5	109	45	78
SC3-2	2268235	52	0.081	52	65												52	65	85
SC3-3	6564791	151	0.235	22	80	129	60										151	63	84
SC3-4	2257593	52	0.081	52	75												52	75	89
SC3-5	4321647	99	0.155	57	75	29	65	14	5								100	62	84
SC3-6	5740100	132	0.206	41	85	82	65	9	5								132	67	86
SC3-7	4321767	99	0.155	54	72	45	65										99	69	86
SC3-8	6304207	145	0.226	90	20	55	65										145	37	75
SC3-9	13918099	320	0.499	10	95	310	25										320	27	71
SC3-10	1783280	41	0.064	41	20												41	20	68
SC3-11	7948613	182	0.285	160	65	22	85										182	67	86
SC3-12	5760024	132	0.207	7	10	126	65										133	62	84
SC3-13	9384035	215	0.337	215	20												215	20	68
SC3-14	9516999	218	0.341	19	85	170	65	29	78								218	68	86
SC3-15	5841499	134	0.210	114	65	9	90	11	42								134	65	85
SC3-16	8348443	192	0.299	192	65												192	65	85
SC3-17	4965928	114	0.178	98	65	6	10	10	5								114	57	82
SC3-18	7450132	171	0.267	171	41												171	41	76
SC3-19	8378314	192	0.301	192	41												192	41	76
SC3-20	8533821	196	0.306	196	20												196	20	68
SC6-1	2998486	69	0.108	69	70												69	70	87
SC6-2	6677001	153	0.240	138	55	9	50	6	5								153	53	81
SC6-3	4844183	111	0.174	111	60												111	60	83
SC6-4	2778021	64	0.100	64	90												64	90	94
SC6-5	7823208	180	0.281	102	50	5	80	19	80	38	90	16	5				180	58	83
SC6-6	1637856	38	0.059	38	84												38	84	92
SC2-29	924654	21	0.033	21	90												21	90	94
61	3379177	78	0.121	78	20												78	20	68
73		90	0.141	90	15												90	15	67
74		120	0.188	120	15												120	15	67
75		79	0.123	79	15												79	15	67
76		86	0.134	86	15												86	15	67
77	7494857	172	0.269	172	20												172	20	68
78		156	0.244	156	15												156	15	67
79		189	0.295	189	15												189	15	67
80		148	0.231	148	15												148	15	67
81		263	0.411	263	15												263	15	67
82		118	0.184	118	15												118	15	67
88	3805480	87	0.137	87	20												87	20	68

Sand Creek Basin Above Woodmen

Lag Time Calculation

06/26/2009

BASIN	OVERLAND				SHALLOW GUTTER FLOW				STORM SEWER FLOW				CHANNEL FLOW				GRASS LINED SWALE				Tc	Tc	Lag
	C(10)	Length (ft)	Slope (%)	Tt (min)	Length (ft)	Slope (%)	Vel (fps)	Tt (min)	Length (ft)	Slope (%)	Vel (fps)	Tt (min)	Length (ft)	Slope (%)	Vel (fps)	Tt (min)	Length (ft)	Slope (%)	Vel (fps)	Tt (min)	TOTAL (min)	TOTAL (hours)	
SC3-1	0.25	50	2.00	8.9	1100	2.0%	5.0	3.7	800	2.0%	12.0	0.8									13.4	0.224	0.134
SC3-2	0.25	100	2.00	12.6	900	2.5%	5.5	2.7	1200	1.1%	11.0	1.8									17.2	0.287	0.172
SC3-3	0.25	100	2.00	12.6	1400	2.0%	5.0	4.7	1000	2.0%	12.0	1.4									18.7	0.312	0.187
SC3-4	0.25	50	2.00	8.9	900	2.7%	5.6	2.7	800	1.5%	11.0	1.2									12.8	0.214	0.128
SC3-5	0.25	50	2.00	8.9	1200	2.0%	5.0	4.0	500	2.0%	12.0	0.7									13.6	0.227	0.136
SC3-6	0.25	70	2.00	10.6	1200	2.5%	5.5	3.6	1300	2.5%	13.0	1.7									15.9	0.265	0.159
SC3-7	0.25	80	2.00	11.3	1600	2.8%	5.6	4.8	1600	2.0%	12.0	2.2									18.3	0.305	0.183
SC3-8	0.25	300	2.50	20.3					2400	2.5%	13.0	3.1					2000	1.3%	2.0	16.7	40.1	0.668	0.401
SC3-9	0.25	300	4.00	17.4													5200	2.6%	3.3	26.3	43.7	0.728	0.437
SC3-10	0.25	300	3.00	19.2													900	3.0%	3.5	4.3	23.4	0.391	0.234
SC3-11	0.25	100	2.00	12.6	1300	2.0%	5.0	4.3	800	2.0%	12.0	1.1	3100	0.5%	5.0	10.3					28.4	0.474	0.284
SC3-12	0.25	100	2.00	12.6	1700	2.5%	5.5	5.2	2600	2.5%	13.0	3.3									21.1	0.352	0.211
SC3-13	0.25	300	5.00	16.2					4300	3.0%	14.0	5.1					1200	7.0%	5.2	3.8	25.2	0.419	0.252
SC3-14	0.25	80	2.00	11.3	1200	2.5%	5.5	3.6	1300	1.3%	11.0	2.0	1800	1.4%	5.0	6.0					22.9	0.382	0.229
SC3-15	0.25	100	2.00	12.6	1900	2.5%	5.5	5.8	2400	2.0%	12.0	3.3									21.7	0.362	0.217
SC3-16	0.25	100	2.00	12.6	1500	2.5%	5.5	4.5	1500	1.0%	10.0	2.5	1500	0.5%	5.0	5.0					24.7	0.412	0.247
SC3-17	0.25	100	2.00	12.6	1500	2.0%	5.0	5.0	1100	3.0%	14.0	1.3									19.0	0.316	0.190
SC3-18	0.25	100	2.00	12.6	1600	4.5%	7.0	3.8	5400	2.5%	13.0	6.9									23.4	0.390	0.234
SC3-19	0.25	100	2.00	12.6	1700	3.5%	6.5	4.4	2300	3.5%	14.0	2.7									19.7	0.329	0.197
SC3-20	0.25	300	4.00	17.4													2400	4.0%	4.0	10.0	27.4	0.457	0.274
77.00	0.25	300	5.00	16.2													3700	3.5%	3.5	17.6	33.8	0.563	0.338
88.00	0.25	300	4.00	17.4													2700	3.0%	3.3	13.6	31.1	0.518	0.311
SC3-61	0.25	300	2.80	20.1													3100	2.5%	3.0	17.2	37.3	0.622	0.373
SC6-1	0.25	100	2.00	12.6	600 1300	2.7% 1.0%	3.4 2.2	2.9 9.8													25.4	0.424	0.254
SC6-2	0.25	100	2.00	12.6	900	2.2%	3.0	5.0	2000	1.0%	9.0	3.7									21.3	0.356	0.213
SC6-3	0.25	50	2.00	8.9	1000	1.8%	2.6	6.4	800	2.5%	3.3	4.0									19.4	0.323	0.194
SC6-4	0.25	50	2.00	8.9	1200	5.0%	4.6	4.3	1200	1.0%	9.0	2.2									15.5	0.259	0.155
SC6-5	0.25	100	2.00	12.6	1000	2.8%	3.5	4.8	1100	1.5%	12.0	1.5									18.9	0.316	0.189
SC6-6	0.25	50	2.00	8.9	1400	2.0%	5.0	4.7	1200	2.0%	12.0	1.7									15.3	0.255	0.153
SC2-29	0.25	50	2.00	8.9	1200	2.7%	3.4	5.9													14.8	0.247	0.148

Sand Creek Basin Above Woodmen Lag Time Calculation

[illegible]

POND DISCHARGE AND VOLUME CALCULATIONS

Culvert Calculator Report

Double 8'r x 8' s CBC

DBSMN OUTLET

Solve For: Discharge

Sterling Ranch North Pond

Culvert Summary

Allowable HW Elevation	7,114.00 ft	Headwater Depth/Height	2.50
Computed Headwater Elev.	7,114.00 ft	Discharge	2,374.05 cfs
Inlet Control HW Elev.	7,114.00 ft	Tailwater Elevation	7,094.00 ft
Outlet Control HW Elev.	7,110.02 ft	Control Type	Inlet Control

Grades

Upstream Invert	7,094.00 ft	Downstream Invert	7,090.00 ft
Length	100.00 ft	Constructed Slope	0.040000 ft/ft

Hydraulic Profile

Profile	S2	Depth, Downstream	5.75 ft
Slope Type	Steep	Normal Depth	4.07 ft
Flow Regime	Supercritical	Critical Depth	8.00 ft
Velocity Downstream	25.80 ft/s	Critical Slope	0.010449 ft/ft

Section

Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	8.00 ft
Section Size	8 x 8 ft	Rise	8.00 ft
Number Sections	2		

Outlet Control Properties

Outlet Control HW Elev.	7,110.02 ft	Upstream Velocity Head	5.35 ft
Ke	0.50	Entrance Loss	2.67 ft

Inlet Control Properties

Inlet Control HW Elev.	7,114.00 ft	Flow Control	Submerged
Inlet Type	90 and 15° wingwall flares	Area Full	128.0 ft²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	2
C	0.04000	Equation Form	1
Y	0.80000		

Rating Table Report

Double 8'r x 8' s CBC

Range Data:

	Minimum	Maximum	Increment
Allowable HW E	7,094.00	7,114.00	2.00 ft

HW Elev. (ft)	Discharge (cfs)
7,094.00	0.00
7,096.00	110.88
7,098.00	313.62
7,100.00	576.16
7,102.00	887.05
7,104.00	1,239.70
7,106.00	1,536.00
7,108.00	1,782.83
7,110.00	1,999.42
7,112.00	2,194.74
7,114.00	2,374.05

Type,... Vol: Elev-Area
Name.... POND 1

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Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sqr(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
7094.00	-----	1.4200	.0000	.000	.000
7096.00	-----	3.0980	6.6154	4.410	4.410
7098.00	-----	4.8600	11.8382	7.892	12.302
7100.00	-----	6.3700	16.7940	11.196	23.498
7102.00	-----	6.7000	19.6029	13.069	36.567
7104.00	-----	7.0400	20.6079	13.739	50.306
7106.00	-----	7.3800	21.6280	14.419	64.724
7108.00	-----	7.7290	22.6615	15.108	79.832
7110.00	-----	8.0860	23.7205	15.814	95.646
7112.00	-----	8.4470	24.7975	16.532	112.177
7114.00	-----	9.1020	26.3174	17.545	129.722

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

Volume = $(1/3) * (EL2 - EL1) * (Area1 + Area2 + \text{sq.rt.}(Area1 * Area2))$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

SR
North Pond

Copied from Sterling Ranch MDDP
Dated Oct 2007

Culvert Calculator Report

8'r x 8's CBC DBPSMS

Solve For: Discharge

Sterling Ranch South Pond

Culvert Summary

Allowable HW Elevation	7,024.00 ft	Headwater Depth/Height	2.50
Computed Headwater Elev.	7,024.00 ft	Discharge	2,380.94 cfs
Inlet Control HW Elev.	7,024.00 ft	Tailwater Elevation	7,002.00 ft
Outlet Control HW Elev.	7,020.07 ft	Control Type	Inlet Control

Grades

Upstream Invert	7,004.00 ft	Downstream Invert	6,998.00 ft
Length	100.00 ft	Constructed Slope	0.060000 ft/ft

Hydraulic Profile

Profile	S2	Depth, Downstream	5.21 ft
Slope Type	Steep	Normal Depth	3.50 ft
Flow Regime	Supercritical	Critical Depth	8.00 ft
Velocity Downstream	28.54 ft/s	Critical Slope	0.010510 ft/ft

Section

Section Shape	Box	Mannings Coefficient	0.013
Section Material	Concrete	Span	8.00 ft
Section Size	8 x 8 ft	Rise	8.00 ft
Number Sections	2		

Outlet Control Properties

Outlet Control HW Elev.	7,020.07 ft	Upstream Velocity Head	5.38 ft
Ke	0.50	Entrance Loss	2.69 ft

Inlet Control Properties

Inlet Control HW Elev.	7,024.00 ft	Flow Control	Submerged
Inlet Type	90 and 15° wingwall flares	Area Full	128.0 ft²
K	0.06100	HDS 5 Chart	8
M	0.75000	HDS 5 Scale	2
C	0.04000	Equation Form	1
Y	0.80000		

Rating Table Report **8'r x 8's CBC DBPSMS**

Range Data:

	Minimum	Maximum	Increment
Allowable HW E	7,004.00	7,018.00	2.00 ft

HW Elev. (ft)	Discharge (cfs)
7,004.00	0.00
7,006.00	110.88
7,008.00	313.62
7,010.00	576.16
7,012.00	887.05
7,014.00	1,239.70
7,016.00	1,546.63
7,018.00	1,792.00

Name.... POND 2

File.... G:\Projects\09001\Dines Ranch\Pondpack\Ponds Middle Final.ppw

Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sq(A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
7004.00	-----	.9240	.0000	.000	.000
7006.00	-----	1.8250	4.0476	2.698	2.698
7008.00	-----	3.1990	7.4402	4.960	7.659
7010.00	-----	5.0460	12.2627	8.175	15.834
7012.00	-----	7.4670	18.6513	12.434	28.268
7014.00	-----	9.7820	25.7955	17.197	45.465
7016.00	-----	10.6060	30.5737	20.382	65.847
7018.00	-----	11.3700	32.9574	21.972	87.819
7020.00	-----	12.1880	35.3299	23.553	111.372
7022.00	-----	13.0990	37.9223	25.282	136.654
7024.00	-----	13.7190	40.2234	26.816	163.469

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sq.rt.}(\text{Area1} * \text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
 Area1, Area2 = Areas computed for EL1, EL2, respectively
 Volume = Incremental volume between EL1 and EL2

SR - South Pond

Copied from Sterling Ranch MDDP
 Dated Oct 2007

Culvert Calculator Report

West Pond

Solve For: Discharge

Sterling Ranch West Pond

Culvert Summary

Allowable HW Elevation	6,984.00 ft	Headwater Depth/Height	4.00
Computed Headwater Elev.	6,984.00 ft	Discharge	230.24 cfs
Inlet Control HW Elev.	6,984.00 ft	Tailwater Elevation	6,968.00 ft
Outlet Control HW Elev.	6,980.35 ft	Control Type	Inlet Control

Grades

Upstream Invert	6,968.00 ft	Downstream Invert	6,966.00 ft
Length	100.00 ft	Constructed Slope	0.020000 ft/ft

Hydraulic Profile

Profile	CompositeM2PressureProfile	Depth, Downstream	3.91 ft
Slope Type	Mild	Normal Depth	N/A ft
Flow Regime	Subcritical	Critical Depth	3.91 ft
Velocity Downstream	18.43 ft/s	Critical Slope	0.022898 ft/ft

Section

Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	4.00 ft
Section Size	48 Inch	Rise	4.00 ft
Number Sections	1		

Outlet Control Properties

Outlet Control HW Elev.	6,980.35 ft	Upstream Velocity Head	5.22 ft
Ke	0.50	Entrance Loss	2.61 ft

Inlet Control Properties

Inlet Control HW Elev.	6,984.00 ft	Flow Control	Submerged
Inlet Type	Square edge w/headwall	Area Full	12.6 ft²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Rating Table Report West Pond

Range Data:

	Minimum	Maximum	Increment
Allowable HW E	6,968.00	6,984.00	0.50 ft

HW Elev. (ft)	Discharge (cfs)
6,968.00	0.00
6,968.50	1.33
6,969.00	5.18
6,969.50	11.28
6,970.00	19.37
6,970.50	29.17
6,971.00	40.39
6,971.50	52.70
6,972.00	65.77
6,972.50	79.27
6,973.00	92.87
6,973.50	106.26
6,974.00	115.46
6,974.50	123.75
6,975.00	131.53
6,975.50	138.86
6,976.00	145.83
6,976.50	152.48
6,977.00	158.85
6,977.50	164.98
6,978.00	170.89
6,978.50	176.60
6,979.00	182.13
6,979.50	187.49
6,980.00	192.71
6,980.50	197.79
6,981.00	202.74
6,981.50	207.58
6,982.00	212.30
6,982.50	216.93
6,983.00	221.45
6,983.50	225.89
6,984.00	230.24

OUTLET STRUCTURE INPUT DATA

Structure ID = C1
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 4.0000 ft
Upstream Invert = 6968.00 ft
Dnstream Invert = 6966.00 ft
Horiz. Length = 100.00 ft
Barrel Length = 100.02 ft
Barrel Slope = .02000 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0130
Ke = .0100 (forward entrance loss)
Kb = .004925 (per ft of full flow)
Kr = .0100 (reverse entrance loss)
HW Convergence = .001 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0098
Inlet Control M = 2.0000
Inlet Control c = .03980
Inlet Control Y = .6700
T1 ratio (HW/D) = .000
T2 ratio (HW/D) = 1.297
Slope Factor = -.500
Calc inlet only = Yes

Use unsubmerged inlet control Form 1 equ. below T1 elev.

Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...

At T1 Elev = 6968.00 ft ---> Flow = 87.96 cfs

At T2 Elev = 6973.19 ft ---> Flow = 100.53 cfs

SR West Pond

Type.... Vol: Elev-Area

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Name.... POND

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Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sq ^r (A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
6968.00	-----	.0900	.0000	.000	.000
6970.00	-----	1.9440	2.4523	1.635	1.635
6972.00	-----	4.1320	8.9102	5.940	7.575
6974.00	-----	4.3800	12.7662	8.511	16.086
6976.00	-----	4.6400	13.5281	9.019	25.105
6978.00	-----	4.9050	14.3157	9.544	34.648
6980.00	-----	5.1770	15.1212	10.081	44.729
6982.00	-----	5.4540	15.9447	10.630	55.359
6984.00	-----	5.7400	16.7892	11.193	66.552

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sq. rt.}(\text{Area1} * \text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

SR-West POND

Pond 3

Regional Pond SC3

Stage	Length (ft)	Width (ft)	Area (sf)	Incremental Volume (cf)	Cumulative Volume (cf)	Cumulative Volume (ac*ft)	93% Cumulative Volume (ac*ft)
77			0	0	0	0.00	0.00
78			62087	31043.5	31043.5	0.71	0.66
79			243916	153001.5	184045	4.23	3.93
80			486121	365018.5	549063.5	12.60	11.72
81			714713	600417	1149480.5	26.39	24.54
82			750407	732560	1882040.5	43.21	40.18
84			774555	1524962	3407002.5	78.21	72.74
85			788301	781428	4188430.5	96.15	89.42
86			802155	795228	4983658.5	114.41	106.40
88			830155	1632310	6615968.5	151.88	141.25
90			859511	1689666	8305634.5	190.67	177.32
92			888368	1747879	10053513.5	230.80	214.64

Pond 3

Worksheet for Broad Crested Weir - 1

Project Description

Solve For

Discharge

Input Data

Headwater Elevation	86.00	ft
Crest Elevation	85.00	ft
Tailwater Elevation	78.00	ft
Crest Surface Type	Paved	
Crest Breadth	80.00	ft
Crest Length	35.00	ft

Results

Discharge	106.29	ft ³ /s
Headwater Height Above Crest	1.00	ft
Tailwater Height Above Crest	-7.00	ft
Weir Coefficient	3.04	US
Submergence Factor	1.00	
Adjusted Weir Coefficient	3.04	US
Flow Area	35.00	ft ²
Velocity	3.04	ft/s
Wetted Perimeter	37.00	ft
Top Width	35.00	ft

Pond 3

Rating Table for Broad Crested Weir - 1

Project Description

Solve For

Discharge

Input Data

Headwater Elevation	90.00	ft
Crest Elevation	85.00	ft
Tailwater Elevation	78.00	ft
Crest Surface Type	Paved	
Crest Breadth	80.00	ft
Crest Length	35.00	ft

Headwater Elevation (ft)

Discharge (ft³/s)

Velocity (ft/s)

85.00		
86.00	106.29	3.04
87.00	300.94	4.30
88.00	552.87	5.27
89.00	851.20	6.08
90.00	1189.59	6.80
91.00	1563.75	7.45
92.00	1970.56	8.04

Culvert Calculator Report

Pond 3 - 18" Outlet Pipe

Pond 3

Solve For: Discharge

Culvert Summary

Allowable HW Elevation	90.00 ft	Headwater Depth/Height	8.67
Computed Headwater Elev.	90.00 ft	Discharge	26.27 cfs
Inlet Control HW Elev.	84.55 ft	Tailwater Elevation	76.00 ft
Outlet Control HW Elev.	90.00 ft	Control Type	Outlet Control

Grades

Upstream Invert	77.00 ft	Downstream Invert	75.00 ft
Length	150.00 ft	Constructed Slope	0.013333 ft/ft

Hydraulic Profile

Profile	CompositeM2PressureProfile	Depth, Downstream	1.49 ft
Slope Type	Mild	Normal Depth	N/A ft
Flow Regime	Subcritical	Critical Depth	1.49 ft
Velocity Downstream	14.88 ft/s	Critical Slope	0.058237 ft/ft

Section

Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	1.50 ft
Section Size	18 Inch	Rise	1.50 ft
Number Sections	1		

Outlet Control Properties

Outlet Control HW Elev.	90.00 ft	Upstream Velocity Head	3.43 ft
Ke	0.20	Entrance Loss	0.69 ft

Inlet Control Properties

Inlet Control HW Elev.	84.55 ft	Flow Control	Submerged
Inlet Type	Groove end w/headwall	Area Full	1.8 ft ²
K	0.00180	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	2
C	0.02920	Equation Form	1
Y	0.74000		

Pond 3

Rating Table Report Pond 3 - 18" Outlet Pipe

Range Data:

	Minimum	Maximum	Increment
Allowable HW E	77.00	92.00	1.00 ft

HW Elev. (ft)	Discharge (cfs)
77.00	0.00
78.00	3.23
79.00	9.81
80.00	13.56
81.00	15.23
82.00	16.80
83.00	18.25
84.00	19.59
85.00	20.85
86.00	22.04
87.00	23.17
88.00	24.25
89.00	25.28
90.00	26.27
91.00	27.22
92.00	28.15

Type.... Vol: Elev-Area
Name.... POND 6

Pond 6

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Elevation (ft)	Planimeter (sq.in)	Area (acres)	A1+A2+sq ^r (A1*A2) (acres)	Volume (ac-ft)	Volume Sum (ac-ft)
6860.00	-----	.0010	.0000	.000	.000
6862.00	-----	.0680	.0772	.051	.051
6864.00	-----	.3370	.5564	.371	.422
6866.00	-----	.8610	1.7367	1.158	1.580
6868.00	-----	1.5390	3.5511	2.367	3.948
6870.00	-----	2.9650	6.6402	4.427	8.374
6872.00	-----	4.0100	10.4231	6.949	15.323
6874.00	-----	5.2070	13.7865	9.191	24.514
6876.00	-----	6.4580	17.4639	11.643	36.157
6878.00	-----	8.0150	21.6675	14.445	50.602
6880.00	-----	10.0320	27.0140	18.009	68.611

POND VOLUME EQUATIONS

* Incremental volume computed by the Conic Method for Reservoir Volumes.

$$\text{Volume} = (1/3) * (\text{EL2} - \text{EL1}) * (\text{Area1} + \text{Area2} + \text{sq.rt.}(\text{Area1} * \text{Area2}))$$

where: EL1, EL2 = Lower and upper elevations of the increment
Area1, Area2 = Areas computed for EL1, EL2, respectively
Volume = Incremental volume between EL1 and EL2

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
 Project Engineer: Benjamin E. Sheets
 Project Title: SAND CREEK DETENTION POND NO. 6
 Project Comments:
 ULTIMATE DEVELOPMENT FLOWS
 ULTIMATE POND CONSTRUCTION

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
6860.00	.00	Free Outfall		(no Q: 02,03,04,Ib,01,Cv)
6860.20	.02	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6860.40	.32	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6860.60	.55	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6860.80	1.53	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6861.00	2.24	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6861.20	3.15	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6861.40	5.15	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6861.60	6.10	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6861.80	7.76	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6862.00	7.82	Free Outfall		01,Cv (no Q: 02,03,04,Ib)
6862.20	8.50	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6862.40	9.24	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6862.60	10.90	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6862.80	12.56	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6863.00	14.31	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6863.20	16.18	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6863.40	17.98	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6863.60	20.05	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6863.80	22.42	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6864.00	21.43	Free Outfall		02,01,Cv (no Q: 03,04,Ib)
6864.20	22.82	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6864.40	24.55	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6864.60	25.82	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6864.80	27.86	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6865.00	29.90	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6865.20	31.94	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6865.40	34.31	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6865.60	36.05	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6865.80	37.38	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6866.00	38.72	Free Outfall		02,03,01,Cv (no Q: 04,Ib)
6866.20	40.05	Free Outfall		02,03,04,01,Cv (no Q: Ib)

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
 Project Engineer: Benjamin E. Sheets
 Project Title: SAND CREEK DETENTION POND NO. 6
 Project Comments:
 ULTIMATE DEVELOPMENT FLOWS
 ULTIMATE POND CONSTRUCTION

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
6866.40	42.13	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6866.60	43.78	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6866.80	46.21	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6867.00	48.07	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6867.20	50.07	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6867.40	52.74	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6867.60	55.41	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6867.80	57.25	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6868.00	58.75	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6868.20	60.62	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6868.40	61.96	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6868.60	63.42	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6868.80	64.75	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6869.00	66.09	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6869.20	67.42	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6869.40	68.76	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6869.60	70.67	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6869.80	71.39	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6870.00	72.10	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6870.20	73.42	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6870.40	74.74	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6870.60	75.44	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6870.80	76.10	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6871.00	77.46	Free Outfall		02,03,04,01,Cv (no Q: Ib)
6871.20	82.17	Free Outfall		02,03,04,Ib,01,Cv
6871.40	91.05	Free Outfall		02,03,04,Ib,01,Cv
6871.60	107.62	Free Outfall		02,03,04,Ib,01,Cv
6871.80	119.27	Free Outfall		02,03,04,Ib,01,Cv
6872.00	132.06	Free Outfall		02,03,04,Ib,01,Cv
6872.20	146.29	Free Outfall		02,03,04,Ib,01,Cv
6872.40	161.34	Free Outfall		02,03,04,Ib,01,Cv
6872.60	172.61	Free Outfall		02,03,04,Ib,01,Cv

Type.... Composite Rating Curve

Page 2.50

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
Project Engineer: Benjamin E. Sheets
Project Title: SAND CREEK DETENTION POND NO. 6
Project Comments:
ULTIMATE DEVELOPMENT FLOWS
ULTIMATE POND CONSTRUCTION

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
6872.80	178.53	Free Outfall		02,03,04,Ib,01,Cv
6873.00	184.20	Free Outfall		02,03,04,Ib,01,Cv
6873.20	189.48	Free Outfall		02,03,04,Ib,01,Cv
6873.40	194.88	Free Outfall		02,03,04,Ib,01,Cv
6873.60	199.97	Free Outfall		02,03,04,Ib,01,Cv
6873.80	204.88	Free Outfall		02,03,04,Ib,01,Cv
6874.00	209.59	Free Outfall		02,03,04,Ib,01,Cv
6874.20	214.24	Free Outfall		02,03,04,Ib,01,Cv
6874.40	218.74	Free Outfall		02,03,04,Ib,01,Cv
6874.60	223.25	Free Outfall		02,03,04,Ib,01,Cv
6874.80	227.39	Free Outfall		02,03,04,Ib,01,Cv
6875.00	231.44	Free Outfall		02,03,04,Ib,01,Cv
6875.20	235.57	Free Outfall		02,03,04,Ib,01,Cv
6875.40	239.48	Free Outfall		02,03,04,Ib,01,Cv
6875.60	243.45	Free Outfall		02,03,04,Ib,01,Cv
6875.80	247.31	Free Outfall		02,03,04,Ib,01,Cv
6876.00	251.00	Free Outfall		02,03,04,Ib,01,Cv
6876.20	254.67	Free Outfall		02,03,04,Ib,01,Cv
6876.40	258.27	Free Outfall		02,03,04,Ib,01,Cv
6876.60	262.11	Free Outfall		02,03,04,Ib,01,Cv
6876.80	265.64	Free Outfall		02,03,04,Ib,01,Cv
6877.00	269.17	Free Outfall		02,03,04,Ib,01,Cv
6877.20	272.40	Free Outfall		02,03,04,Ib,01,Cv
6877.40	275.88	Free Outfall		02,03,04,Ib,01,Cv
6877.60	279.24	Free Outfall		02,03,04,Ib,01,Cv
6877.80	282.36	Free Outfall		02,03,04,Ib,01,Cv
6878.00	285.54	Free Outfall		02,03,04,Ib,01,Cv
6878.20	288.68	Free Outfall		02,03,04,Ib,01,Cv
6878.40	291.77	Free Outfall		02,03,04,Ib,01,Cv
6878.60	294.84	Free Outfall		02,03,04,Ib,01,Cv
6878.80	297.87	Free Outfall		02,03,04,Ib,01,Cv
6879.00	300.86	Free Outfall		02,03,04,Ib,01,Cv

Type.... Composite Rating Curve

Page 2.51

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
Project Engineer: Benjamin E. Sheets
Project Title: SAND CREEK DETENTION POND NO. 6
Project Comments:
ULTIMATE DEVELOPMENT FLOWS
ULTIMATE POND CONSTRUCTION

***** COMPOSITE OUTFLOW SUMMARY *****

WS Elev, Total Q		Converge		Notes
Elev. ft	Q cfs	TW Elev ft	Error +/-ft	Contributing Structures
6879.20	303.82	Free Outfall		02,03,04,Ib,01,Cv
6879.40	306.75	Free Outfall		02,03,04,Ib,01,Cv
6879.60	309.75	Free Outfall		02,03,04,Ib,01,Cv
6879.80	312.52	Free Outfall		02,03,04,Ib,01,Cv
6880.00	315.37	Free Outfall		02,03,04,Ib,01,Cv

Type.... Outlet Input Data

Page 2.01

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
Project Engineer: Benjamin E. Sheets
Project Title: SAND CREEK DETENTION POND NO. 6
Project Comments:
ULTIMATE DEVELOPMENT FLOWS
ULTIMATE POND CONSTRUCTION

REQUESTED POND WS ELEVATIONS:

Min. Elev.= 6860.00 ft
Increment = .20 ft
Max. Elev.= 6880.00 ft

OUTLET CONNECTIVITY

---> Forward Flow Only (UpStream to DnStream)
<--- Reverse Flow Only (DnStream to UpStream)
<---> Forward and Reverse Both Allowed

Structure	No.		Outfall	E1, ft	E2, ft
Orifice-Circular	02	--->	CV	6862.000	6880.000
Orifice-Circular	03	--->	CV	6864.000	6880.000
Orifice-Circular	04	--->	CV	6866.000	6880.000
Inlet Box	Ib	--->	CV	6871.000	6880.000
Orifice-Circular	01	--->	CV	6860.000	6880.000
Culvert-Circular	Cv	--->	TW	6860.000	6880.000
TW SETUP, DS Channel					

Type.... Outlet Input Data

Page 2.02

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007

Project Engineer: Benjamin E. Sheets

Project Title: SAND CREEK DETENTION POND NO. 6

Project Comments:

ULTIMATE DEVELOPMENT FLOWS

ULTIMATE POND CONSTRUCTION

OUTLET STRUCTURE INPUT DATA

Structure ID	=	02
Structure Type	=	Orifice-Circular

# of Openings	=	1
Invert Elev.	=	6862.00 ft
Diameter	=	1.5000 ft
Orifice Coeff.	=	.600

Structure ID	=	03
Structure Type	=	Orifice-Circular

# of Openings	=	1
Invert Elev.	=	6864.00 ft
Diameter	=	1.5000 ft
Orifice Coeff.	=	.600

Structure ID	=	04
Structure Type	=	Orifice-Circular

# of Openings	=	1
Invert Elev.	=	6866.00 ft
Diameter	=	1.5000 ft
Orifice Coeff.	=	.600

Type.... Outlet Input Data

Page 2.03

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
Project Engineer: Benjamin E. Sheets
Project Title: SAND CREEK DETENTION POND NO. 6
Project Comments:
ULTIMATE DEVELOPMENT FLOWS
ULTIMATE POND CONSTRUCTION

OUTLET STRUCTURE INPUT DATA

Structure ID = Ib
Structure Type = Inlet Box

of Openings = 1
Invert Elev. = 6871.00' ft
Orifice Area = 14.0625' sq.ft
Orifice Coeff. = .600
Weir Length = 15.00' ft
Weir Coeff. = 3.000
K, Reverse = 1.000
Mannings n = .0000
Kev, Charged Riser = .000'
Weir Submergence = No

Structure ID = 01
Structure Type = Orifice-Circular

of Openings = 1
Invert Elev. = 6860.00 ft
Diameter = 1.5000 ft
Orifice Coeff. = .600

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
Project Engineer: Benjamin E. Sheets
Project Title: SAND CREEK DETENTION POND NO. 6
Project Comments:
ULTIMATE DEVELOPMENT FLOWS
ULTIMATE POND CONSTRUCTION

OUTLET STRUCTURE INPUT DATA

Structure ID = Cv
Structure Type = Culvert-Circular

No. Barrels = 1
Barrel Diameter = 6.0000 ft
Upstream Invert = 6860.00 ft
Dnstream Invert = 6850.00 ft
Horiz. Length = 350.00 ft
Barrel Length = 350.14 ft
Barrel Slope = .02857 ft/ft

OUTLET CONTROL DATA...

