

FINAL REPORT

THE BOULDERS BROADMOOR MASTER DEVELOPMENT DRAINAGE PLAN

Prepared for
The Boulders Broadmoor
5110 Langdale Way
Colorado Springs, Colorado 80906

January 7, 1998 (Revised: February 24, 1998) *REVISED OCT. 21, 1998*

Woodward-Clyde 

Woodward-Clyde International-Americas
4582 South Ulster Street
Stanford Place 3, Suite 1000
Denver, CO 80237
Project No. 24506

URS Greiner Woodward Clyde

A Division of URS Corporation

October 21, 1998

Stanford Place 3, Suite 600
4582 South Ulster Street
Denver, CO 80237
Tel: 303.740.2600
Fax: 303.740.2650

Offices Worldwide

Mr. Robin Kidder
Stormwater and Engineering Review
City of Colorado Springs
101 West Costilla Street, Suite 122
Colorado Springs, CO 80901-1575

Subject: Addendum No. 1 to Boulders Broadmoor Final MDDP
Project No. 24506

Dear Rick:

As a result of review comments provided by the Colorado Geologic Survey (CGS) regarding the Master Development Drainage Plan (MDDP) for The Boulders Broadmoor development, we offer the following comments and proposed revisions related to the MDDP:

- Page 3-2, Section 3.1, Existing Drainages and Channels, 3rd paragraph--The following should be added: *"A 30-inch corrugated metal pipe (CMP) culvert and riser pipe located at the switchback on the main NORAD access road conveys runoff to Basin 500 from upstream Basin 1A. The peak 100-year discharge from this basin is relatively small, however, overflows from Basins 1B, 1C, and 1D would cause increased runoff to the culvert and riser as a result of clogging of upstream conveyance systems. Similarly, should cross culverts along the NORAD south portal access road become clogged, increased flows would be conveyed to the Boulders Broadmoor property below upstream Basins 3 and 4. Hydrologic analysis have been performed for both the existing conditions and assuming upstream conveyance facilities are clogged. The maximum flow rates determined assuming clogging of upstream conveyance systems is not recommended for design but rather to analyze the impacts to proposed on-site drainage facilities."*
- Page 5-2, Section 5.2, 1st paragraph--The following should be added in front of the last sentence: *"However, discussions with NORAD and USACOE personnel indicate that a debris flow likely occurred on NORAD property but was limited to one or both of the portal entrances and the upper parking lot areas. There is no evidence or indication that this debris flow overtopped the main NORAD access road onto the Boulders Broadmoor property. There is some indication, however, of hyperconcentrated sediment flows and significant runoff amounts which caused some channel erosion below the NORAD access road. Review of available photographs of the site, aerial photographs from 1947 and*

Mr. Robin Kidder
City of Colorado Springs
October 22, 1998
Page 2

1966/67 and a site reconnaissance of the areas below the NORAD access road support this conclusion."

- Page 5-2, Section 5.2, 5th paragraph--The following should be revised: *The correct equation for the bulking factor is $BF = 1/(1-C_v)$ and "segment" should be changed to "sediment"*
- Page 5-2, Section 5.2, 5th paragraph--The following should be added to the last sentence: *"..., however, some hyperconcentrated sediment flows may occur."*
- Page 6-1, Last paragraph, Item 3--The following should be added: *"A minimum bulking factor of 1.2 should be utilized to account for potential hyperconcentrated sediment flows."*
- Page 6-1, Last paragraph, Item 4--The following should be added: *"Natural upstream channels should be upgraded where existing capacities are not adequate to convey the peak 100-year design flow rate bulked for potential debris where appropriate."*
- Page 6-1, Last paragraph--The following should be added as Item 5: *"To the extent possible, existing flow paths and drainages should be maintained for conveyance of flood flows. Where culverts are installed or filling of the natural drainageways is proposed, overflow provisions should be designed."*
- Page 7-1 and 7-2--The following references should be added:
 - *Lincoln-DeVore, Inc., 1987. Letter regarding Star Ranch Subdivision prepared for Berry & Boyle dated May 13, 1987.*
 - *CTL/Thompson, Inc., 1996. "Preliminary Geologic Hazards Investigation, The Boulders Broadmoor, Filing 1 and 1A, Colorado Springs, Colorado". Prepared for Berry & Boyle, Inc. Job No. CS-6155 dated February 14, 1996.*
 - *CTL/Thompson, Inc., 1996. "Preliminary Geologic Hazards Investigation, The Boulders Broadmoor, Phase II, Colorado Springs, Colorado". Prepared for Berry & Boyle, Inc. Job No. CS-6155A dated March 15, 1996.*

If you have any questions regarding this proposed addendum or if it is necessary to revise the MDDP text and issue updated reports, please contact me at 303-740-3962.

Mr. Robin Kidder
City of Colorado Springs
October 22, 1998
Page 3

Sincerely,



Stephen W. Rogers, P.E.
Project Engineer



John W. Andrew, Ph.D.
Senior Consulting Engineer

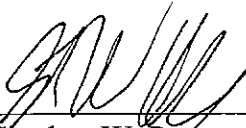
SWR:swr

cc: Mr. Rick O'Conner (City of Colorado Springs)
Mr. Earl C. Robertson (The Boulders Broadmoor)
Mr. Andrew McCord (Kiowa Engineering Corporation)
Mr. Jon Lovekin (CTL/Thompson, Inc.)
File

The Boulders Broadmoor Master Development Drainage Plan

Engineer's Statement:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City/County for master drainage plans. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.



Stephen W. Rogers, P.E.




Developer's Statement:

I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

The Boulders Broadmoor

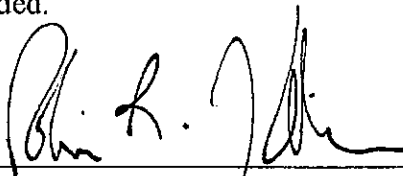
By: 

Title: 

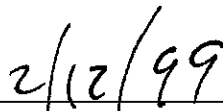
Address: 5110 Langdale Way
Colorado Springs, CO 80906

City of Colorado Springs

Filed in accordance with Section 15-3-906 of the code of the City of Colorado Springs, 1980, as amended.



City Engineer



Date

TABLE OF CONTENTS

Section 1	Introduction	1-1
Section 2	General.....	2-1
	2.1 General Location and Description	2-1
	2.2 Proposed Development	2-1
Section 3	Hydrologic Analysis	3-1
	3.1 Drainage Basin Description	3-1
	3.2 Design Criteria	3-2
	3.3 Rainfall-Runoff Modeling	3-3
Section 4	Drainage and Conveyance Facilities.....	4-1
	4.1 Existing Drainage Facilities	4-1
	4.2 Proposed Facilities	4-1
	4.3 Regional Detention Basins.....	4-4
Section 5	Potential Drainage Problems and Flooding Hazards.....	5-1
	5.1 Flood Hazards	5-1
	5.2 Debris Flow Hazards.....	5-1
Section 6	Conclusions and Recommendations	6-1
Section 7	References.....	7-1

List of Tables

Table 1	Summary of Hydrologic Parameters
Table 2	5-Year Peak Design Discharges
Table 3	100-Year Peak Design Discharges
Table 4	Existing Drainage and Conveyance Facilities
Table 5	Proposed Drainage and Conveyance Facilities

List of Figures

Figure 1	Vicinity Map
----------	--------------

List of Exhibits

Exhibit A	Boulders Broadmoor-Drainage and Facilities Map
-----------	--

TABLE OF CONTENTS

List of Appendixes

Appendix A	HEC-1 Output
A.1	Existing Conditions
A.2	Developed Conditions
Appendix B	Hydrologic/Hydraulic Computations

The Boulders Broadmoor development property is located on a series of old coalescing alluvial fans located at the eastern toe of Cheyenne Mountain west of Colorado Springs, Colorado. Residential development has occurred on the old alluvial fan surfaces below the steep canyons on the eastern slope of Cheyenne Mountain. The scenic beauty and raised elevations are desirable for residential development and, as such, The Boulders Broadmoor is planning for continued phased development. However, alluvial fans have unique hydrologic and hydraulic features. The fan surface typically displays characteristics whereby conveyance channels are neither well-defined nor stable.

Based on the presence of numerous old trees on the fan surface, it is believed that the alluvial surfaces at the base of Cheyenne Mountain are somewhat stable but, a large infrequent precipitation event and the associated runoff could cause channel instability. Engineering controls, as recommended herein, will minimize the potential for channel instability on the downstream fan surfaces.

This Master Development Drainage Plan (MDDP) has been prepared to identify drainage and conveyance issues associated with the proposed development and is required by the City of Colorado Springs for developments within any drainage basin. The purpose of the MDDP is to address major drainage planning for the proposed development before individual phases of development are undertaken. The planning is required so that drainage and hazards associated with flooding can be addressed in the design of the proposed development. This MDDP should be compatible with the Neil Ranch MDDP (KLH, 1982) and the Broadmoor Resort South MDDP (Woodward-Clyde, 1998) as updated by various drainage plans for individual developments (Watts, 1988; Watts, 1989; Kiowa Engineering, 1996) and should serve as a guide for future planning and drainage facility design. Site specific hydraulic design will be completed with the individual preliminary and final drainage reports submitted for approval.

This report presents estimates of off-site peak storm runoff discharges, estimates on-site peak runoff rates, and makes recommendations for major drainage facilities associated with the proposed development. Site specific hydrologic analysis and hydraulic design will be completed concurrently with individual preliminary and final drainage plans and reports submitted for approval.

Previous studies have been reviewed as part of the preparation of this report and have been utilized to the extent possible to maintain consistency. These studies reviewed are included in the references section of this report.

Previous analysis have identified debris flows or hyperconcentrated sediment flows to be a major design issue for drainage and conveyance facilities. This report will address the debris flow issues and make recommendations for drainage/conveyance facility design.

2.1 GENERAL LOCATION AND DESCRIPTION

The Boulders Broadmoor is located in the southwestern corner of the City of Colorado Springs in portions of Sections 12 and 13 of Township 15 South, Range 67 West of the 6th Prime Meridian. The watersheds affecting this property extend into Sections 10, 11, 14 and 15 of Township 15 South, Range 67 West of the 6th Prime Meridian. Figure 1 presents a vicinity map of the area. The proposed development is bordered on the west by Broadmoor Residential Resort Community property, to the west and south by U.S. Government property (NORAD), to the east by the JL Ranch and Neil Ranch, and to the north by Star Ranch and Broadmoor Residential Resort Community property.

All of the watershed drainages studied are tributary to Fountain Creek. Fountain Creek is a major tributary to the Arkansas River which travels south through the City of Colorado Springs.

2.2 PROPOSED DEVELOPMENT

The Boulders Broadmoor development is zoned as a Planned Urban Development (PUD) and will consist of single family residential structures. The proposed development will encompass approximately 230 acres south of Star Ranch. The proposed residential lots range from 1/4 acre to 1 acres in size.

In order to adequately identify and delineate hazards associated with extreme flood and debris flow events and determine design flow rates for conveyance facility design, an estimation of the design flood magnitude must be developed. This can be performed based on statistical flood frequency analysis of gauged stream flows or by estimation of the design rainfall event in conjunction with flood simulation via a rainfall-runoff model. Where stream gage records are limited or unavailable, regression equations may be applied to the analysis in order to supplement the lack of available data.

Review of available information indicates that there are no stream gages in or adjacent to the Boulders Broadmoor development watersheds. A hydrologic analysis for a portion of the off-site basins adjacent to the Boulders Broadmoor development was performed by FLO Engineering in 1995 (FLO Engineering, 1995) as part of the North Master Development Drainage Plan for the Broadmoor Resort Community Development (FLO Engineering, 1997). This study was reviewed and approved by the City of Colorado Springs Engineering Division, and the hydrologic analysis contained therein was reviewed as part of the preparation of this MDDP.

Additional hydrologic analysis for off-site basins were performed utilizing the HEC-1 (USACOE, 1990) computer model for both the off-site and on-site watersheds for both the existing and proposed conditions. Results of the HEC-1 computer model are included in Appendix A and further hydrologic and hydraulic computations are included in Appendix B.

3.1 DRAINAGE BASIN DESCRIPTION

The following section describes the existing hydrologic characteristics of the drainage basins related to the proposed development. The hydrologic characteristics evaluated include soils, precipitation and major drainages and channels. Exhibit A presents a general plan of the drainage basin affecting the proposed development.

Soils

The Boulders Broadmoor property soils can be described as loamy with some significant percentages of clay in some locations. The development area soils have been described in previous reports (KLH, 1982; Weiss, 1987; RCI, 1989; Muller, 1991; FLO Engineering, 1997). Rock outcrops are prevalent in the canyon drainages of Cheyenne Mountain. Along the base of Cheyenne Mountain, coalescing alluvial fans have created non-homogeneous soil groups. The majority of soils in the areas to be developed consist of the Bresser and Jarre-Tecolote Complex which are typically deep and well-drained. The clayey soils in the eastern portion of the property consist of Razor and Razor-Midway complex. Other soils found in the western vicinity of the proposed development include the Coldcreek-Tolman complex which consists of a mix of shallow to deep, well-drained soils.

Precipitation

Based on precipitation records for the Broadmoor Mountain Golf Course from 1932-1997, the average annual precipitation in the area is approximately 20 inches. The City of Colorado Springs Drainage Criteria Manual (City of Colorado Springs, 1994) suggests utilizing rainfall

depths determined from National Oceanic and Atmospheric Administration (NOAA), a branch of the National Weather Service, for design purposes. Further, intense precipitation events could occur in the steep canyons of Cheyenne Mountain.

Existing Drainages and Channels

Several existing drainages and channels convey flow across the proposed development. These flow paths have shifted over time as evidenced by numerous old channel scars which exist on the fan surface and have apparent small drainage areas. This movement of a drainage channel is typical of an active alluvial fan and is an indication of potential debris flow or hyperconcentrated sediment flow.

The largest watershed, Fishers Canyon, lies upstream of Basin 300. This water shed has potential to generate debris flows. Current flow paths suggest that the majority of Fishers Canyon runoff would be conveyed to the South of Star Ranch and conveyed to the South of Star Ranch and through Boulder Broadmoor Filings 1 and 1A.

Another large off-site drainage basin exists west of the NORAD facility and has the potential to generate large runoff and potential debris flows. The NORAD facility intercepts a majority of runoff from this and adjacent watersheds. Runoff and debris are controlled by existing NORAD facilities and several conduits beneath NORAD Road.

3.2 DESIGN CRITERIA

The following discussion relates to the design criteria selected for use in determining peak runoff flow rates. Table 1 provides a summary of the hydrologic characteristics for each drainage basin evaluated. In general, the criteria outlined in the City of Colorado Springs and El Paso County Drainage Criteria Manual (City of Colorado Springs, 1994) were utilized in determining drainage basin characteristics.

Precipitation

The 100-year return period, 2-hour duration design storm of 3.10 inches was utilized for determining peak design discharges for this study. The 2-hour duration precipitation event was previously determined to produce the highest peak runoff rates within adjacent upper watersheds (FLO Engineering, 1995).

The design storm of 3.10 inches is consistent with available 24-hour precipitation records available for the Mountain Golf Course. During the last five years, the area has experienced several large extreme precipitation events. On May 29, 1995, the Mountain Golf Course precipitation gage recorded a maximum 24-hour precipitation depth of 4.30 inches. In 1994, the maximum recorded 24-hour precipitation depth at the Mountain Golf Course was 3.5 inches. Assuming a standard SCS Type II precipitation distribution, the most intense 2-hour increment of the design storm would receive on the order of 55% of the total 24-hour precipitation depth. This corresponds to an equivalent 2-hour precipitation of approximately 2.37 inches for the May 29, 1995 event. Therefore, a 100-year return period, 2-hour duration design storm of 3.10 inches appears to be reasonable.

Drainage Basin Delineation

The major basins described in previous sections were divided into numerous subbasins for hydrologic modeling purposes. In general, each subbasin represents an area of essentially similar hydrologic characteristics and was identified to determine a design flow rate at a particular point of interest including the discharge points at NORAD Road. Drainage basins were determined from available topographic mapping supplemented by a site reconnaissance of the watersheds. Available topography above elevation 7000 feet is limited in accuracy. The majority of the topography is based on the City of Colorado Springs FIMS mapping with 2-foot contours. Some of the drainage basin areas for the major off-site drainages were adopted from previous reports (FLO Engineering, 1995) after reviewing the drainages areas for consistency with available data.

Infiltration

Infiltration within the drainage basins was simulated utilizing the Natural Resources Conservation Service (formerly the Soil Conservation Service (SCS)) SCS curve number approach. The hydrologic classification of these soils is generally in Hydrologic Soil Group C to D for the upper watersheds and Hydrologic Soil Group B for the lower watersheds. The NRCS suggest a runoff curve numbers for the existing soils in the development area to be approximately 58. The weighted average curve numbers utilized in previous studies for the developed condition in the development area generally range from 72 to 78 (KLH, 1982; RCI, 1991). The effects of the development on infiltration were estimated by weighting the curve number based on the percent of development within a given basin or subbasin using an average CN of 72 for the residential areas of the development. A curve number of 72 was estimated assuming an average lot size between 0.5 and 1 acre.

Time of Concentration

The time of concentration of a watershed is defined as the time it takes for a particle of water to travel from the most remote point in the watershed to the design point. The time of concentration within the drainage basins evaluated were estimated utilizing the Kirpich method for the upper watersheds and utilizing the SCS travel time for lower watersheds. For developed conditions, the time of concentration was reduced by a minimum of 10 percent to account for decreased travel times as a result of the developed conditions. Channel routing was utilized where applicable, however, most of the drainages were determined to be too steep for any significant attenuation of flow rates created by channel storage.

3.3 RAINFALL-RUNOFF MODELING

The runoff for the 5-year and 100-year return period precipitation events were simulated using the U.S. Army Corps of Engineers HEC-1 computer program (USACOE, 1990). A rainfall-runoff model was developed for the off-site drainages at the apex of the canyons of the watersheds. The model was developed utilizing the SCS unit hydrograph methodology incorporating SCS runoff curve numbers and time of concentration parameters. The hydrologic parameters were determined for the additional drainage basins as described in the previous section.

Hydrologic Results

Peak runoff rates for the 5-year and 100-year frequency events are provided in Tables 2 and 3, respectively for both the existing and developed conditions. In general, the effects of the development on the peak runoff rates is small. However, for some of the smaller subbasins where development is planned for the majority of the watershed, peak flows may increase as much as 30 percent due to the development. Estimated developed flows are not likely to significantly impact the existing drainage facilities or private property which lie between the project area and regional detention basins. Regional detention basins have been designed to account for a portion of the increased volume created as a result of the development. More information regarding the design of the regional detention basins is provided in Section 4.3.

Peak runoff rates for developed conditions are provided on Exhibit A. Exhibit A also provides ultimate developed flow rates at specific design points as determined from previous studies. In general, the flow rates determined herein are considerably less than those previously determined. Review of previous studies indicates that this is due to 1) smaller drainage areas, 2) methodology utilized and 3) lower curve number created by less than projected development rates.

The proposed debris basin structure at the apex of Fishers Canyon has been designed to attenuate flows up to the 100-year event. As such, flow rates downstream of the structure have been greatly minimized. Further discussion of the design of this structure is included in Appendix D of the Final Report for the Broadmoor Resort Community South Master Development Drainage Plan, February 1998 (Woodward-Clyde, 1998).

A discussion of existing and proposed drainage and conveyance facilities is provided in this section.

4.1 EXISTING DRAINAGE FACILITIES

Most of the existing drainage facilities consist of culverts of various types located downstream of the development and off the project site. Existing drainage facilities are present along Star Ranch Road, Broadmoor Bluffs Drive and NORAD Road. Table 4 lists the existing drainage and conveyance facilities and their estimated capacities.

In general, most downstream existing conveyance facilities were designed to convey larger developed flows than those determined as part of this study.

4.2 PROPOSED FACILITIES

The majority of major drainage improvements are planned to be constructed in 1998. Detailed design plans for the proposed drainage improvements will be submitted with the Preliminary and Final Drainage Plans and Reports for the different filings of the proposed phased development.

The majority of proposed facilities are required to ensure that existing major drainages through the development are maintained. This can usually be performed by providing drainage easements in conjunction with minor channel improvements or by providing some sort of engineered structures to convey the flow. Because the development is located on an old alluvial fan, upstream channel stability is a concern for some of the watersheds. The proposed drainage facilities generally include flood conveyance structures within major drainage ways, local drainage improvements to convey runoff from overland flow areas to collector and main channels/culverts, and mitigation measures for hyperconcentrated sediment flows (mud and debris flows). Table 5 lists the proposed drainage and conveyance facilities for the development. The proposed facilities, discussed by basin area, are presented below.

Basin 300

A debris control structure at the apex of Fishers Canyon is being designed to eliminate the majority of sediment and debris transported to the canyon apex during the 100-year event and reduce the peak discharge downstream through the attenuation of flood waters. As a result, bulking of the water flows downstream of the structure is not expected during the 100-year event and conveyance structures will be designed to convey the peak developed 100-year discharge.

The drainage and conveyance facilities currently being constructed at the Boulders Broadmoor Filing No. 1 and 1A development are capable of passing the peak attenuated developed 100-year water flood from Fishers Canyon of approximately 362 cfs. These facilities were designed to convey runoff from a "split" of Fishers Canyon flow assuming part of the flow would be conveyed to the "north" channel and enter either Star Ranch or property north of Star Ranch including bulking of the water flow created by debris. Current conditions indicate that the majority of flow will be conveyed to the south channel.

The total 100-year design discharge bulked for sediment at the extents of Boulders Broadmoor Filing No. 1 and 1-A without the debris control/flood attenuation structure is estimated to be approximately 790 cfs, (Kiowa Engineering, 1997). The proposed structure will reduce the peak 100-year water discharge from Fishers Canyon assuming attenuation and flow conveyed to the south to an estimated peak discharge of 399 cfs.

Drainage easements will be required for the Fishers Canyon channel and will be maintained by providing either easements, channels or conveyance structures for Drainages 9A, 9B and 300B. Drainage 300 is conveyed to an existing 60-inch CMP at both Ellsworth Street and Broadmoor Bluffs Drive. These structures are estimated to have maximum capacities of approximately 366 and 390 cfs, respectively. Therefore, approximately 33 cfs would overtop the Ellsworth Street curb. Excess flow will be conveyed by Ellsworth Street curb and gutter to the sump in Broadmoor Bluffs Drive at the existing 78-inch CMP culvert crossing. No overtopping of Broadmoor Bluffs Drive is expected during the 100-year frequency design event as flood waters will spill into Basin 400B and be conveyed by the 78-inch CMP culvert crossing at Broadmoor Bluffs Drive. The existing 78-inch CMP has adequate capacity to convey peak 100-year discharges from Basin 400 as well as overflow from Basin 300 at Broadmoor Bluffs Drive.

Basin 400

Proposed improvements for Basin 400 include maintaining conveyance for Drainages 8A, 7A and 400A. Because this drainage accepts runoff from the steep canyons of Cheyenne Mountain, the potential for a debris flow exists and therefore conveyance facilities should be designed accordingly utilizing a minimum bulking factor of 2.0. The existing structures at Ellsworth Street (60" RCP) and Broadmoor Bluffs Drive (78" CMP) should be adequate to convey the peak 100-year design discharge bulked for debris flow.

Basin 500

Proposed improvements for Basin 500 include maintaining conveyance for Drainages 500A, 500C and 500B, designing street conveyance structures to pass potential debris flows and providing a larger culvert (or similar) for Ellsworth Street (if extended). Discussions with the Boulders Broadmoor indicates that the crossing will consist of a 48-inch RCP with a small detention area. This should be capable of passing the 100-year peak discharge bulked for debris flow. A berm or rechannelization may be required for Drainage 500A at approximately Elevation 6780 feet.

Drainage 500C crosses Star Ranch Road via a 24-inch pipe. Drainage 500 is ultimately conveyed to an existing 90-inch CMP riser structure with a 78-inch orifice cutout (insert) at Broadmoor Bluffs Drive. The structure was designed to store approximately 0.93 acre-feet of storage. Upon construction of regional Detention Pond No. 2, the insert is recommended (based on previous studies) to be removed thus reducing temporary storage.

Basin A

Basin A discharges through the proposed Boulders Broadmoor development south of Star Ranch. Drainage paths from the upper portions of Basin A cross proposed Buttermere Drive and

Gladstone Street. Development within Basin A will occupy approximately 60 percent of the basin area, however, the small areal extent of Basin A will likely result in limited drainage. Runoff from the steep canyons of Cheyenne Mountain is expected. The potential for debris flow is expected to be minimal as Basin A lies below the NORAD facility, which has historically intercepted the majority of debris flow off Cheyenne Mountain from past storm events.

Proposed improvements for Basin A include maintaining conveyance for Drainages 1A, 1B, 1C and 1D, which lie above NORAD Road. As Buttermere Drive and Gladstone Street transect portions of Basin A, drainage and flood control facilities will be required to convey runoff as part of the street improvements. The developed 5-year and 100-year flows from the upstream watershed were estimated to be about 71 and 310 cfs, respectively. As such, an 84-inch CMP should be adequate to convey this design flow. The downstream natural channel would require an easement from adjacent properties to maintain flow paths through Basin A. The natural channel should be adequate to convey the developed flows from the upstream watershed.

Basin B

Basin B lies below Basins A and C, however, no runoff from the adjacent upper watersheds is expected to enter Basin B. Basin B outlets to Broadmoor Bluffs Drive and the areal extent of the watershed is limited. Additionally, it is unlikely that any significant debris flow will impact Basin B as potential debris flows are likely to deposit sediment and debris on NORAD property thus limiting the amount of debris transported downstream.

Proposed improvements for Basin B include maintaining flow conveyance from this basin, and ensuring adequate conveyance for the 5-year and 100-year storm events. The 5-year and 100-year peak discharges were estimated to be 2 cfs and 12 cfs, respectively at Design Point DP8.