Mannings n = .0150
Ke = .5000 (forward entrance loss)
Kb = .003819 (per ft of full flow)
Kr = .2000 (reverse entrance loss)
HW Convergence = .010 +/- ft

INLET CONTROL DATA...

Equation form = 1
Inlet Control K = .0098
Inlet Control M = 2.0000
Inlet Control c = .03980
Inlet Control Y = .6700
T1 ratio (HW/D) = .000
T2 ratio (HW/D) = 1.327
Slope Factor = .700

Use unsubmerged inlet control Form 1 equ. below T1 elev.

Use submerged inlet control Form 1 equ. above T2 elev.

In transition zone between unsubmerged and submerged inlet control,
interpolate between flows at T1 & T2...

At T1 Elev = 6860.00 ft ---> Flow = 242.40 cfs
At T2 Elev = 6867.96 ft ---> Flow = 277.03 cfs

Type.... Outlet Input Data

Page 2.05

Name.... FINAL

File.... C:\Temp\Sand Creek Alt\Pond Pack\POND 6 ULT DEV FINAL.ppw

Title... Project Date: 6/6/2007
Project Engineer: Benjamin E. Sheets
Project Title: SAND CREEK DETENTION POND NO. 6
Project Comments:
ULTIMATE DEVELOPMENT FLOWS
ULTIMATE POND CONSTRUCTION

OUTLET STRUCTURE INPUT DATA

Structure ID = TW
Structure Type = TW SETUP, DS Channel

FREE OUTFALL CONDITIONS SPECIFIED

CONVERGENCE TOLERANCES...

Maximum Iterations= 30
Min. TW tolerance = .01 ft
Max. TW tolerance = .01 ft
Min. HW tolerance = .01 ft
Max. HW tolerance = .01 ft
Min. Q tolerance = .10 cfs
Max. Q tolerance = .10 cfs

APPENDIX C

HEC-1 MODEL OUTPUT

100 YEAR OUTPUT
With Runoff Summary

Upstream of Woodmen Road

```
*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*
* RUN DATE 06/25/2009 TIME 11:40:26
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

```

X   X   XXXXXXX   XXXXX   X
X   X   X       X   X   XX
X   X   X       X   X   X
XXXXXXX   XXXX   X       XXXXX   X
X   X   X       X   X   X
X   X   X       X   X   X
X   X   XXXXXXX   XXXXX   XXX

```

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::::::::::::::::::::::::::::::::::::
::::::::::::::::::::::::::::::::::::
:::
::: Full Microcomputer Implementation :::
::: by :::
::: Haestad Methods, Inc. :::
:::
::::::::::::::::::::::::::::::::::::

```

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS;WRITE STAGE FREQUENCY,
 DSS;READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Sand Creek Basin Upstream of Woodmen Road									
2	ID	Model prepared by WCI for Upper Sand Creek Basin									
3	ID	Detention Evaluation Report									
4	ID	FUTURE DEVELOPMENT CONDITION Including Sterling Ranch									
5	ID	Utilizes Extended Detention of Excess 2 Year Volume in									
6	ID	Sand Creek Regional Pond 3									
7	ID	100 Year, 24 hr Type IIA Storm fn:SCBUW100.dat									
8	IT	4	0	0	300						
9	IO	5									
10	KK	SB82									
11	KM	RUNOFF FROM SUB-BASIN 82									
12	BA	.184									
13	IN	30									
14	PB	4.4									
15	PC	0.0000	0.0025	0.0050	0.0075	0.0100	0.0150	0.0200	0.0250	0.0300	0.0500
16	PC	0.0600	0.1000	0.7000	0.7500	0.7800	0.7980	0.8200	0.8300	0.8400	0.8500
17	PC	0.8600	0.8650	0.8700	0.8850	0.8900	0.9000	0.9050	0.9100	0.9150	0.9210
18	PC	0.9270	0.9330	0.9400	0.9450	0.9500	0.9525	0.9550	0.9600	0.9650	0.9700
19	PC	0.9750	0.9800	0.9830	0.9850	0.9880	0.9900	0.9930	0.9950	0.9980	1.0000
20	LS	0	67								
21	UD	.282									
22	KK	RT-4									
23	KM	ROUTE FLOW FROM SUB-BASIN 82 TO DP 74									
24	RK	2300	.030	.035	TRAP		10	10			
25	KK	SB74									
26	KM	RUNOFF FROM SUB-BASIN 74									
27	BA	.188									
28	LS	0	67								
29	UD	0.318									
30	KK	DP74									
31	KM	DESIGN POINT 74 COMBINE RUNOFF FROM SUB-BASIN 74 AND RT-4									
32	HC	2									
33	KK	RT-3									
34	KM	ROUTE FLOW FROM DP 74 TO DP 75									
35	RK	2800	.025	.035	TRAP		10				
36	KK	SB80									
37	KM	RUNOFF FROM SUB-BASIN 80									
38	BA	.231									
39	LS	0	67								
40	UD	.379									
41	KK	RT-2									
42	KM	ROUTE FLOW FROM SB 80 TO DP 75									
43	RK	4100	.035	.035	TRAP		10				

HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
86	KK DF78
87	KM DESIGN POINT 78 COMBINE RUNOFF FROM SUB-BASIN 78 AND RT-7
88	HC 2
89	KK RT-6
90	KM ROUTE FLOW FROM DESIGN POINT 78 TO DP 77
91	RK 3350 .025 .035 TRAP 10 6
92	KK SB77
93	KM RUNOFF FROM SUB-BASIN 77
94	BA .269
95	LS 0 68
96	UD .338
97	KK SB88
98	KM RUNOFF FROM SUB-BASIN 88
99	BA .137
100	LS 0 67
101	UD .311
102	KK DP77
103	KM DESIGN POINT 77 COMBINE RUNOFF FROM SUB-BASINS 77 AND 88 AND RT-6, AND RT-5
104	HC 4
105	KK RT-8
106	KM ROUTE FLOW FROM DESIGN POINT 77 TO DP 71
107	RK 4600 .018 .035 TRAP 25 4
108	KK SC3-20
109	KM RUNOFF FROM SUB-BASIN SC3-20
110	BA .306
111	LS 0 68
112	UD .274
113	KK DP71
114	KM DESIGN POINT 71 COMBINE RUNOFF FROM SUB-BASIN SC3-20 AND RT-8
115	HC 2
116	KK RT-9
117	KM ROUTE FLOW FROM DESIGN POINT 71 TO DP 70
118	RK 980 .020 .035 TRAP 40 4
119	KK SB3-18

120 KM RUNOFF FROM SUB-BASIN SC3-18
 121 BA .267
 122 LS 0 76
 123 UD .234
 124 KK DP70
 125 KM DESIGN POINT 70 COMBINE RUNOFF FROM SUB-BASIN SC3-18 AND RT-9
 126 HC 2

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

127 KK RT-11
 128 KM ROUTE FLOW FROM DESIGN POINT 70 TO DP 69 AT DB-SRMN
 129 RK 1100 .018 .035 TRAP 40 5
 130 KK SC3-19
 131 KM RUNOFF FROM SUB-BASIN SC3-19
 132 BA .301
 133 LS 0 76
 134 UD .197
 135 KK RT-10
 136 KM ROUTE FLOW FROM SUB-BASIN SC3-19 TO DESIGN POINT 69 AT DB-SRMN
 137 RK 1720 .020 .013 CIRC 6
 138 KK SC3-17
 139 KM RUNOFF FROM SUB-BASIN SB3-17
 140 BA .178
 141 LS 0 82
 142 UD .190
 143 KK DP69
 144 KM DESIGN POINT 69 COMBINE RUNOFF FROM SUB-BASIN SC3-17 RT-10 AND RT-11
 145 HC 3
 146 KK DBSRMN
 147 KM ROUTE FLOW THROUGH THE STERLING RANCH MIDDLE NORTH POND
 148 KM STAGE STORAGE CURVE PER DRAFT MDDP FOR STERLING RANCH DATED 10-2007
 149 KM DISCHARGE CURVE ASSUMES A DOUBLE 8'X 8'CBC Ke=0.5 US INV=7094
 150 KO 3
 151 RS 1 ELEV 94
 152 SV 0 4.4 12.3 23.5 36.6 50.3 64.7 79.8 95.6 112
 153 SQ 0 111 314 576 887 1239 1536 1782 2000 2195
 154 SE 94 96 98 100 102 104 106 108 110 112
 155 KK RT-12
 156 KM ROUTE FLOW FROM BASIN SC3-15 TO DP 87
 157 RK 2000 .015 .035 TRAP 50 6
 158 KK SC3-15
 159 KM RUNOFF FROM SUB-BASIN SC3-15
 160 BA .210
 161 LS 0 85.0
 162 UD .217
 163 KK RT-10A
 164 KM ROUTE FLOW FROM SC3-3-14 TO DP 87
 165 RK 1600 .019 .013 CIRC 6.0
 166 KK DP87
 167 KM DESIGN POINT 87 COMBINE RUNOFF FROM RT-10A AND RT-11
 168 HC 2

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

169 KK RT-13
 170 KM ROUTE FLOW FROM DESIGN POINT 87 TO DP 68 AT DBSRMS
 171 RK 2560 .012 .035 TRAP 50 4
 172 KK SC3-14
 173 KM RUNOFF FROM SUB-BASIN SC3-14
 174 BA .341
 175 LS 0 86
 176 UD .229
 177 KK SC3-16
 178 KM RUNOFF FROM SUB-BASIN SC3-16
 179 BA .300
 180 LS 0 85
 181 UD .247
 182 KK RT-13A
 183 KM ROUTE FLOW FROM SUB-BASIN SC3-16 TO DESIGN POINT 68 AT DBSRMS
 184 RK 3060 .005 .035 TRAP 20 4
 185 KK DP68
 186 KM DESIGN POINT 68 COMBINE RUNOFF FROM SUB-BASIN SC3-14 RT-13 AND RT-13A
 187 HC 3
 188 KK DBSRMS
 189 KM ROUTE FLOW THROUGH THE STERLING RANCH MIDDLE SOUTH POND
 190 KM STAGE STORAGE CURVE PER DRAFT MDDP FOR STERLING RANCH DATED 10-2007
 191 KM DISCHARGE CURVE ASSUMES A DOUBLE 8s'X 8r'CBC Ke=0.5 US INV=7024
 192 KO 3
 193 RS 1 ELEV 4
 194 SV 0 2.7 7.6 15.8 28.2 45.4 65.8 87.8
 195 SQ 0 110 313 576 887 1239 1546 1792
 196 SE 4 6 8 10 12 14 16 18
 197 KK RT-17

198	KM	ROUTE FLOW FROM DESIGN POINT 68 TO DP 63				
199	RK	1750 .009 .035 TRAP	40	4		
200	KK	SC3-13				
201	KM	RUNOFF FROM SUB-BASIN SC3-13				
202	BA	.337				
203	LS	0 68				
204	UD	.252				
205	KK	RT-19				
206	KM	ROUTE FLOW FROM SUB-BASIN SC3-13 TO DESIGN POINT 66				
207	RK	4360 .023 .013 CIRC	4.5			
208	KK	SC3-12				
209	KM	RUNOFF FROM SUB-BASIN SC3-12				
210	BA	.206				
211	LS	0 84.0				
212	UD	.211				
		HEC-1 INPUT				PAGE 6
LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10					
213	KK	DP66				
214	KM	DESIGN POINT 66 COMBINE RUNOFF FROM SUB-BASIN SC3-12 AND RT-19				
215	HC	2				
216	KK	RT-20				
217	KM	ROUTE FLOW FROM DESIGN POINT 66 TO DESIGN POINT 63A				
218	RK	5760 .005 .030 TRAP	25	4		
219	KK	SC3-11				
220	KM	RUNOFF FROM SUB-BASIN SC3-11				
221	BA	.285				
222	LS	0 86.0				
223	UD	.284				
224	KK	DP63A				
225	KM	DESIGN POINT 63A COMBINE RUNOFF FROM SUB-BASIN SC3-11 AND RT-20				
226	HC	2				
227	KK	DP63				
228	KM	DESIGN POINT 63 COMBINE RUNOFF FROM RT-17 AND DP-63A				
229	HC	2				
230	KK	RT-17A				
231	KM	ROUTE FLOW FROM DESIGN POINT 63 TO 61				
232	RK	2080 .021 .035 TRAP	40	4		
233	KK	SC3-10				
234	KM	RUNOFF FROM SUB-BASIN SC3-10				
235	BA	.064				
236	LS	0 68.0				
237	UD	.234				
238	KK	SC3-9				
239	KM	RUNOFF FROM SUB-BASIN SC3-9				
240	BA	.499				
241	LS	0 71.0				
242	UD	.437				
243	KK	RT-14				
244	KM	ROUTE FLOW FROM BASIN SC3-9 TO DP 65				
245	RK	1700 .005 .030 TRAP	10	4		
246	KK	SC3-8				
247	KM	RUNOFF FROM SUB-BASIN SC3-8				
248	BA	.226				
249	LS	0 75.0				
250	UD	.401				
251	KK	DP65				
252	KM	DESIGN POINT 65 COMBINE RUNOFF FROM SUB-BASIN SC3-8 AND RT-14				
253	HC	2				
		HEC-1 INPUT				PAGE 7
LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10					
254	KK	RT-15				
255	KM	ROUTE FLOW FROM DP 65 TO DESIGN POINT 64				
256	RK	1550 .020 .013 CIRC	6			
257	KK	SC3-7				
258	KM	RUNOFF FROM SUB-BASIN SC3-7				
259	BA	.155				
260	LS	0 86.0				
261	UD	.183				
262	KK	RT-15A				
263	KM	ROUTE FLOW FROM BASIN SC3-7 TO DP 64				
264	RK	2030 .015 .013 CIRC	5.0			
265	KK	DP64				
266	KM	DESIGN POINT 64 COMBINE RUNOFF FROM RT-15 AND RT-15A				
267	HC	2				
268	KK	RT-16				
269	KM	ROUTE FLOW FROM BASIN DESIGN POINT 64 TO DESIGN POINT 62				
270	RK	1030 .020 .013 CIRC	7			
271	KK	SC3-6				
272	KM	RUNOFF FROM SUB-BASIN SC3-6				
273	BA	.206				

274 LS 0 86.0
 275 UD .159

 276 KK DP62
 277 KM DESIGN POINT 62 COMBINE RUNOFF FROM RT-16 AND BASIN SC3-6
 278 HC 2

 279 KK DBSRW
 280 KM ROUTE FLOW THROUGH THE STERLING RANCH WEST POND
 281 KO 3
 282 RS 1 ELEV 4
 283 SV 0 1.6 7.5 16.0 25.1 34.6 44.7 55.3 66.5
 284 SQ 0 19 66 115 146 171 192 212 230
 285 SE 68 70 72 74 76 78 80 82 84

 286 KK DP61
 287 KM DESIGN POINT 61 COMBINE RUNOFF FROM SUB-BASIN SC3-10 AND RT-17A AND DB-SRW
 288 HC 3

 289 KK RT-17B
 290 KM ROUTE FLOW FROM DP 61 TO DESIGN POINT 60
 291 RK 2250 .013 .035 TRAP 40 4

 292 KK SB61
 293 KM RUNOFF FROM SUB-BASIN 61
 294 BA .121
 295 LS 0 68.0
 296 UD .373

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

297 KK RT-20
 298 KM ROUTE FLOW FROM BASIN 61 TO DESIGN POINT 60
 299 RK 1460 .005 .035 CIRC 3.5

 300 KK SC3-5
 301 KM RUNOFF FROM SUB-BASIN SC3-5
 302 BA .155
 303 LS 0 84.0
 304 UD .136

 305 KK DP60
 306 KM DESIGN POINT 60 COMBINE RUNOFF FROM SUB-BASINS SC3-5 AND 61 AND RT-15
 307 HC 3

 308 KK RT-18
 309 KM ROUTE FLOW FROM DESIGN POINT 60 TO DESIGN POINT 53
 310 RK 1870 .014 .035 TRAP 40 4

 311 KK SC3-4
 312 KM RUNOFF FROM SUB-BASIN SC3-4
 313 BA .081
 314 LS 0 89.0
 315 UD .128

 316 KK RT-21
 317 KM ROUTE FLOW FROM BASIN 4 TO DESIGN POINT 53
 318 RK 1350 .013 .013 CIRC 4.5

 319 KK SC3-1
 320 KM RUNOFF FROM SUB-BASIN SC3-1
 321 BA .170
 322 LS 0 78.0
 323 UD .134

 324 KK SC3-3
 325 KM RUNOFF FROM SUB-BASIN SC3-3
 326 BA .236
 327 LS 0 84.0
 328 UD .189

 329 KK DBSC33
 330 KM ROUTE FLOW THROUGH AN APPROXIMATE POND TO SIMULATE POTENTIAL DETENTION
 331 KO 3
 332 RS 1 ELEV 0
 333 SV 0 2.3 5.0 8.0 11.5 15.4
 334 SQ 0 37 104 195 280 340
 335 SE 0 2 4 6 8 10

 336 KK RT-21B
 337 KM ROUTE FLOW FROM SUB-BASIN SC3-3 TO DESIGN POINT 53
 338 RK 2640 .017 .013 CIRC 6

 HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

339 KK SC3-2
 340 KM RUNOFF FROM SUB-BASIN SC3-2
 341 BA .081
 342 LS 0 85.0
 343 UD .172

 344 KK DP53
 345 KM DESIGN POINT 53 COMBINE RUNOFF FROM SUB-BASINS SC3-1 AND SC3-2 WITH
 346 KM RT-18, RT-21 AND RT-21B
 347 KO 3
 348 HC 5

 349 KK DB-3
 350 KM MODEL AS AN EXTENDED RELEASE DETENTION POND WITH 8' DEPTH TO 35' WIDE BROAD
 351 KM CRESTED WIER WITH CREST ELEVATION AT 85 AND 18" EXTENDED RELEASE OUTLET

```

352      KM PIPE. EXTENDED RELEASE STORAGE SIZED TO CONTAIN THE DELTA OF THE 2YEAR
353      KM RUNOFF BETWEEN THE UNDEVELOPED AND PROPOSED DEVELOPED CONDITIONS
354      KO      1      0      0      10      2      0      0      0.083
355      RS      1      ELEV      77
356      SV      0      .6      11.7      24.5      40.2      72.7      89.2      106.4      141.2
357      SV 177.3      214.6
358      SQ      0      3      14      15      17      20      21      126      572
359      SQ 1200      1990
360      SE      77      78      80      81      82      84      85      86      88
361      SE      90      92
362      KM *****
363      KM BEGIN POND 6 WATERSHED
364      KM *****

365      KK SC6-1
366      KM RUNOFF FROM SUB-BASIN SC6-1
367      BA      .108
368      LS      0      87.0
369      UD      .254

370      KK DBSC61
371      KM ROUTE FLOW THROUGH AN APPROXIMATE POND TO SIMULATE POTENTIAL DETENTION
372      KO      3
373      RS      1      ELEV      0
374      SV      0      2.2      5.0      8.0      11.2
375      SQ      0      23      64      90      117
376      SE      0      2      4      6      8

377      KK RT-23A
378      KM ROUTE FLOW FROM SUB-BASIN SC6-1 TO DESIGN POINT 54A
379      RK 3700      .020      .013      CIRC      4

380      KK SC6-2
381      KM RUNOFF FROM SUB-BASIN SC6-2
382      BA      .239
383      LS      0      81.0
384      UD      .213

```

HEC-1 INPUT

PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

385      KK SC6-3
386      KM RUNOFF FROM SUB-BASIN SC6-3
387      BA      .173
388      LS      0      83.0
389      UD      .194

390      KK DBSC63
391      KM ROUTE FLOW THROUGH AN APPROXIMATE POND TO SIMULATE POTENTIAL DETENTION
392      KO      3
393      RS      1      ELEV      0
394      SV      0      2.3      4.8      7.6      11.7
395      SQ      0      20      48      70      100
396      SE      0      2      4      6      8

397      KK RT-23B
398      KM ROUTE FLOW FROM SUB-BASIN SC6-3 TO DESIGN POINT 54A
399      RK 1500      .005      .013      CIRC      4.5

400      KK DP54A
401      KM DESIGN POINT 54A COMBINE RUNOFF FROM SUB-BASIN SC6-2 WITH ROUTED FLOW
402      KM RT-23A AND RT 23B
403      HC      3

404      KK RT-23C
405      KM ROUTE FLOW FROM DESIGN POINT 54A TO DESIGN POINT 54 AT POND 6
406      RK 1600      .024      .013      CIRC      7

407      KK SC6-4
408      KM RUNOFF FROM SUB-BASIN SC6-4
409      BA      .100
410      LS      0      94.0
411      UD      .155

412      KK DBSC64
413      KM ROUTE FLOW THROUGH AN APPROXIMATE POND TO SIMULATE POTENTIAL DETENTION
414      KO      3
415      RS      1      ELEV      0
416      SV      0      3.30      7.1      11.4      15.90
417      SQ      0      16      37      53      65
418      SE      0      2      4      6      8

419      KK RT-24
420      KM ROUTE FLOW FROM SUB-BASIN SC6-4 TO DESIGN POINT 54 AT POND 6
421      RK 1350      .024      .013      CIRC      3.5

422      KK SC6-5
423      KM RUNOFF FROM SUB-BASIN SC6-5
424      BA      .281
425      LS      0      83.0
426      UD      .189

```

HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

427      KK SC6-6
428      KM RUNOFF FROM SUB-BASIN SC6-6 (WOODMEN HEIGHTS MDDP BASINS 9 AND 10)
429      BA      .059
430      LS      0      92.0
431      UD      .153
432      KK DP54

```

```

433      KM  DESIGN POINT 54 COMBINE RUNOFF FROM SUB-BASIN SC6-5 WITH ROUTED FLOW
434      KM  RT-23C AND RT 24
435      KO    3
436      HC    4

437      KK  DB-6
438      KM  ROUTE FLOW THROUGH THE M & S CIVIL CONSULTANTS PROPOSED POND 6
439      KM  STAGE STORAGE AND DISCHARGE CURVES ARE FROM M&S POND PACK MODEL
440      KM  DATED MARCH 15, 2008
441      KO    1      0      0      10      2      0      0      0.083333
442      RS    1      ELEV    60
443      SV    0      0.05    0.42    1.58    3.95    8.37    11.5    15.32    24.51
444      SV  36.16    50.60    68.61
445      SQ    0      8      22      41      62      78      84      132      210
446      SQ  251      286      316
447      SE    60      62      64      66      68      70      71      72      74
448      SE    76      78      80
449      KM  *****
450      KM  BEGIN THE DIRECT DISCHARGE WATERSHED OF POND 2
451      KM  *****

452      KK  SC2-29
453      KM  RUNOFF FROM SUB-BASIN SC2-29
454      KO    1      0      0      10      2      0      0      0.083
455      BA    .033
456      LS    0      94.0
457      UD    .148
458      ZZ

```

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	{--->} DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
10	SB82	
	V	
	V	
22	RT-4	
	.	
25	.	SB74
	.	
30	DP74.....	
	V	
	V	
33	RT-3	
	.	
36	.	SB80
	.	V
	.	V
41	.	RT-2
	.	
44	.	.
	.	SB81
	.	V
49	.	RT-1
	.	
52	.	.
	.	SB73
	.	.
57	.	.
	.	SE75
	.	.
62	.	.
	.	.
	.	SB76
	.	.
67	DP75.....	
	V	
	V	
70	RT-5	
	.	
73	.	SB79
	.	V
	.	V
78	.	RT-7
	.	
81	.	.
	.	SB78
	.	.
86	.	.
	.	DP78.....
	.	V
	.	V
89	.	RT-6
	.	
92	.	.
	.	SB77
	.	.
97	.	.
	.	SB88
	.	.
102	DP77.....	
	V	
	V	
105	RT-8	
	.	
108	.	SC3-20
	.	.
113	DP71.....	
	V	
	V	
116	RT-9	
	.	
119	.	SB3-18
	.	.
124	DP70.....	
	V	
	V	
127	RT-11	
	.	
130	.	SC3-19
	.	V
	.	V
135	.	RT-10
	.	.
138	.	.
	.	SC3-17
	.	.
143	DP69.....	
	V	

146	V			
	DBSRMN			
	V			
	V			
155	RT-12			
	.			
158	.	SC3-15		
	.	V		
	.	V		
163	.	RT-10A		
	.			
166	DP67			
	V			
	V			
169	RT-13			
	.			
172	.	SC3-14		
	.			
177	.		SC3-16	
	.		V	
	.		V	
182	.		RT-13A	
	.			
185	DP68			
	V			
	V			
188	DBSRMS			
	V			
	V			
197	RT-17			
	.			
200	.	SC3-13		
	.	V		
	.	V		
205	.	RT-19		
	.			
208	.		SC3-12	
	.			
213	.	DP66		
	.	V		
	.	V		
216	.	RT-20		
	.			
219	.		SC3-11	
	.			
224	.	DP63A		
	.			
227	DP63			
	V			
	V			
230	RT-17A			
	.			
233	.	SC3-10		
	.			
238	.		SC3-9	
	.		V	
	.		V	
243	.		RT-14	
	.			
246	.			SC3-8
	.			
251	.		DP65	
	.		V	
	.		V	
254	.		RT-15	
	.			
257	.			SC3-7
	.			V
	.			V
262	.			RT-15A
	.			
265	.		DP64	
	.		V	
	.		V	
268	.		RT-16	
	.			
271	.			SC3-6
	.			
276	.		DP62	
	.		V	
	.		V	
279	.		DBSRW	
	.			
286	DP61			

289	V			
	V			
	RT-17B			
292	.	SB61		
	.	V		
	.	V		
297	.	RT-20		
	.	.		
300	.	.	SC3-5	
	.	.	.	
305	DP60			
	V			
	V			
308	RT-18			
	.			
311	.	SC3-4		
	.	V		
	.	V		
316	.	RT-21		
	.	.		
319	.	.	SC3-1	
	.	.	.	
324	.	.	.	SC3-3
	.	.	V	
	.	.	V	
329	.	.	DBSC33	
	.	.	V	
	.	.	V	
336	.	.	RT-21B	
	.	.	.	
339	.	.	.	SC3-2
	.	.	.	.
344	DP53			
	V			
	V			
349	DB-3			
	.			
365	.	SC6-1		
	.	V		
	.	V		
370	.	DBSC61		
	.	V		
	.	V		
377	.	RT-23A		
	.	.		
380	.	.	SC6-2	
	.	.	.	
385	.	.	.	SC6-3
	.	.	V	
	.	.	V	
390	.	.	DBSC63	
	.	.	V	
	.	.	V	
397	.	.	RT-23B	
	.	.	.	
	.	.	.	
400	.	DP54A		
	.	V		
	.	V		
404	.	RT-23C		
	.	.		
407	.	.	SC6-4	
	.	.	V	
	.	.	V	
412	.	.	DBSC64	
	.	.	V	
	.	.	V	
419	.	.	RT-24	
	.	.	.	
422	.	.	.	SC6-5
	.	.	.	.
427	.	.	.	SC6-6
	.	.	.	.
432	.	DP54		
	.	V		
	.	V		
437	.	DB-6		
	.	.		
452	.	.	SC2-29	

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```
*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*
* RUN DATE 06/25/2009 TIME 11:40:26
*
*****
```

```
*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

Sand Creek Basin Upstream of Woodmen Road
 Model prepared by WCI for Upper Sand Creek Basin
 Detention Evaluation Report
 FUTURE DEVELOPMENT CONDITION Including Sterling Ranch
 Utilizes Extended Detention of Excess 2 Year Volume in
 Sand Creek Regional Pond 3
 100 Year, 24 hr Type IIA Storm fn:SCBUW100.dat

```
9 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMN        4  MINUTES IN COMPUTATION INTERVAL
          IDATE      1  0  STARTING DATE
          ITIME      0000 STARTING TIME
          NQ         300 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     1  0  ENDING DATE
          NDTIME     1956 ENDING TIME
          ICENT      19  CENTURY MARK

          COMPUTATION INTERVAL 0.07 HOURS
          TOTAL TIME BASE     19.93 HOURS
```

```
ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW              CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-Feet
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT
```

*** **

```
*****
*
* DBSRMN
*
*****
```

```
150 KO      OUTPUT CONTROL VARIABLES
          IPRNT      3  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE
```

HYDROGRAPH ROUTING DATA

```
151 RS      STORAGE ROUTING
          NSTPS      1  NUMBER OF SUBREACHES
          ITYP      ELEV TYPE OF INITIAL CONDITION
          RSVRIC     94.00 INITIAL CONDITION
          X          0.00 WORKING R AND D COEFFICIENT

152 SV      STORAGE      0.0      4.4      12.3      23.5      36.6      50.3      64.7      79.8      95.6      112.0

153 SQ      DISCHARGE      0.      111.      314.      576.      887.      1239.      1536.      1782.      2000.      2195.

154 SE      ELEVATION      94.00      96.00      98.00      100.00      102.00      104.00      106.00      108.00      110.00      112.00
```

*** *** *** *** ***

HYDROGRAPH AT STATION DBSRMN

PEAK FLOW	TIME	6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)				
1507.	6.67	457.	170.	170.	170.
	(INCHES)	1.248	1.538	1.538	1.538
	(AC-FT)	227.	280.	280.	280.
PEAK STORAGE	TIME	6-HR	24-HR	72-HR	19.93-HR
(AC-FT)	(HR)				
63.	6.67	19.	7.	7.	7.
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			

(FEET)	(HR)	6-HR	24-HR	72-HR	19.93-HR
105.80	6.67	98.67	95.98	95.98	95.98

CUMULATIVE AREA = 3.41 SQ MI

*** **

188 KK *****
 * DBSRMS *
 * *

192 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

193 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP ELEV TYPE OF INITIAL CONDITION
 RSVRIC 4.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

194 SV	STORAGE	0.0	2.7	7.6	15.8	28.2	45.4	65.8	87.8
195 SQ	DISCHARGE	0.	110.	313.	576.	887.	1239.	1546.	1792.
196 SE	ELEVATION	4.00	6.00	8.00	10.00	12.00	14.00	16.00	18.00

*** **

HYDROGRAPH AT STATION DBSRMS

PEAK FLOW	TIME	6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)				
1561.	7.00	671.	246.	246.	246.
(INCHES)		1.464	1.781	1.781	1.781
(AC-FT)		333.	405.	405.	405.

PEAK STORAGE	TIME	6-HR	24-HR	72-HR	19.93-HR
(AC-FT)	(HR)				
67.	7.00	24.	8.	8.	8.

PEAK STAGE	TIME	6-HR	24-HR	72-HR	19.93-HR
(FEET)	(HR)				
16.12	7.00	10.21	6.63	6.63	6.63

CUMULATIVE AREA = 4.26 SQ MI

*** **

279 KK *****
 * DBSRW *
 * *

281 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

282 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP ELEV TYPE OF INITIAL CONDITION
 RSVRIC 4.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

283 SV	STORAGE	0.0	1.6	7.5	16.0	25.1	34.6	44.7	55.3	66.5
284 SQ	DISCHARGE	0.	19.	66.	115.	146.	171.	192.	212.	230.
285 SE	ELEVATION	68.00	70.00	72.00	74.00	76.00	78.00	80.00	82.00	84.00

*** **

HYDROGRAPH AT STATION DBSRW

PEAK FLOW	TIME	6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)				

219.	7.07	(CFS)	177.	74.	74.	74.
		(INCHES)	1.518	2.091	2.091	2.091
		(AC-FT)	88.	121.	121.	121.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE	
(AC-FT)	(HR)	6-HR	24-HR	72-HR
60.	7.07	40.	14.	14.
				19.93-HR

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE	
(FEET)	(HR)	6-HR	24-HR	72-HR
82.79	7.07	78.88	72.62	72.62
				19.93-HR

CUMULATIVE AREA = 1.09 SQ MI

*** **

 * *
 329 KK * DBSC33 *
 * *

331 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

332 RS	STORAGE ROUTING		1	NUMBER OF SUBREACHES	
	NSTPS		ELEV	TYPE OF INITIAL CONDITION	
	ITYP		0.00	INITIAL CONDITION	
	RSVRIC		0.00	WORKING R AND D COEFFICIENT	
	X				

333 SV	STORAGE	0.0	2.3	5.0	8.0	11.5	15.4
334 SQ	DISCHARGE	0.	37.	104.	195.	280.	340.
335 SE	ELEVATION	0.00	2.00	4.00	6.00	8.00	10.00

*** **

HYDROGRAPH AT STATION DBSC33

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW	
(CFS)	(HR)	6-HR	24-HR	72-HR
280.	6.27	(CFS)	58.	21.
		(INCHES)	2.297	2.687
		(AC-FT)	29.	34.
				21.
				2.687
				34.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE	
(AC-FT)	(HR)	6-HR	24-HR	72-HR
12.	6.27	3.	1.	1.
				1.
				19.93-HR

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE	
(FEET)	(HR)	6-HR	24-HR	72-HR
8.01	6.27	2.20	0.82	0.82
				19.93-HR

CUMULATIVE AREA = 0.24 SQ MI

*** **

 * *
 344 KK * DP53 *
 * *

347 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

348 HC HYDROGRAPH COMBINATION
 ICOMP 5 NUMBER OF HYDROGRAPHS TO COMBINE

*** **

HYDROGRAPH AT STATION DP53

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW	
(CFS)	(HR)	6-HR	24-HR	72-HR
		(CFS)		
				19.93-HR

2967. 6.47 1200. 448. 448. 448.
(INCHES) 1.576 1.953 1.953 1.953
(AC-FT) 595. 738. 738. 738.

CUMULATIVE AREA = 7.08 SQ MI

*** **

349 KK *****
* *
* DE-3 *
* *

354 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPILOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE
IPNCH 10 PUNCH COMPUTED HYDROGRAPH
IOUT 20 SAVE HYDROGRAPH ON THIS UNIT
ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
ISAV2 300 LAST ORDINATE PUNCHED OR SAVED
TIMINT 0.083 TIME INTERVAL IN HOURS

BEGIN POND 6 WATERSHED

HYDROGRAPH ROUTING DATA

355 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 77.00 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

356 SV STORAGE 0.0 0.6 11.7 24.5 40.2 72.7 89.2 106.4 141.2 0.0
177.3 214.6

358 SQ DISCHARGE 0. 3. 14. 15. 17. 20. 21. 126. 572. 0.
1200. 1990.

360 SE ELEVATION 77.00 78.00 80.00 81.00 82.00 84.00 85.00 86.00 88.00 0.00
90.00 92.00

HYDROGRAPH AT STATION DB-3

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*
1	0000	1	0.	0.0	77.0	*	1	0640	101	1055.	155.9	79.1	*	1	1320	201	323.	121.8	61.1				
1	0004	2	0.	0.0	77.0	*	1	0644	102	1111.	164.1	83.3	*	1	1324	202	320.	121.5	61.0				
1	0008	3	0.	0.0	77.0	*	1	0648	103	1161.	171.5	87.1	*	1	1328	203	317.	121.3	60.9				
1	0012	4	0.	0.0	77.0	*	1	0652	104	1218.	178.2	90.0	*	1	1332	204	313.	121.0	60.7				
1	0016	5	0.	0.0	77.0	*	1	0656	105	1341.	184.0	90.4	*	1	1336	205	310.	120.8	60.6				
1	0020	6	0.	0.0	77.0	*	1	0700	106	1445.	188.9	90.6	*	1	1340	206	307.	120.5	60.5				
1	0024	7	0.	0.0	77.0	*	1	0704	107	1532.	193.0	90.8	*	1	1344	207	303.	120.2	60.3				
1	0028	8	0.	0.0	77.0	*	1	0708	108	1605.	196.4	91.0	*	1	1348	208	299.	119.9	60.2				
1	0032	9	0.	0.0	77.0	*	1	0712	109	1664.	199.2	91.2	*	1	1352	209	296.	119.6	60.0				
1	0036	10	0.	0.0	77.0	*	1	0716	110	1712.	201.5	91.3	*	1	1356	210	292.	119.3	59.9				
1	0040	11	0.	0.0	77.0	*	1	0720	111	1750.	203.3	91.4	*	1	1400	211	288.	119.1	59.7				
1	0044	12	0.	0.0	77.0	*	1	0724	112	1779.	204.6	91.5	*	1	1404	212	284.	118.8	59.6				
1	0048	13	0.	0.0	77.0	*	1	0728	113	1799.	205.6	91.5	*	1	1408	213	281.	118.5	59.4				
1	0052	14	0.	0.0	77.0	*	1	0732	114	1812.	206.2	91.5	*	1	1412	214	277.	118.2	59.3				
1	0056	15	0.	0.0	77.0	*	1	0736	115	1818.	206.5	91.6	*	1	1416	215	273.	117.9	59.1				
1	0100	16	0.	0.0	77.0	*	1	0740	116	1819.	206.5	91.6	*	1	1420	216	269.	117.6	59.0				
1	0104	17	0.	0.0	77.0	*	1	0744	117	1814.	206.3	91.6	*	1	1424	217	266.	117.3	58.8				
1	0108	18	0.	0.0	77.0	*	1	0748	118	1805.	205.9	91.5	*	1	1428	218	262.	117.0	58.7				
1	0112	19	0.	0.0	77.0	*	1	0752	119	1791.	205.2	91.5	*	1	1432	219	258.	116.7	58.5				
1	0116	20	0.	0.0	77.0	*	1	0756	120	1774.	204.4	91.5	*	1	1436	220	255.	116.5	58.4				
1	0120	21	0.	0.0	77.0	*	1	0800	121	1752.	203.4	91.4	*	1	1440	221	252.	116.2	58.2				
1	0124	22	0.	0.0	77.0	*	1	0804	122	1727.	202.2	91.3	*	1	1444	222	248.	115.9	58.1				
1	0128	23	0.	0.0	77.0	*	1	0808	123	1697.	200.8	91.3	*	1	1448	223	245.	115.7	58.0				
1	0132	24	0.	0.0	77.0	*	1	0812	124	1665.	199.3	91.2	*	1	1452	224	242.	115.4	57.9				
1	0136	25	0.	0.0	77.0	*	1	0816	125	1630.	197.6	91.1	*	1	1456	225	239.	115.2	57.7				
1	0140	26	0.	0.0	77.0	*	1	0820	126	1592.	195.8	91.0	*	1	1500	226	236.	115.0	57.6				
1	0144	27	0.	0.0	77.0	*	1	0824	127	1553.	194.0	90.9	*	1	1504	227	233.	114.8	57.5				
1	0148	28	0.	0.0	77.0	*	1	0828	128	1513.	192.1	90.8	*	1	1508	228	230.	114.5	57.4				
1	0152	29	0.	0.0	77.0	*	1	0832	129	1471.	190.1	90.7	*	1	1512	229	228.	114.3	57.3				
1	0156	30	0.	0.0	77.0	*	1	0836	130	1429.	188.1	90.6	*	1	1516	230	225.	114.2	57.2				
1	0200	31	0.	0.0	77.0	*	1	0840	131	1386.	186.1	90.5	*	1	1520	231	223.	114.0	57.1				
1	0204	32	0.	0.0	77.0	*	1	0844	132	1343.	184.1	90.4	*	1	1524	232	221.	113.8	57.0				
1	0208	33	0.	0.0	77.0	*	1	0848	133	1301.	182.0	90.3	*	1	1528	233	219.	113.7	56.9				
1	0212	34	0.	0.0	77.0	*	1	0852	134	1258.	180.1	90.1	*	1	1532	234	217.	113.5	56.9				
1	0216	35	0.	0.0	77.0	*	1	0856	135	1216.	178.1	90.0	*	1	1536	235	215.	113.4	56.8				
1	0220	36	0.	0.0	77.0	*	1	0900	136	1192.	176.1	89.4	*	1	1540	236	214.	113.2	56.7				
1	0224	37	0.	0.0	77.0	*	1	0904	137	1177.	173.9	88.3	*	1	1544	237	212.	113.1	56.7				
1	0228	38	0.	0.0	77.0	*	1	0908	138	1162.	171.7	87.2	*	1	1548	238	211.	113.0	56.6				
1	0232	39	0.	0.0	77.0	*	1	0912	139	1147.	169.4	86.0	*	1	1552	239	210.	112.9	56.6				
1	0236	40	0.	0.0	77.0	*	1	0916	140	1130.	167.0	84.8	*	1	1556	240	209.	112.9	56.5				
1	0240	41	0.	0.0	77.0	*	1	0920	141	1114.	164.6	83.5	*	1	1600	241	208.	112.8	56.5				
1	0244	42	0.	0.0	77.0	*	1	0924	142	1097.	162.1	82.3	*	1	1604	242	207.	112.7	56.5				
1	0248	43	0.	0.0	77.0	*	1	0928	143	1080.	159.5	81.0	*	1	1608	243	206.	112.6	56.4				
1	0252	44	0.	0.0	77.0	*	1	0932	144	1063.	157.0	79.7	*	1	1612	244	205.	112.6	56.4				
1	0256	45	0.	0.0	77.0	*	1	0936	145	1045.	154.5	78.4	*	1	1616	245	205.	112.5	56.4				

1	0300	46	0.	0.0	77.0	*	1	0940	146	1028.	151.9	77.1	*	1	1620	246	204.	112.5	56.3
1	0304	47	0.	0.0	77.0	*	1	0944	147	1011.	149.3	75.8	*	1	1624	247	203.	112.4	56.3
1	0308	48	0.	0.0	77.0	*	1	0948	148	994.	146.8	74.5	*	1	1628	248	202.	112.4	56.3
1	0312	49	0.	0.0	77.0	*	1	0952	149	976.	144.2	73.2	*	1	1632	249	202.	112.3	56.2
1	0316	50	0.	0.0	77.0	*	1	0956	150	959.	141.7	71.9	*	1	1636	250	201.	112.3	56.2
1	0320	51	0.	0.0	77.0	*	1	1000	151	560.	140.3	70.7	*	1	1640	251	200.	112.2	56.2
1	0324	52	0.	0.0	77.0	*	1	1004	152	555.	139.9	70.5	*	1	1644	252	200.	112.2	56.2
1	0328	53	0.	0.0	77.0	*	1	1008	153	550.	139.5	70.2	*	1	1648	253	199.	112.1	56.1
1	0332	54	0.	0.0	77.0	*	1	1012	154	544.	139.0	70.0	*	1	1652	254	198.	112.1	56.1
1	0336	55	0.	0.0	77.0	*	1	1016	155	537.	138.5	69.8	*	1	1656	255	198.	112.0	56.1
1	0340	56	0.	0.0	77.0	*	1	1020	156	531.	138.0	69.5	*	1	1700	256	197.	112.0	56.1
1	0344	57	0.	0.0	77.0	*	1	1024	157	524.	137.4	69.2	*	1	1704	257	197.	111.9	56.0
1	0348	58	0.	0.0	77.0	*	1	1028	158	517.	136.9	68.9	*	1	1708	258	196.	111.9	56.0
1	0352	59	0.	0.0	77.0	*	1	1032	159	510.	136.3	68.6	*	1	1712	259	195.	111.8	56.0
1	0356	60	0.	0.0	77.0	*	1	1036	160	503.	135.8	68.4	*	1	1716	260	194.	111.7	55.9
1	0400	61	0.	0.0	77.0	*	1	1040	161	495.	135.2	68.1	*	1	1720	261	193.	111.6	55.9
1	0404	62	0.	0.0	77.0	*	1	1044	162	488.	134.7	67.8	*	1	1724	262	192.	111.5	55.8
1	0408	63	0.	0.0	77.0	*	1	1048	163	481.	134.1	67.5	*	1	1728	263	190.	111.4	55.8
1	0412	64	0.	0.0	77.0	*	1	1052	164	474.	133.6	67.2	*	1	1732	264	189.	111.3	55.7
1	0416	65	0.	0.0	77.0	*	1	1056	165	467.	133.0	66.9	*	1	1736	265	187.	111.2	55.7
1	0420	66	0.	0.0	77.0	*	1	1100	166	460.	132.4	66.6	*	1	1740	266	185.	111.0	55.6
1	0424	67	0.	0.0	77.0	*	1	1104	167	452.	131.9	66.3	*	1	1744	267	184.	110.9	55.5
1	0428	68	0.	0.0	77.0	*	1	1108	168	445.	131.3	66.1	*	1	1748	268	182.	110.8	55.4
1	0432	69	0.	0.0	77.0	*	1	1112	169	439.	130.8	65.8	*	1	1752	269	180.	110.6	55.4
1	0436	70	0.	0.0	77.0	*	1	1116	170	433.	130.3	65.5	*	1	1756	270	178.	110.5	55.3
1	0440	71	0.	0.0	77.0	*	1	1120	171	427.	129.9	65.3	*	1	1800	271	176.	110.3	55.2
1	0444	72	0.	0.0	77.0	*	1	1124	172	422.	129.5	65.1	*	1	1804	272	174.	110.1	55.1
1	0448	73	0.	0.0	77.0	*	1	1128	173	417.	129.1	64.9	*	1	1808	273	172.	110.0	55.0
1	0452	74	0.	0.0	77.0	*	1	1132	174	412.	128.7	64.7	*	1	1812	274	170.	109.8	55.0
1	0456	75	0.	0.0	77.0	*	1	1136	175	407.	128.4	64.5	*	1	1816	275	168.	109.7	54.9
1	0500	76	0.	0.0	77.0	*	1	1140	176	403.	128.0	64.3	*	1	1820	276	166.	109.5	54.8
1	0504	77	0.	0.0	77.0	*	1	1144	177	398.	127.7	64.2	*	1	1824	277	164.	109.4	54.7
1	0508	78	0.	0.0	77.0	*	1	1148	178	394.	127.3	64.0	*	1	1828	278	162.	109.2	54.6
1	0512	79	0.	0.0	77.0	*	1	1152	179	389.	126.9	63.8	*	1	1832	279	160.	109.1	54.6
1	0516	80	0.	0.0	77.0	*	1	1156	180	385.	126.6	63.6	*	1	1836	280	158.	108.9	54.5
1	0520	81	0.	0.0	77.0	*	1	1200	181	381.	126.3	63.4	*	1	1840	281	156.	108.8	54.4
1	0524	82	0.	0.0	77.1	*	1	1204	182	376.	125.9	63.3	*	1	1844	282	155.	108.6	54.3
1	0528	83	0.	0.0	77.1	*	1	1208	183	373.	125.6	63.1	*	1	1848	283	153.	108.5	54.3
1	0532	84	0.	0.1	77.1	*	1	1212	184	369.	125.4	63.0	*	1	1852	284	151.	108.4	54.2
1	0536	85	1.	0.2	77.4	*	1	1216	185	367.	125.2	62.9	*	1	1856	285	150.	108.3	54.2
1	0540	86	3.	0.8	78.0	*	1	1220	186	364.	125.0	62.8	*	1	1900	286	148.	108.1	54.1
1	0544	87	5.	2.2	78.3	*	1	1224	187	362.	124.8	62.7	*	1	1904	287	147.	108.0	54.0
1	0548	88	7.	4.9	78.8	*	1	1228	188	359.	124.6	62.6	*	1	1908	288	146.	107.9	54.0
1	0552	89	12.	9.2	79.5	*	1	1232	189	357.	124.4	62.5	*	1	1912	289	144.	107.8	53.9
1	0556	90	14.	15.4	80.3	*	1	1236	190	355.	124.3	62.4	*	1	1916	290	143.	107.8	53.9
1	0600	91	15.	24.2	81.0	*	1	1240	191	352.	124.1	62.3	*	1	1920	291	142.	107.7	53.9
1	0604	92	16.	35.5	81.7	*	1	1244	192	350.	123.9	62.2	*	1	1924	292	142.	107.6	53.8
1	0608	93	18.	48.3	82.5	*	1	1248	193	347.	123.6	62.1	*	1	1928	293	141.	107.6	53.8
1	0612	94	19.	61.7	83.3	*	1	1252	194	344.	123.4	62.0	*	1	1932	294	140.	107.5	53.8
1	0616	95	20.	75.8	84.2	*	1	1256	195	341.	123.2	61.9	*	1	1936	295	140.	107.5	53.8
1	0620	96	30.	90.7	85.2	*	1	1300	196	338.	123.0	61.7	*	1	1940	296	140.	107.5	53.7
1	0624	97	125.	106.3	86.1	*	1	1304	197	335.	122.7	61.6	*	1	1944	297	140.	107.5	53.7
1	0628	98	318.	121.4	86.9	*	1	1308	198	332.	122.5	61.5	*	1	1948	298	140.	107.5	53.7
1	0632	99	496.	135.3	88.1	*	1	1312	199	329.	122.2	61.4	*	1	1952	299	140.	107.5	53.7
1	0636	100	993.	146.7	74.5	*	1	1316	200	326.	122.0	61.2	*	1	1956	300	140.	107.5	53.7

PEAK FLOW	TIME		6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)	(CFS)				
1819.	7.67		988.	383.	383.	383.
		{INCHES}	1.298	1.668	1.668	1.668
		{AC-FT}	490.	630.	630.	630.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	19.93-HR
{AC-FT}	(HR)					
207.	7.67		160.	93.	93.	93.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	19.93-HR
{FEET}	(HR)					
91.57	7.67		82.47	69.79	69.79	69.79

CUMULATIVE AREA = 7.08 SQ MI

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370 KK	*****					
	*					
	*	DBSC61	*			
	*		*			

372 KO	OUTPUT CONTROL VARIABLES					
	IPRNT	3	PRINT CONTROL			
	IPLST	0	PLOT CONTROL			
	OSCAL	0.	HYDROGRAPH PLOT SCALE			
	HYDROGRAPH ROUTING DATA					
373 RS	STORAGE ROUTING					
	NSTPS	1	NUMBER OF SUBREACHES			
	ITYP	ELEV	TYPE OF INITIAL CONDITION			
	RSVRIC	0.00	INITIAL CONDITION			
	X	0.00	WORKING R AND D COEFFICIENT			
374 SV	STORAGE	0.0	2.2	5.0	8.0	11.2

375 SQ DISCHARGE 0. 23. 64. 90. 117.
376 SE ELEVATION 0.00 2.00 4.00 6.00 8.00

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HYDROGRAPH AT STATION DBSC61

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	19.93-HR
(CFS)	(HR)						
84.	6.47	(CFS)	29.	10.	10.	10.	10.
		(INCHES)	2.511	2.946	2.946	2.946	2.946
		(AC-FT)	14.	17.	17.	17.	17.

PEAK STORAGE	TIME		6-HR	MAXIMUM AVERAGE STORAGE	24-HR	72-HR	19.93-HR
(AC-FT)	(HR)						
7.	6.47		3.	1.	1.	1.	1.

PEAK STAGE	TIME		6-HR	MAXIMUM AVERAGE STAGE	24-HR	72-HR	19.93-HR
(FEET)	(HR)						
5.54	6.47		2.09	0.76	0.76	0.76	0.76

CUMULATIVE AREA = 0.11 SQ MI

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390 KK * DBSC63 *
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392 KO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

393 RS	STORAGE ROUTING		1	NUMBER OF SUBREACHES			
	NSTPS		ELEV	TYPE OF INITIAL CONDITION			
	ITYP		0.00	INITIAL CONDITION			
	RSVRIC		0.00	WORKING R AND D COEFFICIENT			
	X						
394 SV	STORAGE	0.0	2.3	4.8	7.6	11.7	
395 SQ	DISCHARGE	0.	20.	48.	70.	100.	
396 SE	ELEVATION	0.00	2.00	4.00	6.00	8.00	

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HYDROGRAPH AT STATION DBSC63

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	19.93-HR
(CFS)	(HR)						
96.	6.40	(CFS)	40.	14.	14.	14.	14.
		(INCHES)	2.145	2.574	2.574	2.574	2.574
		(AC-FT)	20.	24.	24.	24.	24.

PEAK STORAGE	TIME		6-HR	MAXIMUM AVERAGE STORAGE	24-HR	72-HR	19.93-HR
(AC-FT)	(HR)						
11.	6.40		4.	2.	2.	2.	2.

PEAK STAGE	TIME		6-HR	MAXIMUM AVERAGE STAGE	24-HR	72-HR	19.93-HR
(FEET)	(HR)						
7.72	6.40		3.46	1.28	1.28	1.28	1.28

CUMULATIVE AREA = 0.17 SQ MI

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412 KK * DBSC64 *
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414 KO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

415 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP ELEV TYPE OF INITIAL CONDITION
 RSVRIC 0.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

416 SV STORAGE 0.0 3.3 7.1 11.4 15.9

417 SQ DISCHARGE 0. 16. 37. 53. 65.

418 SE ELEVATION 0.00 2.00 4.00 6.00 8.00

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HYDROGRAPH AT STATION DBSC64

PEAK FLOW	TIME		6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)	(CFS)				
53.	6.40		30.	12.	12.	12.
		(INCHES)	2.829	3.588	3.588	3.588
		(AC-FT)	15.	19.	19.	19.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	19.93-HR
(AC-FT)	(HR)					
11.	6.40		6.	2.	2.	2.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	19.93-HR
(FEET)	(HR)					
6.00	6.40		3.46	1.35	1.35	1.35

CUMULATIVE AREA = 0.10 SQ MI

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 * DP54 *
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435 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

436 HC HYDROGRAPH COMBINATION
 ICOMP 4 NUMBER OF HYDROGRAPHS TO COMBINE

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HYDROGRAPH AT STATION DP54

PEAK FLOW	TIME		6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)	(CFS)				
1382.	6.07		240.	86.	86.	86.
		(INCHES)	2.324	2.760	2.760	2.760
		(AC-FT)	119.	141.	141.	141.