Basin C

Proposed improvements for Basin C include maintaining conveyance for Basin 2, designing street conveyance structures and adequately sized culvert(s) to pass flows. The basin outlet at Broadmoor Bluffs Drive, and the existing crossing for this road should adequately pass the estimated 100-year peak design discharge of 72 cfs. However, while it is anticipated that the majority of debris during past storm events has deposited on the NORAD property, there is still the potential for minor debris flows to be generated through Basin C. As such, it is recommended that peak discharges be bulked by a factor of 1.2 for on-site drainage design purposes.

Basin D

Proposed improvements for Basin D include maintaining adequate drainage through the basin, and implementation of any required flow conveyance structures required to facilitate efficient drainage. Developed peak flows for the 5-year and 100-year frequency events were estimated to be approximately 2 cfs and 6 cfs, respectively. Where Basin D lies below the majority of residential development and is situated at the easternmost portion of the development it is unlikely that significant debris flow would arise from the steep canyons in the upper watershed.

Basin E

Proposed improvements for Basin E include maintaining adequate drainage through the basin, with implementation of any required flow conveyance structures required to facilitate efficient drainage. Where Basin E lies below the majority of residential development and is situated at the easternmost portion of the development, significant debris flow arising from the steep canyons in the upper watershed is not expected. The 5-year and 100-year peak discharges were computed as 1 and 3 cfs, respectively.

Basin F

Proposed improvements for Basin F include maintaining conveyance throughout the basin and providing adequate off-basin drainage for the developed 5-year and 100-year design flows of 1 and 6 cfs, respectively. The potential for debris flow through this basin is expected to be minimal.

Basin G

Proposed improvements for Basin G include maintaining conveyance through the basin and providing adequate off-basin drainage for the developed 5-year and 100-year design flows of 3 and 17 cfs, respectively. A 24-inch CMP will be adequate to convey the 100-year design discharge. The potential for debris flow through this basin is expected to be minimal.

Basin H

Proposed improvements for Basin H include maintaining conveyance of Basins 3 and 4. As the NORAD property has intercepted the majority of debris flows from past storm events, it is unlikely that significant levels of debris will be conveyed to Basin H from the upper watersheds. However, because there is minimal potential for debris flow to pass through the basin, it is suggested that design flows for on-site runoff conveyance be bulked by a factor of 1.2. A 36-inch CMP will be adequate to convey the peak, bulked 100-year design discharge.

4.3 REGIONAL DETENTION BASINS

Regional Detention Pond No. 2 is located at Route 115 and NORAD Road and accepts flow from a portion of The Boulders Broadmoor development. The structure has been designed for a total release rate of 1280 cfs which is comparable to the historic undeveloped 100-year, 24-hour runoff event. The available storage is approximately 43 acre-feet.

A portion of runoff from the Boulders Broadmoor development is not intercepted by Regional Detention Pond No. 2. Runoff volumes will be maintained to historic levels by on-site routing to detention facilities.

The Regional Detention Ponds were designed assuming a developed Curve Number of 68 for the upstream developments, consistent with the Curve Numbers utilized in this study.

In order to minimize potential drainage problems associated with the development, adequate design of conveyance structures including addressing the effects of debris flows is required. Potential drainage problems associated with the proposed development include the following:

- flooding associated with the inability of a natural or improved channel to convey upstream runoff
- local flooding associated with the inability of a conveyance structure (i.e., culvert, bridge, etc.) to convey upstream runoff
- local flooding associated with the “clogging” of conveyance structures as a result of debris flows
- structural damage associated with high-velocity water and/or debris flows
- structural damage associated with erosion
- maintenance associated with debris and sediment deposition

The conveyance structures within the development should be designed for the 100-year debris flow (where specified) and will greatly reduce the above potential hazards. All housing structures should be located above the 100-year water and debris flooding elevation within a channel.

5.1 FLOOD HAZARDS

FEMA Flood Insurance Rate Maps (FIRM) show the study area to be classified as “Other Areas”, Flood Hazard Zone X. As such, this area is determined to be outside the limits of the 500-year floodplain. Fishers Canyon was studied in detail as part of the FEMA Flood Insurance Study (FIS), however, the limits of the study appears to extend to just west of Colorado Route 115.

A statistical procedure exists for estimating the flooding hazards on alluvial fans (FEMA 1990). The evaluation is risk based and is primarily utilized for flood insurance purposes. This evaluation may be initiated by regulatory agencies for the proposed development area in the future.

In general, the natural drainages within the development area are deeply incised as a result of active alluvial fan development and are scattered across the fan surface. The flow from the major canyon watersheds have historically migrated across the alluvial fans leaving behind deeply incised channels as the flow paths have shifted over time. Current topographic conditions indicate large natural channels that appear to be “oversized” compared to the upstream watershed area.

5.2 DEBRIS FLOW HAZARDS

As recently as 1965, significant floods generated from strong, convective thunderstorms, have occurred in the vicinity of Ski Hill and Fishers Canyon. The 1965 storm generated significant runoff and conveyed mud and debris downstream of the upper watersheds north of the ski hill. There was no documentation of debris flow in Fishers Canyon or other drainages to the south

from the 1965 flood event, but some minor flows may have occurred. There is a potential for future debris flow where debris flows occurred in 1965 and in adjacent basins.

Estimates of the available sediment within the eastern Cheyenne Mountain watersheds indicate that the available sediment is less than the amount that could be transported during an extreme runoff event (on the order of 10 percent). However, due to the steepness of the watersheds and low vegetative cover in the upper portions of the watersheds, a large amount of sediment and debris could become available during larger than the 25-year frequency event and it is prudent to consider that debris flows will not be sediment limited. Additionally, fire in the watersheds could reduce the vegetative cover and increase the probability of erosion and available sediment. The hazard is associated primarily with 25-year size storms and larger. Mitigation measures following fire should be implemented immediately following such an event to reduce erosion within the watershed and the amount of sediment transported downstream.

Evaluation of the watersheds above the proposed development indicate that the potential for a debris flow exists in the steep canyons of Cheyenne Mountain. However, the existing NORAD facilities located near the apex of the majority of canyons above the proposed development would intercept debris flows passing through the facility. The existing facilities will essentially act as an energy barrier limiting the velocities of flow and minimizing the sediment transport capabilities of the runoff. The NORAD parking lot will spread out concentrated flows and cause deposition of suspended/transported material. Similarly, the small culverts under NORAD Road will cause velocities to decrease and deposition to occur.

A debris control/flood attenuation structure is proposed to be constructed at the apex of Fishers Canyon. This structure will greatly reduce the debris flow hazard and peak flow rates downstream through the Boulder Broadmoor Filing 1 and 1A. The structure has been designed to store the debris flow volume transported during the 100-year frequency event.

The volume of debris transported during a flood is an average of 25-40 percent of the water volume, and the peak debris discharge is estimated at 45-55 percent of the peak water discharge (O'Brien and Julien 1997). Previous analyses have recommended, for conveyance design purposes, a minimum sediment concentration of 50 percent by volume which results in a bulking factor of 2.00 for the peak discharge and runoff volume (FLO Engineering, 1995). The bulking factor is given as $BF=1 (Cl-Cv)$ where Cv is the average segment concentration by volume. The bulking factor accounts for increased volume taken up by sediment particles. A bulking factor of 2.00 is the recommended minimum for conveyance design of the larger drainages within the study area where the potential for a debris flow exists. A bulking factor of 1.20 is recommended for the smaller drainages below the existing NORAD facilities where the potential for debris flow is minor.

The debris flow hazard can be mitigated by avoidance, conveyance and storage. Avoidance and conveyance can be achieved by designing conveyance structures to adequately pass debris flows, maintaining the stability of debris flow channels and placing structures as high as possible above drainage channels.

This Master Development Drainage Plan provides peak design flow rates through the proposed development and indicates the need for drainage improvements for specific drainages affected by the proposed development. The off-site structures located downstream of the proposed development should be adequate to convey the peak 100-year discharge rates (bulked for sediment where appropriate) after development. Detailed design plans for proposed drainage facilities will be submitted following approval of submitted Preliminary and Final Drainage Reports and Plans.

A debris control/attenuation structure are recommended for Fishers Canyon in order to minimize debris flows and flooding during the design event. The structure will convey flows to the south of Star Ranch through the Boulders Broadmoor Filing 1 and 1A development. Flood attenuation provided by the structure will reduce peak discharges downstream. The proposed improvements for the Boulders Broadmoor Filings No. 1 and 1A development and downstream conveyance structures will be adequate to convey the 100-year design water flow from Fishers Canyon. The other drainages within the study area are much smaller in aerial extent and the probability of these drainages to generate a debris flow during a large, infrequent rainfall event is low. In addition, the existing NORAD facilities located above the proposed development will limit the amount of debris transported downstream. However, conveyance structures in drainages which originate in the steep canyons of Cheyenne Mountain (Basins 400 and 500) should be designed to carry a potential debris flood during the 100-year frequency event. In general, the structures should be designed using a peak bulking factor of 2.00 which corresponds to a peak sediment concentration of 50 percent.

In general, the peak flows determined by this MDDP are less than those previously determined. As such, downstream conveyance structures should be capable of passing the 100-year peak discharges bulked for a potential debris flood, where appropriate.

The following recommendations are made regarding the drainage facilities for the proposed development:

1. A debris control/flood attenuation structure will be constructed at the apex of Fishers Canyon. This structure will reduce peak discharges downstream and minimize flooding/debris flow hazards through the Boulders Broadmoor Filing 1 and 1A.
2. Conveyance structures within Basins 400 and 500 will be designed to convey potential debris flows by utilizing a minimum bulking factor of 2.0.
3. The existing NORAD facility will intercept the majority of runoff debris flows generated in the upper watersheds of the Basins 1, 2, 3 and 4. As such, the potential for debris flows through the development is expected to be minimal.
4. Drainage facilities should be designed to pass the peak 100-year discharges as presented in Table 5.

- Chen and Associates, 1980. "Preliminary Subsoil and Engineering Geology Investigation - The Broadmoor, Colorado Springs, Colorado."
- City of Colorado Springs and El Paso County, 1994. "Drainage Criteria Manual."
- Drexel Barrell, Inc. 1989. "Master Development Drainage Plan for a Portion of Broadmoor Bluffs at Cheyenne Mountain Ranch and Contiguous Drainage Areas, Colorado Springs, Colorado."
- Drexel Barrell, Inc., 1991. "Master Development Drainage Plan for Regional Detention Ponds at Broadmoor Bluffs."
- Federal Emergency Management Agency, 1997. Flood Insurance Rate Maps (FIRM), El Paso County, Colorado and Incorporated Areas. Panels 737, 739, 741, and 743. Effective date: March 17, 1997.
- FEMA, 1990. Fan: An Alluvial Fan Flooding Computer Program. Federal Emergency Management Agency, Washington, D.C.
- FLO Engineering, Inc., 1995. "Broadmoor South Property Design Flood Hydrology."
- FLO Engineering, Inc., 1996. "Broadmoor Residential Resort Community, Filing No. 1 - Final Drainage Report."
- FLO Engineering, Inc., 1996. Broadmoor Residential Resort Community, Phase 1 - Preliminary and Final Drainage Report. Revised, June 1997.
- FLO Engineering, Inc., 1997. "Broadmoor Residential Resort Community - North Master Development Drainage Plan."
- French, R.H., 1987. "Hydraulic Processes on Alluvial Fans. Elsevier Publishing.
- Jeffries, Donell, 1988. "Preliminary and Final Drainage Study, Glen Oaks at Broadmoor, Country Walk at Broadmoor, Glen Oaks at Broadmoor Filing No. 2."
- Julien, P.Y. and Lan, Y.Q., 1991. "On the Rheology of Hyperconcentration." Journal of Hydraulic Engineering, ASCE, 117 (3), 346-353.
- Julien, P.Y. and O'Brien, J.S., 1997. "On the Importance of Mud and Debris Flow Rheology in Structural Design." Proceedings of First International Conference on Debris-Flow Hazards Mitigation, San Francisco, California, August 7-9, 1997.
- KLH Engineering Consultants, Inc., 1982. "Master Drainage Plan for Broadmoor South - Neal Ranch Drainage Basin."
- Kiowa Engineering Corporation, 1996. "Preliminary and Final Drainage Study - Boulders Broadmoor Filing No. 1 and 1A, Colorado Springs, Colorado."
- KKBNA, Inc., 1984. "Drainage Study for Neil Ranch Filing 2, Broadmoor Bluffs Park Filing No. 8 and Farthing Drive."
- Lincoln Devore Testing Laboratory, Inc., 1981. "Debris Flow Hazard Analysis - Vicinity of Ski Broadmoor, The Broadmoor Property, Colorado Springs, Colorado."
- Muller Engineering Company, 1991. "Fisher's Canyon Drainage Basin Planning Study."

- O'Brien, J.S. and Julien, P.Y., 1988. "Laboratory Analysis of Mudflow Properties." *Journal of Hydraulic Engineering*, ASCE, 114 (8), 877-887.
- O'Brien, J.S., Julien, P.Y. and Fullerton, W.T., 1993. "Two-dimensional Water Flood and Mudflow Simulation." *ASCE Journal of Hydraulic Engineering*, Vol. 119, No. 2, pp. 244-259.
- O'Brien, J.S. and Julien, P.Y., 1997. "On the Importance of Mudflow Routing." *Proceedings of First International Conference on Debris-Flow Hazards Mitigation*, San Francisco, California, August 7-9, 1997.
- Resource Consultants, Inc., 1989. "Flood Insurance Study for the City of Colorado Springs and El Paso County, Colorado."
- United States Army Corps of Engineers, 1990. "HEC-1, Flood Hydrograph Package-User's Manual", Hydrologic Engineering Center, September.
- U.S. Department of Agriculture, 1972. "National Engineering Handbook, Section 4, Hydrology Soil Conservation Service."
- U.S. Department of Agriculture, 1981. "Soil Survey of El Paso County Area, Colorado." Soil Conservation Service.
- United States Department of Transportation, 1971. "Debris Control Structures," *Hydraulic Engineering Circular No. 9*, Federal Highway Administration.
- United States Geological Survey, 1994. "7.5 Minute Series Topographic Map - Colorado Springs Quadrangle, Colorado."
- United States Geological Survey, 1994. "7.5 Minute Series Topographic Map - Cheyenne Mountain Quadrangle, Colorado"
- Watts, Oliver E., 1988. "Neil Ranch Master Drainage Plan Update, Broadmoor Oaks Filings 3, 4, 7, 8, 9, and 10."
- Watts, Oliver E., 1989. "Preliminary Drainage Plan and Report - Broadmoor Bluffs No. 11 and Neil Ranch Master Drainage Plan Update."
- Weiss Consulting Engineers, Inc., 1987. "Master Drainage Study for Neil Ranch."
- Woodward-Clyde International-Americas, 1998. "Broadmoor Resort Community South Master Development Drainage Plan, Final Report, February 13, 1998."

**TABLE 1
SUMMARY OF HYDROLOGIC PARAMETERS**

Basin ID	Area (mi ²)	Curve Number		Lag Time		% Impervious
		Existing	Developed	Existing (hours)	Developed (hours)	
34	0.023	74	74	0.125	0.125	7
5	0.042	74	74	0.131	0.131	7
6	0.022	74	74	0.131	0.131	7
7	0.014	74	74	0.107	0.107	7
8-1	0.056	74	74	0.113	0.113	7
8-2	0.016	74	74	0.086	0.086	7
8-3	0.021	74	74	0.202	0.202	7
9	0.085	74	74	0.125	0.125	4
1011-1	0.035	74	74	0.160	0.160	7
1011-2	0.037	74	74	0.160	0.160	7
12-1	0.089	76	76	0.126	0.126	6
12-2	0.028	76	76	0.113	0.113	6
12-3	0.027	76	76	0.145	0.145	6
12-4	0.038	75	75	0.110	0.110	6
12-5	0.081	76	76	0.154	0.154	10
12-6	0.061	76	76	0.129	0.129	10
12-7	0.016	75	75	0.134	0.134	5
12-8	0.023	75	75	0.077	0.077	5
12-9	0.021	75	75	0.112	0.112	4
12-10	0.069	75	75	0.170	0.170	5
12-11	0.041	74	74	0.073	0.073	10
1A	0.023	78	78	0.15	0.150	5
1B	0.231	78	78	0.27	0.270	5
1C	0.037	78	78	0.22	0.220	5
1D	0.05	78	78	0.04	0.040	5
2	0.043	78	78	0.19	0.190	5
2X	0.005	78	78	0.13	0.130	5
3	0.069	78	78	0.26	0.260	5
4	0.096	78	78	0.21	0.210	5
A	0.091	58	72	0.25	0.230	5
B	0.018	58	72	0.15	0.140	5
C	0.088	58	72	0.19	0.170	5
D	0.023	58	72	0.160	0.160	5
E	0.01	58	72	0.14	0.140	5
F	0.009	58	72	0.13	0.130	5
G	0.034	58	72	0.19	0.190	5
H	0.03	58	72	0.19	0.170	5
300A	0.049	58	65	0.313	0.162	4
300B	0.026	58	68	0.247	0.126	4
300C	0.06	58	72	0.288	0.150	4
400A	0.02	58	72	0.406	0.210	4
400B	0.01	58	72	0.406	0.210	4
500A	0.036	58	72	0.313	0.162	4
500B	0.009	58	72	0.208	0.108	4
500C	0.015	58	72	0.208	0.108	4

**TABLE 2
5-YEAR PEAK DISCHARGE RESULTS**

Basin ID	Developed		Existing	
	Q _{5YR-2HR} (cfs)	5 Year Unit Peak Runoff (cfs/mi ²)	Q _{5YR-2HR} (cfs)	5 Year Unit Peak Runoff (cfs/mi ²)
Basin 500				
34	5	217	5	217
500C	2	133	1	67
5	9	214	9	214
6	5	227	5	227
500A	5	139	2	56
DP500	23	167	19	138
500B	1	11	1	11
DP501	24	163	20	136
Basin 400				
7	3	250	3	250
8-1	12	214	12	214
8-2	4	250	4	250
8-3	4	190	4	190
DP7A	22	210	22	210
400A	2	100	1	50
DP401	24	192	22	176
400B	1	33	<1	<100
DP402	24	178	22	163
Basin 300				
12-1	22	247	22	247
12-2	7	250	7	250
12-3	6	222	6	222
12-4	9	237	9	237
12-5	23	284	23	284
12-6	18	295	18	295
12-7	3	188	3	188
12-8	5	217	5	217
12-9	4	191	4	191
12-10	13	188	13	188
Fishers Canyon	57	126	--	--
12-11	11	275	11	275
DP6	61	124	109	221
I011-1	7	200	7	200
I011-2	7	189	7	189
DP8	70	124	121	214
300A	3	61	3	61
9	15	176	15	176
300B	2	77	2	77
DP301	84	109	136	177
300C	8	133	3	50
DP302	90	108	137	165
Basin A				
1A	6	261	6	261

**TABLE 2
5-YEAR PEAK DISCHARGE RESULTS**

Basin ID	Developed		Existing	
	Q _{5YR-2HR} (cfs)	5 Year _{Unit Peak Runoff} (cfs/mi ²)	Q _{5YR-2HR} (cfs)	5 Year _{Unit Peak Runoff} (cfs/mi ²)
1B	52	225	52	225
1C	9	243	9	243
DP 1BC	61	--	61	--
1D	18	360	18	360
DP 1ABCD	68	--	68	--
A	11	121	7	77
DP 1	70	--	63	--
Basin C				
2	11	256	11	256
2X	1	200	1	200
C	12	136	8	91
DP 2	18	--	11	--
Basin H				
3	16	232	16	232
4	24	250	24	250
H	4	133	3	100
DP 3	28	--	25	--
Basin B				
B	3	167	2	111
Basin D				
D	3	130	2	87
Basin E				
E	1	100	1	100
Basin F				
F	18	2000	1	111
Basin G				
G	4	118	3	88

**TABLE 3
100-YEAR PEAK DISCHARGE RESULTS**

Basin ID	Developed			Existing		
	Q _{100YR-2HR} (cfs)	V _{100YR-2HR} (ac-ft)	100 Year Unit Peak Runoff (cfs/mi ²)	Q _{100YR-2HR} (cfs)	V _{100YR-2HR} (ac-ft)	100 Year Unit Peak Runoff (cfs/mi ²)
Basin 500						
34	26	1	1130	26	1	1130
500C	15	1	1000	4	0	267
5	47	2	1119	47	2	1119
6	25	1	1136	25	1	1136
500A	31	2	861	8	1	222
DP500	133	7	964	98	6	710
500B	9	1	1000	2	0	222
DP501	141	8	959	100	6	680
Basin 400						
7	17	1	1417	17	1	1417
8-1	66	3	1179	66	3	1179
8-2	20	1	1250	20	1	1250
8-3	20	2	952	20	2	952
DP7A	117	7	1114	117	7	1114
400A	16	1	800	4	0	200
DP401	128	8	1024	118	8	944
400B	8	1	800	2	0	200
DP402	134	9	993	119	8	881
Basin 300						
12-1	112	5	1258	112	5	1258
12-2	37	2	1321	37	2	1321
12-3	32	2	1185	32	2	1185
12-4	47	3	1237	47	3	1237
12-5	102	5	1259	102	5	1259
12-6	82	3	1344	82	3	1344
12-7	18	1	1125	18	1	1125
12-8	31	1	1348	31	1	1348
12-9	25	2	1196	25	2	1196
12-10	72	4	1043	72	4	1043
Fishers Canyon	222	28	490	--	--	--
12-11	57	3	1390	57	3	1390
DP6	240	23	486	564	31	1142
1011-1	37	2	1057	37	2	1057
1011-2	39	3	1054	39	3	1054
DP8	274	28	484	630	36	113
300A	25	2	510	11	1	224
9	91	5	1071	91	2	1071
300B	18	1	692	6	1	231
DP301	362	36	471	718	42	935
300C	53	3	883	14	1	233
DP302	399	39	481	721	43	871
Basin A						
1A	30	2	1304	30	2	1304
1B	234	16	1013	234	16	1013
1C	41	3	1108	41	3	1108
DP 1BC	273	19	--	273	19	--
1D	93	3	1860	93	3	1860

**TABLE 3
100-YEAR PEAK DISCHARGE RESULTS**

Basin ID	Developed			Existing		
	Q _{100YR-2HR} (cfs)	V _{100YR-2HR} (ac-ft)	100 Year Unit Peak Runoff (cfs/mi ²)	Q _{100YR-2HR} (cfs)	V _{100YR-2HR} (ac-ft)	100 Year Unit Peak Runoff (cfs/mi ²)
DP 1 ABCD	302	22	--	302	22	--
A	69	5	758	24	2	264
DP 1	335	27	--	299	24	--
Basin C						
2	51	3	1186	51	3	1186
2X	7	<1	1400	7	<1	1400
C	78	5	886	25	2	284
DP 2	99	8	--	52	5	--
Basin H						
3	70	5	1014	70	5	1014
4	109	7	1135	109	7	1135
H	26	2	867	8	1	267
DP 3	135	13	--	124	12	--
Basin B						
B	18	1	1000	6	<1	333
Basin D						
D	18	1	783	6	<1	261
Basin E						
E	9	1	900	3	<1	300
Basin F						
F	123	7	13667	3	<1	333
Basin G						
G	25	2	735	8	1	235

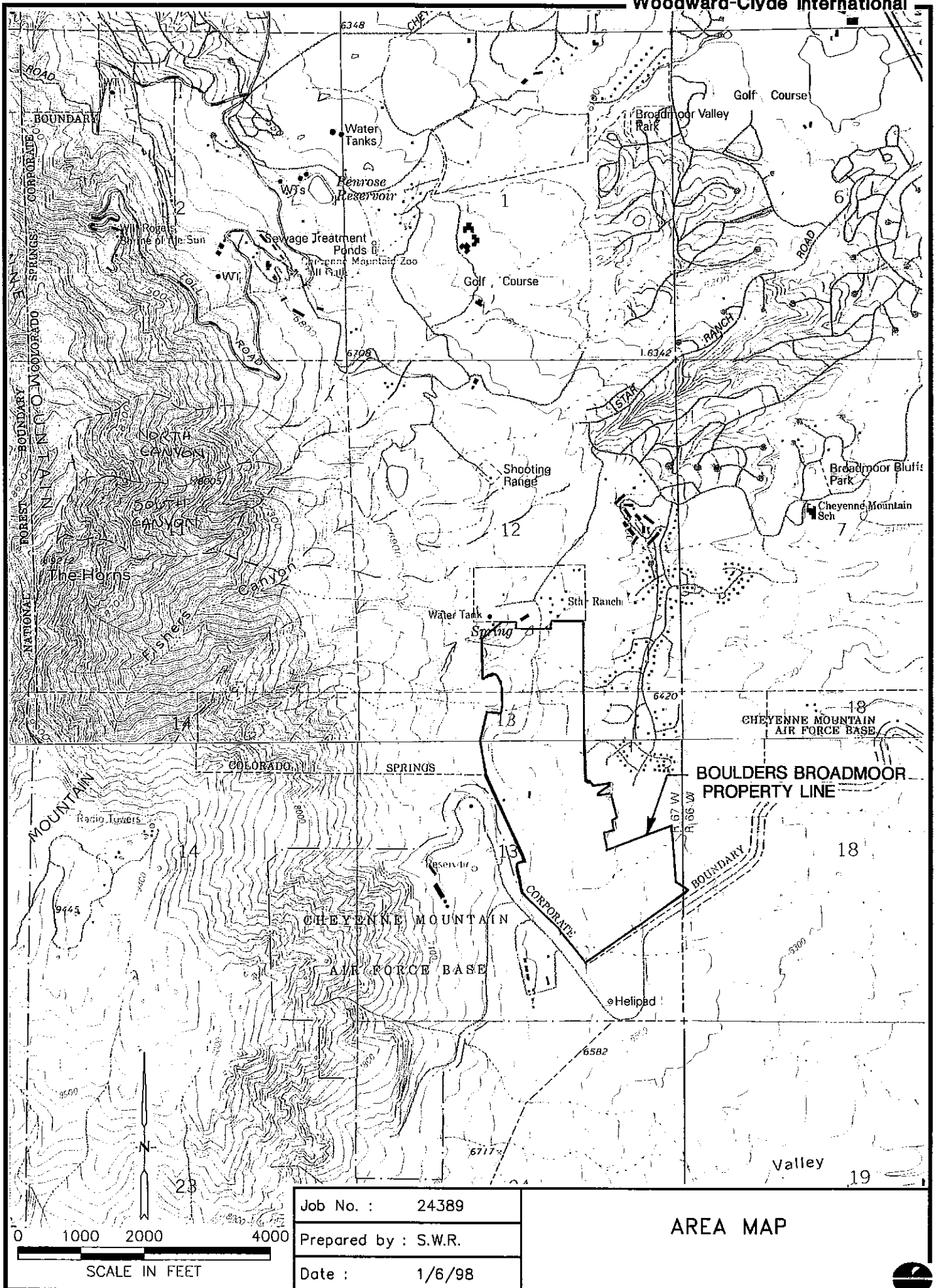
**TABLE 4
EXISTING DRAINAGE AND CONVEYANCE FACILITIES**

Location	Type & Size	Approximate Capacity (cfs)	100-Year Developed Capacity (cfs)
Basin 1			
DPIABCD	66"-CMP	380	310
Basin 2			
DPB2	30"-CMP	93	47
DPB2X	24"-CMP	45	6
Basin 3			
DP3	42"-CMP	69	66
Basin 4			
DP4	42"-CMP	119	101
Basin A			
DP1	84"-CMP	477	330
Basin B			
DP3	36"-CMP	124	17
Basin 300			
Jarman Street	2x66"-RCP	750	362
Stonebeck Lane	66"-RCP	450	362
Ellsworth Street	60"-RCP	366	399
Broadmoor Bluffs Drive	60"-CMP	390	399
Basin 400			
Broadmoor Bluffs Drive	78"-CMP	646	134
Star Ranch Road	18"-CMP	13	134
Ellsworth Street	60"-CMP	310	134
Basin 500			
Star Ranch Road	18"-CMP	14	72
Star Ranch Road	24"-HDPE	31	41
Star Ranch Road	18"-CMP	14	9
Ellsworth Street	48"-CMP	200	133