CUMULATIVE AREA = 0.96 SQ MI

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 * DB-6 *
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441 KO OUTPUT CONTROL VARIABLES
 IPRNT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE
 IPNCH 10 PUNCH COMPUTED HYDROGRAPH
 IOUT 20 SAVE HYDROGRAPH ON THIS UNIT
 ISAV1 1 FIRST ORDINATE PUNCHED OR SAVED
 ISAV2 300 LAST ORDINATE PUNCHED OR SAVED
 TIMINT 0.083 TIME INTERVAL IN HOURS

 BEGIN THE DIRECT DISCHARGE WATERSHED OF POND 2

HYDROGRAPH ROUTING DATA

442 RS	STORAGE ROUTING										
	NSTPS	1	NUMBER OF SUBREACHES								
	ITYP	ELEV	TYPE OF INITIAL CONDITION								
	RSVRIC	60.00	INITIAL CONDITION								
	X	0.00	WORKING R AND D COEFFICIENT								
443 SV	STORAGE	0.0 36.2	0.1 50.6	0.4 68.6	1.6	4.0	8.4	11.5	15.3	24.5	0.0
445 SQ	DISCHARGE	0. 251.	8. 286.	22. 316.	41.	62.	78.	84.	132.	210.	0.
447 SE	ELEVATION	60.00 76.00	62.00 78.00	64.00 80.00	66.00	68.00	70.00	71.00	72.00	74.00	0.00

HYDROGRAPH AT STATION DB-6

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE
1	0000	1	0.	0.	0.0	60.0	*	1	0640	101	283.	49.3	77.8	*	1	1320	201	76.	7.7	16.3		
1	0004	2	0.	0.	0.0	60.0	*	1	0644	102	284.	49.8	77.9	*	1	1324	202	75.	7.5	15.9		
1	0008	3	0.	0.	0.0	60.0	*	1	0648	103	285.	50.1	77.9	*	1	1328	203	74.	7.3	15.4		
1	0012	4	0.	0.	0.0	60.0	*	1	0652	104	285.	50.3	78.0	*	1	1332	204	73.	7.1	15.0		
1	0016	5	0.	0.	0.0	60.0	*	1	0656	105	286.	50.5	78.0	*	1	1336	205	72.	6.8	14.5		
1	0020	6	0.	0.	0.0	60.0	*	1	0700	106	286.	50.5	78.0	*	1	1340	206	72.	6.6	14.1		
1	0024	7	0.	0.	0.0	60.0	*	1	0704	107	286.	50.5	78.0	*	1	1344	207	71.	6.4	13.6		
1	0028	8	0.	0.	0.0	60.0	*	1	0708	108	286.	50.5	78.0	*	1	1348	208	70.	6.2	13.2		
1	0032	9	0.	0.	0.0	60.0	*	1	0712	109	286.	50.4	78.0	*	1	1352	209	69.	6.0	12.8		
1	0036	10	0.	0.	0.0	60.0	*	1	0716	110	285.	50.2	77.9	*	1	1356	210	69.	5.8	12.4		
1	0040	11	0.	0.	0.0	60.0	*	1	0720	111	285.	50.0	77.9	*	1	1400	211	68.	5.6	11.9		
1	0044	12	0.	0.	0.0	60.0	*	1	0724	112	284.	49.7	77.9	*	1	1404	212	67.	5.4	11.5		
1	0048	13	0.	0.	0.0	60.0	*	1	0728	113	283.	49.4	77.8	*	1	1408	213	67.	5.2	11.1		
1	0052	14	0.	0.	0.0	60.0	*	1	0732	114	282.	49.0	77.8	*	1	1412	214	66.	5.0	10.7		
1	0056	15	0.	0.	0.0	60.0	*	1	0736	115	281.	48.6	77.7	*	1	1416	215	65.	4.8	10.4		
1	0100	16	0.	0.	0.0	60.0	*	1	0740	116	280.	48.1	77.7	*	1	1420	216	65.	4.7	10.0		
1	0104	17	0.	0.	0.0	60.0	*	1	0744	117	279.	47.7	77.6	*	1	1424	217	64.	4.5	9.6		
1	0108	18	0.	0.	0.0	60.0	*	1	0748	118	278.	47.3	77.5	*	1	1428	218	63.	4.3	9.3		
1	0112	19	0.	0.	0.0	60.0	*	1	0752	119	277.	46.8	77.5	*	1	1432	219	63.	4.2	9.0		
1	0116	20	0.	0.	0.0	60.0	*	1	0756	120	276.	46.3	77.4	*	1	1436	220	62.	4.0	8.6		
1	0120	21	0.	0.	0.0	60.0	*	1	0800	121	275.	45.9	77.3	*	1	1440	221	61.	3.8	8.3		
1	0124	22	0.	0.	0.0	60.0	*	1	0804	122	273.	45.4	77.3	*	1	1444	222	60.	3.7	8.0		
1	0128	23	0.	0.	0.0	60.0	*	1	0808	123	272.	44.9	77.2	*	1	1448	223	58.	3.6	7.7		
1	0132	24	0.	0.	0.0	60.0	*	1	0812	124	271.	44.3	77.1	*	1	1452	224	57.	3.4	7.4		
1	0136	25	0.	0.	0.0	60.0	*	1	0816	125	269.	43.7	77.0	*	1	1456	225	56.	3.3	7.1		
1	0140	26	0.	0.	0.0	60.0	*	1	0820	126	268.	43.0	76.9	*	1	1500	226	55.	3.2	6.8		
1	0144	27	0.	0.	0.0	60.0	*	1	0824	127	266.	42.3	76.9	*	1	1504	227	54.	3.0	6.6		
1	0148	28	0.	0.	0.0	60.0	*	1	0828	128	264.	41.6	76.8	*	1	1508	228	53.	2.9	6.3		
1	0152	29	0.	0.	0.0	60.0	*	1	0832	129	262.	40.8	76.6	*	1	1512	229	52.	2.8	6.1		
1	0156	30	0.	0.	0.0	60.0	*	1	0836	130	261.	40.1	76.5	*	1	1516	230	51.	2.7	5.9		
1	0200	31	0.	0.	0.0	60.0	*	1	0840	131	259.	39.3	76.4	*	1	1520	231	50.	2.6	5.7		
1	0204	32	0.	0.	0.0	60.0	*	1	0844	132	257.	38.5	76.3	*	1	1524	232	49.	2.5	5.5		
1	0208	33	0.	0.	0.0	60.0	*	1	0848	133	255.	37.7	76.2	*	1	1528	233	48.	2.4	5.3		
1	0212	34	0.	0.	0.0	60.0	*	1	0852	134	253.	36.9	76.1	*	1	1532	234	48.	2.3	5.1		
1	0216	35	0.	0.	0.0	60.0	*	1	0856	135	251.	36.1	76.0	*	1	1536	235	47.	2.3	4.9		
1	0220	36	0.	0.	0.0	60.0	*	1	0900	136	245.	35.3	74.3	*	1	1540	236	46.	2.2	4.7		
1	0224	37	0.	0.	0.0	60.0	*	1	0904	137	240.	34.6	72.6	*	1	1544	237	46.	2.1	4.6		
1	0228	38	0.	0.	0.0	60.0	*	1	0908	138	235.	33.8	71.0	*	1	1548	238	45.	2.0	4.5		
1	0232	39	0.	0.	0.0	60.0	*	1	0912	139	229.	33.1	69.5	*	1	1552	239	45.	2.0	4.3		
1	0236	40	0.	0.	0.0	60.0	*	1	0916	140	224.	32.3	67.9	*	1	1556	240	44.	1.9	4.2		
1	0240	41	0.	0.	0.0	60.0	*	1	0920	141	219.	31.6	66.4	*	1	1600	241	44.	1.9	4.1		
1	0244	42	0.	0.	0.0	60.0	*	1	0924	142	215.	30.9	65.0	*	1	1604	242	43.	1.8	4.0		
1	0248	43	0.	0.	0.0	60.0	*	1	0928	143	210.	30.2	63.5	*	1	1608	243	43.	1.8	3.9		
1	0252	44	0.	0.	0.0	60.0	*	1	0932	144	205.	29.6	62.1	*	1	1612	244	42.	1.7	3.8		
1	0256	45	0.	0.	0.0	60.0	*	1	0936	145	201.	28.9	60.8	*	1	1616	245	42.	1.7	3.7		
1	0300	46	0.	0.	0.0	60.0	*	1	0940	146	196.	28.3	59.4	*	1	1620	246	41.	1.6	3.6		
1	0304	47	0.	0.	0.0	60.0	*	1	0944	147	192.	27.7	58.1	*	1	1624	247	41.	1.6	3.4		
1	0308	48	0.	0.	0.0	60.0	*	1	0948	148	188.	27.1	56.9	*	1	1628	248	40.	1.5	3.3		
1	0312	49	0.	0.	0.0	60.0	*	1	0952	149	184.	26.5	55.7	*	1	1632	249	39.	1.4	3.2		
1	0316	50	0.	0.	0.0	60.0	*	1	0956	150	180.	25.9	54.5	*	1	1636	250	38.	1.4	3.1		
1	0320	51	0.	0.	0.0	60.0	*	1	1000	151	176.	25.4	53.3	*	1	1640	251	37.	1.3	3.0		
1	0324	52	0.	0.	0.0	60.0	*	1	1004	152	172.	24.8	52.2	*	1	1644	252	36.	1.3	2.9		
1	0328	53	0.	0.	0.0	60.0	*	1	1008	153	207.	24.2	51.0	*	1	1648	253	36.	1.3	2.8		
1	0332	54	0.	0.	0.0	60.0	*	1	1012	154	201.	23.4	49.5	*	1	1652	254	35.	1.2	2.7		
1	0336	55	0.	0.	0.0	60.0	*	1	1016	155	195.	22.7	47.9	*	1	1656	255	34.	1.2	2.6		
1	0340	56	0.	0.	0.0	60.0	*	1	1020	156	189.	22.0	46.4	*	1	1700	256	34.	1.1	2.6		
1	0344	57	0.	0.	0.0	60.0	*	1	1024	157	183.	21.3	45.0	*	1	1704	257	33.	1.1	2.5		
1	0348	58	0.	0.	0.0	60.0	*	1	1028	158	177.	20.6	43.5	*	1	1708	258	33.	1.1	2.4		
1	0352	59	0.	0.	0.0	60.0	*	1	1032	159	171.	20.0	42.1	*	1	1712	259	32.	1.0	2.3		
1	0356	60	0.	0.	0.0	60.0	*	1	1036	160	166.	19.3	40.8	*	1	1716	260	31.	1.0	2.2		
1	0400	61	0.	0.	0.0	60.0	*	1	1040	161	161.	18.7	39.5	*	1	1720	261	31.	0.9	2.1		
1	0404	62	0.	0.	0.0	60.0	*	1	1044	162	156.	18.1	38.3	*	1	1724	262	30.	0.9	2.0		
1	0408	63	0.	0.	0.0	60.0	*	1	1048	163	151.	17.6	37.1	*	1	1728	263	29.	0.9	1.9		
1	0412	64	0.	0.	0.0	60.0	*	1	1052	164	146.	17.0	35.9	*	1	1732	264	28.	0.8	1.8		
1	0416	65	0.	0.	0.0	60.0	*	1	1056	165	142.	16.5	34.8	*	1	1736	265	28.	0.8	1.7		
1	0420	66	0.	0.	0.0	60.0	*	1	1100	166	138.	16.0	33.7	*	1	1740	266	27.	0.7	1.6		
1	0424	67	0.	0.	0.0	60.0	*	1	1104	167	133.	15.5	32.7	*	1	1744	267	26.	0.7	1.5		
1	0428	68	0.	0.	0.0	60.0	*	1	1108	16												

[illegible]

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

456 LS SCS LOSS RATE
 STRTL 0.13 INITIAL ABSTRACTION
 CRVNER 94.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

457 UD SCS DIMENSIONLESS UNITGRAPH
 TLRG 0.15 LAG

WARNING *** TIME INTERVAL IS GREATER THAN .29*LAG

UNIT HYDROGRAPH									
13 END-OF-PERIOD ORDINATES									
24.	75.	87.	62.	32.	18.	10.	5.	3.	2.
1.	1.	0.							

HYDROGRAPH AT STATION SC2-29

DA	MON	HRMN	ORD	RAIN	LOSS	EXCESS	COMP Q	*	DA	MON	HRMN	ORD	RAIN	LOSS	EXCESS	COMP Q
1		0000	1	0.00	0.00	0.00	0.	*	1		1000	151	0.01	0.00	0.01	2.
1		0004	2	0.00	0.00	0.00	0.	*	1		1004	152	0.00	0.00	0.00	2.
1		0008	3	0.00	0.00	0.00	0.	*	1		1008	153	0.00	0.00	0.00	2.
1		0012	4	0.00	0.00	0.00	0.	*	1		1012	154	0.00	0.00	0.00	1.
1		0016	5	0.00	0.00	0.00	0.	*	1		1016	155	0.00	0.00	0.00	1.
1		0020	6	0.00	0.00	0.00	0.	*	1		1020	156	0.00	0.00	0.00	1.
1		0024	7	0.00	0.00	0.00	0.	*	1		1024	157	0.00	0.00	0.00	1.
1		0028	8	0.00	0.00	0.00	0.	*	1		1028	158	0.00	0.00	0.00	1.
1		0032	9	0.00	0.00	0.00	0.	*	1		1032	159	0.00	0.00	0.00	1.
1		0036	10	0.00	0.00	0.00	0.	*	1		1036	160	0.00	0.00	0.00	1.
1		0040	11	0.00	0.00	0.00	0.	*	1		1040	161	0.00	0.00	0.00	1.
1		0044	12	0.00	0.00	0.00	0.	*	1		1044	162	0.00	0.00	0.00	1.
1		0048	13	0.00	0.00	0.00	0.	*	1		1048	163	0.00	0.00	0.00	1.
1		0052	14	0.00	0.00	0.00	0.	*	1		1052	164	0.00	0.00	0.00	1.
1		0056	15	0.00	0.00	0.00	0.	*	1		1056	165	0.00	0.00	0.00	1.
1		0100	16	0.00	0.00	0.00	0.	*	1		1100	166	0.00	0.00	0.00	1.
1		0104	17	0.00	0.00	0.00	0.	*	1		1104	167	0.01	0.00	0.01	1.
1		0108	18	0.00	0.00	0.00	0.	*	1		1108	168	0.01	0.00	0.01	2.
1		0112	19	0.00	0.00	0.00	0.	*	1		1112	169	0.01	0.00	0.01	2.
1		0116	20	0.00	0.00	0.00	0.	*	1		1116	170	0.01	0.00	0.01	2.
1		0120	21	0.00	0.00	0.00	0.	*	1		1120	171	0.01	0.00	0.01	3.
1		0124	22	0.00	0.00	0.00	0.	*	1		1124	172	0.01	0.00	0.01	3.
1		0128	23	0.00	0.00	0.00	0.	*	1		1128	173	0.01	0.00	0.01	3.
1		0132	24	0.00	0.00	0.00	0.	*	1		1132	174	0.01	0.00	0.01	3.
1		0136	25	0.00	0.00	0.00	0.	*	1		1136	175	0.00	0.00	0.00	2.
1		0140	26	0.00	0.00	0.00	0.	*	1		1140	176	0.00	0.00	0.00	2.
1		0144	27	0.00	0.00	0.00	0.	*	1		1144	177	0.00	0.00	0.00	2.
1		0148	28	0.00	0.00	0.00	0.	*	1		1148	178	0.00	0.00	0.00	1.
1		0152	29	0.00	0.00	0.00	0.	*	1		1152	179	0.00	0.00	0.00	1.
1		0156	30	0.00	0.00	0.00	0.	*	1		1156	180	0.00	0.00	0.00	1.
1		0200	31	0.00	0.00	0.00	0.	*	1		1200	181	0.00	0.00	0.00	1.
1		0204	32	0.00	0.00	0.00	0.	*	1		1204	182	0.01	0.00	0.01	1.
1		0208	33	0.00	0.00	0.00	0.	*	1		1208	183	0.01	0.00	0.01	1.
1		0212	34	0.00	0.00	0.00	0.	*	1		1212	184	0.01	0.00	0.01	1.
1		0216	35	0.00	0.00	0.00	0.	*	1		1216	185	0.01	0.00	0.01	2.
1		0220	36	0.00	0.00	0.00	0.	*	1		1220	186	0.01	0.00	0.01	2.
1		0224	37	0.00	0.00	0.00	0.	*	1		1224	187	0.01	0.00	0.01	2.
1		0228	38	0.00	0.00	0.00	0.	*	1		1228	188	0.01	0.00	0.01	2.
1		0232	39	0.00	0.00	0.00	0.	*	1		1232	189	0.00	0.00	0.00	2.
1		0236	40	0.00	0.00	0.00	0.	*	1		1236	190	0.00	0.00	0.00	2.
1		0240	41	0.00	0.00	0.00	0.	*	1		1240	191	0.00	0.00	0.00	1.
1		0244	42	0.00	0.00	0.00	0.	*	1		1244	192	0.00	0.00	0.00	1.
1		0248	43	0.00	0.00	0.00	0.	*	1		1248	193	0.00	0.00	0.00	1.
1		0252	44	0.00	0.00	0.00	0.	*	1		1252	194	0.00	0.00	0.00	1.
1		0256	45	0.00	0.00	0.00	0.	*	1		1256	195	0.00	0.00	0.00	1.
1		0300	46	0.00	0.00	0.00	0.	*	1		1300	196	0.00	0.00	0.00	1.
1		0304	47	0.00	0.00	0.00	0.	*	1		1304	197	0.00	0.00	0.00	1.
1		0308	48	0.00	0.00	0.00	0.	*	1		1308	198	0.00	0.00	0.00	1.
1		0312	49	0.00	0.00	0.00	0.	*	1		1312	199	0.00	0.00	0.00	1.
1		0316	50	0.00	0.00	0.00	0.	*	1		1316	200	0.00	0.00	0.00	1.
1		0320	51	0.00	0.00	0.00	0.	*	1		1320	201	0.00	0.00	0.00	1.
1		0324	52	0.00	0.00	0.00	0.	*	1		1324	202	0.00	0.00	0.00	1.
1		0328	53	0.00	0.00	0.00	0.	*	1		1328	203	0.00	0.00	0.00	1.
1		0332	54	0.00	0.00	0.00	0.	*	1		1332	204	0.00	0.00	0.00	1.
1		0336	55	0.00	0.00	0.00	0.	*	1		1336	205	0.00	0.00	0.00	1.
1		0340	56	0.00	0.00	0.00	0.	*	1		1340	206	0.00	0.00	0.00	1.
1		0344	57	0.00	0.00	0.00	0.	*	1		1344	207	0.00	0.00	0.00	1.
1		0348	58	0.00	0.00	0.00	0.	*	1		1348	208	0.00	0.00	0.00	1.
1		0352	59	0.00	0.00	0.00	0.	*	1		1352	209	0.00	0.00	0.00	1.
1		0356	60	0.00	0.00	0.00	0.	*	1		1356	210	0.00	0.00	0.00	1.
1		0400	61	0.00	0.00	0.00	0.	*	1		1400	211	0.00	0.00	0.00	1.
1		0404	62	0.01	0.01	0.00	0.	*	1		1404	212	0.00	0.00	0.00	1.
1		0408	63	0.01	0.01	0.00	0.	*	1		1408	213	0.00	0.00	0.00	1.
1		0412	64	0.01	0.01	0.00	0.	*	1		1412	214	0.00	0.00	0.00	1.
1		0416	65	0.01	0.01	0.00	0.	*	1		1416	215	0.00	0.00	0.00	1.
1		0420	66	0.01	0.01	0.00	0.	*	1		1420	216	0.00	0.00	0.00	1.
1		0424	67	0.01	0.01	0.00	0.	*	1		1424	217	0.00	0.00	0.00	1.
1		0428	68	0.01	0.01	0.00	1.	*	1		1428	218	0.00	0.00	0.00	1.
1		0432	69	0.01	0.01	0.00	1.	*	1		1432	219	0.00	0.00	0.00	1.
1		0436	70	0.01	0.00	0.00	1.	*	1		1436	220	0.00	0.00	0.00	1.
1		0440	71	0.01	0.00	0.00	1.	*	1		1440	221	0.00	0.00	0.00	1.
1		0444	72	0.01	0.00	0.00	1.	*	1		1444	222	0.00	0.00	0.00	1.
1		0448	73	0.01	0.00	0.00	1.	*	1		1448	223	0.00	0.00	0.00	1.
1		0452	74	0.01	0.00	0.00	1.	*	1		1452	224	0.00	0.00	0.00	1.

1	0456	75	0.01	0.00	0.00	1.	*	1	1456	225	0.00	0.00	0.00	1.
1	0500	76	0.01	0.00	0.00	1.	*	1	1500	226	0.00	0.00	0.00	1.
1	0504	77	0.02	0.02	0.01	1.	*	1	1504	227	0.00	0.00	0.00	1.
1	0508	78	0.02	0.01	0.01	1.	*	1	1508	228	0.00	0.00	0.00	1.
1	0512	79	0.02	0.01	0.01	2.	*	1	1512	229	0.00	0.00	0.00	1.
1	0516	80	0.02	0.01	0.01	3.	*	1	1516	230	0.00	0.00	0.00	1.
1	0520	81	0.02	0.01	0.01	3.	*	1	1520	231	0.00	0.00	0.00	1.
1	0524	82	0.02	0.01	0.01	3.	*	1	1524	232	0.00	0.00	0.00	1.
1	0528	83	0.02	0.01	0.01	4.	*	1	1528	233	0.00	0.00	0.00	1.
1	0532	84	0.19	0.07	0.12	6.	*	1	1532	234	0.00	0.00	0.00	1.
1	0536	85	0.36	0.09	0.28	18.	*	1	1536	235	0.00	0.00	0.00	1.
1	0540	86	0.36	0.05	0.31	40.	*	1	1540	236	0.00	0.00	0.00	1.
1	0544	87	0.36	0.04	0.33	63.	*	1	1544	237	0.00	0.00	0.00	1.
1	0548	88	0.36	0.03	0.34	81.	*	1	1548	238	0.00	0.00	0.00	1.
1	0552	89	0.36	0.02	0.34	92.	*	1	1552	239	0.00	0.00	0.00	1.
1	0556	90	0.36	0.02	0.35	100.	*	1	1556	240	0.00	0.00	0.00	1.
1	0600	91	0.36	0.01	0.35	104.	*	1	1600	241	0.00	0.00	0.00	1.
1	0604	92	0.03	0.00	0.03	100.	*	1	1604	242	0.00	0.00	0.00	1.
1	0608	93	0.03	0.00	0.03	78.	*	1	1608	243	0.00	0.00	0.00	1.
1	0612	94	0.03	0.00	0.03	51.	*	1	1612	244	0.00	0.00	0.00	1.
1	0616	95	0.03	0.00	0.03	32.	*	1	1616	245	0.00	0.00	0.00	1.
1	0620	96	0.03	0.00	0.03	22.	*	1	1620	246	0.00	0.00	0.00	1.
1	0624	97	0.03	0.00	0.03	16.	*	1	1624	247	0.00	0.00	0.00	1.
1	0628	98	0.03	0.00	0.03	13.	*	1	1628	248	0.00	0.00	0.00	1.
1	0632	99	0.02	0.00	0.02	11.	*	1	1632	249	0.00	0.00	0.00	1.
1	0636	100	0.02	0.00	0.02	10.	*	1	1636	250	0.00	0.00	0.00	1.
1	0640	101	0.02	0.00	0.02	8.	*	1	1640	251	0.00	0.00	0.00	1.
1	0644	102	0.02	0.00	0.02	7.	*	1	1644	252	0.00	0.00	0.00	1.
1	0648	103	0.02	0.00	0.02	6.	*	1	1648	253	0.00	0.00	0.00	1.
1	0652	104	0.02	0.00	0.02	6.	*	1	1652	254	0.00	0.00	0.00	1.
1	0656	105	0.02	0.00	0.02	6.	*	1	1656	255	0.00	0.00	0.00	1.
1	0700	106	0.02	0.00	0.02	6.	*	1	1700	256	0.00	0.00	0.00	1.
1	0704	107	0.01	0.00	0.01	6.	*	1	1704	257	0.00	0.00	0.00	1.
1	0708	108	0.01	0.00	0.01	5.	*	1	1708	258	0.00	0.00	0.00	1.
1	0712	109	0.01	0.00	0.01	4.	*	1	1712	259	0.00	0.00	0.00	1.
1	0716	110	0.01	0.00	0.01	4.	*	1	1716	260	0.00	0.00	0.00	1.
1	0720	111	0.01	0.00	0.01	4.	*	1	1720	261	0.00	0.00	0.00	1.
1	0724	112	0.01	0.00	0.01	4.	*	1	1724	262	0.00	0.00	0.00	1.
1	0728	113	0.01	0.00	0.01	3.	*	1	1728	263	0.00	0.00	0.00	0.
1	0732	114	0.01	0.00	0.01	3.	*	1	1732	264	0.00	0.00	0.00	0.
1	0736	115	0.01	0.00	0.01	4.	*	1	1736	265	0.00	0.00	0.00	0.
1	0740	116	0.01	0.00	0.01	4.	*	1	1740	266	0.00	0.00	0.00	0.
1	0744	117	0.01	0.00	0.01	4.	*	1	1744	267	0.00	0.00	0.00	0.
1	0748	118	0.01	0.00	0.01	4.	*	1	1748	268	0.00	0.00	0.00	0.
1	0752	119	0.01	0.00	0.01	4.	*	1	1752	269	0.00	0.00	0.00	0.
1	0756	120	0.01	0.00	0.01	4.	*	1	1756	270	0.00	0.00	0.00	0.
1	0800	121	0.01	0.00	0.01	4.	*	1	1800	271	0.00	0.00	0.00	0.
1	0804	122	0.01	0.00	0.01	4.	*	1	1804	272	0.00	0.00	0.00	1.
1	0808	123	0.01	0.00	0.01	3.	*	1	1808	273	0.00	0.00	0.00	1.
1	0812	124	0.01	0.00	0.01	3.	*	1	1812	274	0.00	0.00	0.00	1.
1	0816	125	0.01	0.00	0.01	2.	*	1	1816	275	0.00	0.00	0.00	1.
1	0820	126	0.01	0.00	0.01	2.	*	1	1820	276	0.00	0.00	0.00	1.
1	0824	127	0.01	0.00	0.01	2.	*	1	1824	277	0.00	0.00	0.00	1.
1	0828	128	0.01	0.00	0.01	2.	*	1	1828	278	0.00	0.00	0.00	1.
1	0832	129	0.01	0.00	0.01	2.	*	1	1832	279	0.00	0.00	0.00	1.
1	0836	130	0.01	0.00	0.01	2.	*	1	1836	280	0.00	0.00	0.00	1.
1	0840	131	0.01	0.00	0.01	2.	*	1	1840	281	0.00	0.00	0.00	1.
1	0844	132	0.01	0.00	0.01	2.	*	1	1844	282	0.00	0.00	0.00	1.
1	0848	133	0.01	0.00	0.01	2.	*	1	1848	283	0.00	0.00	0.00	1.
1	0852	134	0.01	0.00	0.01	2.	*	1	1852	284	0.00	0.00	0.00	1.
1	0856	135	0.01	0.00	0.01	2.	*	1	1856	285	0.00	0.00	0.00	1.
1	0900	136	0.01	0.00	0.01	2.	*	1	1900	286	0.00	0.00	0.00	1.
1	0904	137	0.01	0.00	0.01	2.	*	1	1904	287	0.00	0.00	0.00	1.
1	0908	138	0.01	0.00	0.01	2.	*	1	1908	288	0.00	0.00	0.00	1.
1	0912	139	0.01	0.00	0.01	2.	*	1	1912	289	0.00	0.00	0.00	1.
1	0916	140	0.01	0.00	0.01	2.	*	1	1916	290	0.00	0.00	0.00	1.
1	0920	141	0.01	0.00	0.01	2.	*	1	1920	291	0.00	0.00	0.00	1.
1	0924	142	0.01	0.00	0.01	2.	*	1	1924	292	0.00	0.00	0.00	1.
1	0928	143	0.01	0.00	0.01	2.	*	1	1928	293	0.00	0.00	0.00	1.
1	0932	144	0.01	0.00	0.01	2.	*	1	1932	294	0.00	0.00	0.00	1.
1	0936	145	0.01	0.00	0.01	2.	*	1	1936	295	0.00	0.00	0.00	1.
1	0940	146	0.01	0.00	0.01	2.	*	1	1940	296	0.00	0.00	0.00	1.
1	0944	147	0.01	0.00	0.01	2.	*	1	1944	297	0.00	0.00	0.00	1.
1	0948	148	0.01	0.00	0.01	2.	*	1	1948	298	0.00	0.00	0.00	1.
1	0952	149	0.01	0.00	0.01	2.	*	1	1952	299	0.00	0.00	0.00	1.
1	0956	150	0.01	0.00	0.01	2.	*	1	1956	300	0.00	0.00	0.00	1.

TOTAL RAINFALL = 4.40, TOTAL LOSS = 0.68, TOTAL EXCESS = 3.72

PEAK FLOW	TIME	6-HR	24-HR	72-HR	19.93-HR
(CFS)	(HR)				
104.	6.00	11.	4.	4.	4.
		(INCHES)	3.228	3.708	3.708
		(AC-FT)	6.	7.	7.

CUMULATIVE AREA = 0.03 SQ MI

100 YEAR RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB82	163.	6.20	22.	8.	8.	0.18		
ROUTED TO	RT-4	161.	6.20	22.	8.	8.	0.18		
HYDROGRAPH AT	SB74	156.	6.20	23.	8.	8.	0.19		
2 COMBINED AT	DP74	317.	6.20	45.	17.	17.	0.37		
ROUTED TO	RT-3	314.	6.27	45.	17.	17.	0.37		
HYDROGRAPH AT	SB80	170.	6.27	28.	10.	10.	0.23		
ROUTED TO	RT-2	168.	6.33	28.	10.	10.	0.23		
HYDROGRAPH AT	SB81	279.	6.33	50.	18.	18.	0.41		
ROUTED TO	RT-1	278.	6.40	49.	18.	18.	0.41		
HYDROGRAPH AT	SB73	122.	6.20	17.	6.	6.	0.14		
HYDROGRAPH AT	SB75	94.	6.27	15.	6.	6.	0.12		
HYDROGRAPH AT	SB76	101.	6.27	16.	6.	6.	0.13		
6 COMBINED AT	DP75	1022.	6.33	170.	63.	63.	1.41		
ROUTED TO	RT-5	1021.	6.33	170.	63.	63.	1.41		
HYDROGRAPH AT	SB79	252.	6.20	36.	13.	13.	0.29		
ROUTED TO	RT-7	249.	6.27	36.	13.	13.	0.29		
HYDROGRAPH AT	SB78	205.	6.20	30.	11.	11.	0.24		
2 COMBINED AT	DP78	446.	6.20	65.	24.	24.	0.54		
ROUTED TO	RT-6	446.	6.27	65.	24.	24.	0.54		
HYDROGRAPH AT	SB77	226.	6.20	34.	13.	13.	0.27		
HYDROGRAPH AT	SB88	115.	6.20	17.	6.	6.	0.14		
4 COMBINED AT	DP77	1761.	6.27	285.	106.	106.	2.36		
ROUTED TO	RT-8	1757.	6.40	284.	105.	105.	2.36		
HYDROGRAPH AT	SC3-20	291.	6.13	39.	14.	14.	0.31		
2 COMBINED AT	DP71	1922.	6.40	320.	119.	119.	2.66		
ROUTED TO	RT-9	1917.	6.40	320.	119.	119.	2.66		
HYDROGRAPH AT	SB3-18	410.	6.13	49.	18.	18.	0.27		
2 COMBINED AT	DP70	2078.	6.40	367.	137.	137.	2.93		
ROUTED TO	RT-11	2077.	6.40	367.	137.	137.	2.93		
HYDROGRAPH AT	SC3-19	500.	6.07	56.	20.	20.	0.30		
ROUTED TO	RT-10	491.	6.07	56.	20.	20.	0.30		
HYDROGRAPH AT	SC3-17	379.	6.07	42.	15.	15.	0.18		

3 COMBINED AT	DP69	2372.	6.33	463.	171.	171.	3.41		
ROUTED TO	DBSRMN	1507.	6.67	457.	170.	170.	3.41	105.80	6.67
ROUTED TO	RT-12	1501.	6.67	457.	169.	169.	3.41		
HYDROGRAPH AT	SC3-15	471.	6.07	55.	19.	19.	0.21		
ROUTED TO	RT-10A	466.	6.07	55.	19.	19.	0.21		
2 COMBINED AT	DP87	1569.	6.67	509.	188.	188.	3.62		
ROUTED TO	RT-13	1564.	6.73	508.	188.	188.	3.62		
HYDROGRAPH AT	SC3-14	772.	6.07	92.	32.	32.	0.34		
HYDROGRAPH AT	SC3-16	634.	6.13	78.	27.	27.	0.30		
ROUTED TO	RT-13A	632.	6.27	78.	27.	27.	0.30		
3 COMBINED AT	DP68	2242.	6.20	675.	247.	247.	4.26		
ROUTED TO	DBSRMS	1561.	7.00	671.	246.	246.	4.26	16.12	7.00
ROUTED TO	RT-17	1560.	7.00	671.	245.	245.	4.26		
HYDROGRAPH AT	SC3-13	340.	6.13	43.	16.	16.	0.34		
ROUTED TO	RT-19	330.	6.20	43.	16.	16.	0.34		
HYDROGRAPH AT	SC3-12	452.	6.07	52.	18.	18.	0.21		
2 COMBINED AT	DP66	756.	6.13	95.	34.	34.	0.54		
ROUTED TO	RT-20	735.	6.40	94.	34.	34.	0.54		
HYDROGRAPH AT	SC3-11	593.	6.13	77.	27.	27.	0.28		
2 COMBINED AT	DP63A	1114.	6.33	169.	60.	60.	0.83		
2 COMBINED AT	DP63	2292.	6.40	837.	305.	305.	5.09		
ROUTED TO	RT-17A	2283.	6.40	837.	305.	305.	5.09		
HYDROGRAPH AT	SC3-10	67.	6.13	8.	3.	3.	0.06		
HYDROGRAPH AT	SC3-9	416.	6.33	73.	27.	27.	0.50		
ROUTED TO	RT-14	413.	6.33	73.	27.	27.	0.50		
HYDROGRAPH AT	SC3-8	246.	6.27	40.	14.	14.	0.23		
2 COMBINED AT	DP65	650.	6.33	113.	41.	41.	0.73		
ROUTED TO	RT-15	648.	6.33	113.	41.	41.	0.73		
HYDROGRAPH AT	SC3-7	380.	6.07	42.	15.	15.	0.16		
ROUTED TO	RT-15A	377.	6.07	42.	15.	15.	0.16		
2 COMBINED AT	DP64	833.	6.20	154.	56.	56.	0.88		
ROUTED TO	RT-16	830.	6.20	154.	56.	56.	0.88		
HYDROGRAPH AT	SC3-6	517.	6.07	56.	19.	19.	0.21		
2 COMBINED AT	DP62	1218.	6.13	209.	75.	75.	1.09		

	DB-6	286.	7.07	216.	85.	85.	0.96	77.99	7.07
HYDROGRAPH AT	SC2-29	104.	6.00	11.	4.	4.	0.03		

Downstream of Woodmen Road

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 06/25/2009 TIME 16:45:31 *
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
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::::::::::::::::::::::::::::::::::::
::: Full Microcomputer Implementation :::
::: by :::
::: Haestad Methods, Inc. :::
:::
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37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Sand Creek Detention Basin No.2 final design**Mod n. of Woodmen
2 ID FUTURE DEVELOPMENT CONDITION WITH SAND CREEK NO. 1
3 ID DETENTION BASIN MODELED PER DBPS WITH SPRINGS RANCH DETENTION
4 ID Model prepared by Kiowa Engineering has been modified by WCI
5 ID for the Upper sand Creek Detention Evaluation Report
6 ID 100 Year, 24 hr Type IIA Storms fn:SCBDW.DAT
7 ID File Modified By WCI To Remove Routing Segment RT-40 and delete
8 ID the model above Woodmen Road and substitute hydrographs from a
9 ID revised model SCBUW.dat for the watershed above Woodmen.
10 ID Basin 25 reduced by 16.4 ac and Basin 26 reduced by 21.2 acres to
11 ID reflect commercial area near Woodmen and Black Forrest Rd. that is
12 ID routed through Pond 6.
13 ID *DIAGRAM
IT 5 0 0 300
14 KK DP53
15 KM *****
16 KM IMPORT OUTFLOW HYDROGRAPH FROM POND 3 LOCATED NORTH OF WOODMEN ROAD
17 KM *****
18 BI DB-3 20
19 KO 1
20 KK RT-27
21 KM ROUTE FLOW FROM DESIGN POINT 53 (DETENTION BASIN 3) TO DESIGN POINT 50
22 RK 1380 .022 .035 TRAP 10 10
23 IO 5
24 KK SC2-24
25 KM RUNOFF FROM SUB-BASIN SC2-24
26 BA .0660
27 IN 30
28 PB 4.4
29 PC 0.0000 0.0025 0.0050 0.0075 0.0100 0.0150 0.0200 0.0250 0.0300 0.0500
30 PC 0.0600 0.1000 0.7000 0.7500 0.7800 0.7980 0.8200 0.8300 0.8400 0.8500
31 PC 0.8600 0.8650 0.8700 0.8850 0.8900 0.9000 0.9050 0.9100 0.9150 0.9210
32 PC 0.9270 0.9330 0.9400 0.9450 0.9500 0.9525 0.9550 0.9600 0.9650 0.9700
33 PC 0.9750 0.9800 0.9830 0.9850 0.9880 0.9900 0.9930 0.9950 0.9980 1.0000
34 LS 0 90.7
35 UD .190
36 KK RT-29
37 KM ROUTE FLOW FROM SUB-BASIN SC2-24 TO DESIGN POINT 50
38 RK 330 .020 .013 CIRC 4
39 KK SC2-27
40 KM RUNOFF FROM SUB-BASIN SC2-27
41 BA .0780
42 LS 0 92.0
43 UD .210

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HEC-1 INPUT

PAGE 2

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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44      KK      DP50
45      KM      DESIGN POINT 50 COMBINE RUNOFF FROM SC2-27, RT-27 AND RT-29
46      HC      3

47      KK      RT-31
48      KM      ROUTE FLOW FROM DESIGN POINT 50 TO DESIGN POINT 49
49      RK      940      .020      .035      TRAP      40      6
50      KM      *****
51      KM      BEGIN THE DIRECT DISCHARGE WATERSHED OF POND 2
52      KM      *****

53      KK      DP51
54      KM      *****
55      KM      IMPORT THE HYDROGRAPH FOR BASIN SC2-29 LOCATED NORTH OF WOODMEN ROAD
56      KM      *****
57      BI      SC2-29      20
58      KO      1

59      KK      RT-30
60      KM      ROUTE FLOW FROM DESIGN POINT 51 TO DESIGN POINT 49
61      RK      1590      .019      .035      TRAP      20      10

62      KK      DP49
63      KM      DESIGN POINT 49 COMBINE RUNOFF FROM RT-31 AND RT-30
64      HC      2

65      KK      RT-32
66      KM      ROUTE FLOW FROM DESIGN POINT 49 TO DESIGN POINT 48
67      RK      4930      .017      .035      TRAP      40      6

68      KK      DP54
69      KM      *****
70      KM      INPUT HYDROGRAPH FOR OUTFLOW FROM POND 6 LOCATED NORTH OF WOODMEN ROAD
71      KM      *****
72      BI      DB-6      20
73      KO      1

74      KK      RT-26
75      KM      ROUTE FLOW FROM DESIGN POINT 54 (DETENTION BASIN 92 OUTLET) TO DESIGN POINT 47
76      RK      4680      .028      .035      TRAP      40      10

77      KK      SC2-26
78      KM      RUNOFF FROM SUB-BASIN SC2-26
79      KM      Basin area reduced by 21.2 ac to reflect commercial area near Balck Forrest
80      KM      RD and Woodmen Rd that is routed through Pond 6
81      BA      .1530
82      LS      0      79
83      UD      .320

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HEC-1 INPUT

PAGE 3

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

84      KK      RT-34
85      KM      ROUTE FLOW FROM SUB-BASIN SC2-26 TO DESIGN POINT 47
86      RK      1620      .014      .035      TRAP      4      4

87      KK      SC2-25
88      KM      RUNOFF FROM SUB-BASIN SC2-25
89      KM      Basin area reduced by 16.4 ac to reflect commercial area near Balck Forrest
90      KM      RD and Woodmen Rd that is routed through Pond 6
91      BA      .4180
92      LS      0      79
93      UD      .440

94      KK      DP47
95      KM      DESIGN POINT 47 COMBINE RUNOFF FROM SUB-BASIN SC3-26 AND RT-26 AND RT-34
96      HC      3

97      KK      RT-33
98      KM      ROUTE FLOW FROM DESIGN POINT 47 TO DESIGN POINT 48
99      RK      510      .020      .035      TRAP      6      4

100      KK      SC2-28
101      KM      RUNOFF FROM SUB-BASIN SC2-28
102      BA      .2740
103      LS      0      67.1
104      UD      .510

105      KK      SC2-21
106      KM      RUNOFF FROM SUB-BASIN SC2-21
107      BA      .1930
108      LS      0      79.0
109      UD      .390

110      KK      DP48
111      KM      DESIGN POINT 48 COMBINE RUNOFF FROM SB'S SC2-21 AND SC2-28, RT-33 AND RT-32
112      HC      4

113      KK      RT-37
114      KM      ROUTE FLOW FROM DESIGN POINT 48 TO DESIGN POINT 42
115      RK      1080      .022      .035      TRAP      60      6

116      KK      SC2-23
117      KM      RUNOFF FROM SUB-BASIN SC2-23
118      BA      .1960
119      LS      0      79
120      UD      .330

121      KK      RT-35
122      KM      ROUTE FLOW FROM SUB-BASIN SC2-23 TO DESIGN POINT 46
123      RK      1180      .015      .013      CIRC      5

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PAGE 5

LINE	ID	1.....2.....3.....4.....5.....6.....7.....8.....9.....10
164	KK	DP85
165	KM	DESIGN POINT 85 COMBINE RUNOFF FROM RT-41, RT-38, RT-39 AND
166	KM	SUB-BASIN SC2-13
167	HC	4
168	KK	RT-42
169	KM	ROUTE FLOW FROM DESIGN POINT 85 TO DESIGN POINT 37
170	RK	2900 .016 .035 TRAP 60 6
171	KK	SC2-14
172	KM	RUNOFF FROM SUB-BASIN SC2-14
173	BA	.2650
174	LS	0 76.2
175	UD	.450
176	KK	SC2-16
177	KM	RUNOFF FROM SUB-BASIN SC2-16
178	BA	.2120
179	LS	0 76.2
180	UD	.310
181	KK	DP37
182	KM	DESIGN POINT 37 COMBINE RUNOFF FROM RT-42, SUB-BASINS SC2-14 AND SC2-16
183	HC	3
184	KK	RT-43
185	KM	ROUTE FLOW FROM DESIGN POINT 37 TO DESIGN POINT 36
186	RK	3380 .016 .035 TRAP 60 4
187	KK	SC2-12
188	KM	RUNOFF FROM SUB-BASIN SC2-12
189	BA	.1790
190	LS	0 70.6
191	UD	.280
192	KK	RT-46
193	KM	ROUTE FLOW FROM SUB-BASIN SC2-12 TO DESIGN POINT 33
194	RK	2280 .024 .035 TRAP 20 4
195	KK	SC2-8
196	KM	RUNOFF FROM SUB-BASIN SC2-8
197	BA	.3480
198	LS	0 79.0
199	UD	.440
200	KK	DP33
201	KM	DESIGN POINT 33 COMBINE RUNOFF FROM RT-46 AND SUB-BASIN SC2-8
202	HC	2

LINE	ID	1	2	3	4	5	6	7	8	9	10
203	KK	RT-45									
204	KM	ROUTE FLOW FROM DESIGN POINT 33 TO DESIGN POINT 36									
205	RK	850	.021	.035	TRAP	20	4				
206	KK	DP36									
207	KM	DESIGN POINT 36 COMBINE RUNOFF FROM RT-45 AND RT-43									
208	HC	2									
209	KK	RT-44									
210	KM	ROUTE FLOW FROM DESIGN POINT 36 TO DESIGN POINT 35									
211	RK	1740	.016	.035	TRAP	60	4				
212	KK	SC2-11									
213	KM	RUNOFF FROM SUB-BASIN SC2-11									
214	BA	.2100									
215	LS	0	79.0								
216	UD	.240									
217	KK	RT-52									
218	KM	ROUTE FLOW FROM SUB-BASIN SC2-11 TO DESIGN POINT 39									
219	RK	2680	.008	.035	TRAP	30	4				
220	KK	SC2-19									
221	KM	RUNOFF FROM SUB-BASIN SC2-19									
222	BA	.1820									
223	LS	0	86.0								
224	UD	.300									
225	KK	RT-51									
226	KM	ROUTE FLOW FROM SUB-BASIN SC2-19 TO DESIGN POINT 39									
227	RK	3100	.022	.013	CIRC	6					
228	KK	SC2-18									
229	KM	RUNOFF FROM SUB-BASIN SC2-18									
230	BA	.1980									
231	LS	0	88.0								
232	UD	.290									
233	KK	DP39									
234	KM	DESIGN POINT 39 COMBINE RUNOFF FROM RT-52, RT-51 AND SUB-BASIN SC2-18									
235	HC	3									
236	KK	RT-49									
237	KM	ROUTE FLOW FROM DESIGN POINT 39 TO DESIGN POINT 34									
238	RK	1160	.024	.035	TRAP	20	4				
239	KK	SC2-17									
240	KM	RUNOFF FROM SUB-BASIN SC2-17									
241	BA	.1600									
242	LS	0	79.0								
243	UD	.320									

LINE	ID	1	2	3	4	5	6	7	8	9	10
244	KK	RT-50									
245	KM	ROUTE FLOW FROM SUB-BASIN SC2-17 TO DESIGN POINT 34									
246	RK	1840	.025	.035	TRAP	20	4				
247	KK	SC2-29									
248	KM	RUNOFF FROM SUB-BASIN SC2-29									
249	BA	.1510									
250	LS	0	88.0								
251	UD	.210									
252	KK	DP34									
253	KM	DESIGN POINT 34 COMBINE RUNOFF FROM RT-49, RT-50 AND SUB-BASIN SC2-29									
254	HC	3									
255	KK	RT-48									
256	KM	ROUTE FLOW FROM DESIGN POINT 34 TO DESIGN POINT 32									
257	RK	1470	.020	.04	TRAP	20	4				
258	KK	SC2-10									
259	KM	RUNOFF FROM SUB-BASIN SC2-10									
260	BA	.3290									
261	LS	0	78.4								
262	UD	.390									
263	KK	DP32									
264	KM	DESIGN POINT 32 COMBINE RUNOFF FROM RT-48 AND SUB-BASIN SC2-10									
265	HC	2									
266	KK	RT-47									
267	KM	ROUTE FLOW FROM SUB-BASIN DESIGN POINT 32 TO DESIGN POINT 35									
268	RK	991	.028	.031	TRAP	20	4				
269	KK	SC2-9									
270	KM	RUNOFF FROM SUB-BASIN SC2-9									
271	BA	.2060									
272	LS	0	73.0								
273	UD	.520									
274	KK	DP35									
275	KM	DESIGN POINT 35 COMBINE RUNOFF FROM RT-47, RT-44 AND SUB-BASIN SC2-9									
276	HC	3									
277	KK	RT-53									
278	KM	ROUTE FLOW FROM DESIGN POINT 35 TO DESIGN POINT 29									
279	RK	1120	.020	.035	TRAP	60	6				

280 KK SC2-7
 281 KM RUNOFF FROM SUB-BASIN SC2-7
 282 BA .1440
 283 LS 0 79.0
 284 UD .320

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

285 KK DP29
 286 KM DESIGN POINT 29 COMBINE RUNOFF FROM RT-53 AND SUB-BASIN SC2-7
 287 HC 2
 288 KK RT-54
 289 KM ROUTE FLOW FROM DESIGN POINT 29 TO DESIGN POINT 28
 290 RK 1560 .011 .035 TRAP 60 4
 291 KK SC2-6
 292 KM RUNOFF FROM SUB-BASIN SC2-6
 293 BA .1500
 294 LS 0 81.6
 295 UD .240
 296 KK RT-55
 297 KM ROUTE FLOW FROM SUB-BASIN SC2-6 TO DESIGN POINT 28
 298 RD 950 .020 .035 TRAP 10 4
 299 KK SC2-2
 300 KM RUNOFF FROM SUB-BASIN SC2-2
 301 BA .1670
 302 LS 0 80.8
 303 UD .280
 304 KK DP28
 305 KM DESIGN POINT 28 COMBINE RUNOFF FROM RT-54, RT-55 AND SUB-BASIN SC2-2
 306 HC 3
 307 KK RT-56
 308 KM ROUTE FLOW FROM DESIGN POINT 28 TO DESIGN POINT 21
 309 RK 1280 .017 .035 TRAP 60 4
 310 KK SC2-5
 311 KM RUNOFF FROM SUB-BASIN SC2-5
 312 BA .1610
 313 LS 0 92.0
 314 UD .280
 315 KK RT-58
 316 KM ROUTE FLOW FROM SUB-BASIN SC2-5 TO DESIGN POINT 21
 317 RK 1670 .027 .024 CIRC 5
 318 KK SC2-0
 319 KM RUNOFF FROM SUB-BASIN SC2-0
 320 BA .1500
 321 LS 0 73.4
 322 UD .320
 323 KK RT-57
 324 KM ROUTE FLOW FROM SUB-BASIN SC2-0 TO DESIGN POINT 21
 325 RK 800 .032 .035 TRAP 10 4

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

326 KK DP21
 327 KM DESIGN POINT 21 COMBINE RUNOFF FROM RT-58, RT-56 AND RT- 57
 328 HC 3
 329 KK RT-59
 330 KM ROUTE FLOW FROM DESIGN POINT 21 TO DESIGN POINT 20
 331 RK 830 .01 .035 TRAP 80 4
 332 KK SC2-4
 333 KM RUNOFF FROM SUB-BASIN SC2-4
 334 BA .2850
 335 LS 0 92.0
 336 UD .320
 337 KK RT-61
 338 KM ROUTE FLOW FROM SUB-BASIN SC2-4 TO DESIGN POINT 22
 339 RK 1670 .019 .035 TRAP 20 3
 340 KK SC2-3
 341 KM RUNOFF FROM SUB-BASIN SC2-3
 342 BA .0710
 343 LS 0 92.0
 344 UD .210
 345 KK DP22
 346 KM DESIGN POINT 22 COMBINE RUNOFF FROM RT-61 AND SC2-3
 347 HC 2
 348 KK RT-60
 349 KM ROUTE FLOW FROM DESIGN POINT 22 TO DESIGN POINT 20
 350 RK 1010 .018 .035 TRAP 20 3
 351 KK SC2-1
 352 KM RUNOFF FROM SUB-BASIN SC2-1
 353 BA .1850
 354 LS 0 80.0
 355 UD .290

356	KK	DP20										
357	KM	DESIGN POINT 20 COMBINE RUNOFF FROM RT-60, RT-59 AND SB SC2-1										
358	KO	1										
359	HC	3										
360	KK	DBSC2										
361	KM	ROUTE FLOW FROM DP 20 THROUGH SAND CREEK DETENTION BASIN DBSC2										
362	KM	TRIPLE 12 x 8 CBC										
363	RS	1	ELEV	6495.5								
364	SV	0	.41	8.35	28.53	54.91	83.35	115.75	153.35	194.03	237.59	
365	SV	283.9	349									
366	SQ	0	75	150	930	1530	2300	2810	3360	4050	4250	
367	SQ	4550	7150									
368	SE	6495.5	6496	6498	6500	6502	6504	6506	6508	6510	6512	
369	SE	6514	6516									

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PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

370	KK	RT-63										
371	KM	ROUTE FLOW FROM DETENTION BASIN SC2 TO DESIGN POINT 18										
372	KO	1										
373	RK	1290	.007	.035	TRAP	100	3					
374	KK	SC1-10										
375	KM	RUNOFF FROM SUB-BASIN SC1-10										
376	BA	.2880										
377	LS	0	78.2									
378	UD	.390										
379	KK	DP18										
380	KM	DESIGN POINT 18 COMBINE RUNOFF FROM RT-63 AND SUB-BASIN SC1-10										
381	HC	2										
382	KK	RT-66										
383	KM	ROUTE FLOW FROM DESIGN POINT 18 TO DESIGN POINT 17										
384	RK	320	.007	.035	TRAP	100	3					
385	KK	SC1-13										
386	KM	RUNOFF FROM SUB-BASIN SC1-13										
387	BA	.1300										
388	LS	0	87.3									
389	UD	.270										
390	KK	RT-64										
391	KM	ROUTE FLOW FROM SUB-BASIN SC1-13 TO DESIGN POINT 19										
392	RK	1970	.026	.013	CIRC	4.5						
393	KK	SC1-14										
394	KM	RUNOFF FROM SUB-BASIN SC1-14										
395	BA	.1100										
396	LS	0	92.0									
397	UD	.280										
398	KK	DP19										
399	KM	DESIGN POINT 19 COMBINE RUNOFF FROM RT-64 AND SUB-BASIN SC1-14										
400	HC	2										
401	KK	RT-65										
402	KM	ROUTE FLOW FROM DESIGN POINT 19 TO DESIGN POINT 17										
403	RK	700	.022	.013	TRAP	10	.1					
404	KK	DP17										
405	KM	DESIGN POINT 17 COMBINE RUNOFF FROM RT-65 AND RT-66										
406	HC	2										
407	KK	RT-67										
408	KM	ROUTE FLOW FROM DESIGN POINT 17 TO DESIGN POINT 13										
409	RK	910	.007	.035	TRAP	100	3					

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PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

410	KK	SC1-9										
411	KM	RUNOFF FROM SUB-BASIN SC1-9										
412	BA	.2400										
413	LS	0	76.9									
414	UD	.240										
415	KK	DBSR29										
416	KM	ROUTE FLOW FROM SB SC1-9 THROUGH SPRINGS RANCH DETENTION BASIN DBSR29										
417	RS	1	ELEV	100								
418	SV	0	.03	0.47	1.20	2.60	6.70	8.8	10.7			
419	SQ	0	.2	7.8	22	40	73	97	113			
420	SE	100	100.5	101.5	102.5	103.5	104.5	105.5	106.5			
421	KK	RT-70										
422	KM	ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR29 TO DESIGN POINT 14										
423	RD	2750	.013	.013	CIRC	5.5						
424	KK	SC1-8										
425	KM	RUNOFF FROM SUB-BASIN SC1-8										
426	BA	.2100										
427	LS	0	79.5									
428	UD	.320										
429	KK	RT-71										
430	KM	ROUTE FLOW FROM SUB-BASIN SC1-8 TO DESIGN POINT 14										
431	RK	600	.010	.013	CIRC	5						
432	KK	DP14										
433	KM	DESIGN POINT 14 COMBINE RUNOFF FROM RT-71 AND RT-70										
434	HC	2										

435 KK RT-69
 436 KM ROUTE FLOW FROM DESIGN POINT 14 TO DESIGN POINT 12
 437 RK 1550 .015 .013 TRAP 7 .1

 438 KK SC1-7
 439 KM RUNOFF FROM SUB-BASIN SC1-7
 440 BA .0960
 441 LS 0 80.1
 442 UD .230

 443 KK DP12
 444 KM DESIGN POINT 12 COMBINE RUNOFF FROM SUB-BASIN SC1-7 AND RT-69
 445 HC 2

 446 KK RT-68
 447 KM ROUTE FLOW FROM DESIGN POINT 12 TO DESIGN POINT 13
 448 RK 390 .025 .013 TRAP 7 .1

HEC-1 INPUT

PAGE 12

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

449 KK DP13
 450 KM DESIGN POINT 13 COMBINE RUNOFF FROM RT-67 AND RT-68
 451 HC 2

 452 KK RT-72
 453 KM ROUTE FLOW FROM DESIGN POINT 13 TO DESIGN POINT 10
 454 RK 1150 .007 .04 TRAP 100 3

 455 KK SC1-6
 456 KM RUNOFF FROM SUB-BASIN SC1-6
 457 BA .1590
 458 LS 0 80.6
 459 UD .220

 460 KK DBSR15
 461 KM ROUTE FLOW FROM SB SC1-6 THROUGH SPRINGS RANCH DETENTION BASIN DBSR15
 462 RS 1 ELEV 100
 463 SV 0 1.50 3.80 6.50 9.70 13.80
 464 SQ 0 80 125 150 180 210
 465 SE 100 102 104 106 108 110

 466 KK RT-74
 467 KM ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR15 TO DESIGN POINT 11
 468 RK 2080 .021 .013 CIRC 3.5

 469 KK SC1-5
 470 KM RUNOFF FROM SUB-BASIN SC1-5
 471 BA .1730
 472 LS 0 79.0
 473 UD .310

 474 KK DP11
 475 KM DESIGN POINT 11 COMBINE RUNOFF FROM SUB-BASIN SC1-5 AND RT-74
 476 HC 2

 477 KK RT-73
 478 KM ROUTE FLOW FROM DESIGN POINT 11 TO DESIGN POINT 10
 479 RK 580 .024 .013 CIRC 3.5

 480 KK DP10
 481 KM DESIGN POINT 10 COMBINE RUNOFF FROM RT-72 AND RT-73
 482 HC 2

 483 KK RT-75
 484 KM ROUTE FLOW FROM DESIGN POINT 10 TO DESIGN POINT 9
 485 RK 2270 .007 .035 TRAP 100 3

 486 KK SC1-2
 487 KM RUNOFF FROM SUB-BASIN SC1-2
 488 BA .1360
 489 LS 0 79.0
 490 UD .320

HEC-1 INPUT

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

491 KK RT-76
 492 KM ROUTE FLOW FROM SUB-BASIN SC1-2 TO DESIGN POINT 9
 493 RD 500 .039 .013 CIRC 3

 494 KK DP9
 495 KM DESIGN POINT 9 COMBINE RUNOFF FROM RT-75 AND RT-76
 496 HC 2

 497 KK RT-78
 498 KM ROUTE FLOW FROM DESIGN POINT 9 TO DESIGN POINT 4
 499 RK 1530 .007 .035 TRAP 100 3

 500 KK SC1-12
 501 KM RUNOFF FROM SUB-BASIN SC1-12
 502 BA .2480
 503 LS 0 84.0
 504 UD .370

 505 KK RT-81
 506 KM ROUTE FLOW FROM SUB-BASIN SC1-12 TO DESIGN POINT 8
 507 RK 1400 .008 .035 TRAP 30 4

 508 KK SC1-11
 509 KM RUNOFF FROM SUB-BASIN SC1-11
 510 BA .1600

511 LS 0 92.0
 512 UD .290

 513 KK DP8
 514 KM DESIGN POINT 8 COMBINE RUNOFF FROM SUB-BASIN SC1-11 AND RT-81
 515 HC 2

 516 KK RT-77
 517 KM ROUTE FLOW FROM DESIGN POINT 8 TO DESIGN POINT 4
 518 RK 3100 .007 .035 TRAP 100 3

 519 KK SC1-4
 520 KM RUNOFF FROM SUB-BASIN SC1-4
 521 BA .1010
 522 LS 0 92.0
 523 UD .270

 524 KK RT-80
 525 KM ROUTE FLOW FROM SUB-BASIN SC1-4 TO DESIGN POINT 4
 526 RK 720 .014 .013 CIRC 5

 527 KK SC1-1
 528 KM RUNOFF FROM SUB-BASIN SC1-1
 529 BA .1090
 530 LS 0 79.0
 531 UD .280

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

 532 KK RT-79
 533 KM ROUTE FLOW FROM SUB-BASIN SC1-1 TO DESIGN POINT 4
 534 RK 1115 .027 .013 CIRC 3

 535 KK SC1-3
 536 KM RUNOFF FROM SUB-BASIN SC1-3
 537 BA .3530
 538 LS 0 65.0
 539 UD .460

 540 KK DP4
 541 KM DESIGN POINT 4 COMBINE RUNOFF FROM SB SC1-3, RT-80, RT-77, RT-78 AND RT-79
 542 HC 5

 543 KK DBSC1
 544 KM ROUTE FLOW FROM DP 4 THROUGH SAND CREEK DETENTION BASIN DBSC1
 545 KO 0
 546 RS 1 ELEV 6393
 547 SV 0 3.06 14.95 31.12 53.59 69.95 86.31 106.5 126.7 150.4
 548 SV 174.1 200.4 226.7 254.9 283.2 313.4 343.6 375.2 406.8 439.5
 549 SQ 0 3 5 189 954 1465 2055 2431 2755 3045
 550 SQ 3310 3555 3785 4037 5518 7863 10797 14211 18042 22245
 551 SE 6393 6394 6396 6398 6400 6401 6402 6403 6404 6405
 552 SE 6406 6407 6408 6409 6410 6411 6412 6413 6414 6415
 553 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT
 LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW

 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

 14 DP53
 V
 V
 20 RT-27
 .
 .
 24 . SC2-24
 .
 V
 V
 36 . RT-29
 .
 .
 39 . SC2-27
 .
 .
 44 DP50.....
 V
 V
 47 RT-31
 .
 .
 53 . DP51
 .
 V
 V
 59 . RT-30
 .
 .
 62 DP49.....
 V
 V
 65 RT-32
 .
 .
 68 . DP54
 .
 V
 V
 74 . RT-26
 .
 .
 77 . SC2-26
 .
 V
 V
 84 . RT-34

87	.	.	.	SC2-25
	.	.	.	.
94	.	DP47		.
	.	V	.	.
	.	V	.	.
97	.	RT-33	.	.
	.	.	.	.
100	.	.	SC2-28	.
	.	.	.	.
105	.	.	.	SC2-21
	.	.	.	.
110	DP48		.	.
	V	.	.	.
	V	.	.	.
113	RT-37	.	.	.
	.	.	.	.
116	.	SC2-23	.	.
	.	V	.	.
	.	V	.	.
121	.	RT-35	.	.
	.	.	.	.
124	.	.	SC2-22	.
	.	.	.	.
129	.	DP46		.
	.	V	.	.
	.	V	.	.
132	.	RT-36	.	.
	.	.	.	.
135	DP42		.	.
	V	.	.	.
	V	.	.	.
138	RT-38	.	.	.
	.	.	.	.
141	.	SC2-20	.	.
	.	V	.	.
	.	V	.	.
146	.	RT-41	.	.
	.	.	.	.
149	.	.	SC2-15	.
	.	.	V	.
	.	.	V	.
154	.	.	RT-39	.
	.	.	.	.
157	.	.	.	SC2-13
	.	.	.	.
164	DP85		.	.
	V	.	.	.
	V	.	.	.
168	RT-42	.	.	.
	.	.	.	.
171	.	SC2-14	.	.
	.	.	.	.
176	.	.	SC2-16	.
	.	.	.	.
181	DP37		.	.
	V	.	.	.
	V	.	.	.
184	RT-43	.	.	.
	.	.	.	.
187	.	SC2-12	.	.
	.	V	.	.
	.	V	.	.
192	.	RT-46	.	.
	.	.	.	.
195	.	.	SC2-8	.
	.	.	.	.
200	.	DP33		.
	.	V	.	.
	.	V	.	.
203	.	RT-45	.	.
	.	.	.	.
206	DP36		.	.
	V	.	.	.
	V	.	.	.
209	RT-44	.	.	.
	.	.	.	.
212	.	SC2-11	.	.
	.	V	.	.
	.	V	.	.
217	.	RT-52	.	.
	.	.	.	.
220	.	.	SC2-19	.
	.	.	V	.
	.	.	V	.