**TABLE 5
PROPOSED DRAINAGE AND CONVEYANCE FACILITIES**

Location	Type & Size	5-Year Developed Capacity (cfs)	100-Year Developed Capacity (cfs)	Minimum Capacity (cfs)	Debris Flow Bulking Factor
Basin B					
DPB	Culvert Crossing	2	12	14	1.2
Basin C					
DPC	Culvert Crossing	12	72	86	1.2
Basin D					
DPD	Culvert Crossing	2	6	7	1.2
Basin E					
DPE	Culvert Crossing	1	3	4	1.2
Basin F					
DPF	Culvert Crossing	1	6	7	1.2
Basin G					
DPG	24"-CMP ⁽¹⁾	3	17	20	1.2
Basin 300					
Berm Adj. to 300B	Earthfill/riprap channel	17	91	180	2
Fishers Canyon Dam	Debris Basin	110	564	1120	2
--	Drainage Easement	121	370	370	1
--	Drainage Easement	17	109	218	2
Basin 400					
--	Drainage Easement	20	128	260	2
Basin 500					
Ellsworth Street Expansion	Culvert Crossing	16	133	270	2
Berm Adj. to 500A	Earthfill/riprap channel	12	72	144	2
--	Drainage Easement	12	72	144	2
--	Drainage Easement	4	26	52	2

⁽¹⁾ Installation of 24"-CMP at proposed NORAD entry facility



4389TB

AREA MAP

FIG. 1


```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* SEPTEMBER 1990
* VERSION 4.0
*
* RUN DATE 02/24/1998 TIME 09:38:21
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN									
2	ID	5-YR,2-HR EXISTING				PART 1					
3	ID	FULL BASIN RUNOFF STUDY				5BX1.DAT					
	*DIAGRAM										
4	IT	1	01DEC97	1200	1500						
5	IN	5									
6	IO	5	0								
7	KK	B34									
8	KM	RUNOFF FROM SDA				34					
9	BA	.023									
10	PB	1.6									
11	PI	.01	.03	.04	.07	.12	.22	.12	.07	.05	.04
12	PI	.03	.03	.03	.02	.02	.01	.01	.01	.01	.01
13	PI	.01	.01	.01	.01						
14	LS	0	74	7							
15	UD	.125									
16	KK	J-J									
17	KM	ROUTE FLOW FROM B3 THRU 500C									
18	RS	1	STOR	0							
19	RC	0.12	.08	0.12	1500	.21					
20	RX	0	5	10	20	28	38	43	48		
21	RY	15	12.5	10	0	0	10	12.5	15		
22	KK	500C									
23	KM	RUNOFF FROM 500C									
24	BA	.015									
25	LS	0	58	4							
26	UD	.208									
27	KK	DP345									
28	KM	COMBINE SDAs 34 & 500C									
29	HC	2									
30	KK	B5									
31	KM	RUNOFF FROM SDA 5									
32	BA	.042									
33	LS	0	74	7							
34	UD	.131									

35 KK B6
 36 KM RUNOFF FROM SDA 6
 37 BA .022
 38 LS 0 74 7
 39 UD .131

 40 KK DP56
 41 KM COMBINE FLOWS FROM SDAs 5&6
 42 HC 2

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

43 KK K-K
 44 KM ROUTE FLOW FROM DP 56 THRU 500A
 45 RS 1 STOR 0
 46 RC 0.12 .08 0.12 2100 .22
 47 RX 0 5 10 20 28 38 43 48
 48 RY 15 12.5 10 0 0 10 12.5 15

49 KK 500A
 50 KM RUNOFF FROM SDA 500A
 51 BA .036
 52 LS 0 58 4
 53 UD .313

54 KK DP500
 55 KM COMBINE FLOWS FROM CP56&500A
 56 HC 3

57 KK 500B
 58 KM RUNOFF FROM SDA 500B
 59 BA .009
 60 LS 0 58 4
 61 UD .208

62 KK DP501
 63 KM COMBINE FLOWS FROM CP5A&500B
 64 HC 2

65 KK B7
 66 KM RUNOFF FROM SDA 7
 67 BA .014
 68 LS 0 74 7
 69 UD .107

70 KK B8-1
 71 KM RUNOFF FROM SDA 8-1
 72 BA .056
 73 LS 0 74 7
 74 UD .113

75 KK B8-2
 76 KM RUNOFF FROM SDA 8-2
 77 BA .016
 78 LS 0 74 7
 79 UD .086

80 KK DP7
 81 KM COMBINE FLOWS FROM SDAs 8-1, 8-2
 82 HC 2

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83 KK B8-3
 84 KM RUNOFF FROM SDA 8-3
 85 BA .021
 86 LS 0 74 7
 87 UD .202

88 KK DP7A
 89 KM COMBINE FLOW FROM CP7 & SDA 8-3 &B7
 90 HC 3

91 KK 400A
 92 KM RUNOFF FROM SDA 400A
 93 BA .02

```

94      LS      0      58      4
95      UD      .506

96      KK      DP401
97      KM      COMBINE FLOWS FROM DP7A & SDA 400A
98      HC      2

99      KK      400B
100     KM      RUNOFF FROM SDA      400B
101     BA      .01
102     LS      0      58      4
103     UD      .406

104     KK      DP402
105     KM      COMBINE FLOWS FROM DP402& SDA 400B
106     HC      2

107     KK      B1A
108     KM      RUNOFF FROM BASIN 1A
109     BA      0.023
110     LS      0      78      5
111     UD      0.15

112     KK      B1B
113     KM      RUNOFF FROM BASIN 1B
114     BA      0.231
115     LS      0      78      5
116     UD      0.27

117     KK      B1C
118     KM      RUNOFF FROM BASIN 1C
119     BA      0.037
120     LS      0      78      5
121     UD      0.22

122     KK      DP1BC
123     KM      COMBINE FLOWS FROM BASIN 1B & 1C
124     HC      2

```

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

125     KK      A-A
126     KM      ROUTE FLOW FROM DP1BC THRU BASIN 1D
127     RS      1      STOR      0
128     RC      0.12    0.08    0.12    1600    0.21
129     RX      0      5      10     20     28     38     43     48
130     RY      15     12.5    10     0      0      10     12.5    15

131     KK      B1D
132     KM      RUNOFF FROM BASIN 1D
133     BA      0.050
134     LS      0      78      5
135     UD      0.04

136     KK      DP1ABCD
137     KM      COMBINE FLOWS FROM BASINS 1A THRU 1D
138     HC      2

139     KK      C-C
140     KM      ROUTE FLOW FROM BASINS 1A-1D THRU BASIN A
141     RS      1      STOR      0
142     RC      0.12    0.08    0.12    4000    .08
143     RX      0      5      10     20     28     38     43     48
144     RY      15     12.5    10     0      0      10     12.5    15

145     KK      BA
146     KM      RUNOFF FROM BASIN A
147     BA      0.09
148     LS      0      58      5
149     UD      0.25

150     KK      DP1
151     KM      COMBINE FLOWS FROM BASINS 1A-1D & BASIN A
152     HC      2

153     KK      B2
154     KM      RUNOFF FROM BASIN 2
155     BA      0.043

```

156	LS	0	78	5					
157	UD	0.19							
158	KK	D-D							
159	KM	ROUTE FLOW FROM BASIN 2 THRU BASIN C							
160	RS	1	STOR	0					
161	RC	0.12	0.08	0.12	6400	0.07			
162	RX	0	5	10	20	28	38	43	48
163	RY	15	12.5	10	0	0	10	12.5	15
164	KK	B2X							
165	KM	RUNOFF FROM BASIN 2X							
166	BA	.005							
167	LS	0	78	5					
168	UD	0.13							

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

169	KK	X-X							
170	KM	ROUTE FLOW FROM BASIN 2X THRU BASIN C							
171	RS	1	STOR	0					
172	RC	0.12	0.08	0.12	490	0.49			
173	RX	0	5	10	20	28	38	43	48
174	RY	15	12.5	10	0	0	10	12.5	15
175	KK	BC							
176	KM	RUNOFF FROM BASIN C							
177	BA	0.09							
178	LS	0	58	5					
179	UD	0.19							
180	KK	DP2							
181	KM	COMBINE FLOWS FROM BASINS 2, 2X & C							
182	HC	3							
183	KK	B3							
184	KM	RUNOFF FROM BASIN 3							
185	BA	0.0687							
186	LS	0	78	5					
187	UD	0.26							
188	KK	E-E							
189	KM	ROUTE FLOW FROM BASIN 3 THRU BASIN H							
190	RS	1	STOR	0					
191	RC	0.12	0.08	0.12	7200	0.03			
192	RX	0	5	10	20	28	38	43	48
193	RY	15	12.5	10	0	0	10	12.5	15
194	KK	B4							
195	KM	RUNOFF FROM BASIN 4							
196	BA	0.096							
197	LS	0	78	5					
198	UD	0.21							
199	KK	F-F							
200	KM	ROUTE FLOW FROM BASIN 4 THRU BASIN H							
201	RS	1	STOR	0					
202	RC	0.12	0.08	0.12	2800	0.05			
203	RX	0	5	10	20	28	38	43	48
204	RY	15	12.5	10	0	0	10	12.5	15
205	KK	BH							
206	KM	RUNOFF FROM BASIN H							
207	BA	0.03							
208	LS	0	58	5					
209	UD	0.19							

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

210	KK	DP3							
211	KM	COMBINE FLOWS FROM BASINS 3, 4 & H							
212	HC	3							
213	KK	BB							
214	KM	RUNOFF FROM BASIN B							
215	BA	0.02							

216	LS	0	58	5
217	UD	0.15		
218	KK	BD		
219	KM	RUNOFF FROM BASIN D		
220	BA	0.02		
221	LS	0	58	5
222	UD	0.16		
223	ZZ			

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW	
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW	
7	B34		
	V		
	V		
16	J-J		
	.		
22	500C		
	.		
27	DP345	
	.		
30	B5		
	.		
35		B6	
	.	.	
40	DP56	
	V		
	V		
43	K-K		
	.		
49		500A	
	.	.	
54	DP500	
	.		
57	500B		
	.		
62	DP501	
	.		
65	B7		
	.		
70		B8-1	
	.	.	
75		B8-2	
	.	.	
80		DP7
	.	.	
83		B8-3	
	.	.	
88	DP7A	
	.	.	
91		400A	
	.	.	
96	DP401	
	.	.	
99		400B	
	.	.	
104	DP402	

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
 5-YR,2-HR EXISTING PART 1
 FULL BASIN RUNOFF STUDY 5BX1.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

6 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 1 MINUTES IN COMPUTATION INTERVAL
 IDATE 1DEC97 STARTING DATE
 ITIME 1200 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1DEC97 ENDING DATE
 NDTIME 1659 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .02 HOURS
 TOTAL TIME BASE 4.98 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	B34	5.	.70	1.	1.	1.	.02		
ROUTED TO	J-J	5.	.77	1.	1.	1.	.02	.08	.77
HYDROGRAPH AT	500C	1.	.68	0.	0.	0.	.01		
2 COMBINED AT	DP345	5.	.75	1.	1.	1.	.04		
HYDROGRAPH AT	B5	9.	.72	2.	2.	2.	.04		
HYDROGRAPH AT	B6	5.	.72	1.	1.	1.	.02		
2 COMBINED AT	DP56	13.	.72	2.	2.	2.	.06		
ROUTED TO	K-K	12.	.80	2.	2.	2.	.06	.20	.80
HYDROGRAPH AT	500A	2.	.80	0.	0.	0.	.04		
3 COMBINED AT	DP500	19.	.78	4.	4.	4.	.14		

+	HYDROGRAPH AT	500B	1.	.68	0.	0.	0.	.01		
+	2 COMBINED AT	DP501	20.	.78	4.	4.	4.	.15		
+	HYDROGRAPH AT	B7	3.	.68	1.	1.	1.	.01		
+	HYDROGRAPH AT	BB-1	12.	.68	2.	2.	2.	.06		
+	HYDROGRAPH AT	BB-2	4.	.65	1.	1.	1.	.02		
+	2 COMBINED AT	DP7	15.	.68	3.	3.	3.	.07		
+	HYDROGRAPH AT	BB-3	4.	.80	1.	1.	1.	.02		
+	3 COMBINED AT	DP7A	22.	.70	4.	4.	4.	.11		
+	HYDROGRAPH AT	400A	1.	1.00	0.	0.	0.	.02		
+	2 COMBINED AT	DP401	22.	.70	4.	4.	4.	.13		
+	HYDROGRAPH AT	400B	0.	.88	0.	0.	0.	.01		
+	2 COMBINED AT	DP402	22.	.70	4.	4.	4.	.14		
+	HYDROGRAPH AT	B1A	6.	.73	1.	1.	1.	.02		
+	HYDROGRAPH AT	B1B	52.	.90	10.	10.	10.	.23		
+	HYDROGRAPH AT	B1C	9.	.83	2.	2.	2.	.04		
+	2 COMBINED AT	DP1BC	61.	.88	12.	12.	12.	.27		
+	ROUTED TO	A-A	60.	.93	12.	12.	12.	.27	.90	.93
+	HYDROGRAPH AT	B1D	18.	.60	2.	2.	2.	.05		
+	2 COMBINED AT	DP1ABC	68.	.93	14.	14.	14.	.32		
+	ROUTED TO	C-C	60.	1.12	14.	14.	14.	.32	1.19	1.12
+	HYDROGRAPH AT	BA	7.	.73	1.	1.	1.	.09		
+	2 COMBINED AT	DP1	63.	1.10	15.	15.	15.	.41		
+	HYDROGRAPH AT	B2	11.	.78	2.	2.	2.	.04		
+	ROUTED TO	D-D	7.	1.23	2.	2.	2.	.04	.19	1.23
+	HYDROGRAPH AT	B2X	1.	.70	0.	0.	0.	.00		
+	ROUTED TO	X-X	1.	.72	0.	0.	0.	.00		

									.02	.72
+										
+	HYDROGRAPH AT	BC	8.	.67	1.	1.	1.	.09		
+	3 COMBINED AT	DP2	11.	.73	3.	3.	3.	.14		
+	HYDROGRAPH AT	B3	16.	.88	3.	3.	3.	.07		
+	ROUTED TO	E-E	8.	1.47	3.	3.	3.	.07	.36	1.47
+	HYDROGRAPH AT	B4	24.	.82	4.	4.	4.	.10		
+	ROUTED TO	F-F	18.	1.07	4.	4.	4.	.10	.64	1.07
+	HYDROGRAPH AT	BH	3.	.67	0.	0.	0.	.03		
+	3 COMBINED AT	DP3	25.	1.15	8.	8.	8.	.19		
+	HYDROGRAPH AT	BB	2.	.62	0.	0.	0.	.02		
+	HYDROGRAPH AT	BD	2.	.63	0.	0.	0.	.02		

*** NORMAL END OF HEC-1 ***

36	KK	DP1								
37	KM	COMBINE FLOWS FROM SDAs 12-1, 12-2								
38	HC	2								
39	KK	A-A								
40	KM	ROUTE FLOWS FROM CP1 TO CP2								
41	RS	1	STOR	0						
42	RC	0.12	.08	0.12	630	0.24				
43	RX	0	5	10	20	28	38	43	48	
44	RY	15	12.5	10	0	0	10	12.5	15	

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

45	KK	B12-3								
46	KM	RUNOFF FROM SDA 12-3								
47	BA	.027								
48	LS	0	76	6						
49	UD	.145								
50	KK	B12-4								
51	KM	RUNOFF FROM SDA 12-4								
52	BA	.038								
53	LS	0	75	6						
54	UD	.110								

55	KK	B12-7								
56	KM	RUNOFF FROM SDA 12-7								
57	BA	.016								
58	LS	0	75	5						
59	UD	.134								

60	KK	DP2								
61	KM	COMBINE CP1 & SDAs 12-3, 12-4, 12-7								
62	HC	4								

63	KK	B-B								
64	KM	ROUTE FLOWS FROM CP2 TO CP3								
65	RS	1	STOR	0						
66	RC	0.12	.08	0.12	790	0.19				
67	RX	0	5	10	20	28	38	43	48	
68	RY	15	12.5	10	0	0	10	12.5	15	

69	KK	B12-5								
70	KM	RUNOFF FROM SDA 12-5								
71	BA	.081								
72	LS	0	76	10						
73	UD	.154								

74	KK	B12-8								
75	KM	RUNOFF FROM SDA 12-8								
76	BA	.023								
77	LS	0	75	5						
78	UD	.077								

79	KK	DP3								
80	KM	COMBINE FLOWS FROM CP2 & SDAs 12-5, 12-8								
81	HC	3								

82	KK	C-C								
83	KM	ROUTE FLOW FROM CP3 TO CP4								
84	RS	1	STOR	0						
85	RC	0.12	.08	0.12	650	0.26				
86	RX	0	5	10	20	28	38	43	48	
87	RY	15	12.5	10	0	0	10	12.5	15	

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	B12-6								
89	KM	RUNOFF FROM SDA 12-6								
90	BA	.061								
91	LS	0	76	10						
92	UD	.129								

93	KK	B12-9								
94	KM	RUNOFF FROM SDA 12-9								

95	BA	.021							
96	LS	0	75	4					
97	UD	.112							
98	KK	DP4							
99	KM	COMBINE FLOWS FROM CP3 & SDAs 12-6, 12-9							
100	HC	3							
101	KK	D-D							
102	KM	ROUTE FLOWS FROM CP4 TO CP5							
103	RS	1	STOR	0					
104	RC	0.12	.08	0.12	1420	0.19			
105	RX	0	5	10	20	28	38	43	48
106	RY	15	12.5	10	0	0	10	12.5	15
107	KK	B1210							
108	KM	RUNOFF FROM SDA 12-10							
109	BA	.069							
110	LS	0	75	5					
111	UD	.170							
112	KK	DP5							
113	KM	COMBINE FLOWS FROM CP4 & SDA 1210							
114	HC	2							
115	KK	E-E							
116	KM	ROUTE FLOW FROM CP5 TO CP6							
117	RS	1	STOR	0					
118	RC	0.12	.08	0.12	1420	0.11			
119	RX	0	5	10	20	28	38	43	48
120	RY	15	12.5	10	0	0	10	12.5	15
121	KK	B1211							
122	KM	RUNOFF FROM SDA 12-11							
123	BA	.041							
124	LS	0	74	10					
125	UD	.073							
126	KK	DP6							
127	KM	COMBINE DP5 & 1211							
128	HC	2							

HEC-1 INPUT

1
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

129	KK	F-F							
130	KM	ROUTE FLOW FROM 12-11 TO 1011-1							
131	RS	1	STOR	0					
132	RC	0.12	.08	0.12	1200	.13			
133	RX	0	5	10	20	28	38	43	48
134	RY	15	12.5	10	0	0	10	12.5	15
135	KK	B10111							
136	KM	RUNOFF FROM SDA 1011-1							
137	BA	.035							
138	LS	0	74	7					
139	UD	.160							
140	KK	B10112							
141	KM	RUNOFF FROM SDA 1011-2							
142	BA	.037							
143	LS	0	74	7					
144	UD	.160							
145	KK	DP8							
146	KM	COMBINE FLOW FROM SDA 1012 & CP6A							
147	HC	3							
148	KK	G-G							
149	KM	ROUTE FLOW FROM DP8 TO DP300							
150	RS	1	STOR	0					
151	RC	0.12	.08	0.12	1200	.12			
152	RX	0	5	10	20	28	38	43	48
153	RY	15	12.5	10	0	0	10	12.5	15
154	KK	300A							
155	KM	RUNOFF FROM 300A							
156	BA	.049							
157	LS	0	58	4					

158	UD	.313							
159	KK	DP300							
160	KM	COMBINE FLOWS FROM CP8 & SDA 300A							
161	HC	2							
162	KK	B9							
163	KM	RUNOFF FROM SDA 9							
164	BA	.085							
165	LS	0	74	4					
166	UD	.125							
167	KK	H-H							
168	KM	ROUTE FLOW FROM B9 THRU SDA 300B							
169	RS	1	STOR	0					
170	RC	0.12	.08	0.12	1200	.25			
171	RX	0	5	10	20	28	38	43	48
172	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

173	KK	300B							
174	KM	RUNOFF FROM 300B							
175	BA	.026							
176	LS	0	58	4					
177	UD	.247							
178	KK	DP9							
179	KM	COMBINE SDA B9 & 300B							
180	HC	2							
181	KK	DP301							
182	KM	COMBINE FLOWS FROM CP300 & SDA 300B							
183	HC	2							
184	KK	I-I							
185	KM	ROUTE FLOW FROM B9 THRU SDA 300B							
186	RS	1	STOR	0					
187	RC	0.12	.08	0.12	1800	.17			
188	RX	0	4	5.5	6.5	14.5	15.5	17	21
189	RY	7	5	2	0	0	2	5	7
190	KK	300C							
191	KM	RUNOFF FROM SDA 300C							
192	BA	.06							
193	LS	0	58	4					
194	UD	.288							
195	KK	DP302							
196	KM	COMBINE FLOWS FROM CP301 & 300C							
197	HC	2							
198	ZZ								

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (---->) DIVERSION OR PUMP FLOW

NO. (.) CONNECTOR (<----) RETURN OF DIVERTED OR PUMPED FLOW

7	BE								
	.								
16	.	BF							
	.	.							
21	.	.	BG						
	.	.	.						
26	.	.	.	B12-1					
					
31	B12-2				
				
36	DP1.....			
	V				
	V				
39	A-A				

45			B12-3	
50				B12-4
55				B12-7
60			DP2	
			V	
			V	
63			B-B	
69				B12-5
74				B12-8
79			DP3	
			V	
			V	
82			C-C	
88				B12-6
93				B12-9
98			DP4	
			V	
			V	
101			D-D	
107				B1210
112			DP5	
			V	
			V	
115			E-E	
121				B1211
126			DP6	
			V	
			V	
129			F-F	
135				B10111
140				B10112
145			DP8	
			V	
			V	
148			G-G	
154				300A
159			DP300	
162				B9
				V
				V

```

167      .      .      .      .      .      H-H
      .      .      .      .      .      .
173      .      .      .      .      .      .      300B
      .      .      .      .      .      .      .
178      .      .      .      .      .      .      DP9.....
      .      .      .      .      .      .      .
181      .      .      .      .      .      .      DP301.....
      .      .      .      .      .      .      V
      .      .      .      .      .      .      V
184      .      .      .      .      .      .      I-I
      .      .      .      .      .      .      .
190      .      .      .      .      .      .      300C
      .      .      .      .      .      .      .
195      .      .      .      .      .      .      DP302.....

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTMBER 1990 *
* VERSION 4.0 *
* RUN DATE 02/24/1998 TIME 09:38:00 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
5-YR,2-HR EXISTING PART 2
FULL BASIN RUNOFF STUDY 5BX2.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

6 IO OUTPUT CONTROL VARIABLES

```

IPRNT 5 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

```

IT HYDROGRAPH TIME DATA

```

NMIN 1 MINUTES IN COMPUTATION INTERVAL
IDATE 1DEC97 STARTING DATE
ITIME 1200 STARTING TIME
NQ 300 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 1DEC97 ENDING DATE
NDTIME 1659 ENDING TIME
ICENT 19 CENTURY MARK

```

```

COMPUTATION INTERVAL .02 HOURS
TOTAL TIME BASE 4.98 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

```

1

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+									
+	HYDROGRAPH AT								
	BE	1.	.62	0.	0.	0.	.01		
+	HYDROGRAPH AT								
	BF	1.	.60	0.	0.	0.	.01		

+		B10111	7.	.75	1.	1.	1.	.04		
	HYDROGRAPH AT									
+		B10112	7.	.75	1.	1.	1.	.04		
	3 COMBINED AT									
+		DP8	121.	.85	23.	23.	23.	.57		
	ROUTED TO									
+		G-G	120.	.88	23.	23.	23.	.57	1.64	.88
+	HYDROGRAPH AT									
		300A	3.	.80	0.	0.	0.	.05		
+	2 COMBINED AT									
		DP300	122.	.88	23.	23.	23.	.62		
+	HYDROGRAPH AT									
		B9	15.	.73	3.	3.	3.	.09		
+	ROUTED TO									
+		H-H	14.	.78	3.	3.	3.	.09	.22	.78
+	HYDROGRAPH AT									
		300B	2.	.72	0.	0.	0.	.03		
+	2 COMBINED AT									
		DP9	16.	.78	3.	3.	3.	.11		
+	2 COMBINED AT									
		DP301	136.	.88	26.	26.	26.	.73		
+	ROUTED TO									
+		I-I	134.	.92	26.	26.	26.	.73	1.68	.92
+	HYDROGRAPH AT									
		300C	3.	.77	1.	1.	1.	.06		
+	2 COMBINED AT									
		DP302	137.	.92	27.	27.	27.	.79		

*** NORMAL END OF HEC-1 ***

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 09 1992 *
* VERSION 4.0.3E *
*
* RUN DATE 02/24/98 TIME 09:45:17 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 551-1748 *
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

PAGE 1