225	.	.	RT-51	.
	.	.	.	.
228	.	.	.	SC2-18
	.	.	.	.
233	.	DP39		.
	.	V		.
	.	V		.
236	.	RT-49		.
	.	.		.
239	.	.	SC2-17	.
	.	.	V	.
	.	.	V	.
244	.	.	RT-50	.
	.	.	.	.
247	.	.	.	SC2-29
	.	.	.	.
252	.	DP34		.
	.	V		.
	.	V		.
255	.	RT-48		.
	.	.		.
258	.	.	SC2-10	.
	.	.	.	.
263	.	DP32		.
	.	V		.
	.	V		.
266	.	RT-47		.
	.	.		.
269	.	.	SC2-9	.
	.	.	.	.
274	.	DP35		.
	.	V		.
	.	V		.
277	.	RT-53		.
	.	.		.
280	.	.	SC2-7	.
	.	.	.	.
285	.	DP29		.
	.	V		.
	.	V		.
288	.	RT-54		.
	.	.		.
291	.	.	SC2-6	.
	.	.	V	.
	.	.	V	.
296	.	.	RT-55	.
	.	.	.	.
299	.	.	SC2-2	.
	.	.	.	.
304	.	DP28		.
	.	V		.
	.	V		.
307	.	RT-56		.
	.	.		.
310	.	.	SC2-5	.
	.	.	V	.
	.	.	V	.
315	.	.	RT-58	.
	.	.	.	.
318	.	.	.	SC2-0
	.	.	.	V
	.	.	.	V
323	.	.	RT-57	.
	.	.	.	.
326	.	DP21		.
	.	V		.
	.	V		.
329	.	RT-59		.
	.	.		.
332	.	.	SC2-4	.
	.	.	V	.
	.	.	V	.
337	.	.	RT-61	.
	.	.	.	.
340	.	.	.	SC2-3
	.	.	.	.
345	.	.	DP22	
	.	.	V	.
	.	.	V	.
348	.	.	RT-60	.
	.	.	.	.
351	.	.	.	SC2-1
	.	.	.	.
356	.	DP20		.
	.	V		.

360	V	
	DBSC2	
	V	
370	RT-63	
	.	
374	.	SC1-10
	.	
379	DP18.....	
	V	
	V	
382	RT-66	
	.	
385	.	SC1-13
	.	V
	.	V
390	.	RT-64
	.	
393	.	SC1-14
	.	
398	.	DP19.....
	.	V
	.	V
401	.	RT-65
	.	
404	DP17.....	
	V	
	V	
407	RT-67	
	.	
410	.	SC1-9
	.	V
	.	V
415	.	DBSR29
	.	V
	.	V
421	.	RT-70
	.	
424	.	SC1-8
	.	V
	.	V
429	.	RT-71
	.	
432	.	DP14.....
	.	V
	.	V
435	.	RT-69
	.	
438	.	SC1-7
	.	
443	.	DP12.....
	.	V
	.	V
446	.	RT-68
	.	
449	DP13.....	
	V	
	V	
452	RT-72	
	.	
455	.	SC1-6
	.	V
	.	V
460	.	DBSR15
	.	V
	.	V
466	.	RT-74
	.	
469	.	SC1-5
	.	
474	.	DP11.....
	.	V
	.	V
477	.	RT-73
	.	
480	DP10.....	
	V	
	V	
483	RT-75	
	.	
486	.	SC1-2
	.	V
	.	V
491	.	RT-76
	.	
494	DP9.....	
	V	
	V	
497	RT-78	

```

.
.
500      .      SC1-12
.          V
.          V
505      .      RT-81
.          .
.          .
508      .          SC1-11
.          .
.          .
513      .      DP8.....
.          V
.          V
516      .      RT-77
.          .
.          .
519      .          SC1-4
.          .
.          V
.          V
524      .          RT-80
.          .
.          .
527      .          .
.          .
.          .
532      .          .
.          .
.          .
535      .          .
.          .
.          .
540      .      DP4.....
.          V
.          V
543      .      DBSC1

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```
*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
*
* RUN DATE 06/25/2009 TIME 16:45:31
*
*****
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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****
```

Sand Creek Detention Basin No.2 final design**Mod n. of Woodmen
 FUTURE DEVELOPMENT CONDITION WITH SAND CREEK NO. 1
 DETENTION BASIN MODELED PER DBPS WITH SPRINGS RANCH DETENTION
 Model prepared by Kiowa Engineering has been modified by WCI
 for the Upper sand Creek Detention Evaluation Report
 100 Year, 24 hr Type IIA Storms fn:SCBDW.DAT
 File Modified By WCI To Remove Routing Segment RT-40 and delete
 the model above Woodmen Road and substitute hydrographs from a
 revised model SCBUW.dat for the watershed above Woodmen.
 Basin 25 reduced by 16.4 ac and Basin 26 reduced by 21.2 acres to
 reflect commercial area near Woodmen and Black Forrest Rd. that is
 routed through Pond 6.

IT HYDROGRAPH TIME DATA

NMIN	5	MINUTES IN COMPUTATION INTERVAL
IDATE	1 0	STARTING DATE
ITIME	0000	STARTING TIME
NQ	300	NUMBER OF HYDROGRAPH ORDINATES
NDDATE	2 0	ENDING DATE
NDTIME	0055	ENDING TIME
ICENT	19	CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS

DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-Feet
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

*** **

14 KK *****
 * *
 * DP53 *
 * *

 IMPORT OUTFLOW HYDROGRAPH FROM POND 3 LOCATED NORTH OF WOODMEN ROAD

18 BI READ STATION DB-3 HYDROGRAPH FROM UNIT 20

HYDROGRAPH AT STATION DP53

DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*
1	0000	1	0.	*	1	0615	76	20.	*	1	1230	151	358.	*	1	1845	226	154.					
1	0005	2	0.	*	1	0620	77	30.	*	1	1235	152	355.	*	1	1850	227	152.					
1	0010	3	0.	*	1	0625	78	174.	*	1	1240	153	352.	*	1	1855	228	150.					
1	0015	4	0.	*	1	0630	79	407.	*	1	1245	154	349.	*	1	1900	229	148.					
1	0020	5	0.	*	1	0635	80	869.	*	1	1250	155	346.	*	1	1905	230	147.					
1	0025	6	0.	*	1	0640	81	1055.	*	1	1255	156	342.	*	1	1910	231	145.					
1	0030	7	0.	*	1	0645	82	1123.	*	1	1300	157	338.	*	1	1915	232	144.					
1	0035	8	0.	*	1	0650	83	1189.	*	1	1305	158	335.	*	1	1920	233	142.					
1	0040	9	0.	*	1	0655	84	1310.	*	1	1310	159	331.	*	1	1925	234	141.					
1	0045	10	0.	*	1	0700	85	1445.	*	1	1315	160	327.	*	1	1930	235	141.					
1	0050	11	0.	*	1	0705	86	1550.	*	1	1320	161	323.	*	1	1935	236	140.					
1	0055	12	0.	*	1	0710	87	1634.	*	1	1325	162	319.	*	1	1940	237	140.					
1	0100	13	0.	*	1	0715	88	1700.	*	1	1330	163	315.	*	1	1945	238	140.					
1	0105	14	0.	*	1	0720	89	1750.	*	1	1335	164	311.	*	1	1950	239	140.					
1	0110	15	0.	*	1	0725	90	1784.	*	1	1340	165	307.	*	1	1955	240	140.					
1	0115	16	0.	*	1	0730	91	1805.	*	1	1345	166	302.	*	1	2000	241	140.					
1	0120	17	0.	*	1	0735	92	1817.	*	1	1350	167	298.	*	1	2005	242	140.					
1	0125	18	0.	*	1	0740	93	1819.	*	1	1355	168	293.	*	1	2010	243	140.					
1	0130	19	0.	*	1	0745	94	1812.	*	1	1400	169	288.	*	1	2015	244	140.					
1	0135	20	0.	*	1	0750	95	1798.	*	1	1405	170	283.	*	1	2020	245	140.					
1	0140	21	0.	*	1	0755	96	1778.	*	1	1410	171	279.	*	1	2025	246	140.					
1	0145	22	0.	*	1	0800	97	1752.	*	1	1415	172	274.	*	1	2030	247	140.					
1	0150	23	0.	*	1	0805	98	1719.	*	1	1420	173	269.	*	1	2035	248	140.					
1	0155	24	0.	*	1	0810	99	1681.	*	1	1425	174	265.	*	1	2040	249	140.					
1	0200	25	0.	*	1	0815	100	1639.	*	1	1430	175	260.	*	1	2045	250	140.					
1	0205	26	0.	*	1	0820	101	1592.	*	1	1435	176	256.	*	1	2050	251	140.					
1	0210	27	0.	*	1	0825	102	1543.	*	1	1440	177	252.	*	1	2055	252	140.					
1	0215	28	0.	*	1	0830	103	1492.	*	1	1445	178	247.	*	1	2100	253	140.					
1	0220	29	0.	*	1	0835	104	1439.	*	1	1450	179	243.	*	1	2105	254	140.					
1	0225	30	0.	*	1	0840	105	1386.	*	1	1455	180	240.	*	1	2110	255	140.					
1	0230	31	0.	*	1	0845	106	1332.	*	1	1500	181	236.	*	1	2115	256	140.					
1	0235	32	0.	*	1	0850	107	1279.	*	1	1505	182	232.	*	1	2120	257	140.					

1	0240	33	0.	*	1	0855	108	1227.	*	1	1510	183	229.	*	1	2125	258	140.
1	0245	34	0.	*	1	0900	109	1192.	*	1	1515	184	226.	*	1	2130	259	140.
1	0250	35	0.	*	1	0905	110	1174.	*	1	1520	185	223.	*	1	2135	260	140.
1	0255	36	0.	*	1	0910	111	1154.	*	1	1525	186	221.	*	1	2140	261	140.
1	0300	37	0.	*	1	0915	112	1134.	*	1	1530	187	218.	*	1	2145	262	140.
1	0305	38	0.	*	1	0920	113	1114.	*	1	1535	188	216.	*	1	2150	263	140.
1	0310	39	0.	*	1	0925	114	1093.	*	1	1540	189	214.	*	1	2155	264	140.
1	0315	40	0.	*	1	0930	115	1071.	*	1	1545	190	212.	*	1	2200	265	140.
1	0320	41	0.	*	1	0935	116	1050.	*	1	1550	191	210.	*	1	2205	266	140.
1	0325	42	0.	*	1	0940	117	1028.	*	1	1555	192	209.	*	1	2210	267	140.
1	0330	43	0.	*	1	0945	118	1006.	*	1	1600	193	208.	*	1	2215	268	140.
1	0335	44	0.	*	1	0950	119	985.	*	1	1605	194	207.	*	1	2220	269	140.
1	0340	45	0.	*	1	0955	120	963.	*	1	1610	195	206.	*	1	2225	270	140.
1	0345	46	0.	*	1	1000	121	960.	*	1	1615	196	205.	*	1	2230	271	140.
1	0350	47	0.	*	1	1005	122	954.	*	1	1620	197	204.	*	1	2235	272	140.
1	0355	48	0.	*	1	1010	123	947.	*	1	1625	198	203.	*	1	2240	273	140.
1	0400	49	0.	*	1	1015	124	939.	*	1	1630	199	202.	*	1	2245	274	140.
1	0405	50	0.	*	1	1020	125	931.	*	1	1635	200	201.	*	1	2250	275	140.
1	0410	51	0.	*	1	1025	126	922.	*	1	1640	201	200.	*	1	2255	276	140.
1	0415	52	0.	*	1	1030	127	913.	*	1	1645	202	200.	*	1	2300	277	140.
1	0420	53	0.	*	1	1035	128	904.	*	1	1650	203	199.	*	1	2305	278	140.
1	0425	54	0.	*	1	1040	129	895.	*	1	1655	204	198.	*	1	2310	279	140.
1	0430	55	0.	*	1	1045	130	886.	*	1	1700	205	197.	*	1	2315	280	140.
1	0435	56	0.	*	1	1050	131	878.	*	1	1705	206	196.	*	1	2320	281	140.
1	0440	57	0.	*	1	1055	132	869.	*	1	1710	207	195.	*	1	2325	282	140.
1	0445	58	0.	*	1	1100	133	860.	*	1	1715	208	194.	*	1	2330	283	140.
1	0450	59	0.	*	1	1105	134	851.	*	1	1720	209	193.	*	1	2335	284	140.
1	0455	60	0.	*	1	1110	135	842.	*	1	1725	210	191.	*	1	2340	285	140.
1	0500	61	0.	*	1	1115	136	834.	*	1	1730	211	189.	*	1	2345	286	140.
1	0505	62	0.	*	1	1120	137	827.	*	1	1735	212	188.	*	1	2350	287	140.
1	0510	63	0.	*	1	1125	138	820.	*	1	1740	213	185.	*	1	2355	288	140.
1	0515	64	0.	*	1	1130	139	814.	*	1	1745	214	183.	*	2	0000	289	140.
1	0520	65	0.	*	1	1135	140	809.	*	1	1750	215	181.	*	2	0005	290	140.
1	0525	66	0.	*	1	1140	141	803.	*	1	1755	216	179.	*	2	0010	291	140.
1	0530	67	0.	*	1	1145	142	797.	*	1	1800	217	176.	*	2	0015	292	140.
1	0535	68	1.	*	1	1150	143	792.	*	1	1805	218	174.	*	2	0020	293	140.
1	0540	69	3.	*	1	1155	144	786.	*	1	1810	219	171.	*	2	0025	294	140.
1	0545	70	5.	*	1	1200	145	781.	*	1	1815	220	168.	*	2	0030	295	140.
1	0550	71	9.	*	1	1205	146	775.	*	1	1820	221	166.	*	2	0035	296	140.
1	0555	72	14.	*	1	1210	147	771.	*	1	1825	222	163.	*	2	0040	297	140.
1	0600	73	15.	*	1	1215	148	767.	*	1	1830	223	161.	*	2	0045	298	140.
1	0605	74	17.	*	1	1220	149	764.	*	1	1835	224	159.	*	2	0050	299	140.
1	0610	75	18.	*	1	1225	150	761.	*	1	1840	225	156.	*	2	0055	300	140.

PEAK FLOW	TIME		6-HR	24-HR	72-HR	24.92-HR
(CFS)	(HR)	(CFS)				
1819.	7.67		988.	347.	334.	334.
		(INCHES)	1.297	1.820	1.820	1.820
		(AC-FT)	490.	687.	687.	687.

CUMULATIVE AREA = 7.08 SQ MI

19 KO OUTPUT CONTROL VARIABLES
 IPRNT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

20 KK *****
 * RT-27 *
 * *

 ROUTE FLOW FROM DESIGN POINT 53 (DETENTION BASIN 3) TO DESIGN POINT 50

23 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

57 BI READ STATION SC2-29 HYDROGRAPH FROM UNIT 20

*** **

53 KK *****
 * DP51 *
 * *

58 KO OUTPUT CONTROL VARIABLES
 IPRNT 1 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

72 BI READ STATION DB-6 HYDROGRAPH FROM UNIT 20

*** **

* DP54 *

73 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

*** **

* DP20 *

358 KO OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

359 HC HYDROGRAPH COMBINATION
ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

HYDROGRAPH AT STATION DP20
SUM OF 3 HYDROGRAPHS

DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*	DA	MON	HRMN	ORD	FLOW	*
1	0000	1	0.	*	1	0615	76	7484.	*	1	1230	151	678.	*	1	1845	226	304.					
1	0005	2	0.	*	1	0620	77	7409.	*	1	1235	152	678.	*	1	1850	227	308.					
1	0010	3	0.	*	1	0625	78	6772.	*	1	1240	153	680.	*	1	1855	228	311.					
1	0015	4	0.	*	1	0630	79	5898.	*	1	1245	154	680.	*	1	1900	229	313.					
1	0020	5	0.	*	1	0635	80	5028.	*	1	1250	155	677.	*	1	1905	230	315.					
1	0025	6	0.	*	1	0640	81	4285.	*	1	1255	156	669.	*	1	1910	231	315.					
1	0030	7	0.	*	1	0645	82	3739.	*	1	1300	157	657.	*	1	1915	232	315.					
1	0035	8	0.	*	1	0650	83	3398.	*	1	1305	158	642.	*	1	1920	233	314.					
1	0040	9	0.	*	1	0655	84	3199.	*	1	1310	159	626.	*	1	1925	234	314.					
1	0045	10	0.	*	1	0700	85	3060.	*	1	1315	160	610.	*	1	1930	235	312.					
1	0050	11	0.	*	1	0705	86	2946.	*	1	1320	161	596.	*	1	1935	236	311.					
1	0055	12	0.	*	1	0710	87	2861.	*	1	1325	162	583.	*	1	1940	237	310.					
1	0100	13	0.	*	1	0715	88	2810.	*	1	1330	163	572.	*	1	1945	238	309.					
1	0105	14	0.	*	1	0720	89	2781.	*	1	1335	164	563.	*	1	1950	239	308.					
1	0110	15	0.	*	1	0725	90	2759.	*	1	1340	165	555.	*	1	1955	240	307.					
1	0115	16	0.	*	1	0730	91	2739.	*	1	1345	166	548.	*	1	2000	241	306.					
1	0120	17	0.	*	1	0735	92	2718.	*	1	1350	167	541.	*	1	2005	242	305.					
1	0125	18	0.	*	1	0740	93	2700.	*	1	1355	168	535.	*	1	2010	243	305.					
1	0130	19	0.	*	1	0745	94	2685.	*	1	1400	169	529.	*	1	2015	244	304.					
1	0135	20	0.	*	1	0750	95	2675.	*	1	1405	170	524.	*	1	2020	245	304.					
1	0140	21	0.	*	1	0755	96	2668.	*	1	1410	171	519.	*	1	2025	246	304.					
1	0145	22	0.	*	1	0800	97	2660.	*	1	1415	172	514.	*	1	2030	247	304.					
1	0150	23	0.	*	1	0805	98	2648.	*	1	1420	173	510.	*	1	2035	248	304.					
1	0155	24	0.	*	1	0810	99	2625.	*	1	1425	174	507.	*	1	2040	249	303.					
1	0200	25	0.	*	1	0815	100	2586.	*	1	1430	175	504.	*	1	2045	250	302.					
1	0205	26	0.	*	1	0820	101	2530.	*	1	1435	176	501.	*	1	2050	251	299.					
1	0210	27	0.	*	1	0825	102	2459.	*	1	1440	177	499.	*	1	2055	252	295.					
1	0215	28	0.	*	1	0830	103	2380.	*	1	1445	178	496.	*	1	2100	253	291.					
1	0220	29	0.	*	1	0835	104	2298.	*	1	1450	179	494.	*	1	2105	254	285.					
1	0225	30	0.	*	1	0840	105	2217.	*	1	1455	180	490.	*	1	2110	255	279.					
1	0230	31	0.	*	1	0845	106	2140.	*	1	1500	181	487.	*	1	2115	256	273.					
1	0235	32	0.	*	1	0850	107	2068.	*	1	1505	182	483.	*	1	2120	257	267.					
1	0240	33	0.	*	1	0855	108	1999.	*	1	1510	183	479.	*	1	2125	258	260.					
1	0245	34	0.	*	1	0900	109	1935.	*	1	1515	184	474.	*	1	2130	259	254.					
1	0250	35	0.	*	1	0905	110	1874.	*	1	1520	185	469.	*	1	2135	260	249.					
1	0255	36	0.	*	1	0910	111	1818.	*	1	1525	186	464.	*	1	2140	261	244.					
1	0300	37	0.	*	1	0915	112	1768.	*	1	1530	187	460.	*	1	2145	262	240.					
1	0305	38	0.	*	1	0920	113	1727.	*	1	1535	188	455.	*	1	2150	263	237.					
1	0310	39	0.	*	1	0925	114	1692.	*	1	1540	189	451.	*	1	2155	264	236.					
1	0315	40	0.	*	1	0930	115	1662.	*	1	1545	190	448.	*	1	2200	265	235.					
1	0320	41	0.	*	1	0935	116	1635.	*	1	1550	191	445.	*	1	2205	266	235.					
1	0325	42	0.	*	1	0940	117	1608.	*	1	1555	192	444.	*	1	2210	267	236.					
1	0330	43	0.	*	1	0945	118	1581.	*	1	1600	193	443.	*	1	2215	268	237.					
1	0335	44	0.	*	1	0950	119	1554.	*	1	1605	194	442.	*	1	2220	269	238.					
1	0340	45	0.	*	1	0955	120	1528.	*	1	1610	195	442.	*	1	2225	270	238.					
1	0345	46	0.	*	1	1000	121	1501.	*	1	1615	196	440.	*	1	2230	271	237.					
1	0350	47	0.	*	1	1005	122	1469.	*	1	1620	197	437.	*	1	2235	272	236.					
1	0355	48	0.	*	1	1010	123	1415.	*	1	1625	198	433.	*	1	2240	273	235.					
1	0400	49	0.	*	1	1015	124	1327.	*	1	1630	199	428.	*	1	2245	274	233.					
1	0405	50	0.	*	1	1020	125	1215.	*	1	1635	200	421.	*	1	2250	275	232.					
1	0410	51	0.	*	1	1025	126	1105.	*	1	1640	201	415.	*	1	2255	276	232.					
1	0415	52	0.	*	1	1030	127	1021.	*	1	1645	202	408.	*	1	2300	277	232.					
1	0420	53	0.	*	1	1035	128	965.	*	1	1650	203	402.	*	1	2305	278	233.					
1	0425	54	0.	*	1	1040	129	928.	*	1	1655	204	396.	*	1	2310	279	235.					
1	0430	55	0.	*	1	1045	130	899.	*	1	1700	205	391.	*	1	2315	280	236.					
1	0435	56	0.	*	1	1050	131	873.	*	1	1705	206	387.	*	1	2320	281	237.					
1	0440	57	0.	*	1	1055	132	849.	*	1	1710	207	382.	*	1	2325	282	237.					
1	0445	58	0.	*	1	1100	133	828.	*	1	1715	208	377.	*	1	2330	283	237.					
1	0450	59	0.	*	1	1105	134	809.	*	1	1720	209	371.	*	1	2335	284	236.					
1	0455	60	1.	*	1	1110	135	795.	*	1	1725	210	364.	*	1	2340	285	235.					
1	0500	61	2.	*	1	1115	136	788.	*	1	1730	211	356.	*	1	2345	286	233.					
1	0505	62	3.	*	1	1120	137	789.	*	1	1735	212	347.	*	1	2350	287	232.					
1	0510	63	3.	*	1	1125	138	799.	*	1	1740	213	338.	*	1	2355	288	232.					

1	0515	64	5.	*	1	1130	139	815.	*	1	1745	214	329.	*	2	0000	289	232.
1	0520	65	7.	*	1	1135	140	832.	*	1	1750	215	321.	*	2	0005	290	233.
1	0525	66	12.	*	1	1140	141	847.	*	1	1755	216	313.	*	2	0010	291	235.
1	0530	67	18.	*	1	1145	142	852.	*	1	1800	217	306.	*	2	0015	292	236.
1	0535	68	31.	*	1	1150	143	846.	*	1	1805	218	300.	*	2	0020	293	237.
1	0540	69	94.	*	1	1155	144	828.	*	1	1810	219	295.	*	2	0025	294	238.
1	0545	70	297.	*	1	1200	145	802.	*	1	1815	220	292.	*	2	0030	295	237.
1	0550	71	829.	*	1	1205	146	772.	*	1	1820	221	291.	*	2	0035	296	236.
1	0555	72	1748.	*	1	1210	147	741.	*	1	1825	222	291.	*	2	0040	297	234.
1	0600	73	3126.	*	1	1215	148	715.	*	1	1830	223	293.	*	2	0045	298	230.
1	0605	74	5008.	*	1	1220	149	696.	*	1	1835	224	296.	*	2	0050	299	226.
1	0610	75	6693.	*	1	1225	150	684.	*	1	1840	225	300.	*	2	0055	300	221.

PEAK FLOW {CFS}	TIME {HR}	6-HR (CFS)	24-HR (INCHES)	72-HR (AC-FT)	24.92-HR (INCHES)
7484.	6.25	2343.	1.561	798.	769.
		1162.	1583.	1583.	1583.

CUMULATIVE AREA = 13.96 SQ MI

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370 KK

RT-63

372 KO
OUTPUT CONTROL VARIABLES
IPRNT 1 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

373 RK
KINEMATIC WAVE STREAM ROUTING
L 1290. CHANNEL LENGTH
S 0.0070 SLOPE
N 0.035 CHANNEL ROUGHNESS COEFFICIENT
CA 0.00 CONTRIBUTING AREA
SHAPE TRAP CHANNEL SHAPE
WD 100.00 BOTTOM WIDTH OR DIAMETER
Z 3.00 SIDE SLOPE
NDXMIN 2 MINIMUM NUMBER OF DX INTERVALS

COMPUTED KINEMATIC PARAMETERS
VARIABLE TIME STEP
(DT SHOWN IS A MINIMUM)

ELEMENT	ALPHA	M	DT (MIN)	DX (FT)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	MAXIMUM CELERITY (FPS)
MAIN	0.21	1.60	0.59	430.00	4062.91	401.45	2.11	13.62

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1572E+04 EXCESS=0.0000E+00 OUTFLOW=0.1571E+04 BASIN STORAGE=0.1474E+01 PERCENT ERROR= 0.0

INTERPOLATED TO SPECIFIED COMPUTATION INTERVAL

MAIN	0.21	1.60	5.00	4061.78	405.00	2.11
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HYDROGRAPH AT STATION RT-63

DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW	DA	MON	HRMN	ORD	FLOW
1	0000	1	0.	*	1	0615	76	2515.	*	1	1230	151	744.	*	1	1845	226	300.	
1	0005	2	0.	*	1	0620	77	3028.	*	1	1235	152	729.	*	1	1850	227	301.	
1	0010	3	0.	*	1	0625	78	3450.	*	1	1240	153	717.	*	1	1855	228	302.	
1	0015	4	0.	*	1	0630	79	3782.	*	1	1245	154	708.	*	1	1900	229	304.	
1	0020	5	0.	*	1	0635	80	3989.	*	1	1250	155	702.	*	1	1905	230	306.	
1	0025	6	0.	*	1	0640	81	4055.	*	1	1255	156	695.	*	1	1910	231	308.	
1	0030	7	0.	*	1	0645	82	4062.	*	1	1300	157	689.	*	1	1915	232	310.	
1	0035	8	0.	*	1	0650	83	4042.	*	1	1305	158	681.	*	1	1920	233	311.	
1	0040	9	0.	*	1	0655	84	3972.	*	1	1310	159	671.	*	1	1925	234	312.	
1	0045	10	0.	*	1	0700	85	3883.	*	1	1315	160	660.	*	1	1930	235	312.	
1	0050	11	0.	*	1	0705	86	3789.	*	1	1320	161	648.	*	1	1935	236	312.	
1	0055	12	0.	*	1	0710	87	3694.	*	1	1325	162	636.	*	1	1940	237	312.	
1	0100	13	0.	*	1	0715	88	3601.	*	1	1330	163	623.	*	1	1945	238	312.	
1	0105	14	0.	*	1	0720	89	3513.	*	1	1335	164	611.	*	1	1950	239	311.	
1	0110	15	0.	*	1	0725	90	3432.	*	1	1340	165	600.	*	1	1955	240	310.	
1	0115	16	0.	*	1	0730	91	3359.	*	1	1345	166	589.	*	1	2000	241	309.	
1	0120	17	0.	*	1	0735	92	3298.	*	1	1350	167	580.	*	1	2005	242	309.	
1	0125	18	0.	*	1	0740	93	3242.	*	1	1355	168	571.	*	1	2010	243	308.	
1	0130	19	0.	*	1	0745	94	3190.	*	1	1400	169	562.	*	1	2015	244	307.	
1	0135	20	0.	*	1	0750	95	3141.	*	1	1405	170	555.	*	1	2020	245	306.	
1	0140	21	0.	*	1	0755	96	3096.	*	1	1410	171	547.	*	1	2025	246	306.	
1	0145	22	0.	*	1	0800	97	3055.	*	1	1415	172	541.	*	1	2030	247	305.	
1	0150	23	0.	*	1	0805	98	3017.	*	1	1420	173	534.	*	1	2035	248	305.	
1	0155	24	0.	*	1	0810	99	2981.	*	1	1425	174	529.	*	1	2040	249	305.	
1	0200	25	0.	*	1	0815	100	2945.	*	1	1430	175	523.	*	1	2045	250	304.	