```

1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
2 ID 100YR,2HR EXISTING PART 1
3 ID FULL BASIN RUNOFF STUDY 100BX1.DAT
  *DIAGRAM
4 IT 1 01DEC97 1200 1500
5 IN 5
6 IO 5 0
7 KK B34
8 KM RUNOFF FROM SDA 34
9 BA .023
10 PB 3.1
11 PI .01 .03 .04 .07 .12 .22 .12 .07 .05 .04
12 PI .03 .03 .03 .02 .02 .01 .01 .01 .01 .01
13 PI .01 .01 .01 .01
14 LS 0 74 7
15 UD .125
16 KK J-J
17 KM ROUTE FLOW FROM B3 THRU 500C
18 RS 1 STOR 0
19 RC 0.12 .08 0.12 1500 .21
20 RX 0 5 10 20 28 38 43 48
21 RY 15 12.5 10 0 0 10 12.5 15
22 KK 500C
23 KM RUNOFF FROM 500C
24 BA .015
25 LS 0 58 4
26 UD .208
27 KK DP345
28 KM COMBINE SDAs 34 & 500C
29 HC 2
30 KK B5
31 KM RUNOFF FROM SDA 5
32 BA .042
33 LS 0 74 7
34 UD .131

```

35 KK B6
 36 KM RUNOFF FROM SDA 6
 37 BA .022
 38 LS 0 74 7
 39 UD .131

 40 KK DP56
 41 KM COMBINE FLOWS FROM SDAs 5&6
 42 HC 2

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

43 KK K-K
 44 KM ROUTE FLOW FROM DP 56 THRU 500A
 45 RS 1 STOR 0
 46 RC 0.12 .08 0.12 2100 .22
 47 RX 0 5 10 20 28 38 43 48
 48 RY 15 12.5 10 0 0 10 12.5 15

49 KK 500A
 50 KM RUNOFF FROM SDA 500A
 51 BA .036
 52 LS 0 58 4
 53 UD .313

54 KK DP500
 55 KM COMBINE FLOWS FROM CP56&500A
 56 HC 3

57 KK 500B
 58 KM RUNOFF FROM SDA 500B
 59 BA .009
 60 LS 0 58 4
 61 UD .208

62 KK DP501
 63 KM COMBINE FLOWS FROM CP5A&500B
 64 HC 2

65 KK B7
 66 KM RUNOFF FROM SDA 7
 67 BA .014
 68 LS 0 74 7
 69 UD .107

70 KK B8-1
 71 KM RUNOFF FROM SDA 8-1
 72 BA .056
 73 LS 0 74 7
 74 UD .113

75 KK B8-2
 76 KM RUNOFF FROM SDA 8-2
 77 BA .016
 78 LS 0 74 7
 79 UD .086

80 KK DP7
 81 KM COMBINE FLOWS FROM SDAs 8-1, 8-2
 82 HC 2

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83 KK B8-3
 84 KM RUNOFF FROM SDA 8-3
 85 BA .021
 86 LS 0 74 7
 87 UD .202

88 KK DP7A
 89 KM COMBINE FLOW FROM CP7 & SDA 8-3 &B7
 90 HC 3

91 KK 400A
 92 KM RUNOFF FROM SDA 400A
 93 BA .02

```

94      LS      0      58      4
95      UD      .506

96      KK      DP401
97      KM      COMBINE FLOWS FROM DP7A & SDA 400A
98      HC      2

99      KK      400B
100     KM      RUNOFF FROM SDA      400B
101     BA      .01
102     LS      0      58      4
103     UD      .406

104     KK      DP402
105     KM      COMBINE FLOWS FROM DP402& SDA 400B
106     HC      2

107     KK      B1A
108     KM      RUNOFF FROM BASIN 1A
109     BA      0.023
110     LS      0      78      5
111     UD      0.15

112     KK      B1B
113     KM      RUNOFF FROM BASIN 1B
114     BA      0.231
115     LS      0      78      5
116     UD      0.27

117     KK      B1C
118     KM      RUNOFF FROM BASIN 1C
119     BA      0.037
120     LS      0      78      5
121     UD      0.22

122     KK      DP1BC
123     KM      COMBINE FLOWS FROM BASIN 1B & 1C
124     HC      2

```

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

125     KK      A-A
126     KM      ROUTE FLOW FROM DP1BC THRU BASIN 1D
127     RS      1      STOR      0
128     RC      0.12  0.08  0.12  1600  0.21
129     RX      0      5      10      20      28      38      43      48
130     RY      15     12.5  10      0      0      10     12.5  15

131     KK      B1D
132     KM      RUNOFF FROM BASIN 1D
133     BA      0.050
134     LS      0      78      5
135     UD      0.04

136     KK      DP1ABCD
137     KM      COMBINE FLOWS FROM BASINS 1A THRU 1D
138     HC      2

139     KK      C-C
140     KM      ROUTE FLOW FROM BASINS 1A-1D THRU BASIN A
141     RS      1      STOR      0
142     RC      0.12  0.08  0.12  4000  .08
143     RX      0      5      10      20      28      38      43      48
144     RY      15     12.5  10      0      0      10     12.5  15

145     KK      BA
146     KM      RUNOFF FROM BASIN A
147     BA      0.09
148     LS      0      58      5
149     UD      0.25

150     KK      DP1
151     KM      COMBINE FLOWS FROM BASINS 1A-1D & BASIN A
152     HC      2

153     KK      B2
154     KM      RUNOFF FROM BASIN 2
155     BA      0.043

```

156	LS	0	78	5					
157	UD	0.19							
158	KK	D-D							
159	KM	ROUTE FLOW FROM BASIN 2 THRU BASIN C							
160	RS	1	STOR	0					
161	RC	0.12	0.08	0.12	6400	0.07			
162	RX	0	5	10	20	28	38	43	48
163	RY	15	12.5	10	0	0	10	12.5	15
164	KK	B2X							
165	KM	RUNOFF FROM BASIN 2X							
166	BA	.005							
167	LS	0	78	5					
168	UD	0.13							

HEC-1 INPUT

PAGE 5

1
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

169	KK	X-X							
170	KM	ROUTE FLOW FROM BASIN 2X THRU BASIN C							
171	RS	1	STOR	0					
172	RC	0.12	0.08	0.12	490	0.49			
173	RX	0	5	10	20	28	38	43	48
174	RY	15	12.5	10	0	0	10	12.5	15
175	KK	BC							
176	KM	RUNOFF FROM BASIN C							
177	BA	0.09							
178	LS	0	58	5					
179	UD	0.19							
180	KK	DP2							
181	KM	COMBINE FLOWS FROM BASINS 2,2X & C							
182	HC	3							
183	KK	B3							
184	KM	RUNOFF FROM BASIN 3							
185	BA	0.0687							
186	LS	0	78	5					
187	UD	0.26							
188	KK	E-E							
189	KM	ROUTE FLOW FROM BASIN 3 THRU BASIN H							
190	RS	1	STOR	0					
191	RC	0.12	0.08	0.12	7200	0.03			
192	RX	0	5	10	20	28	38	43	48
193	RY	15	12.5	10	0	0	10	12.5	15
194	KK	B4							
195	KM	RUNOFF FROM BASIN 4							
196	BA	0.096							
197	LS	0	78	5					
198	UD	0.21							
199	KK	F-F							
200	KM	ROUTE FLOW FROM BASIN 4 THRU BASIN H							
201	RS	1	STOR	0					
202	RC	0.12	0.08	0.12	2800	0.05			
203	RX	0	5	10	20	28	38	43	48
204	RY	15	12.5	10	0	0	10	12.5	15
205	KK	BH							
206	KM	RUNOFF FROM BASIN H							
207	BA	0.03							
208	LS	0	58	5					
209	UD	0.19							

HEC-1 INPUT

PAGE 6

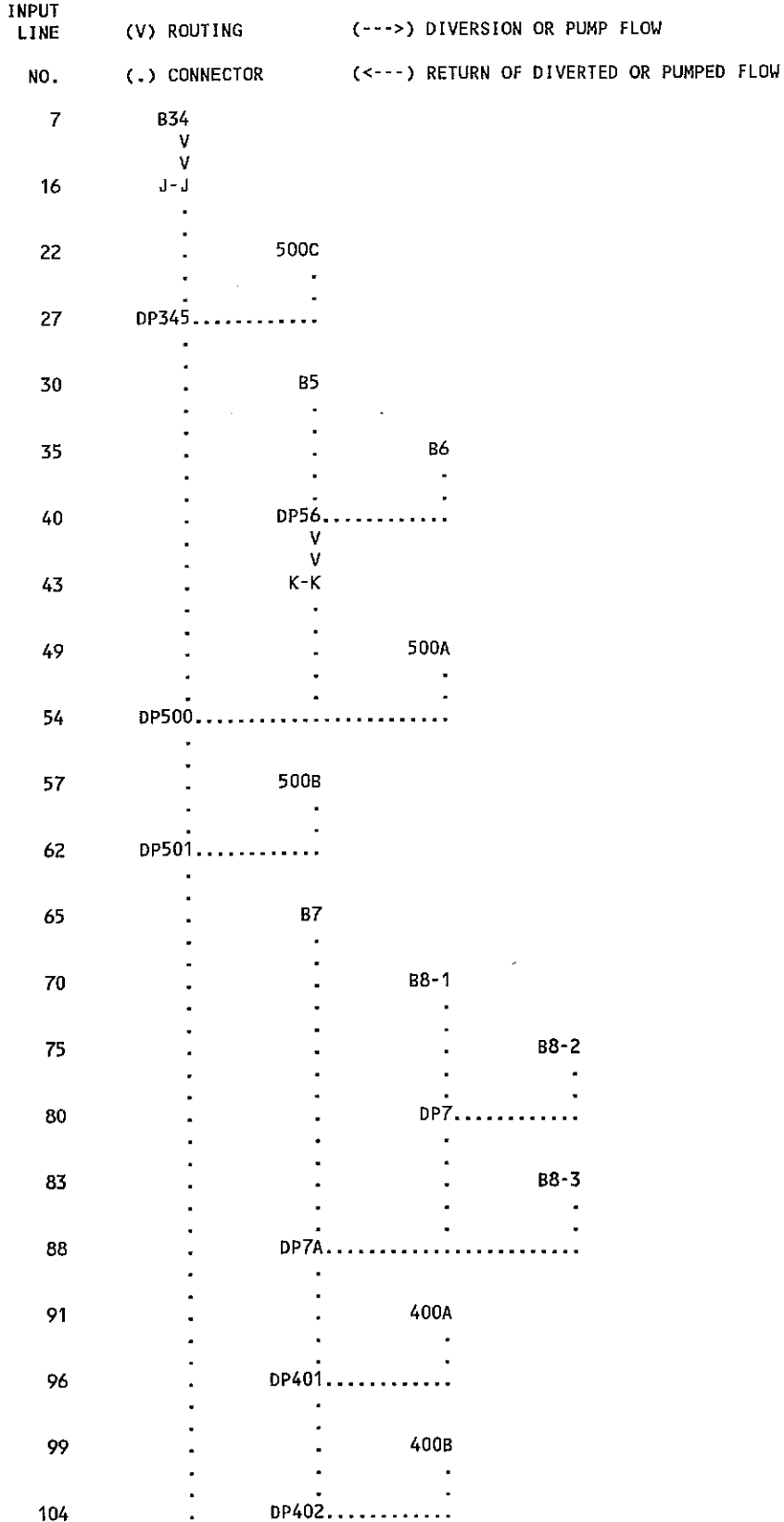
1
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

210	KK	DP3							
211	KM	COMBINE FLOWS FROM BASINS 3, 4 & H							
212	HC	3							
213	KK	BB							
214	KM	RUNOFF FROM BASIN B							
215	BA	0.02							

216	LS	0	58	5
217	UD	0.15		
218	KK	BD		
219	KM	RUNOFF FROM BASIN D		
220	BA	0.02		
221	LS	0	58	5
222	UD	0.16		
223	ZZ			

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



```

107      . . . . . B1A
      .
112      . . . . . B1B
      .
117      . . . . . B1C
      .
122      . . . . . DP1BC.....
      .
      . V
      . V
125      . . . . . A-A
      .
131      . . . . . B1D
      .
136      . . . . . DP1ABC.....
      .
      . V
      . V
139      . . . . . C-C
      .
145      . . . . . BA
      .
150      . . . . . DP1.....
      .
153      . . . . . B2
      .
      . V
      . V
158      . . . . . D-D
      .
164      . . . . . B2X
      .
      . V
      . V
169      . . . . . X-X
      .
175      . . . . . BC
      .
180      . . . . . DP2.....
      .
183      . . . . . B3
      .
      . V
      . V
188      . . . . . E-E
      .
194      . . . . . B4
      .
      . V
      . V
199      . . . . . F-F
      .
205      . . . . . BH
      .
210      . . . . . DP3.....
      .
213      . . . . . BB
      .
218      . . . . . BD

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 09 1992 *
* VERSION 4.0.3E *
*

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *

```

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
 100YR,2HR EXISTING PART 1
 FULL BASIN RUNOFF STUDY 100BX1.DAT

6 IO OUTPUT CONTROL VARIABLES

IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 1 MINUTES IN COMPUTATION INTERVAL
 IDATE 1DEC97 STARTING DATE
 ITIME 1200 STARTING TIME
 NQ 1500 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 2DEC97 ENDING DATE
 NDTIME 1259 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.02 HOURS
 TOTAL TIME BASE 24.98 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	B34	26.	0.65	3.	1.	1.	0.02		
ROUTED TO	J-J	24.	0.73	3.	1.	1.	0.02	0.41	0.73
HYDROGRAPH AT	500C	4.	0.87	1.	0.	0.	0.01		
2 COMBINED AT	DP345	27.	0.73	3.	1.	1.	0.04		
HYDROGRAPH AT	B5	47.	0.67	5.	1.	1.	0.04		
HYDROGRAPH AT	B6	25.	0.67	3.	1.	1.	0.02		
2 COMBINED AT	DP56	72.	0.67	8.	2.	2.	0.06		
ROUTED TO	K-K	66.	0.73	8.	2.	2.	0.06	0.94	0.73
HYDROGRAPH AT	500A	8.	1.02	2.	0.	0.	0.04		
3 COMBINED AT	DP500	98.	0.73	13.	3.	3.	0.14		
HYDROGRAPH AT	500B	2.	0.87	0.	0.	0.	0.01		

+	2 COMBINED AT	DP501	100.	0.73	13.	3.	3.	0.15		
+	HYDROGRAPH AT	B7	17.	0.63	2.	0.	0.	0.01		
+	HYDROGRAPH AT	B8-1	66.	0.65	7.	2.	2.	0.06		
+	HYDROGRAPH AT	B8-2	20.	0.60	2.	0.	0.	0.02		
+	2 COMBINED AT	DP7	86.	0.63	9.	2.	2.	0.07		
+	HYDROGRAPH AT	B8-3	20.	0.77	3.	1.	1.	0.02		
+	3 COMBINED AT	DP7A	117.	0.65	13.	3.	3.	0.11		
+	HYDROGRAPH AT	400A	4.	1.32	1.	0.	0.	0.02		
+	2 COMBINED AT	DP401	118.	0.65	14.	3.	3.	0.13		
+	HYDROGRAPH AT	400B	2.	1.17	0.	0.	0.	0.01		
+	2 COMBINED AT	DP402	119.	0.65	14.	4.	3.	0.14		
+	HYDROGRAPH AT	B1A	30.	0.68	3.	1.	1.	0.02		
+	HYDROGRAPH AT	B1B	234.	0.83	32.	8.	8.	0.23		
+	HYDROGRAPH AT	B1C	41.	0.77	5.	1.	1.	0.04		
+	2 COMBINED AT	DP1BC	273.	0.83	37.	9.	9.	0.27		
+	ROUTED TO	A-A	271.	0.87	37.	9.	9.	0.27	2.24	0.87
+	HYDROGRAPH AT	B1D	93.	0.52	7.	2.	2.	0.05		
+	2 COMBINED AT	DP1ABC	302.	0.85	44.	11.	11.	0.32		
+	ROUTED TO	C-C	275.	0.95	44.	11.	11.	0.32	2.98	0.95
+	HYDROGRAPH AT	BA	24.	0.92	4.	1.	1.	0.09		
+	2 COMBINED AT	DP1	299.	0.95	49.	12.	12.	0.41		
+	HYDROGRAPH AT	B2	51.	0.73	6.	1.	1.	0.04		
+	ROUTED TO	D-D	26.	1.08	6.	1.	1.	0.04	0.79	1.08
+	HYDROGRAPH AT	B2X	7.	0.65	1.	0.	0.	0.00		
+	ROUTED TO	X-X	7.	0.67	1.	0.	0.	0.00	0.08	0.67

+	HYDROGRAPH AT	BC	25.	0.83	4.	1.	1.	0.09		
+	3 COMBINED AT	DP2	52.	0.90	11.	3.	3.	0.14		
+	HYDROGRAPH AT	B3	70.	0.83	9.	2.	2.	0.07		
+	ROUTED TO	E-E	37.	1.27	9.	2.	2.	0.07	1.20	1.27
+	HYDROGRAPH AT	B4	109.	0.77	13.	3.	3.	0.10		
+	ROUTED TO	F-F	92.	0.88	13.	3.	3.	0.10	1.80	0.88
+	HYDROGRAPH AT	BH	8.	0.83	1.	0.	0.	0.03		
+	3 COMBINED AT	DP3	124.	0.95	24.	6.	6.	0.19		
+	HYDROGRAPH AT	BB	6.	0.77	1.	0.	0.	0.02		
+	HYDROGRAPH AT	BD	6.	0.78	1.	0.	0.	0.02		

*** NORMAL END OF HEC-1 ***

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*
* RUN DATE 02/24/1998 TIME 09:41:35 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN									
2	ID	100YR,2HR		EXISTING			PART 2				
3	ID	FULL BASIN RUNOFF STUDY					100BX2.DAT				
	*DIAGRAM										
4	IT	1	01DEC97	1200	1500						
5	IN	5									
6	IO	5	0								
7	KK	BE									
8	KM	RUNOFF FROM BASIN E									
9	BA	0.01									
10	PB	3.1									
11	PI	.01	.03	.04	.07	.12	.22	.12	.07	.05	.04
12	PI	.03	.03	.03	.02	.02	.01	.01	.01	.01	.01
13	PI	.01	.01	.01	.01						
14	LS	0	58	5							
15	UD	0.14									
16	KK	BF									
17	KM	RUNOFF FROM BASIN F									
18	BA	0.01									
19	LS	0	58	5							
20	UD	0.13									
21	KK	BG									
22	KM	RUNOFF FROM BASIN G									
23	BA	0.03									
24	LS	0	58	5							
25	UD	0.19									
26	KK	B12-1									
27	KM	RUNOFF FROM SDA 12-1									
28	BA	.089									
29	LS	0	76	6							
30	UD	.126									
31	KK	B12-2									
32	KM	RUNOFF FROM SDA 12-2									
33	BA	.028									
34	LS	0	76	6							
35	UD	.113									

36	KK	DP1							
37	KM	COMBINE FLOWS FROM SDAs 12-1, 12-2							
38	HC	2							
39	KK	A-A							
40	KM	ROUTE FLOWS FROM CP1 TO CP2							
41	RS	1	STOR	0					
42	RC	0.12	.08	0.12	630	0.24			
43	RX	0	5	10	20	28	38	43	48
44	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

45	KK	B12-3							
46	KM	RUNOFF FROM SDA 12-3							
47	BA	.027							
48	LS	0	76	6					
49	UD	.145							
50	KK	B12-4							
51	KM	RUNOFF FROM SDA 12-4							
52	BA	.038							
53	LS	0	75	6					
54	UD	.110							
55	KK	B12-7							
56	KM	RUNOFF FROM SDA 12-7							
57	BA	.016							
58	LS	0	75	5					
59	UD	.134							
60	KK	DP2							
61	KM	COMBINE CP1 & SDAs 12-3, 12-4, 12-7							
62	HC	4							
63	KK	B-B							
64	KM	ROUTE FLOWS FROM CP2 TO CP3							
65	RS	1	STOR	0					
66	RC	0.12	.08	0.12	790	0.19			
67	RX	0	5	10	20	28	38	43	48
68	RY	15	12.5	10	0	0	10	12.5	15
69	KK	B12-5							
70	KM	RUNOFF FROM SDA 12-5							
71	BA	.081							
72	LS	0	76	10					
73	UD	.154							
74	KK	B12-8							
75	KM	RUNOFF FROM SDA 12-8							
76	BA	.023							
77	LS	0	75	5					
78	UD	.077							
79	KK	DP3							
80	KM	COMBINE FLOWS FROM CP2 & SDAs 12-5, 12-8							
81	HC	3							
82	KK	C-C							
83	KM	ROUTE FLOW FROM CP3 TO CP4							
84	RS	1	STOR	0					
85	RC	0.12	.08	0.12	650	0.26			
86	RX	0	5	10	20	28	38	43	48
87	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	B12-6							
89	KM	RUNOFF FROM SDA 12-6							
90	BA	.061							
91	LS	0	76	10					
92	UD	.129							
93	KK	B12-9							
94	KM	RUNOFF FROM SDA 12-9							

95	BA	.021							
96	LS	0	75	4					
97	UD	.112							
98	KK	DP4							
99	KM	COMBINE FLOWS FROM CP3 & SDAs 12-6, 12-9							
100	HC	3							
101	KK	D-D							
102	KM	ROUTE FLOWS FROM CP4 TO CP5							
103	RS	1	STOR	0					
104	RC	0.12	.08	0.12	1420	0.19			
105	RX	0	5	10	20	28	38	43	48
106	RY	15	12.5	10	0	0	10	12.5	15
107	KK	B1210							
108	KM	RUNOFF FROM SDA 12-10							
109	BA	.069							
110	LS	0	75	5					
111	UD	.170							
112	KK	DP5							
113	KM	COMBINE FLOWS FROM CP4 & SDA 1210							
114	HC	2							
115	KK	E-E							
116	KM	ROUTE FLOW FROM CP5 TO CP6							
117	RS	1	STOR	0					
118	RC	0.12	.08	0.12	1420	0.11			
119	RX	0	5	10	20	28	38	43	48
120	RY	15	12.5	10	0	0	10	12.5	15
121	KK	B1211							
122	KM	RUNOFF FROM SDA 12-11							
123	BA	.041							
124	LS	0	74	10					
125	UD	.073							
126	KK	DP6							
127	KM	COMBINE DP5 & 1211							
128	HC	2							

HEC-1 INPUT

PAGE 4

1
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

129	KK	F-F							
130	KM	ROUTE FLOW FROM 12-11 TO 1011-1							
131	RS	1	STOR	0					
132	RC	0.12	.08	0.12	1200	.13			
133	RX	0	5	10	20	28	38	43	48
134	RY	15	12.5	10	0	0	10	12.5	15
135	KK	B10111							
136	KM	RUNOFF FROM SDA 1011-1							
137	BA	.035							
138	LS	0	74	7					
139	UD	.160							
140	KK	B10112							
141	KM	RUNOFF FROM SDA 1011-2							
142	BA	.037							
143	LS	0	74	7					
144	UD	.160							
145	KK	DP8							
146	KM	COMBINE FLOW FROM SDA 1012 & CP6A							
147	HC	3							
148	KK	G-G							
149	KM	ROUTE FLOW FROM DP8 TO DP300							
150	RS	1	STOR	0					
151	RC	0.12	.08	0.12	1200	.12			
152	RX	0	5	10	20	28	38	43	48
153	RY	15	12.5	10	0	0	10	12.5	15
154	KK	300A							
155	KM	RUNOFF FROM 300A							
156	BA	.049							
157	LS	0	58	4					

158 UD .313
 159 KK DP300
 160 KM COMBINE FLOWS FROM CP8 & SDA 300A
 161 HC 2
 162 KK B9
 163 KM RUNOFF FROM SDA 9
 164 BA .085
 165 LS 0 74 4
 166 UD .125
 167 KK H-H
 168 KM ROUTE FLOW FROM B9 THRU SDA 300B
 169 RS 1 STOR 0
 170 RC 0.12 .08 0.12 1200 .25
 171 RX 0 5 10 20 28 38 43 48
 172 RY 15 12.5 10 0 0 10 12.5 15

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

173 KK 300B
 174 KM RUNOFF FROM 300B
 175 BA .026
 176 LS 0 58 4
 177 UD .247
 178 KK DP9
 179 KM COMBINE SDA B9 & 300B
 180 HC 2
 181 KK DP301
 182 KM COMBINE FLOWS FROM CP300 & SDA 300B
 183 HC 2
 184 KK I-1
 185 KM ROUTE FLOW FROM B9 THRU SDA 300B
 186 RS 1 STOR 0
 187 RC 0.12 .08 0.12 1800 .17
 188 RX 0 4 5.5 6.5 14.5 15.5 17 21
 189 RY 7 5 2 0 0 2 5 7
 190 KK 300C
 191 KM RUNOFF FROM SDA 300C
 192 BA .06
 193 LS 0 58 4
 194 UD .288
 195 KK DP302
 196 KM COMBINE FLOWS FROM CP301 & 300C
 197 HC 2
 198 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

  7  BE
    .
  16 . BF
    .
    .
  21 . BG
    .
    .
  26 . B12-1
    .
    .
  31 . B12-2
    .
    .
  36 . DP1.....
    .
    .
  39 . A-A
  
```

45			B12-3	
50				B12-4
55				B12-7
60		DP2	-----	
		V		
		V		
63		B-B		
69			B12-5	
74				B12-8
79		DP3	-----	
		V		
		V		
82		C-C		
88			B12-6	
93				B12-9
98		DP4	-----	
		V		
		V		
101		D-D		
107			B1210	
112		DP5	-----	
		V		
		V		
115		E-E		
121			B1211	
126		DP6	-----	
		V		
		V		
129		F-F		
135			B10111	
140				B10112
145		DP8	-----	
		V		
		V		
148		G-G		
154			300A	
159		DP300	-----	
162			B9	
			V	
			V	

```

167      .      .      .      .      .      H-H
      .      .      .      .      .      .
173      .      .      .      .      .      .      300B
      .      .      .      .      .      .      .
178      .      .      .      .      .      .      .      DP9.....
      .      .      .      .      .      .      .
181      .      .      .      .      .      .      .      DP301.....
      .      .      .      .      .      .      .      V
      .      .      .      .      .      .      .      V
184      .      .      .      .      .      .      .      I-I
      .      .      .      .      .      .      .
190      .      .      .      .      .      .      .      300C
      .      .      .      .      .      .      .
195      .      .      .      .      .      .      .      DP302.....

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 02/24/1998 TIME 09:41:35 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
100YR,2HR EXISTING PART 2
FULL BASIN RUNOFF STUDY 100BX2.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN      1 MINUTES IN COMPUTATION INTERVAL
          IDATE     1DEC97 STARTING DATE
          ITIME     1200 STARTING TIME
          NQ        300 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    1DEC97 ENDING DATE
          NDTIME    1659 ENDING TIME
          ICENT     19 CENTURY MARK

          COMPUTATION INTERVAL .02 HOURS
          TOTAL TIME BASE      4.98 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+	BE	3.	.75	1.	1.	1.	.01		
+	HYDROGRAPH AT								
+	BF	3.	.73	1.	1.	1.	.01		

+		B10111	37.	.70	5.	5.	5.	.04		
+	HYDROGRAPH AT	B10112	39.	.70	5.	5.	5.	.04		
+	3 COMBINED AT	DP8	630.	.75	87.	87.	87.	.57		
+	ROUTED TO	G-G	624.	.77	87.	87.	87.	.57	4.19	.77
+	HYDROGRAPH AT	300A	11.	1.02	3.	3.	3.	.05		
+	2 COMBINED AT	DP300	632.	.77	90.	90.	90.	.62		
+	HYDROGRAPH AT	B9	91.	.67	12.	12.	12.	.09		
+	ROUTED TO	H-H	89.	.68	12.	12.	12.	.09	1.07	.68
+	HYDROGRAPH AT	300B	6.	.92	1.	1.	1.	.03		
+	2 COMBINED AT	DP9	93.	.70	13.	13.	13.	.11		
+	2 COMBINED AT	DP301	718.	.77	103.	103.	103.	.73		
+	ROUTED TO	I-I	710.	.78	103.	103.	103.	.73	4.31	.78
+	HYDROGRAPH AT	300C	14.	.98	3.	3.	3.	.06		
+	2 COMBINED AT	DP302	721.	.78	106.	106.	106.	.79		

*** NORMAL END OF HEC-1 ***


```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* SEPTEMBER 1990
* VERSION 4.0
*
* RUN DATE 02/24/1998 TIME 09:38:40
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN									
2	ID	5-YR,2-HR DEVELOPED			PART 1						
3	ID	FULL BASIN RUNOFF STUDY					5BD1.DAT				
	*DIAGRAM										
4	IT	1	01DEC97	1200	1500						
5	IN	5									
6	IO	5	0								
7	KK	B34									
8	KM	RUNOFF FROM SDA			34						
9	BA	.023									
10	PB	1.6									
11	PI	.01	.03	.04	.07	.12	.22	.12	.07	.05	.04
12	PI	.03	.03	.03	.02	.02	.01	.01	.01	.01	.01
13	PI	.01	.01	.01	.01						
14	LS	0	74	7							
15	UD	.125									
16	KK	J-J									
17	KM	ROUTE FLOW FROM B3 THRU 500C									
18	RS	1	STOR	0							
19	RC	0.12	.08	0.12	1500	.21					
20	RX	0	5	10	20	28	38	43	48		
21	RY	15	12.5	10	0	0	10	12.5	15		
22	KK	500C									
23	KM	RUNOFF FROM 500C									
24	BA	.015									
25	LS	0	72	4							
26	UD	.108									
27	KK	DP345									
28	KM	COMBINE SDAs 34 & 500C									
29	HC	2									
30	KK	B5									
31	KM	RUNOFF FROM SDA 5									
32	BA	.042									
33	LS	0	74	7							
34	UD	.131									

35 KK B6
 36 KM RUNOFF FROM SDA 6
 37 BA .022
 38 LS 0 74 7
 39 UD .131

 40 KK DP56
 41 KM COMBINE FLOWS FROM SDAs 5&6
 42 HC 2

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

43 KK K-K
 44 KM ROUTE FLOW FROM DP 56 THRU 500A
 45 RS 1 STOR 0
 46 RC 0.12 .08 0.12 2100 .22
 47 RX 0 5 10 20 28
 48 RY 15 12.5 10 0 0 38 43 48
 10 12.5 15

49 KK 500A
 50 KM RUNOFF FROM SDA 500A
 51 BA .036
 52 LS 0 72 4
 53 UD .162

54 KK DP500
 55 KM COMBINE FLOWS FROM CP56&500A
 56 HC 3

57 KK 500B
 58 KM RUNOFF FROM SDA 500B
 59 BA .009
 60 LS 0 72 4
 61 UD .108

62 KK DP501
 63 KM COMBINE FLOWS FROM CP5A&500B
 64 HC 2

65 KK B7
 66 KM RUNOFF FROM SDA 7
 67 BA .014
 68 LS 0 74 7
 69 UD .107

70 KK B8-1
 71 KM RUNOFF FROM SDA 8-1
 72 BA .056
 73 LS 0 74 7
 74 UD .113

75 KK B8-2
 76 KM RUNOFF FROM SDA 8-2
 77 BA .016
 78 LS 0 74 7
 79 UD .086

80 KK DP7
 81 KM COMBINE FLOWS FROM SDAs 8-1, 8-2
 82 HC 2

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83 KK B8-3
 84 KM RUNOFF FROM SDA 8-3
 85 BA .021
 86 LS 0 74 7
 87 UD .202

88 KK DP7A
 89 KM COMBINE FLOW FROM CP7 & SDA 8-3 &B7
 90 HC 3

91 KK 400A
 92 KM RUNOFF FROM SDA 400A
 93 BA .02

94 LS 0 72 4
 95 UD .210

 96 KK DP401
 97 KM COMBINE FLOWS FROM DP7A & SDA 400A
 98 HC 2

 99 KK 400B
 100 KM RUNOFF FROM SDA 400B
 101 BA .01
 102 LS 0 72 4
 103 UD .210

 104 KK DP402
 105 KM COMBINE FLOWS FROM DP402& SDA 400B
 106 HC 2

 107 KK B1A
 108 KM RUNOFF FROM BASIN 1A
 109 BA 0.023
 110 LS 0 78 5
 111 UD 0.15

 112 KK B1B
 113 KM RUNOFF FROM BASIN 1B
 114 BA 0.231
 115 LS 0 78 5
 116 UD 0.27

 117 KK B1C
 118 KM RUNOFF FROM BASIN 1C
 119 BA 0.037
 120 LS 0 78 5
 121 UD 0.22

 122 KK DP1BC
 123 KM COMBINE FLOWS FROM BASIN 1B & 1C
 124 HC 2

REC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

125 KK A-A
 126 KM ROUTE FLOW FROM DP1BC THRU BASIN 1D
 127 RS 1 STOR 0
 128 RC 0.12 0.08 0.12 1600 0.21
 129 RX 0 5 10 20 28 38 43 48
 130 RY 15 12.5 10 0 0 10 12.5 15

 131 KK B1D
 132 KM RUNOFF FROM BASIN 1D
 133 BA 0.050
 134 LS 0 78 5
 135 UD 0.04

 136 KK DP1ABCD
 137 KM COMBINE FLOWS FROM BASINS 1A THRU 1D
 138 HC 2

 139 KK C-C
 140 KM ROUTE FLOW FROM BASINS 1A-1D THRU BASIN A
 141 RS 1 STOR 0
 142 RC 0.12 0.08 0.12 4000 .08
 143 RX 0 5 10 20 28 38 43 48
 144 RY 15 12.5 10 0 0 10 12.5 15

 145 KK BA
 146 KM RUNOFF FROM BASIN A
 147 BA 0.09
 148 LS 0 72 5
 149 UD 0.23

 150 KK DP1
 151 KM COMBINE FLOWS FROM BASINS 1A-1D & BASIN A
 152 HC 2

 153 KK B2
 154 KM RUNOFF FROM BASIN 2
 155 BA 0.043

```

156      LS      0      78      5
157      UD      0.19

158      KK      D-D
159      KM      ROUTE FLOW FROM BASIN 2 THRU BASIN C
160      RS      1      STOR      0
161      RC      0.12  0.08  0.12  6400  0.07
162      RX      0      5      10      20      28      38      43      48
163      RY      15     12.5  10      0      0      10     12.5  15

164      KK      B2X
165      KM      RUNOFF FROM BASIN 2X
166      BA      .005
167      LS      0      78      5
168      UD      0.13

```

HEC-1 INPUT

1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

169      KK      X-X
170      KM      ROUTE FLOW FROM BASIN 2X THRU BASIN C
171      RS      1      STOR      0
172      RC      0.12  0.08  0.12  50     0.49
173      RX      0      5      10      20      28      38      43      48
174      RY      15     12.5  10      0      0      10     12.5  15

175      KK      BC
176      KM      RUNOFF FROM BASIN C
177      BA      0.09
178      LS      0      72      5
179      UD      0.17

180      KK      DP2
181      KM      COMBINE FLOWS FROM BASINS 2, 2X & C
182      HC      3

183      KK      B3
184      KM      RUNOFF FROM BASIN 3
185      BA      0.0687
186      LS      0      78      5
187      UD      0.26

188      KK      E-E
189      KM      ROUTE FLOW FROM BASIN 3 THRU BASIN H
190      RS      1      STOR      0
191      RC      0.12  0.08  0.12  7200  0.03
192      RX      0      5      10      20      28      38      43      48
193      RY      15     12.5  10      0      0      10     12.5  15

194      KK      B4
195      KM      RUNOFF FROM BASIN 4
196      BA      0.096
197      LS      0      78      5
198      UD      0.21

199      KK      F-F
200      KM      ROUTE FLOW FROM BASIN 4 THRU BASIN H
201      RS      1      STOR      0
202      RC      0.12  0.08  0.12  2800  0.05
203      RX      0      5      10      20      28      38      43      48
204      RY      15     12.5  10      0      0      10     12.5  15

205      KK      BH
206      KM      RUNOFF FROM BASIN H
207      BA      0.03
208      LS      0      72      5
209      UD      0.17

```

HEC-1 INPUT

1
LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

210      KK      DP3
211      KM      COMBINE FLOWS FROM BASINS 3, 4 & H
212      HC      3

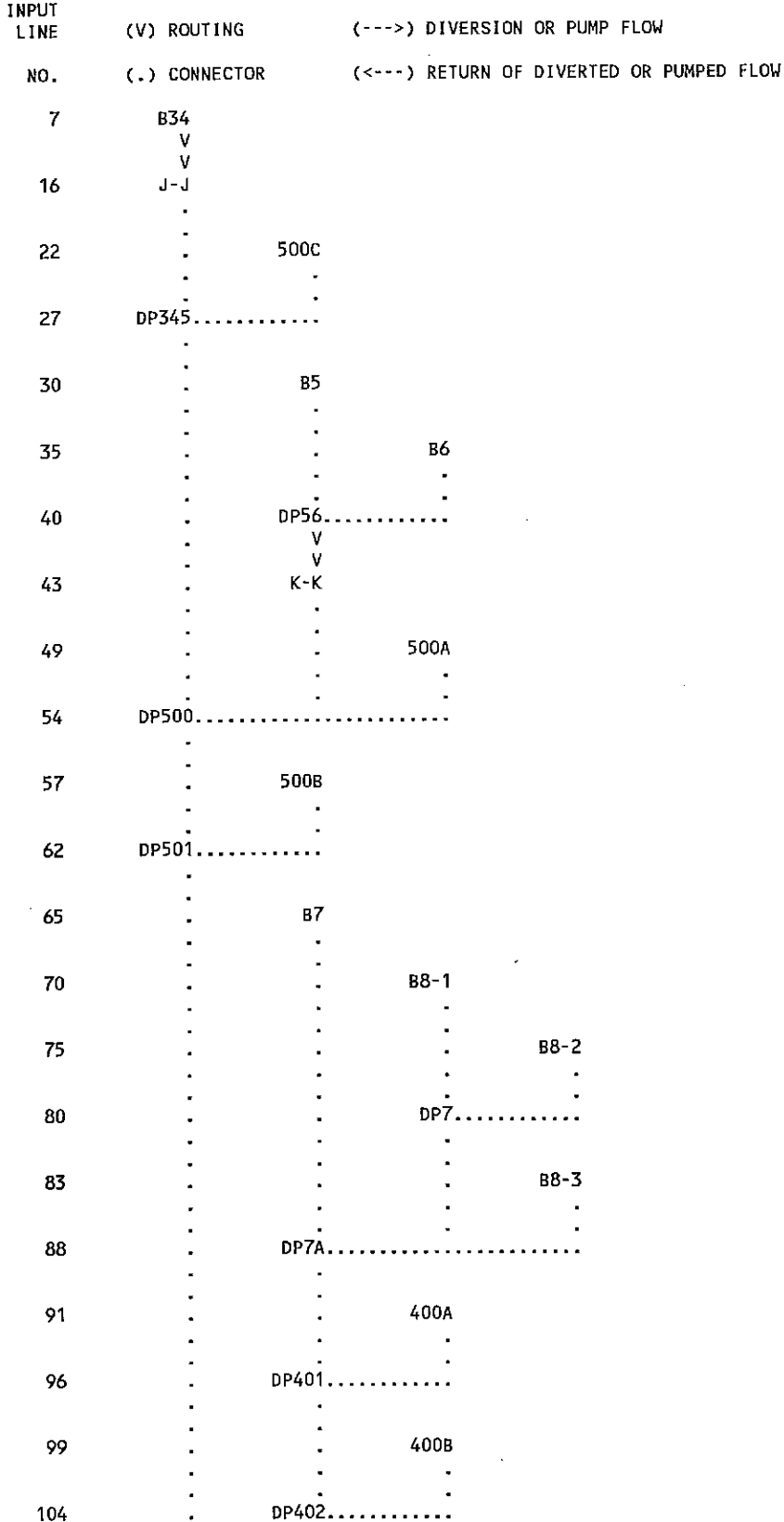
213      KK      BB
214      KM      RUNOFF FROM BASIN B
215      BA      0.02

```

216	LS	0	72	5
217	UD	0.14		
218	KK	BD		
219	KM	RUNOFF FROM BASIN D		
220	BA	0.02		
221	LS	0	72	5
222	UD	0.16		
223	ZZ			

1

SCHEMATIC DIAGRAM OF STREAM NETWORK



BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
 5-YR,2-HR DEVELOPED PART 1
 FULL BASIN RUNOFF STUDY 5BD1.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

6 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 1 MINUTES IN COMPUTATION INTERVAL
 IDATE 1DEC97 STARTING DATE
 ITIME 1200 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1DEC97 ENDING DATE
 NDTIME 1659 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .02 HOURS
 TOTAL TIME BASE 4.98 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

1

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT								
+		B34	5. .70	1.	1.	1.	.02		
+	ROUTED TO								
+		J-J	5. .77	1.	1.	1.	.02	.08	.77
+	HYDROGRAPH AT								
+		500C	2. .73	0.	0.	0.	.01		
+	2 COMBINED AT								
+		DP345	7. .77	1.	1.	1.	.04		
+	HYDROGRAPH AT								
+		B5	9. .72	2.	2.	2.	.04		
+	HYDROGRAPH AT								
+		B6	5. .72	1.	1.	1.	.02		
+	2 COMBINED AT								
+		DP56	13. .72	2.	2.	2.	.06		
+	ROUTED TO								
+		K-K	12. .80	2.	2.	2.	.06	.20	.80
+	HYDROGRAPH AT								
+		500A	5. .82	1.	1.	1.	.04		
+	3 COMBINED AT								
+		DP500	23. .80	5.	5.	5.	.14		

+	HYDROGRAPH AT	500B	1.	.73	0.	0.	0.	.01		
+	2 COMBINED AT	DP501	24.	.78	5.	5.	5.	.15		
+	HYDROGRAPH AT	B7	3.	.68	1.	1.	1.	.01		
+	HYDROGRAPH AT	B8-1	12.	.68	2.	2.	2.	.06		
+	HYDROGRAPH AT	B8-2	4.	.65	1.	1.	1.	.02		
+	2 COMBINED AT	DP7	15.	.68	3.	3.	3.	.07		
+	HYDROGRAPH AT	B8-3	4.	.80	1.	1.	1.	.02		
+	3 COMBINED AT	DP7A	22.	.70	4.	4.	4.	.11		
+	HYDROGRAPH AT	400A	2.	.88	1.	1.	1.	.02		
+	2 COMBINED AT	DP401	24.	.70	4.	4.	4.	.13		
+	HYDROGRAPH AT	400B	1.	.88	0.	0.	0.	.01		
+	2 COMBINED AT	DP402	24.	.72	5.	5.	5.	.14		
+	HYDROGRAPH AT	B1A	6.	.73	1.	1.	1.	.02		
+	HYDROGRAPH AT	B1B	52.	.90	10.	10.	10.	.23		
+	HYDROGRAPH AT	B1C	9.	.83	2.	2.	2.	.04		
+	2 COMBINED AT	DP1BC	61.	.88	12.	12.	12.	.27		
+	ROUTED TO	A-A	60.	.93	12.	12.	12.	.27	.90	.93
+	HYDROGRAPH AT	B1D	18.	.60	2.	2.	2.	.05		
+	2 COMBINED AT	DP1ABC	68.	.93	14.	14.	14.	.32		
+	ROUTED TO	C-C	60.	1.12	14.	14.	14.	.32	1.19	1.12
+	HYDROGRAPH AT	BA	11.	.88	3.	3.	3.	.09		
+	2 COMBINED AT	DP1	70.	1.10	17.	17.	17.	.41		
+	HYDROGRAPH AT	B2	11.	.78	2.	2.	2.	.04		
+	ROUTED TO	D-D	7.	1.23	2.	2.	2.	.04	.19	1.23
+	HYDROGRAPH AT	B2X	1.	.70	0.	0.	0.	.00		
+	ROUTED TO	X-X	1.	.70	0.	0.	0.	.00		

									.02	.70
+	HYDROGRAPH AT	BC	12.	.80	3.	3.	3.	.09		
+	3 COMBINED AT	DP2	18.	.88	5.	5.	5.	.14		
+	HYDROGRAPH AT	B3	16.	.88	3.	3.	3.	.07		
+	ROUTED TO	E-E	8.	1.47	3.	3.	3.	.07	.36	1.47
+	HYDROGRAPH AT	B4	24.	.82	4.	4.	4.	.10		
+	ROUTED TO	F-F	18.	1.07	4.	4.	4.	.10	.64	1.07
+	HYDROGRAPH AT	BH	4.	.80	1.	1.	1.	.03		
+	3 COMBINED AT	DP3	28.	1.15	8.	8.	8.	.19		
+	HYDROGRAPH AT	BB	3.	.77	1.	1.	1.	.02		
+	HYDROGRAPH AT	BD	3.	.78	1.	1.	1.	.02		

*** NORMAL END OF HEC-1 ***

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*
* RUN DATE 02/24/1998 TIME 09:38:59 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN									
2	ID	5-YR,2-HR		DEVELOPED			PART 2				
3	ID	FULL BASIN RUNOFF STUDY					5BD2.DAT				
	*DIAGRAM										
4	IT	1	01DEC97	1200	1500						
5	IN	5									
6	IO	5	0								
7	KK	BE									
8	KM	RUNOFF FROM BASIN E									
9	BA	0.01									
10	PB	1.6									
11	PI	.01	.03	.04	.07	.12	.22	.12	.07	.05	.04
12	PI	.03	.03	.03	.02	.02	.01	.01	.01	.01	.01
13	PI	.01	.01	.01	.01						
14	LS	0	72	5							
15	UD	0.14									
16	KK	BF									
17	KM	RUNOFF FROM BASIN F									
18	BA	0.13									
19	LS	0	72	5							
20	UD	0.13									
21	KK	BG									
22	KM	RUNOFF FROM BASIN G									
23	BA	0.03									
24	LS	0	72	5							
25	UD	0.19									
26	KK	B12-1									
27	KM	RUNOFF FROM SDA 12-1									
28	BA	.089									
29	LS	0	76	6							
30	UD	.126									
31	KK	B12-2									
32	KM	RUNOFF FROM SDA 12-2									
33	BA	.028									
34	LS	0	76	6							
35	UD	.113									

36	KK	DP1							
37	KM	COMBINE FLOWS FROM SDAs 12-1, 12-2							
38	HC	2							
39	KK	A-A							
40	KM	ROUTE FLOWS FROM CP1 TO CP2							
41	RS	1	STOR	0					
42	RC	0.12	.08	0.12	630	0.24			
43	RX	0	5	10	20	28	38	43	48
44	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

45	KK	B12-3							
46	KM	RUNOFF FROM SDA 12-3							
47	BA	.027							
48	LS	0	76	6					
49	UD	.145							
50	KK	B12-4							
51	KM	RUNOFF FROM SDA 12-4							
52	BA	.038							
53	LS	0	75	6					
54	UD	.110							
55	KK	B12-7							
56	KM	RUNOFF FROM SDA 12-7							
57	BA	.016							
58	LS	0	75	5					
59	UD	.134							
60	KK	DP2							
61	KM	COMBINE CP1 & SDAs 12-3, 12-4, 12-7							
62	HC	4							
63	KK	B-B							
64	KM	ROUTE FLOWS FROM CP2 TO CP3							
65	RS	1	STOR	0					
66	RC	0.12	.08	0.12	790	0.19			
67	RX	0	5	10	20	28	38	43	48
68	RY	15	12.5	10	0	0	10	12.5	15
69	KK	B12-5							
70	KM	RUNOFF FROM SDA 12-5							
71	BA	.081							
72	LS	0	76	10					
73	UD	.154							
74	KK	B12-8							
75	KM	RUNOFF FROM SDA 12-8							
76	BA	.023							
77	LS	0	75	5					
78	UD	.077							
79	KK	DP3							
80	KM	COMBINE FLOWS FROM CP2 & SDAs 12-5, 12-8							
81	HC	3							
82	KK	C-C							
83	KM	ROUTE FLOW FROM CP3 TO CP4							
84	RS	1	STOR	0					
85	RC	0.12	.08	0.12	650	0.26			
86	RX	0	5	10	20	28	38	43	48
87	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	B12-6							
89	KM	RUNOFF FROM SDA 12-6							
90	BA	.061							
91	LS	0	76	10					
92	UD	.129							
93	KK	B12-9							
94	KM	RUNOFF FROM SDA 12-9							

95	BA	.021								
96	LS	0	75	4						
97	UD	.112								
98	KK	DP4								
99	KM	COMBINE FLOWS FROM CP3 & SDAs 12-6, 12-9								
100	HC	3								
101	KK	D-D								
102	KM	ROUTE FLOWS FROM CP4 TO CP5								
103	RS	1	STOR	0						
104	RC	0.12	.08	0.12	1420	0.19				
105	RX	0	5	10	20	28	38	43	48	
106	RY	15	12.5	10	0	0	10	12.5	15	
107	KK	B1210								
108	KM	RUNOFF FROM SDA 12-10								
109	BA	.069								
110	LS	0	75	5						
111	UD	.170								
112	KK	DP5								
113	KM	COMBINE FLOWS FROM CP4 & SDA 1210								
114	HC	2								
115	KK	E-E								
116	KM	ROUTE FLOW FROM CP5 TO CP6								
117	RS	1	STOR	0						
118	RC	0.12	.08	0.12	1420	0.11				
119	RX	0	5	10	20	28	38	43	48	
120	RY	15	12.5	10	0	0	10	12.5	15	
121	KK	DAM								
122	KM	ROUTE FLOW THROUGH FISHERS CNAYON DEBRIS CONTROL STURCTURE								
123	RS	1	ELEV	7155						
124	SV	0	0.3	0.75	1.35	2.55	3.6	3.8	6.1	11.0
125	SV	13.5	16.1	18.8						
126	SE	7155	7160	7165	7170	7175	7178	7180	7182	7184
127	SE	7188	7190	7192						
128	SQ	0	19	39	51	61	67	70	158	197
129	SQ	1853	4801	8610						227

HEC-1 INPUT

1 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

130	KK	B1211								
131	KM	RUNOFF FROM SDA 12-11								
132	BA	.041								
133	LS	0	74	10						
134	UD	.073								
135	KK	DP6								
136	KM	COMBINE DP5 & 1211								
137	HC	2								
138	KK	F-F								
139	KM	ROUTE FLOW FROM 12-11 TO 1011-1								
140	RS	1	STOR	0						
141	RC	0.12	.08	0.12	1200	.13				
142	RX	0	5	10	20	28	38	43	48	
143	RY	15	12.5	10	0	0	10	12.5	15	
144	KK	B10111								
145	KM	RUNOFF FROM SDA 1011-1								
146	BA	.035								
147	LS	0	74	7						
148	UD	.160								
149	KK	B10112								
150	KM	RUNOFF FROM SDA 1011-2								
151	BA	.037								
152	LS	0	74	7						
153	UD	.160								
154	KK	DP8								
155	KM	COMBINE FLOW FROM SDA 1012 & CP6A								
156	HC	3								
157	KK	G-G								

158	KM	ROUTE FLOW FROM DP8 TO DP300							
159	RS	1	STOR	0					
160	RC	0.12	.08	0.12	1200	.12			
161	RX	0	5	10	20	28	38	43	48
162	RY	15	12.5	10	0	0	10	12.5	15

163	KK	300A							
164	KM	RUNOFF FROM 300A							
165	BA	.049							
166	LS	0	65	4					
167	UD	.162							

168	KK	DP300							
169	KM	COMBINE FLOWS FROM CP8 & SDA 300A							
170	HC	2							

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

171	KK	B9							
172	KM	RUNOFF FROM SDA 9							
173	BA	.085							
174	LS	0	74	4					
175	UD	.125							

176	KK	H-H							
177	KM	ROUTE FLOW FROM B9 THRU SDA 300B							
178	RS	1	STOR	0					
179	RC	0.12	.08	0.12	1200	.25			
180	RX	0	5	10	20	28	38	43	48
181	RY	15	12.5	10	0	0	10	12.5	15

182	KK	300B							
183	KM	RUNOFF FROM 300B							
184	BA	.026							
185	LS	0	68	4					
186	UD	.126							

187	KK	DP9							
188	KM	COMBINE SDA B9 & 300B							
189	HC	2							

190	KK	DP301							
191	KM	COMBINE FLOWS FROM CP300 & SDA 300B							
192	HC	2							

193	KK	I-I							
194	KM	ROUTE FLOW FROM B9 THRU SDA 300B							
195	RS	1	STOR	0					
196	RC	0.12	.08	0.12	1800	.17			
197	RX	0	4	5.5	6.5	14.5	15.5	17	21
198	RY	7	5	2	0	0	2	5	7

199	KK	300C							
200	KM	RUNOFF FROM SDA 300C							
201	BA	.06							
202	LS	0	72	4					
203	UD	.150							

204	KK	DP302							
205	KM	COMBINE FLOWS FROM CP301 & 300C							
206	HC	2							
207	ZZ								

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
7	BE	
	.	
16	.	BF
	.	.
	.	.
21	.	BG
	.	.
	.	.

26	.	.	.	B12-1	.	
31	B12-2
36	.	.	.	DP1	
39	.	.	.	V		
	.	.	.	V		
	.	.	.	A-A		
45	B12-3
50	B12-4
55	B12-7
60	.	.	.	DP2	
63	.	.	.	V		
	.	.	.	V		
	.	.	.	B-B		
69	B12-5
74	B12-8
79	.	.	.	DP3	
82	.	.	.	V		
	.	.	.	V		
	.	.	.	C-C		
88	B12-6
93	B12-9
98	.	.	.	DP4	
101	.	.	.	V		
	.	.	.	V		
	.	.	.	D-D		
107	B1210
112	.	.	.	DP5	
115	.	.	.	V		
	.	.	.	V		
	.	.	.	E-E		
121	.	.	.	V		
	.	.	.	DAM		
130	B1211
135	.	.	.	DP6	
138	.	.	.	V		
	.	.	.	V		
	.	.	.	F-F		
144	B10111
149	B10112
154	.	.	.	DP8	
	.	.	.	V		

```

157      .      .      .      V
      .      .      .      G-G
      .      .      .      .
163      .      .      .      .      300A
      .      .      .      .      .
168      .      .      .      DP300.....
      .      .      .      .
171      .      .      .      .      B9
      .      .      .      .      V
      .      .      .      .      V
176      .      .      .      .      H-H
      .      .      .      .      .
182      .      .      .      .      .      300B
      .      .      .      .      .
187      .      .      .      .      DP9.....
      .      .      .      .      .
190      .      .      .      DP301.....
      .      .      .      .      V
      .      .      .      .      V
193      .      .      .      .      I-I
      .      .      .      .      .
199      .      .      .      .      300C
      .      .      .      .      .
204      .      .      .      DP302.....