1	0205	26	0.	*	1	0820	101	2909.	*	1	1435	176	519.	*	1	2050	251	304.
1	0210	27	0.	*	1	0825	102	2871.	*	1	1440	177	515.	*	1	2055	252	303.
1	0215	28	0.	*	1	0830	103	2830.	*	1	1445	178	511.	*	1	2100	253	301.
1	0220	29	0.	*	1	0835	104	2782.	*	1	1450	179	507.	*	1	2105	254	299.
1	0225	30	0.	*	1	0840	105	2731.	*	1	1455	180	504.	*	1	2110	255	295.
1	0230	31	0.	*	1	0845	106	2676.	*	1	1500	181	501.	*	1	2115	256	292.
1	0235	32	0.	*	1	0850	107	2619.	*	1	1505	182	498.	*	1	2120	257	288.
1	0240	33	0.	*	1	0855	108	2561.	*	1	1510	183	494.	*	1	2125	258	283.
1	0245	34	0.	*	1	0900	109	2502.	*	1	1515	184	490.	*	1	2130	259	278.
1	0250	35	0.	*	1	0905	110	2442.	*	1	1520	185	487.	*	1	2135	260	272.
1	0255	36	0.	*	1	0910	111	2383.	*	1	1525	186	482.	*	1	2140	261	267.
1	0300	37	0.	*	1	0915	112	2323.	*	1	1530	187	478.	*	1	2145	262	262.
1	0305	38	0.	*	1	0920	113	2242.	*	1	1535	188	474.	*	1	2150	263	257.
1	0310	39	0.	*	1	0925	114	2154.	*	1	1540	189	469.	*	1	2155	264	252.
1	0315	40	0.	*	1	0930	115	2075.	*	1	1545	190	465.	*	1	2200	265	248.
1	0320	41	0.	*	1	0935	116	2004.	*	1	1550	191	461.	*	1	2205	266	245.
1	0325	42	0.	*	1	0940	117	1940.	*	1	1555	192	457.	*	1	2210	267	243.
1	0330	43	0.	*	1	0945	118	1882.	*	1	1600	193	454.	*	1	2215	268	241.
1	0335	44	0.	*	1	0950	119	1830.	*	1	1605	194	452.	*	1	2220	269	240.
1	0340	45	0.	*	1	0955	120	1782.	*	1	1610	195	449.	*	1	2225	270	240.
1	0345	46	0.	*	1	1000	121	1738.	*	1	1615	196	448.	*	1	2230	271	239.
1	0350	47	0.	*	1	1005	122	1696.	*	1	1620	197	446.	*	1	2235	272	239.
1	0355	48	0.	*	1	1010	123	1655.	*	1	1625	198	444.	*	1	2240	273	238.
1	0400	49	0.	*	1	1015	124	1611.	*	1	1630	199	441.	*	1	2245	274	237.
1	0405	50	0.	*	1	1020	125	1558.	*	1	1635	200	438.	*	1	2250	275	237.
1	0410	51	0.	*	1	1025	126	1502.	*	1	1640	201	434.	*	1	2255	276	236.
1	0415	52	0.	*	1	1030	127	1442.	*	1	1645	202	429.	*	1	2300	277	235.
1	0420	53	0.	*	1	1035	128	1380.	*	1	1650	203	424.	*	1	2305	278	234.
1	0425	54	0.	*	1	1040	129	1320.	*	1	1655	204	419.	*	1	2310	279	234.
1	0430	55	0.	*	1	1045	130	1263.	*	1	1700	205	414.	*	1	2315	280	234.
1	0435	56	0.	*	1	1050	131	1209.	*	1	1705	206	408.	*	1	2320	281	235.
1	0440	57	0.	*	1	1055	132	1160.	*	1	1710	207	403.	*	1	2325	282	235.
1	0445	58	0.	*	1	1100	133	1115.	*	1	1715	208	398.	*	1	2330	283	236.
1	0450	59	0.	*	1	1105	134	1073.	*	1	1720	209	393.	*	1	2335	284	236.
1	0455	60	0.	*	1	1110	135	1035.	*	1	1725	210	388.	*	1	2340	285	236.
1	0500	61	0.	*	1	1115	136	1000.	*	1	1730	211	382.	*	1	2345	286	236.
1	0505	62	0.	*	1	1120	137	970.	*	1	1735	212	376.	*	1	2350	287	235.
1	0510	63	0.	*	1	1125	138	944.	*	1	1740	213	369.	*	1	2355	288	235.
1	0515	64	0.	*	1	1130	139	918.	*	1	1745	214	362.	*	2	0000	289	234.
1	0520	65	0.	*	1	1135	140	896.	*	1	1750	215	354.	*	2	0005	290	234.
1	0525	66	1.	*	1	1140	141	881.	*	1	1755	216	347.	*	2	0010	291	234.
1	0530	67	3.	*	1	1145	142	873.	*	1	1800	217	339.	*	2	0015	292	234.
1	0535	68	7.	*	1	1150	143	867.	*	1	1805	218	331.	*	2	0020	293	234.
1	0540	69	15.	*	1	1155	144	861.	*	1	1810	219	324.	*	2	0025	294	235.
1	0545	70	45.	*	1	1200	145	852.	*	1	1815	220	317.	*	2	0030	295	236.
1	0550	71	84.	*	1	1205	146	839.	*	1	1820	221	311.	*	2	0035	296	236.
1	0555	72	161.	*	1	1210	147	823.	*	1	1825	222	307.	*	2	0040	297	236.
1	0600	73	574.	*	1	1215	148	803.	*	1	1830	223	303.	*	2	0045	298	235.
1	0605	74	1143.	*	1	1220	149	782.	*	1	1835	224	301.	*	2	0050	299	234.
1	0610	75	1849.	*	1	1225	150	762.	*	1	1840	225	300.	*	2	0055	300	232.

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	6-HR	24-HR	72-HR	24.92-HR
4062.	6.75	2310.	792.	763.	763.
		(INCHES)	1.539	2.111	2.111
		(AC-FT)	1146.	1571.	1571.
CUMULATIVE AREA =		13.96 SQ MI			

*** FDKRUT - NEWTON RAPHSON FAILEDFIXED POINT ITERATION USED - ITERATION= 1

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543 KK  * DBSC1 *
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545 KO  OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

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	RT-39	290.	6.08	36.	11.	11.	0.18
HYDROGRAPH AT	SC2-13	246.	6.17	32.	10.	10.	0.16
4 COMBINED AT	DP85	2885.	6.25	1572.	552.	532.	10.20
ROUTED TO	RT-42	2833.	6.25	1571.	551.	531.	10.20
HYDROGRAPH AT	SC2-14	264.	6.33	47.	15.	14.	0.26
HYDROGRAPH AT	SC2-16	269.	6.17	38.	12.	11.	0.21
3 COMBINED AT	DP37	3344.	6.25	1653.	577.	556.	10.68
ROUTED TO	RT-43	3302.	6.33	1650.	576.	555.	10.68
HYDROGRAPH AT	SC2-12	182.	6.17	25.	8.	8.	0.18
ROUTED TO	RT-46	175.	6.25	25.	8.	8.	0.18
HYDROGRAPH AT	SC2-8	390.	6.33	69.	21.	21.	0.35
2 COMBINED AT	DP33	573.	6.25	93.	29.	28.	0.53
ROUTED TO	RT-45	566.	6.25	93.	29.	28.	0.53
2 COMBINED AT	DP36	3857.	6.33	1740.	605.	583.	11.21
ROUTED TO	RT-44	3842.	6.33	1740.	605.	582.	11.21
HYDROGRAPH AT	SC2-11	340.	6.08	42.	13.	12.	0.21
ROUTED TO	RT-52	338.	6.17	42.	13.	12.	0.21
HYDROGRAPH AT	SC2-19	350.	6.17	47.	14.	14.	0.18
ROUTED TO	RT-51	347.	6.17	47.	14.	14.	0.18
HYDROGRAPH AT	SC2-18	412.	6.17	55.	17.	16.	0.20
3 COMBINED AT	DP39	1096.	6.17	144.	44.	42.	0.59
ROUTED TO	RT-49	1085.	6.17	144.	44.	42.	0.59
HYDROGRAPH AT	SC2-17	226.	6.17	32.	10.	9.	0.16
ROUTED TO	RT-50	219.	6.25	32.	10.	9.	0.16
HYDROGRAPH AT	SC2-29	360.	6.08	42.	13.	12.	0.15
3 COMBINED AT	DP34	1611.	6.17	217.	66.	64.	0.90
ROUTED TO	RT-48	1594.	6.17	217.	66.	64.	0.90
HYDROGRAPH AT	SC2-10	401.	6.25	64.	20.	19.	0.33
2 COMBINED AT	DP32	1972.	6.17	281.	86.	83.	1.23
ROUTED TO	RT-47	1953.	6.17	281.	86.	83.	1.23
HYDROGRAPH AT	SC2-9	158.	6.42	31.	10.	10.	0.21
3 COMBINED AT	DP35	5718.	6.25	2037.	701.	675.	12.64
ROUTED TO	RT-53	5663.	6.33	2037.	701.	675.	12.64
HYDROGRAPH AT	SC2-7	203.	6.17	29.	9.	9.	0.14
2 COMBINED AT	DP29	5840.	6.25	2065.	710.	683.	12.79
ROUTED TO	RT-54	5829.	6.33	2065.	709.	683.	12.79

HYDROGRAPH AT	SC2-6	271.	6.08	33.	10.	10.	0.15		
ROUTED TO	RT-55	261.	6.17	33.	10.	10.	0.15		
HYDROGRAPH AT	SC2-2	271.	6.17	36.	11.	11.	0.17		
3 COMBINED AT	DP28	6184.	6.33	2129.	730.	703.	13.10		
ROUTED TO	RT-56	6183.	6.33	2130.	730.	703.	13.10		
HYDROGRAPH AT	SC2-5	388.	6.08	51.	15.	15.	0.16		
ROUTED TO	RT-58	381.	6.08	51.	15.	15.	0.16		
HYDROGRAPH AT	SC2-0	162.	6.17	23.	7.	7.	0.15		
ROUTED TO	RT-57	158.	6.25	23.	7.	7.	0.15		
3 COMBINED AT	DP21	6586.	6.33	2199.	753.	725.	13.41		
ROUTED TO	RT-59	6584.	6.33	2199.	753.	725.	13.41		
HYDROGRAPH AT	SC2-4	651.	6.17	90.	27.	26.	0.28		
ROUTED TO	RT-61	646.	6.17	90.	27.	26.	0.28		
HYDROGRAPH AT	SC2-3	190.	6.08	23.	7.	6.	0.07		
2 COMBINED AT	DP22	807.	6.17	113.	34.	32.	0.36		
ROUTED TO	RT-60	805.	6.17	113.	34.	32.	0.36		
HYDROGRAPH AT	SC2-1	287.	6.17	38.	12.	11.	0.19		
3 COMBINED AT	DP20	7484.	6.25	2343.	798.	769.	13.96		
ROUTED TO	DBSC2	4063.	6.67	2312.	793.	764.	13.96	6510.13	6.67
ROUTED TO	RT-63	4062.	6.75	2310.	792.	763.	13.96		
HYDROGRAPH AT	SC1-10	348.	6.25	55.	17.	17.	0.29		
2 COMBINED AT	DP18	4208.	6.67	2362.	809.	779.	14.24		
ROUTED TO	RT-66	4206.	6.67	2362.	809.	779.	14.24		
HYDROGRAPH AT	SC1-13	273.	6.08	35.	11.	10.	0.13		
ROUTED TO	RT-64	269.	6.17	35.	11.	10.	0.13		
HYDROGRAPH AT	SC1-14	265.	6.08	35.	10.	10.	0.11		
2 COMBINED AT	DP19	533.	6.08	70.	21.	20.	0.24		
ROUTED TO	RT-65	530.	6.08	70.	21.	20.	0.24		
2 COMBINED AT	DP17	4321.	6.67	2419.	830.	799.	14.48		
ROUTED TO	RT-67	4321.	6.67	2419.	829.	799.	14.48		
HYDROGRAPH AT	SC1-9	354.	6.08	44.	14.	13.	0.24		
ROUTED TO	DBSR29	108.	6.50	43.	14.	13.	0.24	106.21	6.50
ROUTED TO	RT-70	108.	6.50	43.	14.	13.	0.24		
HYDROGRAPH AT	SC1-8	303.	6.17	43.	13.	13.	0.21		
ROUTED TO	RT-71	301.	6.17	43.	13.	13.	0.21		

2 COMBINED AT	DP14	374.	6.25	85.	27.	26.	0.45		
ROUTED TO	RT-69	373.	6.25	85.	27.	26.	0.45		
HYDROGRAPH AT	SC1-7	167.	6.08	20.	6.	6.	0.10		
2 COMBINED AT	DP12	519.	6.17	105.	33.	32.	0.55		
ROUTED TO	RT-68	517.	6.17	105.	33.	32.	0.55		
2 COMBINED AT	DP13	4565.	6.58	2520.	862.	831.	15.03		
ROUTED TO	RT-72	4555.	6.67	2518.	862.	830.	15.03		
HYDROGRAPH AT	SC1-6	288.	6.08	34.	10.	10.	0.16		
ROUTED TO	DBSR15	140.	6.33	34.	10.	10.	0.16	105.18	6.33
ROUTED TO	RT-74	139.	6.33	34.	10.	10.	0.16		
HYDROGRAPH AT	SC1-5	249.	6.17	34.	11.	10.	0.17		
2 COMBINED AT	DP11	378.	6.17	68.	21.	20.	0.33		
ROUTED TO	RT-73	377.	6.17	68.	21.	20.	0.33		
2 COMBINED AT	DP10	4768.	6.58	2584.	883.	850.	15.36		
ROUTED TO	RT-75	4758.	6.67	2583.	881.	849.	15.36		
HYDROGRAPH AT	SC1-2	192.	6.17	27.	8.	8.	0.14		
ROUTED TO	RT-76	191.	6.17	27.	8.	8.	0.14		
2 COMBINED AT	DP9	4817.	6.67	2607.	890.	857.	15.50		
ROUTED TO	RT-78	4817.	6.67	2607.	889.	856.	15.50		
HYDROGRAPH AT	SC1-12	392.	6.25	60.	18.	17.	0.25		
ROUTED TO	RT-81	391.	6.25	60.	18.	17.	0.25		
HYDROGRAPH AT	SC1-11	379.	6.08	51.	15.	15.	0.16		
2 COMBINED AT	DP8	744.	6.17	110.	33.	32.	0.41		
ROUTED TO	RT-77	736.	6.25	109.	33.	32.	0.41		
HYDROGRAPH AT	SC1-4	248.	6.08	32.	10.	9.	0.10		
ROUTED TO	RT-80	247.	6.08	32.	10.	9.	0.10		
HYDROGRAPH AT	SC1-1	164.	6.17	22.	7.	6.	0.11		
ROUTED TO	RT-79	164.	6.17	22.	7.	6.	0.11		
HYDROGRAPH AT	SC1-3	184.	6.33	36.	12.	12.	0.35		
5 COMBINED AT	DP4	5537.	6.50	2793.	950.	915.	16.47		
ROUTED TO	DBSC1	3815.	7.50	2721.	933.	899.	16.47	6408.12	7.50

10 YEAR OUTPUT SUMMARY

AREA UPSTREAM OF WOODMEN 10 YEAR RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB82	60.	6.20	9.	3.	3.	0.18		
ROUTED TO	RT-4	59.	6.27	9.	3.	3.	0.18		
HYDROGRAPH AT	SB74	56.	6.27	9.	4.	4.	0.19		
2 COMBINED AT	DP74	114.	6.27	18.	7.	7.	0.37		
ROUTED TO	RT-3	114.	6.33	18.	7.	7.	0.37		
HYDROGRAPH AT	SB80	60.	6.27	11.	4.	4.	0.23		
ROUTED TO	RT-2	60.	6.40	11.	4.	4.	0.23		
HYDROGRAPH AT	SB81	100.	6.33	20.	8.	8.	0.41		
ROUTED TO	RT-1	99.	6.47	20.	8.	8.	0.41		
HYDROGRAPH AT	SB73	44.	6.20	7.	3.	3.	0.14		
HYDROGRAPH AT	SB75	34.	6.27	6.	2.	2.	0.12		
HYDROGRAPH AT	SB76	36.	6.27	6.	3.	3.	0.13		
6 COMBINED AT	DP75	365.	6.33	67.	26.	26.	1.41		
ROUTED TO	RT-5	362.	6.40	67.	26.	26.	1.41		
HYDROGRAPH AT	SB79	91.	6.20	14.	6.	6.	0.29		
ROUTED TO	RT-7	89.	6.27	14.	5.	5.	0.29		
HYDROGRAPH AT	SB78	73.	6.20	12.	5.	5.	0.24		
2 COMBINED AT	DP78	162.	6.27	26.	10.	10.	0.54		
ROUTED TO	RT-6	159.	6.33	26.	10.	10.	0.54		
HYDROGRAPH AT	SB77	85.	6.27	14.	5.	5.	0.27		
HYDROGRAPH AT	SB88	41.	6.20	7.	3.	3.	0.14		
4 COMBINED AT	DP77	626.	6.33	113.	44.	44.	2.36		
ROUTED TO	RT-8	625.	6.47	113.	44.	44.	2.36		
HYDROGRAPH AT	SC3-20	111.	6.20	16.	6.	6.	0.31		
2 COMBINED AT	DP71	679.	6.47	127.	50.	50.	2.66		
ROUTED TO	RT-9	671.	6.47	127.	50.	50.	2.66		
HYDROGRAPH AT	SB3-18	195.	6.13	24.	9.	9.	0.27		
2 COMBINED AT	DP70	735.	6.47	149.	59.	59.	2.93		
ROUTED TO	RT-11	721.	6.47	149.	58.	58.	2.93		
HYDROGRAPH AT	SC3-19	235.	6.07	27.	10.	10.	0.30		
ROUTED TO	RT-10	232.	6.13	27.	10.	10.	0.30		
HYDROGRAPH AT	SC3-17	202.	6.07	22.	8.	8.	0.18		

3 COMBINED AT	DP69	827.	6.47	197.	76.	76.	3.41		
ROUTED TO	DBSRMN	571.	6.73	194.	75.	75.	3.41	99.96	6.73
ROUTED TO	RT-12	570.	6.80	194.	75.	75.	3.41		
HYDROGRAPH AT	SC3-15	261.	6.07	30.	11.	11.	0.21		
ROUTED TO	RT-10A	257.	6.13	30.	11.	11.	0.21		
2 COMBINED AT	DP87	600.	6.80	222.	86.	86.	3.62		
ROUTED TO	RT-13	600.	6.80	222.	85.	85.	3.62		
HYDROGRAPH AT	SC3-14	432.	6.07	52.	18.	18.	0.34		
HYDROGRAPH AT	SC3-16	354.	6.13	43.	15.	15.	0.30		
ROUTED TO	RT-13A	350.	6.33	43.	15.	15.	0.30		
3 COMBINED AT	DP68	1100.	6.27	315.	119.	119.	4.26		
ROUTED TO	DBSRMS	762.	6.80	312.	118.	118.	4.26	11.19	6.80
ROUTED TO	RT-17	761.	6.80	312.	118.	118.	4.26		
HYDROGRAPH AT	SC3-13	127.	6.13	18.	7.	7.	0.34		
ROUTED TO	RT-19	127.	6.20	18.	7.	7.	0.34		
HYDROGRAPH AT	SC3-12	246.	6.07	28.	10.	10.	0.21		
2 COMBINED AT	DP66	356.	6.13	46.	17.	17.	0.54		
ROUTED TO	RT-20	346.	6.47	45.	17.	17.	0.54		
HYDROGRAPH AT	SC3-11	332.	6.13	43.	15.	15.	0.28		
2 COMBINED AT	DP63A	507.	6.40	88.	32.	32.	0.83		
2 COMBINED AT	DP63	1183.	6.47	398.	150.	150.	5.09		
ROUTED TO	RT-17A	1176.	6.47	398.	149.	149.	5.09		
HYDROGRAPH AT	SC3-10	26.	6.13	3.	1.	1.	0.06		
HYDROGRAPH AT	SC3-9	169.	6.33	32.	12.	12.	0.50		
ROUTED TO	RT-14	168.	6.40	32.	12.	12.	0.50		
HYDROGRAPH AT	SC3-8	110.	6.27	19.	7.	7.	0.23		
2 COMBINED AT	DP65	272.	6.33	50.	19.	19.	0.73		
ROUTED TO	RT-15	269.	6.40	50.	19.	19.	0.73		
HYDROGRAPH AT	SC3-7	218.	6.07	24.	8.	8.	0.16		
ROUTED TO	RT-15A	214.	6.07	24.	8.	8.	0.16		
2 COMBINED AT	DP64	371.	6.20	74.	27.	27.	0.88		
ROUTED TO	RT-16	370.	6.20	74.	27.	27.	0.88		
HYDROGRAPH AT	SC3-6	300.	6.07	31.	11.	11.	0.21		
2 COMBINED AT	DP62	608.	6.07	105.	38.	38.	1.09		
ROUTED TO	DBSRW	148.	6.93	97.	38.	38.	1.09	76.12	6.93

3 COMBINED AT	DP61	1322.	6.47	498.	188.	188.	6.24		
ROUTED TO	RT-17B	1320.	6.53	498.	188.	188.	6.24		
HYDROGRAPH AT	SB61	35.	6.27	6.	2.	2.	0.12		
ROUTED TO	RT-20	35.	6.33	6.	2.	2.	0.12		
HYDROGRAPH AT	SC3-5	211.	6.07	21.	8.	8.	0.16		
3 COMBINED AT	DP60	1373.	6.53	522.	198.	198.	6.51		
ROUTED TO	RT-18	1361.	6.53	521.	197.	197.	6.51		
HYDROGRAPH AT	SC3-4	141.	6.00	14.	5.	5.	0.08		
ROUTED TO	RT-21	138.	6.00	14.	5.	5.	0.08		
HYDROGRAPH AT	SC3-1	174.	6.07	17.	6.	6.	0.17		
HYDROGRAPH AT	SC3-3	298.	6.07	33.	12.	12.	0.24		
ROUTED TO	DBSC33	146.	6.27	32.	11.	11.	0.24	4.92	6.27
ROUTED TO	RT-21B	144.	6.33	32.	11.	11.	0.24		
HYDROGRAPH AT	SC3-2	111.	6.07	12.	4.	4.	0.08		
5 COMBINED AT	DP53	1534.	6.53	593.	224.	224.	7.08		
ROUTED TO	DB-3	959.	7.73	399.	163.	163.	7.08	84.90	6.73
HYDROGRAPH AT	SC6-1	139.	6.13	17.	6.	6.	0.11		
ROUTED TO	DBSC61	50.	6.47	17.	6.	6.	0.11	3.32	6.47
ROUTED TO	RT-23A	50.	6.53	16.	6.	6.	0.11		
HYDROGRAPH AT	SC6-2	241.	6.07	28.	10.	10.	0.24		
HYDROGRAPH AT	SC6-3	205.	6.07	23.	8.	8.	0.17		
ROUTED TO	DBSC63	55.	6.40	21.	8.	8.	0.17	4.61	6.40
ROUTED TO	RT-23B	55.	6.47	21.	8.	8.	0.17		
3 COMBINED AT	DP54A	294.	6.13	66.	24.	24.	0.52		
ROUTED TO	RT-23C	291.	6.13	66.	24.	24.	0.52		
HYDROGRAPH AT	SC6-4	201.	6.00	22.	8.	8.	0.10		
ROUTED TO	DBSC64	36.	6.40	19.	7.	7.	0.10	3.95	6.40
ROUTED TO	RT-24	36.	6.40	19.	7.	7.	0.10		
HYDROGRAPH AT	SC6-5	337.	6.07	37.	13.	13.	0.28		
HYDROGRAPH AT	SC6-6	110.	6.00	12.	4.	4.	0.06		
4 COMBINED AT	DP54	740.	6.07	133.	48.	48.	0.96		
ROUTED TO	DB-6	206.	6.73	124.	48.	48.	0.96	71.90	6.13
HYDROGRAPH AT	SC2-29	67.	6.00	7.	3.	3.	0.03		

AREA DOWNSTREAM OF WOODMEN 10 YEAR RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	DP53	958.	7.75	398.	154.	148.	7.08		
ROUTED TO	RT-27	953.	7.83	399.	154.	148.	7.08		
HYDROGRAPH AT	SC2-24	106.	6.08	12.	4.	3.	0.07		
ROUTED TO	RT-29	106.	6.08	12.	4.	3.	0.07		
HYDROGRAPH AT	SC2-27	130.	6.08	15.	5.	4.	0.08		
3 COMBINED AT	DP50	964.	7.83	404.	162.	156.	7.23		
ROUTED TO	RT-31	946.	7.83	405.	162.	156.	7.23		
HYDROGRAPH AT	DP51	67.	6.00	7.	2.	2.	0.03		
ROUTED TO	RT-30	65.	6.08	7.	2.	2.	0.03		
2 COMBINED AT	DP49	949.	7.83	406.	164.	158.	7.26		
ROUTED TO	RT-32	916.	7.92	406.	163.	157.	7.26		
HYDROGRAPH AT	DP54	206.	6.75	124.	43.	42.	0.96		
ROUTED TO	RT-26	206.	6.83	124.	43.	41.	0.96		
HYDROGRAPH AT	SC2-26	104.	6.17	15.	5.	5.	0.15		
ROUTED TO	RT-34	103.	6.25	15.	5.	5.	0.15		
HYDROGRAPH AT	SC2-25	233.	6.33	41.	13.	13.	0.42		
3 COMBINED AT	DP47	482.	6.33	179.	61.	59.	1.53		
ROUTED TO	RT-33	478.	6.33	179.	61.	59.	1.53		
HYDROGRAPH AT	SC2-28	53.	6.42	12.	4.	4.	0.27		
HYDROGRAPH AT	SC2-21	117.	6.25	19.	6.	6.	0.19		
4 COMBINED AT	DP48	1141.	7.92	608.	235.	226.	9.26		
ROUTED TO	RT-37	1126.	7.92	608.	234.	226.	9.26		
HYDROGRAPH AT	SC2-23	130.	6.17	20.	6.	6.	0.20		
ROUTED TO	RT-35	130.	6.25	20.	6.	6.	0.20		
HYDROGRAPH AT	SC2-22	68.	6.08	8.	3.	2.	0.08		
2 COMBINED AT	DP46	192.	6.17	28.	9.	9.	0.28		
ROUTED TO	RT-36	187.	6.17	28.	9.	9.	0.28		
2 COMBINED AT	DP42	1141.	7.92	634.	243.	234.	9.53		
ROUTED TO	RT-38	1112.	7.92	634.	243.	234.	9.53		
HYDROGRAPH AT	SC2-20	200.	6.25	32.	10.	10.	0.32		
ROUTED TO	RT-41	198.	6.25	32.	10.	10.	0.32		
HYDROGRAPH AT	SC2-15	143.	6.08	18.	6.	6.	0.18		

ROUTED TO	RT-39	142.	6.17	18.	6.	6.	0.18
HYDROGRAPH AT	SC2-13	121.	6.17	16.	5.	5.	0.16
4 COMBINED AT	DP85	1437.	6.25	696.	264.	254.	10.20
ROUTED TO	RT-42	1425.	6.33	696.	263.	253.	10.20
HYDROGRAPH AT	SC2-14	120.	6.33	22.	7.	7.	0.26
HYDROGRAPH AT	SC2-16	123.	6.17	18.	6.	6.	0.21
3 COMBINED AT	DP37	1648.	6.33	734.	276.	266.	10.68
ROUTED TO	RT-43	1622.	6.33	732.	275.	265.	10.68
HYDROGRAPH AT	SC2-12	73.	6.17	11.	4.	3.	0.18
ROUTED TO	RT-46	72.	6.25	11.	4.	3.	0.18
HYDROGRAPH AT	SC2-8	194.	6.33	35.	11.	11.	0.35
2 COMBINED AT	DP33	260.	6.33	45.	15.	14.	0.53
ROUTED TO	RT-45	260.	6.33	45.	15.	14.	0.53
2 COMBINED AT	DP36	1883.	6.33	775.	290.	279.	11.21
ROUTED TO	RT-44	1858.	6.42	775.	289.	279.	11.21
HYDROGRAPH AT	SC2-11	165.	6.08	21.	7.	6.	0.21
ROUTED TO	RT-52	164.	6.25	21.	7.	6.	0.21
HYDROGRAPH AT	SC2-19	195.	6.17	26.	8.	8.	0.18
ROUTED TO	RT-51	192.	6.17	26.	8.	8.	0.18
HYDROGRAPH AT	SC2-18	238.	6.17	32.	10.	9.	0.20
3 COMBINED AT	DP39	592.	6.17	79.	25.	24.	0.59
ROUTED TO	RT-49	579.	6.17	79.	25.	24.	0.59
HYDROGRAPH AT	SC2-17	109.	6.17	16.	5.	5.	0.16
ROUTED TO	RT-50	108.	6.25	16.	5.	5.	0.16
HYDROGRAPH AT	SC2-29	210.	6.08	24.	7.	7.	0.15
3 COMBINED AT	DP34	863.	6.17	119.	37.	36.	0.90
ROUTED TO	RT-48	842.	6.17	119.	37.	36.	0.90
HYDROGRAPH AT	SC2-10	191.	6.25	32.	10.	10.	0.33
2 COMBINED AT	DP32	1016.	6.17	151.	47.	45.	1.23
ROUTED TO	RT-47	1014.	6.25	151.	47.	45.	1.23
HYDROGRAPH AT	SC2-9	66.	6.42	14.	5.	5.	0.21
3 COMBINED AT	DP35	2810.	6.33	927.	341.	329.	12.64
ROUTED TO	RT-53	2784.	6.33	927.	341.	329.	12.64
HYDROGRAPH AT	SC2-7	98.	6.17	14.	5.	4.	0.14
2 COMBINED AT	DP29	2868.	6.33	941.	346.	333.	12.79
ROUTED TO	RT-54	2815.	6.33	941.	345.	333.	12.79

HYDROGRAPH AT	SC2-6	139.	6.08	17.	5.	5.	0.15		
ROUTED TO	RT-55	137.	6.17	17.	5.	5.	0.15		
HYDROGRAPH AT	SC2-2	139.	6.17	18.	6.	6.	0.17		
3 COMBINED AT	DP28	3007.	6.33	974.	357.	344.	13.10		
ROUTED TO	RT-56	2960.	6.33	974.	356.	343.	13.10		
HYDROGRAPH AT	SC2-5	237.	6.08	31.	9.	9.	0.16		
ROUTED TO	RT-58	235.	6.17	31.	9.	9.	0.16		
HYDROGRAPH AT	SC2-0	69.	6.25	11.	4.	3.	0.15		
ROUTED TO	RT-57	69.	6.25	11.	4.	3.	0.15		
3 COMBINED AT	DP21	3189.	6.33	1012.	369.	356.	13.41		
ROUTED TO	RT-59	3147.	6.33	1011.	369.	356.	13.41		
HYDROGRAPH AT	SC2-4	399.	6.17	55.	17.	16.	0.28		
ROUTED TO	RT-61	393.	6.17	55.	17.	16.	0.28		
HYDROGRAPH AT	SC2-3	118.	6.08	14.	4.	4.	0.07		
2 COMBINED AT	DP22	495.	6.17	69.	21.	20.	0.36		
ROUTED TO	RT-60	492.	6.17	69.	21.	20.	0.36		
HYDROGRAPH AT	SC2-1	144.	6.17	20.	6.	6.	0.19		
3 COMBINED AT	DP20	3646.	6.33	1095.	396.	381.	13.96		
ROUTED TO	DBSC2	2327.	6.67	1075.	392.	378.	13.96	6504.10	6.67
ROUTED TO	RT-63	2319.	6.75	1074.	392.	377.	13.96		
HYDROGRAPH AT	SC1-10	165.	6.25	27.	9.	8.	0.29		
2 COMBINED AT	DP18	2396.	6.67	1099.	400.	386.	14.24		
ROUTED TO	RT-66	2393.	6.67	1099.	400.	386.	14.24		
HYDROGRAPH AT	SC1-13	154.	6.17	20.	6.	6.	0.13		
ROUTED TO	RT-64	154.	6.17	20.	6.	6.	0.13		
HYDROGRAPH AT	SC1-14	162.	6.08	21.	6.	6.	0.11		
2 COMBINED AT	DP19	315.	6.17	42.	13.	12.	0.24		
ROUTED TO	RT-65	314.	6.17	42.	13.	12.	0.24		
2 COMBINED AT	DP17	2465.	6.67	1131.	413.	398.	14.48		
ROUTED TO	RT-67	2459.	6.67	1130.	412.	397.	14.48		
HYDROGRAPH AT	SC1-9	166.	6.17	21.	7.	7.	0.24		
ROUTED TO	DBSR29	54.	6.50	21.	7.	7.	0.24	103.93	6.50
ROUTED TO	RT-70	54.	6.58	21.	7.	7.	0.24		
HYDROGRAPH AT	SC1-8	148.	6.17	22.	7.	7.	0.21		
ROUTED TO	RT-71	147.	6.17	22.	7.	7.	0.21		