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 02/24/1998 TIME 09:38:59 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*
*****

```

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
5-YR,2-HR DEVELOPED PART 2
FULL BASIN RUNOFF STUDY 5BD2.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN      1 MINUTES IN COMPUTATION INTERVAL
          IDATE     1DEC97 STARTING DATE
          ITIME     1200 STARTING TIME
          NQ        300 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    1DEC97 ENDING DATE
          NDTIME    1659 ENDING TIME
          ICENT     19 CENTURY MARK

          COMPUTATION INTERVAL .02 HOURS
          TOTAL TIME BASE      4.98 HOURS

```

ENGLISH UNITS

```

DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+										
+	HYDROGRAPH AT	BE	1.	.75	0.	0.	0.	.01		
+	HYDROGRAPH AT	BF	18.	.75	4.	4.	4.	.13		
+	HYDROGRAPH AT	BG	4.	.83	1.	1.	1.	.03		
+	HYDROGRAPH AT	B12-1	22.	.70	4.	4.	4.	.09		
+	HYDROGRAPH AT	B12-2	7.	.68	1.	1.	1.	.03		
+	2 COMBINED AT	DP1	29.	.70	5.	5.	5.	.12		
+	ROUTED TO	A-A	28.	.73	5.	5.	5.	.12	.46	.73
+	HYDROGRAPH AT	B12-3	6.	.73	1.	1.	1.	.03		
+	HYDROGRAPH AT	B12-4	9.	.68	1.	1.	1.	.04		
+	HYDROGRAPH AT	B12-7	3.	.73	1.	1.	1.	.02		
+	4 COMBINED AT	DP2	46.	.72	8.	8.	8.	.20		
+	ROUTED TO	B-B	46.	.75	8.	8.	8.	.20	.81	.75
+	HYDROGRAPH AT	B12-5	23.	.72	4.	4.	4.	.08		
+	HYDROGRAPH AT	B12-8	5.	.65	1.	1.	1.	.02		
+	3 COMBINED AT	DP3	73.	.73	12.	12.	12.	.30		
+	ROUTED TO	C-C	73.	.75	12.	12.	12.	.30	.94	.75
+	HYDROGRAPH AT	B12-6	18.	.68	3.	3.	3.	.06		
+	HYDROGRAPH AT	B12-9	4.	.72	1.	1.	1.	.02		
+	3 COMBINED AT	DP4	94.	.75	16.	16.	16.	.38		
+	ROUTED TO	D-D	92.	.78	16.	16.	16.	.38	1.19	.78
+	HYDROGRAPH AT	B1210	13.	.78	2.	2.	2.	.07		
+	2 COMBINED AT	DP5	105.	.78	18.	18.	18.	.45		
+	ROUTED TO	E-E	102.	.83	18.	18.	18.	.45		

+									1.53	.83
+	ROUTED TO	DAM	57.	1.32	18.	18.	18.	.45		
+									7172.93	1.32
+	HYDROGRAPH AT	B1211	11.	.62	2.	2.	2.	.04		
+	2 COMBINED AT	DP6	61.	1.13	20.	20.	20.	.49		
+	ROUTED TO	F-F	61.	1.17	20.	20.	20.	.49		
+									1.04	1.17
+	HYDROGRAPH AT	B10111	7.	.75	1.	1.	1.	.04		
+	HYDROGRAPH AT	B10112	7.	.75	1.	1.	1.	.04		
+	3 COMBINED AT	DP8	70.	1.13	23.	23.	23.	.57		
+	ROUTED TO	G-G	70.	1.17	23.	23.	23.	.57		
+									1.16	1.17
+	HYDROGRAPH AT	300A	3.	.63	1.	1.	1.	.05		
+	2 COMBINED AT	DP300	72.	1.17	23.	23.	23.	.62		
+	HYDROGRAPH AT	B9	15.	.73	3.	3.	3.	.09		
+	ROUTED TO	H-H	14.	.78	3.	3.	3.	.09		
+									.22	.78
+	HYDROGRAPH AT	300B	2.	.60	0.	0.	0.	.03		
+	2 COMBINED AT	DP9	16.	.80	3.	3.	3.	.11		
+	2 COMBINED AT	DP301	84.	1.15	27.	27.	27.	.73		
+	ROUTED TO	I-I	84.	1.17	27.	27.	27.	.73		
+									1.25	1.18
+	HYDROGRAPH AT	300C	8.	.80	2.	2.	2.	.06		
+	2 COMBINED AT	DP302	90.	1.15	28.	28.	28.	.79		

*** NORMAL END OF HEC-1 ***

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* SEPTEMBER 1990
* VERSION 4.0
*
* RUN DATE 02/24/1998 TIME 09:40:11
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN									
2	ID	100YR,2HR DEVELOPED				PART 1					
3	ID	FULL BASIN RUNOFF STUDY					100BD1.DAT				
	*DIAGRAM										
4	IT	1	01DEC97	1200	1500						
5	IN	5									
6	IO	5	0								
7	KK	B34									
8	KM	RUNOFF FROM SDA				34					
9	BA	.023									
10	PB	3.1									
11	PI	.01	.03	.04	.07	.12	.22	.12	.07	.05	.04
12	PI	.03	.03	.03	.02	.02	.01	.01	.01	.01	.01
13	PI	.01	.01	.01	.01						
14	LS	0	74	7							
15	UD	.125									
16	KK	J-J									
17	KM	ROUTE FLOW FROM B3 THRU 500C									
18	RS	1	STOR	0							
19	RC	0.12	.08	0.12	1500	.21					
20	RX	0	5	10	20	28	38	43	48		
21	RY	15	12.5	10	0	0	10	12.5	15		
22	KK	500C									
23	KM	RUNOFF FROM 500C									
24	BA	.015									
25	LS	0	72	4							
26	UD	.108									
27	KK	DP345									
28	KM	COMBINE SDAs 34 & 500C									
29	HC	2									
30	KK	B5									
31	KM	RUNOFF FROM SDA 5									
32	BA	.042									
33	LS	0	74	7							
34	UD	.131									

35 KK B6
 36 KM RUNOFF FROM SDA 6
 37 BA .022
 38 LS 0 74 7
 39 UD .131

 40 KK DP56
 41 KM COMBINE FLOWS FROM SDAs 5&6
 42 HC 2

HEC-1 INPUT

PAGE 2

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

43 KK K-K
 44 KM ROUTE FLOW FROM DP 56 THRU 500A
 45 RS 1 STOR 0
 46 RC 0.12 .08 0.12 2100 .22
 47 RX 0 5 10 20 28 38 43 48
 48 RY 15 12.5 10 0 0 10 12.5 15

49 KK 500A
 50 KM RUNOFF FROM SDA 500A
 51 BA .036
 52 LS 0 72 4
 53 UD .162

54 KK DP500
 55 KM COMBINE FLOWS FROM CP56&500A
 56 HC 3

57 KK 500B
 58 KM RUNOFF FROM SDA 500B
 59 BA .009
 60 LS 0 72 4
 61 UD .108

62 KK DP501
 63 KM COMBINE FLOWS FROM CP5A&500B
 64 HC 2

65 KK B7
 66 KM RUNOFF FROM SDA 7
 67 BA .014
 68 LS 0 74 7
 69 UD .107

70 KK B8-1
 71 KM RUNOFF FROM SDA 8-1
 72 BA .056
 73 LS 0 74 7
 74 UD .113

75 KK B8-2
 76 KM RUNOFF FROM SDA 8-2
 77 BA .016
 78 LS 0 74 7
 79 UD .086

80 KK DP7
 81 KM COMBINE FLOWS FROM SDAs 8-1, 8-2
 82 HC 2

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

83 KK B8-3
 84 KM RUNOFF FROM SDA 8-3
 85 BA .021
 86 LS 0 74 7
 87 UD .202

88 KK DP7A
 89 KM COMBINE FLOW FROM CP7 & SDA 8-3 &B7
 90 HC 3

91 KK 400A
 92 KM RUNOFF FROM SDA 400A
 93 BA .02

```

94      LS      0      72      4
95      UD      .210

96      KK      DP401
97      KM      COMBINE FLOWS FROM DP7A & SDA 400A
98      HC      2

99      KK      400B
100     KM      RUNOFF FROM SDA      400B
101     BA      .01
102     LS      0      72      4
103     UD      .210

104     KK      DP402
105     KM      COMBINE FLOWS FROM DP402& SDA 400B
106     HC      2

107     KK      B1A
108     KM      RUNOFF FROM BASIN 1A
109     BA      0.023
110     LS      0      78      5
111     UD      0.15

112     KK      B1B
113     KM      RUNOFF FROM BASIN 1B
114     BA      0.231
115     LS      0      78      5
116     UD      0.27

117     KK      B1C
118     KM      RUNOFF FROM BASIN 1C
119     BA      0.037
120     LS      0      78      5
121     UD      0.22

122     KK      DP1BC
123     KM      COMBINE FLOWS FROM BASIN 1B & 1C
124     HC      2

```

HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

125     KK      A-A
126     KM      ROUTE FLOW FROM DP1BC THRU BASIN 1D
127     RS      1      STOR      0
128     RC      0.12  0.08  0.12  1600  0.21
129     RX      0      5      10      20      28      38      43      48
130     RY      15     12.5  10      0      0      10     12.5  15

131     KK      B1D
132     KM      RUNOFF FROM BASIN 1D
133     BA      0.050
134     LS      0      78      5
135     UD      0.04

136     KK      DP1ABCD
137     KM      COMBINE FLOWS FROM BASINS 1A THRU 1D
138     HC      2

139     KK      C-C
140     KM      ROUTE FLOW FROM BASINS 1A-1D THRU BASIN A
141     RS      1      STOR      0
142     RC      0.12  0.08  0.12  4000  .08
143     RX      0      5      10      20      28      38      43      48
144     RY      15     12.5  10      0      0      10     12.5  15

145     KK      BA
146     KM      RUNOFF FROM BASIN A
147     BA      0.09
148     LS      0      72      5
149     UD      0.23

150     KK      DP1
151     KM      COMBINE FLOWS FROM BASINS 1A-1D & BASIN A
152     HC      2

153     KK      B2
154     KM      RUNOFF FROM BASIN 2
155     BA      0.043

```

```

156      LS      0      78      5
157      UD      0.19

158      KK      D-D
159      KM      ROUTE FLOW FROM BASIN 2 THRU BASIN C
160      RS      1      STOR      0
161      RC      0.12  0.08  0.12  6400  0.07
162      RX      0      5      10     20     28     38     43     48
163      RY      15     12.5  10     0      0      10     12.5  15

164      KK      B2X
165      KM      RUNOFF FROM BASIN 2X
166      BA      .005
167      LS      0      78      5
168      UD      0.13

```

HEC-1 INPUT

PAGE 5

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

169      KK      X-X
170      KM      ROUTE FLOW FROM BASIN 2X THRU BASIN C
171      RS      1      STOR      0
172      RC      0.12  0.08  0.12  50     0.49
173      RX      0      5      10     20     28     38     43     48
174      RY      15     12.5  10     0      0      10     12.5  15

175      KK      BC
176      KM      RUNOFF FROM BASIN C
177      BA      0.09
178      LS      0      72      5
179      UD      0.17

180      KK      DP2
181      KM      COMBINE FLOWS FROM BASINS 2, 2X & C
182      HC      3

183      KK      B3
184      KM      RUNOFF FROM BASIN 3
185      BA      0.0687
186      LS      0      78      5
187      UD      0.26

188      KK      E-E
189      KM      ROUTE FLOW FROM BASIN 3 THRU BASIN H
190      RS      1      STOR      0
191      RC      0.12  0.08  0.12  7200  0.03
192      RX      0      5      10     20     28     38     43     48
193      RY      15     12.5  10     0      0      10     12.5  15

194      KK      B4
195      KM      RUNOFF FROM BASIN 4
196      BA      0.096
197      LS      0      78      5
198      UD      0.21

199      KK      F-F
200      KM      ROUTE FLOW FROM BASIN 4 THRU BASIN H
201      RS      1      STOR      0
202      RC      0.12  0.08  0.12  2800  0.05
203      RX      0      5      10     20     28     38     43     48
204      RY      15     12.5  10     0      0      10     12.5  15

205      KK      BH
206      KM      RUNOFF FROM BASIN H
207      BA      0.03
208      LS      0      72      5
209      UD      0.17

```

HEC-1 INPUT

PAGE 6

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

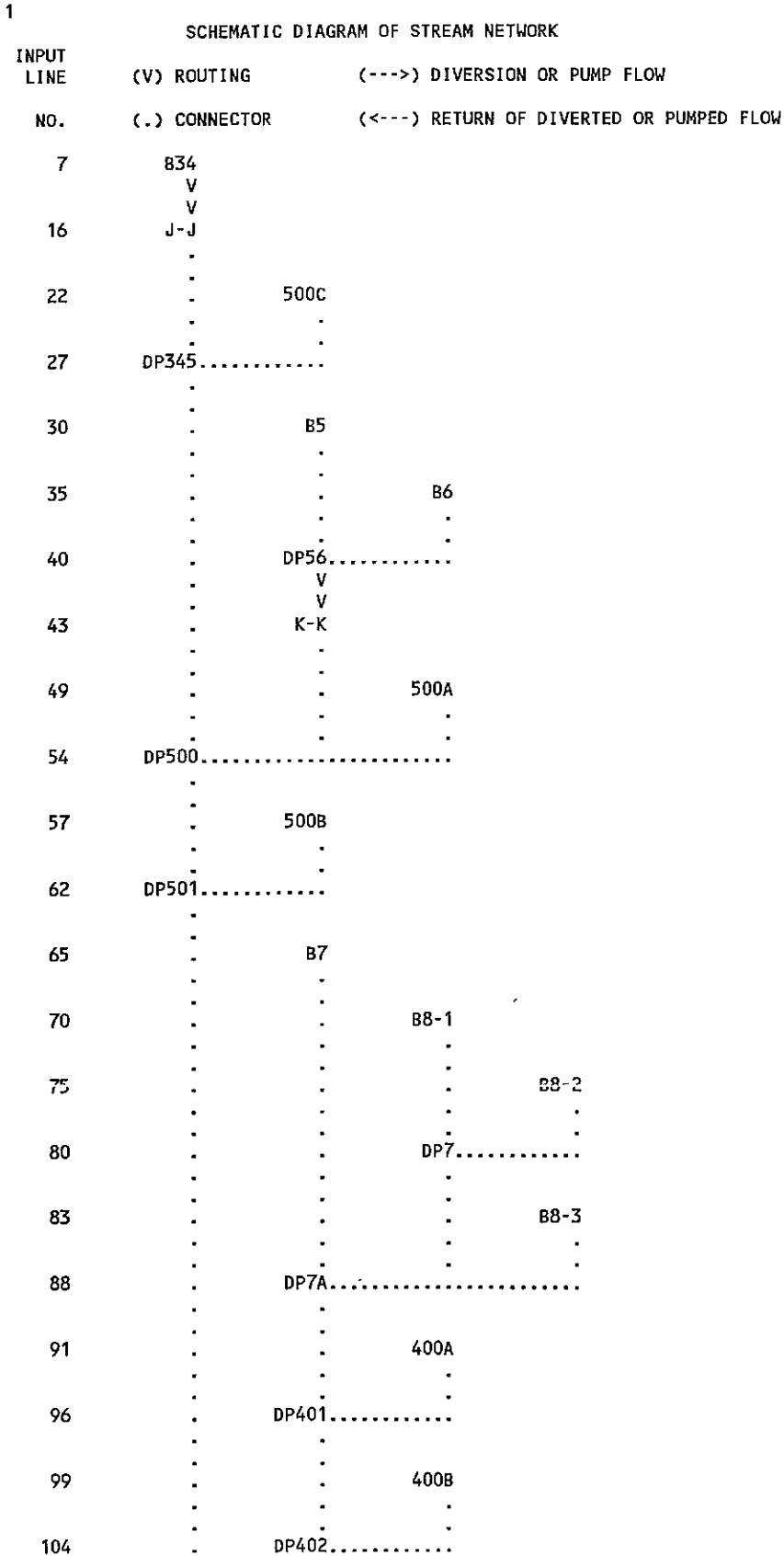
```

210      KK      DP3
211      KM      COMBINE FLOWS FROM BASINS 3, 4 & H
212      HC      3

213      KK      BB
214      KM      RUNOFF FROM BASIN B
215      BA      0.02

```


216	LS	0	72	5
217	UD	0.14		
218	KK	BD		
219	KM	RUNOFF FROM BASIN D		
220	BA	0.02		
221	LS	0	72	5
222	UD	0.16		
223	ZZ			



107	.	.	B1A
112	.	.	B1B
117	B1C
122	.	.	DP1BC
	.	.	V
	.	.	V
125	.	.	A-A
131	B1D
136	.	.	DP1ABC
	.	.	V
	.	.	V
139	.	.	C-C
145	BA
150	.	.	DP1
153	B2
	V
	V
158	D-D
164	B2X	.	.	.
	V	.	.	.
	V	.	.	.
169	X-X	.	.	.
175	BC	.	.
180	DP2
183	B3	.	.	.
	V	.	.	.
	V	.	.	.
188	E-E	.	.	.
194	B4	.	.
	V	.	.
	V	.	.
199	F-F	.	.
205	BH	.
210	DP3
213	BB	.
218	BD

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
*

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
*

```

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
 100YR,2HR DEVELOPED PART 1
 FULL BASIN RUNOFF STUDY 100BD1.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

6 IO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 1 MINUTES IN COMPUTATION INTERVAL
 IDATE 1DEC97 STARTING DATE
 ITIME 1200 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 1DEC97 ENDING DATE
 NDTIME 1659 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL .02 HOURS
 TOTAL TIME BASE 4.98 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	B34	26.	.65	3.	3.	3.	.02		
ROUTED TO	J-J	24.	.73	3.	3.	3.	.02	.41	.73
HYDROGRAPH AT	500C	15.	.65	2.	2.	2.	.01		
2 COMBINED AT	DP345	37.	.70	5.	5.	5.	.04		
HYDROGRAPH AT	B5	47.	.67	6.	6.	6.	.04		
HYDROGRAPH AT	B6	25.	.67	3.	3.	3.	.02		
2 COMBINED AT	DP56	72.	.67	9.	9.	9.	.06		
ROUTED TO	K-K	66.	.73	9.	9.	9.	.06	.94	.73
HYDROGRAPH AT	500A	31.	.72	4.	4.	4.	.04		
3 COMBINED AT	DP500	133.	.72	19.	19.	19.	.14		

+	HYDROGRAPH AT	500B	9.	.65	1.	1.	1.	.01		
+	2 COMBINED AT	DP501	141.	.72	20.	20.	20.	.15		
+	HYDROGRAPH AT	B7	17.	.63	2.	2.	2.	.01		
+	HYDROGRAPH AT	B8-1	66.	.65	8.	8.	8.	.06		
+	HYDROGRAPH AT	B8-2	20.	.60	2.	2.	2.	.02		
+	2 COMBINED AT	DP7	86.	.63	10.	10.	10.	.07		
+	HYDROGRAPH AT	B8-3	20.	.77	3.	3.	3.	.02		
+	3 COMBINED AT	DP7A	117.	.65	16.	16.	16.	.11		
+	HYDROGRAPH AT	400A	16.	.78	2.	2.	2.	.02		
+	2 COMBINED AT	DP401	128.	.65	18.	18.	18.	.13		
+	HYDROGRAPH AT	400B	8.	.78	1.	1.	1.	.01		
+	2 COMBINED AT	DP402	134.	.67	19.	19.	19.	.14		
+	HYDROGRAPH AT	B1A	30.	.68	4.	4.	4.	.02		
+	HYDROGRAPH AT	B1B	234.	.83	39.	39.	39.	.23		
+	HYDROGRAPH AT	B1C	41.	.77	6.	6.	6.	.04		
+	2 COMBINED AT	DP1BC	273.	.83	45.	45.	45.	.27		
+	ROUTED TO	A-A	271.	.87	45.	45.	45.	.27	2.24	.87
+	HYDROGRAPH AT	B1D	93.	.52	8.	8.	8.	.05		
+	2 COMBINED AT	DP1ABC	302.	.85	53.	53.	53.	.32		
+	ROUTED TO	C-C	275.	.95	53.	53.	53.	.32	2.98	.95
+	HYDROGRAPH AT	BA	69.	.82	11.	11.	11.	.09		
+	2 COMBINED AT	DP1	335.	.93	65.	65.	65.	.41		
+	HYDROGRAPH AT	B2	51.	.73	7.	7.	7.	.04		
+	ROUTED TO	D-D	26.	1.08	7.	7.	7.	.04	.79	1.08
+	HYDROGRAPH AT	B2X	7.	.65	1.	1.	1.	.00		
+	ROUTED TO	X-X	7.	.65	1.	1.	1.	.00		

+									.08	.65
+	HYDROGRAPH AT	BC	78.	.73	11.	11.	11.	.09		
+	3 COMBINED AT	DP2	99.	.75	19.	19.	19.	.14		
+	HYDROGRAPH AT	B3	70.	.83	11.	11.	11.	.07		
+	ROUTED TO	E-E	37.	1.27	11.	11.	11.	.07	1.20	1.27
+	HYDROGRAPH AT	B4	109.	.77	16.	16.	16.	.10		
+	ROUTED TO	F-F	92.	.88	16.	16.	16.	.10	1.80	.88
+	HYDROGRAPH AT	BH	26.	.73	4.	4.	4.	.03		
+	3 COMBINED AT	DP3	135.	.92	31.	31.	31.	.19		
+	HYDROGRAPH AT	BB	18.	.68	3.	3.	3.	.02		
+	HYDROGRAPH AT	BD	18.	.72	3.	3.	3.	.02		

*** NORMAL END OF HEC-1 ***

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* SEPTEMBER 1990
* VERSION 4.0
*
* RUN DATE 02/24/1998 TIME 09:39:28
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

```

```

X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN									
2	ID	100YR,2HR DEVELOPED				PART 2					
3	ID	FULL BASIN RUNOFF STUDY						100BD2.DAT			
	*DIAGRAM										
4	IT	1	01DEC97	1200	1500						
5	IN	5									
6	IO	5	0								
7	KK	BE									
8	KM	RUNOFF FROM BASIN E									
9	BA	0.01									
10	PB	3.1									
11	PI	.01	.03	.04	.07	.12	.22	.12	.07	.05	.04
12	PI	.03	.03	.03	.02	.02	.01	.01	.01	.01	.01
13	PI	.01	.01	.01	.01						
14	LS	0	72	5							
15	UD	0.14									
16	KK	BF									
17	KM	RUNOFF FROM BASIN F									
18	BA	0.13									
19	LS	0	72	5							
20	UD	0.13									
21	KK	BG									
22	KM	RUNOFF FROM BASIN G									
23	BA	0.03									
24	LS	0	72	5							
25	UD	0.19									
26	KK	B12-1									
27	KM	RUNOFF FROM SDA 12-1									
28	BA	.089									
29	LS	0	76	6							
30	UD	.126									
31	KK	B12-2									
32	KM	RUNOFF FROM SDA 12-2									
33	BA	.028									
34	LS	0	76	6							
35	UD	.113									

36	KK	DP1							
37	KM	COMBINE FLOWS FROM SDAs 12-1, 12-2							
38	HC	2							
39	KK	A-A							
40	KM	ROUTE FLOWS FROM CP1 TO CP2							
41	RS	1	STOR	0					
42	RC	0.12	.08	0.12	630	0.24			
43	RX	0	5	10	20	28	38	43	48
44	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

45	KK	B12-3							
46	KM	RUNOFF FROM SDA 12-3							
47	BA	.027							
48	LS	0	76	6					
49	UD	.145							
50	KK	B12-4							
51	KM	RUNOFF FROM SDA 12-4							
52	BA	.038							
53	LS	0	75	6					
54	UD	.110							
55	KK	B12-7							
56	KM	RUNOFF FROM SDA 12-7							
57	BA	.016							
58	LS	0	75	5					
59	UD	.134							
60	KK	DP2							
61	KM	COMBINE CP1 & SDAs 12-3, 12-4, 12-7							
62	HC	4							
63	KK	B-B							
64	KM	ROUTE FLOWS FROM CP2 TO CP3							
65	RS	1	STOR	0					
66	RC	0.12	.08	0.12	790	0.19			
67	RX	0	5	10	20	28	38	43	48
68	RY	15	12.5	10	0	0	10	12.5	15
69	KK	B12-5							
70	KM	RUNOFF FROM SDA 12-5							
71	BA	.081							
72	LS	0	76	10					
73	UD	.154							
74	KK	B12-8							
75	KM	RUNOFF FROM SDA 12-8							
76	BA	.023							
77	LS	0	75	5					
78	UD	.077							
79	KK	DP3							
80	KM	COMBINE FLOWS FROM CP2 & SDAs 12-5, 12-8							
81	HC	3							
82	KK	C-C							
83	KM	ROUTE FLOW FROM CP3 TO CP4							
84	RS	1	STOR	0					
85	RC	0.12	.08	0.12	650	0.26			
86	RX	0	5	10	20	28	38	43	48
87	RY	15	12.5	10	0	0	10	12.5	15

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

88	KK	B12-6							
89	KM	RUNOFF FROM SDA 12-6							
90	BA	.061							
91	LS	0	76	10					
92	UD	.129							
93	KK	B12-9							
94	KM	RUNOFF FROM SDA 12-9							

158	KM	ROUTE FLOW FROM DP8 TO DP300								
159	RS	1	STOR	0						
160	RC	0.12	.08	0.12	1200	.12				
161	RX	0	5	10	20	28	38	43	48	
162	RY	15	12.5	10	0	0	10	12.5	15	
163	KK	300A								
164	KM	RUNOFF FROM 300A								
165	BA	.049								
166	LS	0	65	4						
167	UD	.162								
168	KK	DP300								
169	KM	COMBINE FLOWS FROM CP8 & SDA 300A								
170	HC	2								

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

171	KK	B9								
172	KM	RUNOFF FROM SDA 9								
173	BA	.085								
174	LS	0	74	4						
175	UD	.125								
176	KK	H-H								
177	KM	ROUTE FLOW FROM B9 THRU SDA 300B								
178	RS	1	STOR	0						
179	RC	0.12	.08	0.12	1200	.25				
180	RX	0	5	10	20	28	38	43	48	
181	RY	15	12.5	10	0	0	10	12.5	15	
182	KK	300B								
183	KM	RUNOFF FROM 300B								
184	BA	.026								
185	LS	0	68	4						
186	UD	.126								
187	KK	DP9								
188	KM	COMBINE SDA B9 & 300B								
189	HC	2								
190	KK	DP301								
191	KM	COMBINE FLOWS FROM CP300 & SDA 300B								
192	HC	2								
193	KK	I-I								
194	KM	ROUTE FLOW FROM B9 THRU SDA 300B								
195	RS	1	STOR	0						
196	RC	0.12	.08	0.12	1800	.17				
197	RX	0	4	5.5	6.5	14.5	15.5	17	21	
198	RY	7	5	2	0	0	2	5	7	
199	KK	300C								
200	KM	RUNOFF FROM SDA 300C								
201	BA	.06								
202	LS	0	72	4						
203	UD	.150								
204	KK	DP302								
205	KM	COMBINE FLOWS FROM CP301 & 300C								
206	HC	2								
207	ZZ									

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT	(V) ROUTING	(--->)	DIVERSION OR PUMP FLOW
LINE			
NO.	(.) CONNECTOR	(<---)	RETURN OF DIVERTED OR PUMPED FLOW
7	BE		
	.		
16	.	BF	
	.	.	
	.	.	
21	.	.	BG
	.	.	.
	.	.	.

26	.	.	.	B12-1	.	.	.
31	B12-2	.
36	.	.	.	DP1
39	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
	.	.	.	A-A	.	.	.
45	B12-3	.
50	B12-4
55
	B12-7
60	.	.	.	DP2
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
63	.	.	.	B-B	.	.	.
69	B12-5	.
74	B12-8
79	.	.	.	DP3
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
82	.	.	.	C-C	.	.	.
88	B12-6	.
93	B12-9
98	.	.	.	DP4
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
101	.	.	.	D-D	.	.	.
107	B1210	.
112	.	.	.	DP5
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
115	.	.	.	E-E	.	.	.
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
121	.	.	.	DAM	.	.	.
130	B1211	.
135	.	.	.	DP6
	.	.	.	V	.	.	.
	.	.	.	V	.	.	.
138	.	.	.	F-F	.	.	.
144	B10111	.
149	B10112
154	.	.	.	DP8
	.	.	.	V	.	.	.

```

157      .      .      .      V
      .      .      .      G-G
      .      .      .      .
163      .      .      .      .
      .      .      .      300A
      .      .      .      .
168      .      .      .      DP300.....
      .      .      .      .
171      .      .      .      .
      .      .      .      B9
      .      .      .      V
176      .      .      .      V
      .      .      .      H-H
      .      .      .      .
182      .      .      .      .
      .      .      .      300B
      .      .      .      .
187      .      .      .      DP9.....
      .      .      .      .
190      .      .      .      DP301.....
      .      .      .      V
      .      .      .      V
193      .      .      .      I-I
      .      .      .      .
199      .      .      .      .
      .      .      .      300C
      .      .      .      .
204      .      .      .      DP302.....

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   SEPTEMBER 1990             *
*   VERSION 4.0                *
*
* RUN DATE 02/24/1998 TIME 09:39:28 *
*
*****

```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET            *
* DAVIS, CALIFORNIA 95616      *
* (916) 756-1104               *
*
*****

```

BOULDERS BROADMOOR COMMUNITY MASTER DRAINAGE PLAN
100YR,2HR DEVELOPED PART 2
FULL BASIN RUNOFF STUDY 100BD2.DAT

*** ERROR *** SPECIFIED START AND END DATES RESULT IN TOO MANY TIME PERIODS

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN       1 MINUTES IN COMPUTATION INTERVAL
          IDATE      1DEC97 STARTING DATE
          ITIME      1200 STARTING TIME
          NQ         300 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     1DEC97 ENDING DATE
          NDTIME     1659 ENDING TIME
          ICENT      19 CENTURY MARK

          COMPUTATION INTERVAL .02 HOURS
          TOTAL TIME BASE     4.98 HOURS

```

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
					6-HOUR	24-HOUR	72-HOUR			
+	HYDROGRAPH AT	BE	9.	.68	1.	1.	1.	.01		
+	HYDROGRAPH AT	BF	123.	.67	16.	16.	16.	.13		
+	HYDROGRAPH AT	BG	25.	.75	4.	4.	4.	.03		
+	HYDROGRAPH AT	B12-1	112.	.65	14.	14.	14.	.09		
+	HYDROGRAPH AT	B12-2	37.	.63	4.	4.	4.	.03		
+	2 COMBINED AT	DP1	148.	.65	18.	18.	18.	.12		
+	ROUTED TO	A-A	148.	.67	18.	18.	18.	.12	1.51	.67
+	HYDROGRAPH AT	B12-3	32.	.68	4.	4.	4.	.03		
+	HYDROGRAPH AT	B12-4	47.	.63	6.	6.	6.	.04		
+	HYDROGRAPH AT	B12-7	18.	.67	2.	2.	2.	.02		
+	4 COMBINED AT	DP2	244.	.67	30.	30.	30.	.20		
+	ROUTED TO	B-B	242.	.68	30.	30.	30.	.20	2.15	.68
+	HYDROGRAPH AT	B12-5	102.	.68	13.	13.	13.	.08		
+	HYDROGRAPH AT	B12-8	31.	.60	3.	3.	3.	.02		
+	3 COMBINED AT	DP3	369.	.67	47.	47.	47.	.30		
+	ROUTED TO	C-C	369.	.68	47.	47.	47.	.30	2.52	.68
+	HYDROGRAPH AT	B12-6	82.	.65	10.	10.	10.	.06		
+	HYDROGRAPH AT	B12-9	25.	.65	3.	3.	3.	.02		
+	3 COMBINED AT	DP4	472.	.68	60.	60.	60.	.38		
+	ROUTED TO	D-D	466.	.70	60.	60.	60.	.38	3.16	.70
+	HYDROGRAPH AT	B1210	72.	.72	10.	10.	10.	.07		
+	2 COMBINED AT	DP5	537.	.70	71.	71.	71.	.45		
+	ROUTED TO	E-E	527.	.73	71.	71.	71.	.45		

+									3.93	.73
+	ROUTED TO	DAM	222.	1.18	71.	71.	71.	.45		
+									7185.67	1.18
+	HYDROGRAPH AT	B1211	57.	.58	6.	6.	6.	.04		
+	2 COMBINED AT	DP6	240.	1.12	77.	77.	77.	.49		
+	ROUTED TO	F-F	239.	1.13	77.	77.	77.	.49		
+									2.41	1.13
+	HYDROGRAPH AT	B10111	37.	.70	5.	5.	5.	.04		
+	HYDROGRAPH AT	B10112	39.	.70	5.	5.	5.	.04		
+	3 COMBINED AT	DP8	274.	1.10	87.	87.	87.	.57		
+	ROUTED TO	G-G	274.	1.10	87.	87.	87.	.57		
+									2.64	1.10
+	HYDROGRAPH AT	300A	25.	.75	4.	4.	4.	.05		
+	2 COMBINED AT	DP300	293.	.95	91.	91.	91.	.62		
+	HYDROGRAPH AT	B9	91.	.67	12.	12.	12.	.09		
+	ROUTED TO	H-H	89.	.68	12.	12.	12.	.09		
+									1.07	.68
+	HYDROGRAPH AT	300B	18.	.68	3.	3.	3.	.03		
+	2 COMBINED AT	DP9	107.	.68	14.	14.	14.	.11		
+	2 COMBINED AT	DP301	362.	.87	106.	106.	106.	.73		
+	ROUTED TO	I-I	361.	.90	106.	106.	106.	.73		
+									2.96	.90
+	HYDROGRAPH AT	300C	53.	.70	7.	7.	7.	.06		
+	2 COMBINED AT	DP302	399.	.88	113.	113.	113.	.79		

*** NORMAL END OF HEC-1 ***

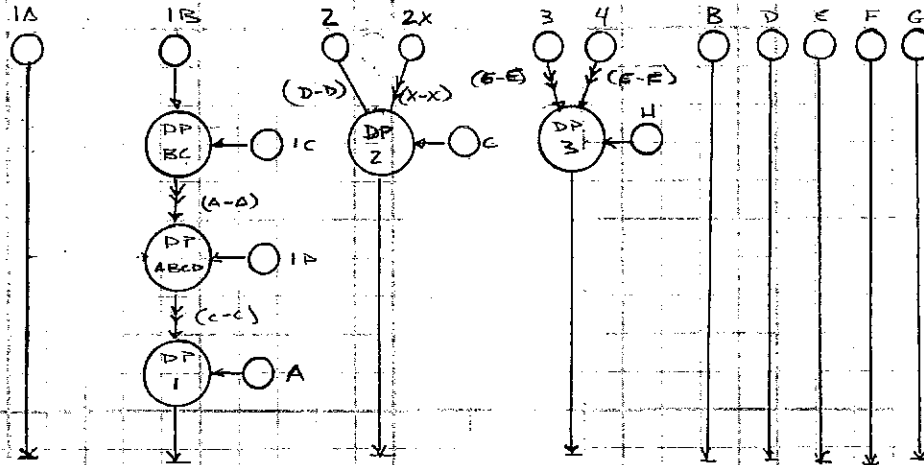
OBJECTIVE:

DEVELOP FLOW SCHEMATIC FOR FULL BASIN ANALYSIS

GIVEN:

SITE TOP

SOLUTION:



NOTE: REVISOR 12/18 TO INCLUDE NEW DRAINAGE AREA 2X.



OBJECTIVE:

ESTIMATE BASIN CHARACTERISTICS FOR SUB-BASIN 2X

GIVEN:

SITE TOP

SOLUTION:

ID	AREA (mi ²)	Avg Slope (F/F)	Losses (EF)	$t_c^{(1)}$ (min)	$t_{channel}^{(2)}$ (FF)	$t_c^{(2)}$ (min)	$t_c^{(3)}$ (min)	LAG (hr)
2X	0.005	0.49	50	12.1	490	1.2	13.3	0.13

$$(1) 167n \left(\frac{0.36}{5} \right)^{0.33} / (5)^{0.2}$$

$$(2) 0.0075 L^{0.77} / 5^{0.885}$$

$$(3) t_1 + t_2$$

$$m = 0.04$$

OBJECTIVE:

DEVELOP SUMMARY OF BASIN HYDROLOGIC CHARACTERISTICS FOR DEVELOPED BOULDERS BROADMOON COMMUNITY

GIVEN:

SITE MAP (KNOWN 1"=100')

SOLUTIONS:

NOTE FROM PREVIOUS KNOWN BASIN DELINEATION: SUB-BASINS TO BE COMBINED UNDER 1 BASIN ID (I.E. A1..A11 → A, B1 → B, C1-10 → C, ETC.)

BASIN ID	AREA (MI ²)	AVG. SLOPE (F/F)	LENGTH (FEET)	$t_{c0}^{(1)}$ (min)	$t_{c1}^{(2)}$ (FE)	$t_{c2}^{(3)}$ (min)	$t_{c3}^{(4)}$ (min)	LAG (min)	LAGS
A ⁽⁵⁾	0.091	0.26	190	21.3	1540	3.4	24.7	0.25	0.23
B ⁽⁶⁾	0.018	0.17	70	14.6	130	0.7	15.3	0.15	0.14
C ⁽⁶⁾	0.088	0.17	140	18.3	2200	1.0	19.3	0.19	0.17
D ⁽⁵⁾	0.023	0.17	40	13.9	400	1.6	15.5	0.16	-
E ⁽⁵⁾	0.010	0.18	40	12.0	360	1.5	13.5	0.14	-
F ⁽⁶⁾	0.009	0.18	50	12.9	-	0	12.9	0.13	0.12
G ⁽⁶⁾	0.034	0.20	100	15.9	560	1.9	17.8	0.18	0.16
H ⁽⁶⁾	0.030	0.22	110	16.1	840	2.5	18.6	0.19	0.17

- (1) $102 \pi ((0.34)^{0.35}) / (L)^{0.2}$
- (2) $0.0078 L^{0.775} \pi^{0.385}$
- (3) $t_{c1} + t_{c2}$
- (4) (3) - 0.6/60
- (5) $\pi = 0.04$
- (6) $\pi = 0.025$ } SUGGESTED π -VALUES (FR-55)
- (7) ADJUSTED LAG TIME, REDUCED BY 10% FOR DEVELOPED CONDITIONS

OBJECTIVE:

DEVELOP WATERSHED CHARACTERISTICS FOR BOULDER BRANCH

GLGWS:

Site topo

SOLUTION:

BASIN	AREA (mi ²)	AUG SLOPE (F/F)	LENGTH (FF)	(1) t _{LO} (min)	(2) L _{CHNL} (FF)	(3) t _{c1} (min)	(5) t _{c2} (min)	(4) L _{AG} (hr)
1A	0.023	0.27	100	10.70	1200	4.17	14.87	0.15
1B	0.231	0.36	1000	21.32	3000	5.39	26.71	0.27
1C	0.037	0.70	1100	19.47	1550	2.56	22.03	0.22
1D	0.050	0.20	0	0.0	1300	3.62	3.62	0.04
2	0.043	0.40	300	14.18	2500	4.59	18.77	0.19
3	0.0667	0.25	600	19.59	2600	6.00	25.59	0.26
4	0.096	0.50	500	16.06	3200	5.09	21.15	0.21

(1) $10.7 \text{ hr} \left(\frac{0.36}{0.37} \right)^{0.37} / (5)^{0.2}$
 (2) $0.0078 \text{ hr} \left(\frac{0.72}{0.36} \right)^{0.72} / (5)^{0.385}$
 (3) $t_{c1} + t_{c2}$
 (4) (3) - 0.6

SUMMARY OF WATERSHED CHARACTERISTICS
BOULDERS BROADLANDS

Watershed	Area ft ²	acres	Average S	Overland ft	tc	Channel ft	tc Kirpich min	Total Min	Time lag hr	
1A	635811.3	0.023	15	0.27	100	10.70	1800	4.17	14.87	0.15
1B	6435613.1	0.231	148	0.38	1000	21.32	3000	5.39	26.71	0.27
1C	1020522	0.037	23	0.70	1100	19.47	1550	2.56	22.03	0.21
1D	1392523	0.050	32	0.20	0	0.00	1300	3.62	3.62	0.04
			218							
2	1193502.3	0.043	27	0.40	300	14.18	2500	4.59	18.77	0.19
3	19145638	0.0687	440	0.25	600	19.59	2800	6.00	25.59	0.26
4	2674065.3	0.096	61	0.50	500	16.06	3200	5.09	21.15	0.21
Total			746							

Overland tc= $107 * n * ((0.3 * L)^{0.33}) / (S^{0.2})$
 Kirpich= $0.0078 * L^{0.77} * (s^{0.385})$

Exhibit A-1, continued: Hydrologic soil groups for United States soils

CHOREE, LIMESTONE	D	CLALLAM	C	CLIPPER	D	CKEL	B	COLVIN, OYERBLOWN,	C
SUBSTRATUM		CLAM GULCH	D	CLIPPER, DRAINED	D	CKER	D	SALINE	
CHOCOLOCCO	B	CLAM	C/D	CLODINE	D	CKESBURY	D	COLWOOD	B/D
CHOCK	D	CLAMP	D	CLONTARP	E	CKEVILLE	B	COLY	B
CHOCORUA	D	CLANA	A	CLOQUALLUM	C	COLAND	B/D	COLYER	D
CHOICE	D	CLANALPINE	C	CLOQUATO	E	COLBAR	C	CONAD	A
CHOOP	D	CLANTON	C	CLOQUET	B	COLBERT	D	CONAR	C
CHOPTIE	D	CLAPPER	B	CLOSKEY	C	COLBURN	C	CONBE	B
CHORALMONT	B	CLARENORE	D	CLOTNO	C/D	COLBY	B	CONBS	B
CHOSKA	B	CLARENCE	D	CLOUD PEAK	C	COLCREEK	B	CONER	B
CHOTEAU	C	CLARENDON	C	CLOUD RIN	B	COLDENT	C	CONETA	D
CHOVAN	D	CLARESON	C	CLOUDCROFT	D	COLLE	C	CONFORT	D
CHRIS	C	CLAREVILLE	C	CLOUDLAND	D	COLEMAN	C	CONFREY	B/D
CHRISHAN	D	CLARINDA	D	CLOUGH	D	COLEPANTOWN	C/D	CONFREY, PONDED	D
CHRISTIAN	C	CLARION	D	CLOVELLY	B	COLESTINE	C	CONITAS	A
CHRISTIANA	C	CLARITA	B	CLOVER SPRINGS	D	COLFAX	C	CONLY	C
CHRISTIANSBURG	C	CLARK	B	CLOVERDALE	D	COLMILL	B	CONMERCE	C
CHRISTINE	D	CLARK FORK	A	CLOVERLAND	C	COLIBRO	B	CONMERI	B
CHRISTOFF	C	CLARKELEN	B	CLOVIS	B	COLINAS	B	CONO	A
CHRISTY	C	CLARKRANGE	C	CLOWERS	B	COLITA	D	CONORABI	D
CHRODER	B	CLARKSBURG	C	CLOWERS, WET	C	COLLAMER	C	CONODDRE	D
CHROME	C	CLARKSDALE	C	CLOWFIN	B	COLLARD	E	CONORD	B
CHRYSLER	C	CLARKSVILLE	B	CLUFF	C	COLLAYOMI	B	COMPASS	B
CHUALAR	B	CLARNO	B	CLUMIE	C	COLLBRAN	D	COMPTCHE	B
CHUBBS	C	CLATO	B	CLURDE	F	COLLEBRAN, COBBLY	C	CONSTOCK	C
CHUCKAMUT	B	CLATSOP	D	CLURO	B	COLLEGEDALE	C	CONUS	C
CHUCKAVALLA	B	CLAUNCH	B	CLYDE	B/D	COLLEGIATE	D	CONA	C
CHUCKLES	B	CLAVERACK	C	CLYNER	E	COLLETT		CONABY	B/D
CHUCKRIDGE	D	CLAVICON	C	COACHMELLA	B	COLLETT, DRAINED	C	CONALB	B
CHUGCREEK	C	CLAWSON	C	COACHMELLA, WET	C	COLLIER	A	CONANT	C
CHUGTER	D	CLAYBURN	B	COANUILA	B	COLLINGTON	B	CONASAUGA	C
CHUIT	B	CLAYSPPRINGS	D	COAL CREEK	D	COLLINS	C	CONATA	D
CHULITNA	B	CLAYTON	B	COALBANK	B	COLLINSTON	D	CONSDY	D
CHUNALL	B	CLE ELUM	C	COALDALE	D	COLLINSVILLE	D	CONCEPCION	D
CHUNNY	D	CLEAR LAKE	D	COALDRAW	D	COLLINWOOD	C	CONCHAS	C
CHUMSTICK	D	CLEAR LAKE,	C	COALMONT	C	COLMA	B	CONCHO	C
CHUPADERA	C	STRATIFIED		COAMO	C	COLNOR	B	CONCONULLY	B
CHURCH	D	SUBSTRATUM		COARSEGOLD	D	COLNEVEE	B	CONCOPD	B
CHURCHMILL	D	CLEAR LAKE,	C	COATSBURG	D	COLO	B/D	CONDA	B
CHURCHVILLE	D	MODERATELY WET		COBAT	B	COLO, DRAINED	B	CONDE	B
CHURN	B	CLEARBROOK	D	CCRATUS	C	COLO, NONFLOODED	B	CONDIT	D
CHUSKA	D	CLEARFIELD	C	CCBB	B	COLOCKUM	B	CONDCHN	C
CHUTE	A	CLEARFORK	D	CCBSFORK	D	COLOMA	A	CONE	A
CIALES	D	CLEARWATER	D	COBEN	D	COLOMBO	B	CONECUM	D
CIBIQUE	B	CLEAVAGE	D	COBEY	F	COLONA	C	CONEJO	B
CIBO	D	CLEAVER	D	COBLE	D	COLONIE	A	CONEJO, WET	C
CIBOLA	B	CLEAYHOR	D	CCBOC	C	COLONYVILLE	C	CONEJO, GRAVELLY	C
CID	C	CLEBIT	D	CCBRE	C	COLORADO	B	SUBSTRATUM	
CIDRAL	C	CLEGG	B	CCBURG	C	COLOROCK	D	CONESTOGA	B
CIENEGA	C	CLEGHORN	C	CCCHETOPA	C	COLOROV	B	CONESUS	B
CIENO	D	CLEMAN	B	CCCHINA	D	COLOSD	D	CONETOE	A
CIERYO, ALKALI	D	CLEMENTINE	C	CCCHITI	C	COLOSSE	A	CONGAREE	B
CIERYO, ALKALI,	D	CLEMENTINE,	B	CCCHPAN	C	COLP	C	CONGER	C
WET		DRAINED		CCCOA	A	COLRAIN	B	CONGER, COBBLY	D
CIERYO, RECLAIMED	C	CLEMS	B	CCCODRIE	C	COLSAYAGE	C	SUBSTRATUM	
CIFIC	C	CLEMVILLE	B	CCCOLALLA	D	COLTER	B	CONGLE	B
CINARRON	C	CLENDENEN	D	CCCOLALLA, DRAINED	C	COLTMRP	D	CONI	D
CINCINNATI	C	CLEONE	B	CCOLEY	B	COLTON	A	CONIC	C
CINCO	A	CLEORA	E	CCODRUS	C	COLTROOP	C	CONLEN	B
CINDERHURST	D	CLERF	C	CCOQUIN	D	COLTS NECK	B	CONLEY	C
CINER	B	CLERGERN	B	CCODYLAKE	B	COLUMBIA, MUCK	B	CONNEAUT	B
CINMADALE	D	CLERNONT	D	CCOE	A	SUBSTRATUM		CONNEL	C
CINNAMON	B	CLEVELAND	C	CCOROCK	D	COLUMBIA, DRAINED,	B	CONNERTON	B
CINTRONA	D	CLEVERLY	B	CCESSE	C/D	CLAY SUBSTRATUM		CONOSTA	C
CIPRIANO	D	CLICK	A	CCPF	C	COLUMBIA,	C	CONOTTON	B
CIRAC	B	CLIFFDELL	B	CCOFFEEN	B	MODERATELY WET		CONOVER	C
CIRCLEBACK	A	CLIFFDOWN	B	CCGGON	B	COLUMBIA, DRAINED	B	CONOWINGO	C
CIRCLEBAR	C	CLIFFHOUSE	C	CCGNA	B	COLUMBIA, FLOODED	C	CONPEAK	D
CIRCLEVILLE	C	CLIFFORD	C	CCGSWELL	C	COLUMBIA, CLAY	C	CONRAD	A/D
CISCO	B	CLIFPSAND	B	CCMAGEN	D	SUBSTRATUM		CONRDE	B
CISNE	D	CLIFTERSON	B	CCMASSET	E	COLUMBIA, SLOPING	B	CONSEJO	C
CISPUS	B	CLIFTON	B	CCMOCTAM	B/D	COLUMBINE	A	CONSER	D
CITADEL	C	CLIFTY	B	CCMOCTAM, SANDY	D	COLUMBUS	C	CONSTABLE	A
CITICO	B	CLIMARA	D	SUBSTRATUM		COLUSA	C	CONSTANCIA	D
CITRONELLE	D	CLINAX	D	CCMDE	B	COLYARD	B	CONSUD	B
CLACKANAS	D	CLINE	C	CCOILS	C	COLVILLE	D	CONTACT	A
CLAIBORNE	B	CLINETOP	D	CCIT	D	COLVILLE, DRAINED	C	CONTEE	D
CLAIRE	A	CLINT	C	CCOKEDALE	D	COLVIN	C/D	CONTYDE	B
CLAIRMONT	B	CLINTON	B	CCOKEDALE, DRAINED	C	COLVIN, SALINE	C	CONTINE	C

NOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS B/C INDICATES THE DRAINED/UNDRAINED SITUATION.
MODIFIERS SHOWN, E.G., BEDROCK SUBSTRATUM; REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.