2 COMBINED AT	DP14	189.	6.25	42.	14.	13.	0.45		
ROUTED TO	RT-69	188.	6.25	42.	14.	13.	0.45		
HYDROGRAPH AT	SC1-7	84.	6.08	10.	3.	3.	0.10		
2 COMBINED AT	DP12	258.	6.17	52.	17.	16.	0.55		
ROUTED TO	RT-68	257.	6.17	52.	17.	16.	0.55		
2 COMBINED AT	DP13	2581.	6.67	1180.	429.	414.	15.03		
ROUTED TO	RT-72	2573.	6.67	1180.	429.	413.	15.03		
HYDROGRAPH AT	SC1-6	147.	6.08	17.	6.	5.	0.16		
ROUTED TO	DBSR15	92.	6.25	17.	5.	5.	0.16	102.52	6.25
ROUTED TO	RT-74	91.	6.33	17.	6.	5.	0.16		
HYDROGRAPH AT	SC1-5	121.	6.17	17.	6.	5.	0.17		
2 COMBINED AT	DP11	207.	6.25	35.	11.	11.	0.33		
ROUTED TO	RT-73	207.	6.25	35.	11.	11.	0.33		
2 COMBINED AT	DP10	2672.	6.67	1213.	440.	424.	15.36		
ROUTED TO	RT-75	2658.	6.67	1212.	439.	423.	15.36		
HYDROGRAPH AT	SC1-2	93.	6.17	14.	4.	4.	0.14		
ROUTED TO	RT-76	92.	6.17	14.	4.	4.	0.14		
2 COMBINED AT	DP9	2689.	6.67	1223.	443.	427.	15.50		
ROUTED TO	RT-78	2683.	6.75	1223.	442.	426.	15.50		
HYDROGRAPH AT	SC1-12	210.	6.25	32.	10.	10.	0.25		
ROUTED TO	RT-81	207.	6.25	32.	10.	10.	0.25		
HYDROGRAPH AT	SC1-11	232.	6.17	31.	9.	9.	0.16		
2 COMBINED AT	DP8	418.	6.17	63.	19.	19.	0.41		
ROUTED TO	RT-77	411.	6.33	63.	19.	19.	0.41		
HYDROGRAPH AT	SC1-4	152.	6.08	20.	6.	6.	0.10		
ROUTED TO	RT-80	151.	6.08	20.	6.	6.	0.10		
HYDROGRAPH AT	SC1-1	81.	6.17	11.	3.	3.	0.11		
ROUTED TO	RT-79	80.	6.17	11.	3.	3.	0.11		
HYDROGRAPH AT	SC1-3	59.	6.42	13.	5.	5.	0.35		
5 COMBINED AT	DP4	2984.	6.67	1320.	476.	458.	16.47		
ROUTED TO	DBSC1	2358.	7.17	1266.	461.	444.	16.47	6402.80	7.17

2 YEAR OUTPUT SUMMARY

	DP69	284.	6.13	72.	30.	30.	3.41		
ROUTED TO	DBSRMN	164.	6.93	71.	30.	30.	3.41	96.52	6.93
ROUTED TO	RT-12	164.	7.00	71.	29.	29.	3.41		
HYDROGRAPH AT	SC3-15	138.	6.13	16.	6.	6.	0.21		
ROUTED TO	RT-10A	138.	6.13	16.	6.	6.	0.21		
2 COMBINED AT	DP87	205.	6.27	86.	35.	35.	3.62		
ROUTED TO	RT-13	203.	6.33	86.	35.	35.	3.62		
HYDROGRAPH AT	SC3-14	237.	6.13	28.	10.	10.	0.34		
HYDROGRAPH AT	SC3-16	189.	6.13	23.	8.	8.	0.30		
ROUTED TO	RT-13A	182.	6.40	23.	8.	8.	0.30		
3 COMBINED AT	DP68	473.	6.40	135.	53.	53.	4.26		
ROUTED TO	DBSRMS	358.	6.60	134.	53.	53.	4.26	8.34	6.60
ROUTED TO	RT-17	357.	6.67	134.	53.	53.	4.26		
HYDROGRAPH AT	SC3-13	35.	6.20	6.	2.	2.	0.34		
ROUTED TO	RT-19	34.	6.27	6.	2.	2.	0.34		
HYDROGRAPH AT	SC3-12	127.	6.13	15.	5.	5.	0.21		
2 COMBINED AT	DP66	153.	6.13	21.	8.	8.	0.54		
ROUTED TO	RT-20	148.	6.53	20.	8.	8.	0.54		
HYDROGRAPH AT	SC3-11	177.	6.13	24.	8.	8.	0.28		
2 COMBINED AT	DP63A	212.	6.53	43.	16.	16.	0.83		
2 COMBINED AT	DP63	560.	6.53	176.	69.	69.	5.09		
ROUTED TO	RT-17A	557.	6.60	176.	69.	69.	5.09		
HYDROGRAPH AT	SC3-10	7.	6.20	1.	0.	0.	0.06		
HYDROGRAPH AT	SC3-9	55.	6.40	12.	5.	5.	0.50		
ROUTED TO	RT-14	55.	6.47	12.	5.	5.	0.50		
HYDROGRAPH AT	SC3-8	43.	6.33	8.	3.	3.	0.23		
2 COMBINED AT	DP65	94.	6.40	20.	8.	8.	0.73		
ROUTED TO	RT-15	94.	6.40	20.	8.	8.	0.73		
HYDROGRAPH AT	SC3-7	119.	6.07	13.	5.	5.	0.16		
ROUTED TO	RT-15A	116.	6.07	13.	5.	5.	0.16		
2 COMBINED AT	DP64	149.	6.20	33.	13.	13.	0.88		
ROUTED TO	RT-16	149.	6.20	33.	13.	13.	0.88		
HYDROGRAPH AT	SC3-6	167.	6.07	17.	6.	6.	0.21		
2 COMBINED AT	DP62	301.	6.07	50.	19.	19.	1.09		
ROUTED TO	DBSRW	83.	6.80	47.	18.	18.	1.09	72.71	6.80

3 COMBINED AT	DP61	641.	6.60	224.	88.	88.	6.24		
ROUTED TO	RT-17B	636.	6.67	224.	87.	87.	6.24		
HYDROGRAPH AT	SB61	10.	6.33	2.	1.	1.	0.12		
ROUTED TO	RT-20	9.	6.40	2.	1.	1.	0.12		
HYDROGRAPH AT	SC3-5	114.	6.07	11.	4.	4.	0.16		
3 COMBINED AT	DP60	654.	6.67	235.	92.	92.	6.51		
ROUTED TO	RT-18	644.	6.67	235.	92.	92.	6.51		
HYDROGRAPH AT	SC3-4	82.	6.00	8.	3.	3.	0.08		
ROUTED TO	RT-21	81.	6.07	8.	3.	3.	0.08		
HYDROGRAPH AT	SC3-1	82.	6.07	8.	3.	3.	0.17		
HYDROGRAPH AT	SC3-3	156.	6.07	17.	6.	6.	0.24		
ROUTED TO	DBSC33	64.	6.33	17.	6.	6.	0.24	2.82	6.33
ROUTED TO	RT-21B	64.	6.33	17.	6.	6.	0.24		
HYDROGRAPH AT	SC3-2	60.	6.07	6.	2.	2.	0.08		
5 COMBINED AT	DP53	718.	6.67	272.	106.	106.	7.08		
ROUTED TO	DB-3	116.	10.20	99.	49.	49.	7.08	84.94	8.13
HYDROGRAPH AT	SC6-1	77.	6.13	10.	3.	3.	0.11		
ROUTED TO	DBSC61	24.	6.53	9.	3.	3.	0.11	2.04	6.53
ROUTED TO	RT-23A	24.	6.60	9.	3.	3.	0.11		
HYDROGRAPH AT	SC6-2	118.	6.13	14.	5.	5.	0.24		
HYDROGRAPH AT	SC6-3	104.	6.07	12.	4.	4.	0.17		
ROUTED TO	DBSC63	26.	6.47	11.	4.	4.	0.17	2.41	6.47
ROUTED TO	RT-23B	26.	6.47	11.	4.	4.	0.17		
3 COMBINED AT	DP54A	136.	6.13	34.	13.	13.	0.52		
ROUTED TO	RT-23C	134.	6.13	34.	13.	13.	0.52		
HYDROGRAPH AT	SC6-4	129.	6.00	14.	5.	5.	0.10		
ROUTED TO	DBSC64	22.	6.40	12.	5.	5.	0.10	2.56	6.40
ROUTED TO	RT-24	22.	6.47	12.	5.	5.	0.10		
HYDROGRAPH AT	SC6-5	172.	6.07	19.	7.	7.	0.28		
HYDROGRAPH AT	SC6-6	68.	6.07	7.	3.	3.	0.06		
4 COMBINED AT	DP54	375.	6.07	72.	27.	27.	0.96		
ROUTED TO	DB-6	93.	7.00	70.	27.	27.	0.96	71.19	7.00
HYDROGRAPH AT	SC2-29	43.	6.00	5.	2.	2.	0.03		

AREA DOWNSTREAM OF WOODMEN 2 YEAR RUNOFF SUMMARY

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	DP53	116.	10.25	99.	51.	49.	7.08		
ROUTED TO	RT-27	116.	10.25	99.	51.	49.	7.08		
HYDROGRAPH AT	SC2-24	64.	6.08	7.	2.	2.	0.07		
ROUTED TO	RT-29	64.	6.08	7.	2.	2.	0.07		
HYDROGRAPH AT	SC2-27	80.	6.08	9.	3.	3.	0.08		
3 COMBINED AT	DP50	150.	6.08	101.	56.	54.	7.23		
ROUTED TO	RT-31	146.	6.08	101.	56.	54.	7.23		
HYDROGRAPH AT	DP51	43.	6.00	5.	1.	1.	0.03		
ROUTED TO	RT-30	42.	6.08	5.	1.	1.	0.03		
2 COMBINED AT	DP49	188.	6.08	102.	57.	55.	7.26		
ROUTED TO	RT-32	183.	6.25	102.	57.	54.	7.26		
HYDROGRAPH AT	DP54	93.	7.00	70.	24.	23.	0.96		
ROUTED TO	RT-26	93.	7.17	70.	24.	23.	0.96		
HYDROGRAPH AT	SC2-26	46.	6.25	7.	2.	2.	0.15		
ROUTED TO	RT-34	46.	6.25	7.	2.	2.	0.15		
HYDROGRAPH AT	SC2-25	102.	6.33	19.	7.	6.	0.42		
3 COMBINED AT	DP47	220.	6.33	96.	33.	32.	1.53		
ROUTED TO	RT-33	219.	6.33	96.	33.	32.	1.53		
HYDROGRAPH AT	SC2-28	13.	6.50	4.	2.	1.	0.27		
HYDROGRAPH AT	SC2-21	51.	6.25	9.	3.	3.	0.19		
4 COMBINED AT	DP48	445.	6.25	193.	94.	90.	9.26		
ROUTED TO	RT-37	443.	6.33	193.	94.	90.	9.26		
HYDROGRAPH AT	SC2-23	58.	6.25	9.	3.	3.	0.20		
ROUTED TO	RT-35	58.	6.25	9.	3.	3.	0.20		
HYDROGRAPH AT	SC2-22	30.	6.08	4.	1.	1.	0.08		
2 COMBINED AT	DP46	84.	6.17	13.	4.	4.	0.28		
ROUTED TO	RT-36	83.	6.25	13.	4.	4.	0.28		
2 COMBINED AT	DP42	518.	6.33	205.	98.	94.	9.53		
ROUTED TO	RT-38	514.	6.33	205.	98.	94.	9.53		
HYDROGRAPH AT	SC2-20	87.	6.25	15.	5.	5.	0.32		
ROUTED TO	RT-41	86.	6.33	15.	5.	5.	0.32		
HYDROGRAPH AT	SC2-15	65.	6.17	9.	3.	3.	0.18		

ROUTED TO	RT-39	65.	6.17	9.	3.	3.	0.18
HYDROGRAPH AT	SC2-13	54.	6.17	8.	3.	2.	0.16
4 COMBINED AT	DP85	684.	6.33	233.	108.	104.	10.20
ROUTED TO	RT-42	670.	6.42	233.	108.	104.	10.20
HYDROGRAPH AT	SC2-14	48.	6.33	10.	3.	3.	0.26
HYDROGRAPH AT	SC2-16	49.	6.25	8.	3.	3.	0.21
3 COMBINED AT	DP37	759.	6.33	249.	114.	109.	10.68
ROUTED TO	RT-43	757.	6.42	248.	113.	109.	10.68
HYDROGRAPH AT	SC2-12	22.	6.25	4.	1.	1.	0.18
ROUTED TO	RT-46	22.	6.33	4.	1.	1.	0.18
HYDROGRAPH AT	SC2-8	85.	6.33	16.	5.	5.	0.35
2 COMBINED AT	DP33	107.	6.33	20.	7.	7.	0.53
ROUTED TO	RT-45	105.	6.33	20.	7.	7.	0.53
2 COMBINED AT	DP36	861.	6.42	267.	120.	115.	11.21
ROUTED TO	RT-44	840.	6.50	266.	119.	115.	11.21
HYDROGRAPH AT	SC2-11	76.	6.17	10.	3.	3.	0.21
ROUTED TO	RT-52	74.	6.25	10.	3.	3.	0.21
HYDROGRAPH AT	SC2-19	104.	6.17	14.	5.	4.	0.18
ROUTED TO	RT-51	101.	6.17	14.	5.	4.	0.18
HYDROGRAPH AT	SC2-18	133.	6.17	18.	6.	5.	0.20
3 COMBINED AT	DP39	294.	6.25	42.	13.	13.	0.59
ROUTED TO	RT-49	293.	6.25	42.	13.	13.	0.59
HYDROGRAPH AT	SC2-17	48.	6.25	7.	3.	2.	0.16
ROUTED TO	RT-50	48.	6.25	7.	2.	2.	0.16
HYDROGRAPH AT	SC2-29	119.	6.08	14.	4.	4.	0.15
3 COMBINED AT	DP34	425.	6.17	63.	20.	19.	0.90
ROUTED TO	RT-48	421.	6.25	63.	20.	19.	0.90
HYDROGRAPH AT	SC2-10	82.	6.33	15.	5.	5.	0.33
2 COMBINED AT	DP32	503.	6.25	77.	25.	24.	1.23
ROUTED TO	RT-47	498.	6.25	77.	25.	24.	1.23
HYDROGRAPH AT	SC2-9	23.	6.50	6.	2.	2.	0.21
3 COMBINED AT	DP35	1259.	6.42	341.	146.	141.	12.64
ROUTED TO	RT-53	1239.	6.42	341.	146.	141.	12.64
HYDROGRAPH AT	SC2-7	43.	6.25	7.	2.	2.	0.14
2 COMBINED AT	DP29	1270.	6.42	348.	148.	143.	12.79
ROUTED TO	RT-54	1242.	6.50	347.	148.	143.	12.79

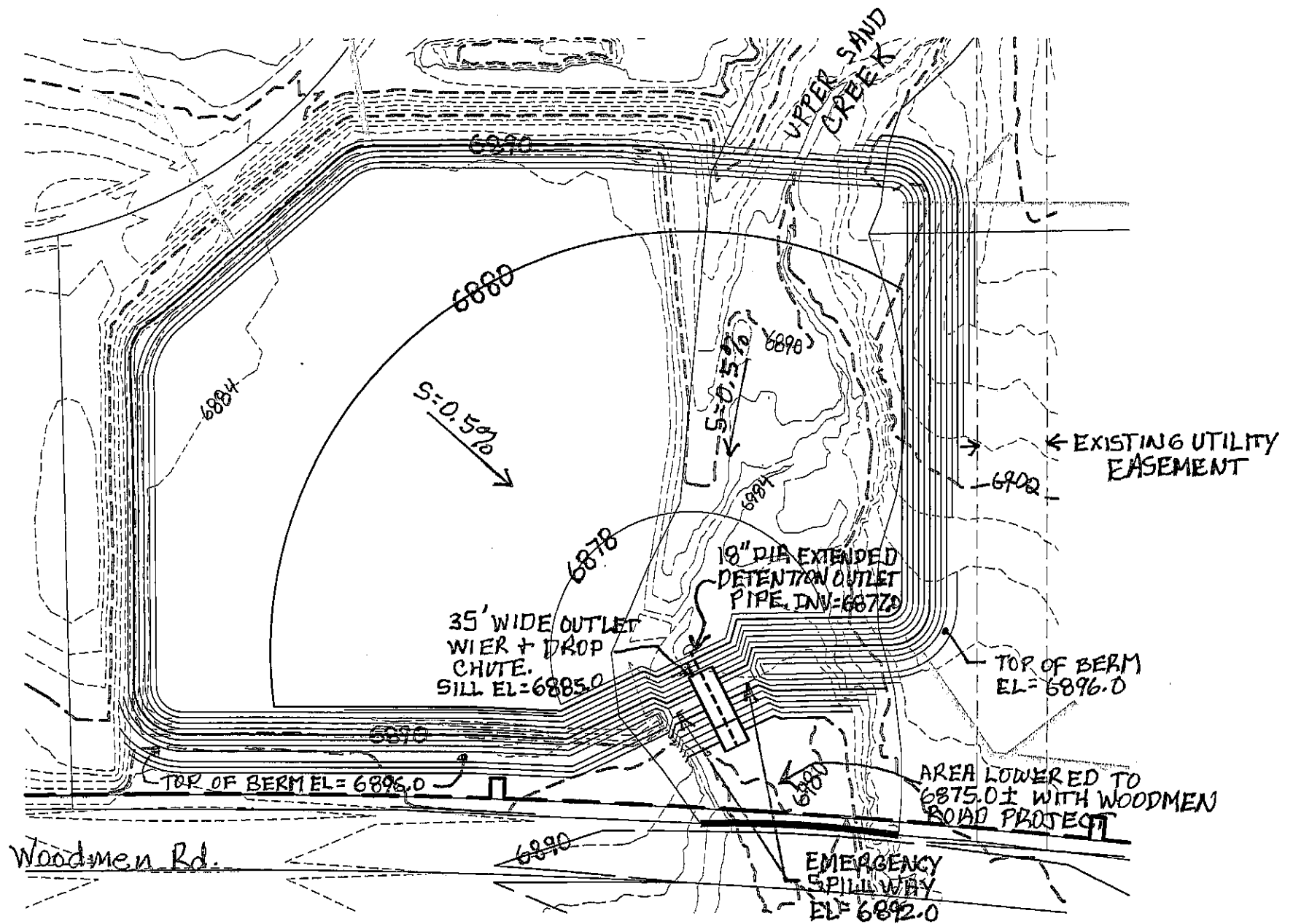
HYDROGRAPH AT	SC2-6	67.	6.17	9.	3.	3.	0.15		
ROUTED TO	RT-55	67.	6.17	9.	3.	3.	0.15		
HYDROGRAPH AT	SC2-2	65.	6.17	9.	3.	3.	0.17		
3 COMBINED AT	DP28	1296.	6.42	363.	154.	148.	13.10		
ROUTED TO	RT-56	1296.	6.50	363.	154.	148.	13.10		
HYDROGRAPH AT	SC2-5	143.	6.17	19.	6.	6.	0.16		
ROUTED TO	RT-58	143.	6.17	19.	6.	6.	0.16		
HYDROGRAPH AT	SC2-0	25.	6.25	4.	2.	1.	0.15		
ROUTED TO	RT-57	25.	6.25	4.	2.	1.	0.15		
3 COMBINED AT	DP21	1370.	6.50	383.	161.	155.	13.41		
ROUTED TO	RT-59	1367.	6.50	383.	161.	155.	13.41		
HYDROGRAPH AT	SC2-4	241.	6.17	34.	10.	10.	0.28		
ROUTED TO	RT-61	235.	6.17	33.	10.	10.	0.28		
HYDROGRAPH AT	SC2-3	72.	6.08	8.	3.	2.	0.07		
2 COMBINED AT	DP22	298.	6.17	42.	13.	12.	0.36		
ROUTED TO	RT-60	294.	6.17	42.	13.	12.	0.36		
HYDROGRAPH AT	SC2-1	66.	6.17	9.	3.	3.	0.19		
3 COMBINED AT	DP20	1557.	6.42	431.	177.	170.	13.96		
ROUTED TO	DBSC2	1110.	6.75	416.	176.	169.	13.96	6500.60	6.75
ROUTED TO	RT-63	1105.	6.75	415.	175.	169.	13.96		
HYDROGRAPH AT	SC1-10	70.	6.33	13.	4.	4.	0.29		
2 COMBINED AT	DP18	1136.	6.75	426.	180.	173.	14.24		
ROUTED TO	RT-66	1135.	6.75	426.	180.	173.	14.24		
HYDROGRAPH AT	SC1-13	86.	6.17	11.	4.	3.	0.13		
ROUTED TO	RT-64	85.	6.17	11.	4.	3.	0.13		
HYDROGRAPH AT	SC1-14	98.	6.17	13.	4.	4.	0.11		
2 COMBINED AT	DP19	183.	6.17	24.	7.	7.	0.24		
ROUTED TO	RT-65	183.	6.17	24.	7.	7.	0.24		
2 COMBINED AT	DP17	1170.	6.75	444.	187.	180.	14.48		
ROUTED TO	RT-67	1167.	6.75	444.	187.	180.	14.48		
HYDROGRAPH AT	SC1-9	71.	6.17	9.	3.	3.	0.24		
ROUTED TO	DESR29	28.	6.50	9.	3.	3.	0.24	102.81	6.50
ROUTED TO	RT-70	27.	6.50	9.	3.	3.	0.24		
HYDROGRAPH AT	SC1-8	66.	6.25	10.	3.	3.	0.21		
ROUTED TO	RT-71	66.	6.25	10.	3.	3.	0.21		

2 COMBINED AT	DP14	86.	6.25	19.	7.	6.	0.45		
ROUTED TO	RT-69	85.	6.25	19.	7.	6.	0.45		
HYDROGRAPH AT	SC1-7	38.	6.17	5.	2.	2.	0.10		
2 COMBINED AT	DP12	116.	6.25	24.	8.	8.	0.55		
ROUTED TO	RT-68	116.	6.25	24.	8.	8.	0.55		
2 COMBINED AT	DP13	1222.	6.75	468.	195.	188.	15.03		
ROUTED TO	RT-72	1218.	6.75	467.	194.	187.	15.03		
HYDROGRAPH AT	SC1-6	68.	6.08	8.	3.	3.	0.16		
ROUTED TO	DBSR15	48.	6.25	8.	3.	3.	0.16	101.21	6.25
ROUTED TO	RT-74	47.	6.33	8.	3.	3.	0.16		
HYDROGRAPH AT	SC1-5	53.	6.17	8.	3.	3.	0.17		
2 COMBINED AT	DP11	100.	6.25	17.	5.	5.	0.33		
ROUTED TO	RT-73	99.	6.25	17.	5.	5.	0.33		
2 COMBINED AT	DP10	1257.	6.75	483.	200.	193.	15.36		
ROUTED TO	RT-75	1253.	6.83	482.	199.	192.	15.36		
HYDROGRAPH AT	SC1-2	41.	6.25	6.	2.	2.	0.14		
ROUTED TO	RT-76	41.	6.25	6.	2.	2.	0.14		
2 COMBINED AT	DP9	1264.	6.83	488.	201.	194.	15.50		
ROUTED TO	RT-78	1263.	6.83	487.	201.	193.	15.50		
HYDROGRAPH AT	SC1-12	106.	6.25	17.	5.	5.	0.25		
ROUTED TO	RT-81	104.	6.33	17.	5.	5.	0.25		
HYDROGRAPH AT	SC1-11	141.	6.17	19.	6.	5.	0.16		
2 COMBINED AT	DP8	227.	6.17	35.	11.	11.	0.41		
ROUTED TO	RT-77	227.	6.33	35.	11.	11.	0.41		
HYDROGRAPH AT	SC1-4	91.	6.08	12.	4.	3.	0.10		
ROUTED TO	RT-80	91.	6.17	12.	4.	3.	0.10		
HYDROGRAPH AT	SC1-1	36.	6.17	5.	2.	2.	0.11		
ROUTED TO	RT-79	36.	6.17	5.	2.	2.	0.11		
HYDROGRAPH AT	SC1-3	11.	6.50	4.	2.	1.	0.35		
5 COMBINED AT	DP4	1395.	6.75	535.	218.	210.	16.47		
ROUTED TO	DBSC1	1125.	7.25	489.	207.	199.	16.47	6400.33	7.25

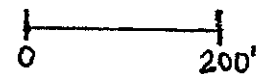


APPENDIX D

SAND CREEK REGIONAL POND NO. 3 CONCEPT PLAN

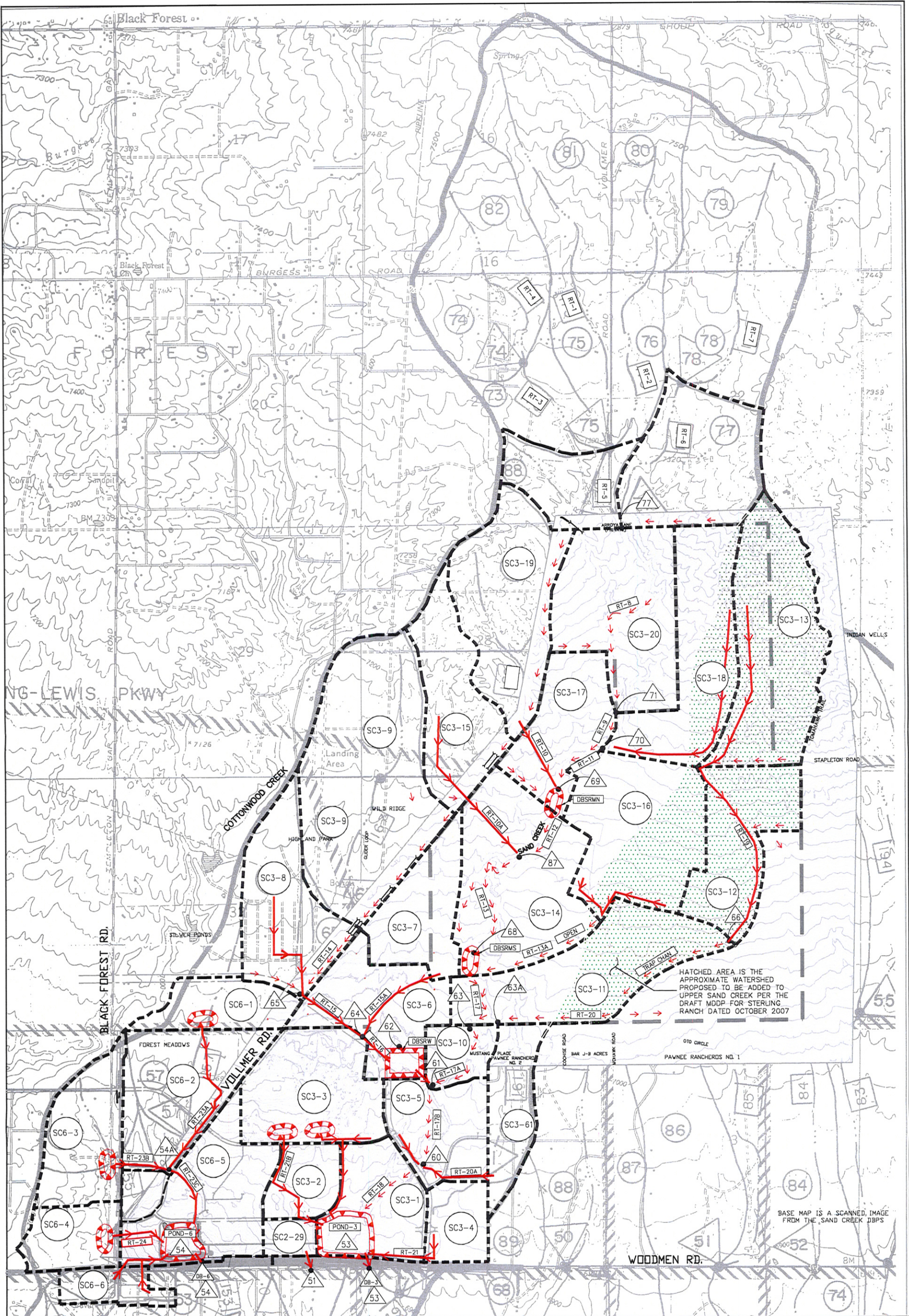


SAND CREEK POND 3
 CONCEPT PLAN



APPENDIX E

WATERSHED MAP



WILSON
& COMPANY
Engineers & Architects



0 1000' 2000'

REFER TO EXHIBIT 2 OF THE "DESIGN ANALYSIS
REPORT FOR "SAND CREEK DETENTION BASIN No. 2"
FOR AREA SOUTH OF WOODMEN ROAD

UPPER SAND CREEK BASIN DETENTION EVALUATION REPORT

JUNE 2009

LEGEND

- HEC 1 BASIN I.D.
- HEC 1 ANALYSIS POINT
- HEC 1 ROUTING ELEMENT I.D.
- STORM SEWER
- DETENTION POND