Exhibit A-1, continued: Hydrologic soil groups for United States soils

HUNNTON	C	ILDECARE	B	IPISH	C	JACAGUAS	B	JEHEMY	D
HUNSINGER	B	ILDEFONSO	B	IPSON	B	JACANA	D	JEKLEY	C
HUNTERS	B	ILES	C	IPSWICH	D	JACEE	C	JELLICO	C
HUNTERSVILLE	B	ILIFF	C	IRA	C	JACINTO	B	JENEZ	C
HUNTNER	C	ILILI	D	IRAAN	B	JACK CREEK	A	JENA	B
HUNTING	C	ILIGN	D	IREDELL	C/D	JACKET	C	JENKINS	C
HUNTINGTON	B	ILLABOT	C	IRELAND	C	JACKLAND	D	JENKINSON	D
HUNTMOUNT	B	ILLANEE	B	IRENE	B	JACKMAN	B	JENKS	B
HUNTPOCK	B	ILLER	B	IRETEBA	B	JACKNIFE	C	JENNESS	B
HUNTSBURG	D	ILLITO	D	IRIGUL	D	JACKPORT	D	JENNINGS	B
HUNTSVILLE	B	ILLTON	C	IRIN	C	JACKPOT	C	JENNY	D
MUPP	B	ILNACO	B	IRNULCO	E	JACKS	C	JENOR	C
MURDS	B	IMA	B	IROCK	E	JACKSON	E	JERAG	D
MURLBUT	C	IMBLER	B	IRON BLOSSOM	C	JACKTONE	D	JERAULD	D
MURLEY	D	IMLAY	D	IRON MOUNTAIN	D	JACOE	D	JERICHO	D
HURRICANE	C	IMHIG	C	IRON RIVER	B	JACOBSEN	D	JEROME	D
HURRY BACK	B	IMMIGRANT	C	IRONCO	B	JACOB	C	JERRY	D
HURRYBACK	B	IMMOKALEE	B/D	IRONDALE	C	JACOT	B	JERRYSLU	C
MURST	D	IMMOKALEE,	D	IRONOYKE	B	JACQUES	C	JERU	B
MURNAL	E	DEPRESSIONAL		IRONSPRINGS	B	JACQUITH	C	JERYAL	B
MUSE	D	IMOGENE	D	IRCHYON	D	JACRAZT	C	JESREL	D
MUSKA	D	IMONIL	B	IRCOUDIS	B/D	JACHIN	D	JESSE CAMP	B
MUSSA	D	IMPACT	A	IRRAWADDY	C	JADIS	B	JESSIETOWN	B
MUSSA, CLAYEY	C	IMPERIAL	D	IRRIGON	C	JAFI	B	JESSO	C
SUBSTRATUM		INARAJAN	D	IRSON	D	JAGUEYES	B	JESSUP	C
MUSSA, MODERATELY	C	INARAJAN,	C	IRVINE	D	JAL	B	JETCOP	D
NET		STRATIFIED		IRVINGTON	C	JALMAR	A	JETSTER	C
MUSSA, DRAINED	B	SUBSTRATUM		IRWIN	D	JAMES	D	JETT	C
MUSSELL	B	INAVALE	A	ISAAC	A	JAMES CANYON	C	JEVES	B
MUSSEMAN	D	INCELL	D	ISABELLA	D	JAMES CANYON,	B	JEWETT	B
MUSUM	B	INCHAU	C	ISAN	A/D	DRAINED		JIGGS	B
MUTCHINSON	C	INCHELTUM	E	ISANTI	A/D	JAMESTON	C/D	JIGSAV	B
MUTCHLEY	D	INCY	A	ISELL	B	JANISE	C	JILSON	C
MUTTON	B	INDARY	C	ISELLA	B	JANISE, OVERBLOWN,	B	JIM	C
MUTT	D	INDEX	A	ISEMI PISHI	B	DRAINED		JIMBO	B
HUTTON	D	INDIANOMA	D	ISPENING	A	JANSEN	B	JIMCREEK	C
MUXLEY	C	INDIAN CREEK	D	ISIDOR	D	JANUDE	B	JIREK	C
MUYSINK	B	INDIANOJ	C	ISKMAT	C	JANUDE, CLAY	C	JIMENEZ	C
MYALL	C	INDIANOLA	A	ISKMAT, CJOJL	D	SUBSTRATUM		JIMLAKE	B
MYANNIS	B	INDIO	B	ISLAMO	B	JARAB	D	JIMMERSON	C
MYATTVILLE	C	INDOLETON	B	ISLES	D	JARBOE	D	JIMSAGE	B
MYDABURG	D	INDOUS	B	ISLES, SLJUCH	A/D	JARDIN	D	JINTOWN	C
MYDE	B/D	INIZ	D	ISLOT	B	JAREALES	C	JIPPER	B
MYDER	D	INFERNAL	D	ISMAY	B	JARITA	C	JIVAS	B
MYDRD	C	INGALLS	B	ISMCD	C	JARMILLO	E	JOACHEN	D
MYE	B	INGENIO	B	ISOLDE	A	JAROLA	C	JOE	C
MYLOC	B	INGERSOLL	B	ISOM	B	JAROSD	B	JOEBS	C
MYNAS	D	INGRAM	D	ISTER	C	JARRE	B/D	JOEPEAK	D
MYRAIRIE	B	INKLER	B	ISTOKPOGA	B/D	JARRON	D	JOCAL	B
MYRUM	B	INKON, DRAINED	D	ITANO	C	JARVIS	E	JOCITY	B
MYSHAM	D	INKOSR	C	ITASCA	B	JASCO	D	JOCITY, LDANY	C
MYSHOT	D	INKS	D	ITAT	F	JASON	D	SURFACE	
MYTOP	D	INKSTEP	B	ITCA	D	JASPER	B	JOCKD	B
MYZEN	D	INLOW	C	ITHACA	C	JAUCA	A	JOCERO	B
IAD	B	INMACHUK	D	ITNAAN	C	JAUCA, SALINE	C	JOEL	B
IBERIA	D	INMAN	C	ITHE	A	JAUURIGA	B	JOENRE	B
ICARIA	D	INMD	A	ITVOOT	B	JAVA	B	JOENEY	D
ICEME	D	INNINGER	C	IUKA	C	JAVDONE	D	JOES	B
ICESLEY	D	INDEPENDENCE	B	IWA	C	JAY	C	JOEVAR	B
ICHOD	D	INSAK	B	IVAN	B	JAYAH	C	JOHNS	C
ICHETUCKNEE	D	INSIDERT	D	IVANELL	C	JAYBEE	D	JOHNSBURG	D
ICICLE	B	INSKIP	C	IVANHCE	D	JAYEL	D	JOHNSON	R
IDA	B	INSULA	O	IYEP	B	JAYE	R	JOHNSTON	D
IDABEL	B	INFERIOR	B	IYERSEN	C	JAYNES	D	JOHNSTOWN	B
IDAHOME	B	INTON	F	IYES, WET	B	JEAGER	C	JOHNWOOD	B
IDAMONT	B	INVERNESS	B	IYIE	A	JEAN	A	JOHNTON	D
IDEE	C	INVERSHIEL	C	IYINS	C	JEAN LAKE	B	JOICE	D
IDLEVILD	D	INVILLE	B	IYNYILD	C	JEANERETTE	D	JOINEP	B
IDLEVILD, DRAINED	C	IO	B	IYRIAN	C	JEBE	B	JOKODOVSKI	D
IDON	B	IOLEAU	C	IYTER	D	JEDO	B	JOLAN	C
IDPELL	C	IONA	B	IYTERA	D	JEDBURG	C	JOLIET	O
IGERT	C	IONIA	B	IYAGORA	C	JEDO	C	JOLLY	C
IGNACIO	C	IOSCO	B	IYAR	D	JEDDITO	C	JONALE	B
IGO	D	IOSEPA	D	IYEE	C	JEDDITO,	B	JONAS	B
IGUALDAD	D	IOTLA	B	IYD	A	SALINE-ALKALI		JONATHAN	B
IMLEN	B	IPAGE	A	IYDOD	D	JFDD	C/D	JONCA	C
IJAM	D	IPAND	C	IYUSER	B	JEFFERS	B/D	JONDA	B
ILACHETONEL	D	IPAVA	B	IYAPU	B	JEFFERSON	B	JONES	B
				IYABU, WET	C	JEFFREY	B	JONESVILLE	B

NOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS B/C INDICATES THE DRAINED/UNDRAINED SITUATION. MODIFIERS SHOWN, E.G., BEDROCK SUBSTRATUM, REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.

Exhibit A-1, continued: Hydrologic soil groups for United States soils

SUNYA	D	SWANTOWN	D	TACOMA	D	TANGUE	B	TENAMA	C
SUN	D	SWANVILLE	C	TACONIC	C/D	TANSEN	B	TENHRAN	A
SUNAPEE	B	SWANWICK	D	TACDOSH	B/D	TANTALUS	B	TEIGEN	C
SUNBURG	B	SWAPPS	C	TADLOCK	B	TANTILE	C/D	TEJA	D
SUNBURST	C	SWARTSMOOD	C	TAFFON	B	TANNAH	D	TEJABE	D
SUNBURY	B	SWARTZ	D	TAFOTA	C	TANNAH, DRAINED	C	TEJANA	B
SUNCITY	D	SWASEY	D	TAFT	C	TANTARD	C	TEKENINK	B
SUNCOOK	A	SWASTIKA	C	TAFTHAM	B	TAOPI	B	TEKISON	C
SUND	C	SWAUK	D	TAFUNA	A	TAPCO	D	TEKLANIKA	A
SUNOANCE	B	SWATNE	C	TAGGART	C	TAPIA	B	TEKOA	C
SUNDAY	A	SWEATMAN	C	TAGLAKE	E	TAPICITONES	D	TEKOA, EXTREMELY	B
SUNDELL	B	SWEDE	E	TANKENITCH	E	TAPPAN	B/D	STONY	C
SUNDOWN	A	SWEEN	C	TANNA	E	TARA	B	TELA	B
SUNEY	B	SWEENEY	B	TANQUILA	D	TARBOPO	A	TELCHER	B
SUNFIELD	B	SWEET	C	TANQUATS	B	TARGHEE	C	TELECAN	B
SUNLIGHT	D	SWEETAPPLE	B	TANTOR	C/D	TARKINGTON	C	TELEFONO	C
SUNNYWAY	D	SWEETGRASS	B	TAJO	C	TARKIO	D	TELEMON	D
SUNNYSIDE	B	SWEETWATER	D	TAKEUCHI	C	TARKLIN	C	TELEPHONE	D
SUNNYVALE	C	SWEITBERG	C	TAKILMA	B	TARLOC	B	TELESCOPE	A
SUNRAY	B	SWEITING	C	TAKOTNA	B	TARNACH	D	TELFER	A
SUNRISE	C	SWEN	C	TAKPOCHAD	D	TARNAY	B	TELFERNER	D
SUNSET	B	SWENODA	E	TALAG	D	TARPLEY	D	TELL	B
SUNSHINE	C	SWIFT	B	TALAMANTES	B	TARR	A	TELLER	B
SUNSMOET	C	SWIFT CREEK	B	TALANTE	D	TARRANT	D	TELLICO	B
SUNUP	D	SWIFTON	B	TALAPUS	F	TARRETE	D	TELLMAN	B
SUNY	D	SWINLEY	C	TALBOTT	C	TARRYALL	C	TELLURA	C
SUONI	C	SWINS	B	TALCO	D	TARRYTOWN	C	TELOS	C
SUP	D	SWINGLER	B	TALCOT	B/D	TASAYA	C	TELSTAD	C
SUPAN	B	SWINGLER, WET	C	TALININA	D	TASCOSA	B	TEMAN	B
SUPERIOR	D	STRONGLY SALINE	E	TALKEETNA	E	TASSEL	D	TEMBLOR	D
SUPERSTITION	A	SWINGLER, WET	C	TALLA	C	TASSELMAN	D	TEMESCAL	D
SUPERVISOR	C	SWINK	D	TALLAC	F	TASSO	B	TEMO	C
SUPPLEE	B	SWINDMISH	C	TALLADEGA	C	TATAI	C	TEMPLE	C
SUR	C	SWINT	B	TALLAPOOSA	C	TATE	B	TEMPLETON	B
SURFSIDE	D	SWISBOR	D	TALLEYVILLE	B	TATERHEAP	B	TEPKIK	B
SURGEN	C	SWISSHELM	E	TALLOONOOK	C	TATIYEE	C	TENAGO	D
SURGH	B	SWISSTAG	B	TALLS	E	TATLUM	D	TENANA	B
SURNUP	B	SWISSVALE	D	TALLULA	E	TATOUCHE	B	TEMAS	C
SURPLUS	C	SWITCHNACK	C	TALLY	B	TATTON	D	TENCEE	D
SURPRISE	B	SWITZERLAND	B	TALMAGE	B	TATUM	E	TENDODY	D
SURRENCY	D	SVOPE	C	TALMO	A	TAUNTON	C	TENERIFFE	A
SURRETT	C	SWORMVILLF	C	TALMOON	D	TAVAPES	A	TEMEK	E
SURVEYORS	B	SVYGERT	C	TALOKA	D	TAVAH	B	TENINO	C
SURYVA	C	SYBLON	D	TALPA	D	TANAS	A/D	TENMILE	C
SUSANNA	C/D	SYCANORE	B	TALQUIN	F/D	TANCAN	C	TENMO	D
SUSANVILLE	D	MODERATELY WET		TALUCE	D	TAYLOR	C	TENORIO	B
SUSIE CREEK	C	SALINE		TAPA	E	TAYLOR CREEK	C	TENOT	C
SUSITHA	B	SYCANORE	C	TAPAMA	D	TAYLORSFLAT	B	TENPIN	D
SUSQUEMANNA	D	MODERATELY WET		TAPALCO	D	TAYLORSFLAT	C	TENRAG	B
SUTA	B	CLAYEY SUBSTRATUM		TAPALPAIS	C	SALINE-ALKALI		TENAS	D
SUTCLIFF	B	SYCANORE	C	TAPANEEEN	E	TAYLORSVILLE	C	TENSED	C
SUTHER	C	MODERATELY WET		TAMBA	D	TAILINA	A	TENSLEEP	B
SUTHERLAND	D	SYCANORE, DRAINED	B	TAMELY	B	TEAGULF	C	TENSHOIR	B
SUTHERLIN	C	SYCANORE, FLOODED	C	TAMFLAT	D	TEAKEAN	B	TENYORRO	D
SUTKIN	B	SYCANORE, CLAY	B	TAMFORD	D	TEALSON	D	TEO	B
SUTLEY	B	SUBSTRATUM		TAMMANY CREEK	B	TEALUMIT	D	TEOCULLI	B
SUTPHEN	D	SYCAN	A	TAMPING	F	TEANAWAY	B	TEPETE	D
SUTRO	C	SYCLE	D	TAMP	E	TEAPO	C	TEQUESTA	B/D
SUTTLE	B	SYCOLINE	B	TAMPICO	B	TEASDALE	E	TERADA	B
SUTTON	B	SYENITE	C	TAPANA	D	TEASPOOM	D	TERBIES	B
SUVER	D	SYLACAUGA	D	TAPANA	D	TEBAY	F	TERENCE	B
SUWANEE	B	SYLCO	C	TAPANA, THAWED	E	TEBBS	B	TERESA	D
SVEA	B	SYLVAN	B	TAPANA, MODERATELY	C	TEBO	B	TERINO	D
SVENSEN	B	SYLVANIAN	C	NET		TECHADO	D	TERLAN	D
SVERDRUP	B	SYLVESTER	B	TANASSEE	B	TECHICK	B	TERLCO	B
SWAGER	C	SYLVIA	C	TANAZZA	E	TECO	E	TERLINGUA	D
SWAINOV	B	SYNCD	C	TANBARK	D	TECOLOTE	B	TERMINAL	D
SWAKANE	D	SYNERTON	B	TANDY	D	TECONAN	D	TERMO	D
SWALER	D	SYNAREP	E	TANEUP	E	TECOPA	D	TEROMOTE	B
SWALESILVER	D	SYRACUSE	B	TANEY	C	TECROW	B	TEROUGE	D
SWAMPYDRAH	D	SYRENE	B/D	TANGAIR	C	TEEL	B	TERRA CIA	B/D
SWAN	D	SPETT	C	TANGI	C	TEELER	B	TERRA CIA, TIDAL	D
SWANBOY	D	TABECHEMING	C	TANGLE	C	TEEMAT	B	TERRA CIA	D
SWANDAO	B	TABERNASH	B	TANNA	D	TEESTO	D	FREQUENTLY	
SWANLAKE	B	TABLE MOUNTAIN	B	TANNANHILL	B	TEETERS	C	FLOODED	
SWANNER	D	TABLER	D	TANNER	D	TEEWINDT	C	TERRAD	C
SWANSEA	D	TABOR	D	TANNER, LOW	D	TEFTON	D	TERRETON	D
SWANSON	C	TACAN	B	PRECIPITATION	B	TEGUND	D	TERRETION, STONY	C
SWANTON	C/D	TACHI	D	TANDP	E	TEHACNAPI	C	TERRIL	B

NOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS B/C INDICATES THE DRAINED/UNDRAINED SITUATION. MODIFIERS SHOWN, E.G., BEDROCK SUBSTRATUM, REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.

Exhibit A-1, continued: Hydrologic soil groups for United States soils

TERRO	C	TMURLONI	C	TINTON	A	TCLTEC	C	TORSIDO	D
TERRY	C	TMURLOY	B	TINYTOWN	B	TOLUCA	B	TORTUGAS	D
TERT	D	TMURNAN	A	TIOCAND	D	TOLVAR	B	TORULL	D
TERMILLIGER	C	TMURNONT	B	TIOGA	B	TONAH	B	TOSCA	B
TESAJO	B	TMWOOD	C	TIPPAH	C	TONAHAWK	A	TOSBER	B
TESSFIVE	D	TIAGOS	B	TIPPECANOE	B	TONALES	D	TOSTON	C
TETMRICK	B	TIAK	C	TIPPER	C	TONASAKI	C	TOTAVI	A
TETON	C	TIBAN	B	TIPPERARY	A	TONAST	C	TOTELAKE	B
TETONIA	B	TIBBITTS	B	TIPPYPAH	B	TONRAF	C	TOTEM	B
TETONKA	C/D	TISS	C	TIPPO	C	TONBSTONE	C	TOTIER	C
TETONVIEW	D	TIBSON	B	TIPTON	B	TOHE	B	TOTO	B/D
TETONVILLE	D	TIBURONES	D	TIPTONVILLE	B	TOHEL	D	TOTYEN	C/D
TETONVILLE, GRAVELLY	C	TICA	D	TIPTOP	B	TONERA	C	TOUCHET	C
TETOTUM	C	TICE	B	TIRO	C	TONERA, CEMENTED SUBSTRATUM	D	TOUHEY	B
TEVIS	B	TICELL	D	TISEBURY	B	TONI	A	TOULON	B
TEV	C	TICMOP	D	TISCH	D	TONIACHI	B	TOURN	C
TEVA	B	TICINO	C	TISDALE	C	TONKA	B/D	TOURNOUIST	B
TEX	B	TICKAPOO	D	TISHAR	B	TONOTLEY	B/D	TOURS	B
TEXAMA	D	TICKASON	B	TISONIA	B	TONS	D	TOUTLE	A
TEXARK	D	TIDINGS	B	TISWORTH	C	TONSHERRY	C	TOUTLE, FLOODED	B
TEXLINE	B	TIDWELL	D	TITUS	E/D	TONTY	D	TOVAR	C
TEXROY	B	TIERRA	D	TITUSVILLE	C	TONALEA	C	TOVAY	C
TEZUHA	C	TIERRANEGRE	B	TIVOLI	A	TONASKET	B	TOVAYE	B
THACKER	D	TIESIDE	D	TIVY	C	TONATA	D	TOVHEE	D
THACKERY	B	TIETON	E	TGA	B	TONCANA	B	TOWNER	B
THADEK	C	TIFFANY	F/D	TGADLAKE	B	TONY	D	TOWNLEY	C
THAGE	C	TIFTON	B	TDANC	B	TONGUE RIVER	C	TOWNSEND	C
THATCHER	B	TIGER CREEK	F	TDANO	E	TONIO	E	TOVOSANGY	B
THATUNA	C	TIGERON	B	TDICO	A/D	TONKA	C/D	TOXAVAY	B/D
THAYNE	B	TIGIT	C	TDGIN	F	TONKAVAR	A	TOY	D
THESES	B	TIGIVON	B	TDISH	C	TONKAWA	A	TOYAM	B
THEBO	D	TIGLEY	B	TDLER	E	TONKEY	B/D	TOYUSKA	B
THEDALUND	C	TIGON	D	TDOSA	D	TONKIN	B	TOZE	B
THEEDLE	C	TIGUA	D	TDY	E	TONKIN, MODERATELY WET	C	TRABUCC	C
THENAS	C	TIJERAS	E	TDAL	C	TONKS	C	TRACHUTE	B
THEODOR	D	TIKI	D	TDALONA	C	TONOPAH	A	TRACK	C
THEON	B	TIKIF	B/D	TDCCA	B	TONOR	B	TRACK, DRAINED	O
THERESA	D	TILFER	B	TDCCA	B	TONOR	C	TRACOSA	D
THERIOT	D	TILFORD	B	TDCCA	B	TONOVEK	B	TRACY	B
THERMO	D	TILLEDA	B	TDCCA	B	TONOVA	B/D	TRAEEDOLLAR	B
TIERRA	D	TILLICUM	B	TDCCA	B	TONSINA	E	TRAEER	B/D
THERMOPOLIS	D	TILLMAN	C	TDCCA	B	TONSINA	E	TRAG	B
THESE	B	TILLMONT	B	TDCCA	B	TONUCCO	D	TRAG, COOL	C
THEYFORD	A	TILLOU	C	TDCCA	B	TOOLE	D	TRAHAM	C
THEYIS	B	TILMA	C	TDCCA	B	TOOLESBORD	B	TRAIL	A
THIEPRIVER	B/D	TILSIT	C	TDCCA	B	TOONES	D	TRAILAMP	D
THIEL	B	TILTUN	E	TDCCA	B	TONSIF	C	TRAILCREEK	C
THIESSEN	C	TIMBALIER	D	TDCCA	B	TOONE, LOAMY SUBSTRATUM, STONY	B	TRAILHEAD	B
THIKE	D	TIMBERC	R	TDCCA	B	TOP	C	TRAINER	B
THIKOL	B	TIMBERHEAD	C	TDCCA	B	TOP	C	TRAITORS	D
THIRST	D	TIMBERLY	B	TDCCA	B	TOP	C	TRAMPAS	C
THISTLEBURN	B	TIMBERVILLE	B	TDCCA	B	TOP	C	TRANNAY	B
THISTLEDEW	B	TIMBLIN	D	TDCCA	B	TOP	C	TRANQUILAR	C
THONY	D	TIMBUCTOO	C	TDCCA	B	TOP	C	TRANSYLVANIA	B
THONAS	B/D	TIMENTWA	B	TDCCA	B	TOP	C	TRAPPER	B
THONHILL	B	TIMHILL	B	TDCCA	B	TOP	C	TRAPPIST	C
THONS	D	TIMNUS	B	TDCCA	B	TOP	C	TRAPPS	B
THORNBOURGH	B	TIMKEN	C	TDCCA	B	TOP	C	TRASK	C
THORNDALE	D	TIMMERMAN	B	TDCCA	B	TOP	C	TRAVELERS	D
THORNDIKE	C/D	TIMMONS	B	TDCCA	B	TOP	C	TRAYER	B
THORNDICK	D	TIMPANUTE	C	TDCCA	B	TOP	C	TRAVERTINE	C
THORNTON	D	TIMPANOGOS	B	TDCCA	B	TOP	C	TRAVESSILLA	D
THORNTONFARE	B	TIMPANOGOS, MODERATELY WET	C	TDCCA	B	TOP	C	TRAVIS	D
THOUT	C	TIN	B	TDCCA	B	TOP	C	TRAYSON	C
THOM	B	TINPEP	D	TDCCA	B	TOP	C	TRAWICK	B
THOMSON	B	TINPULA	B	TDCCA	B	TOP	C	TRAY	C
THRASH	B	TINA	C	TDCCA	B	TOP	C	TREADWAY	D
THREDCGILL	B	TINAJA	C	TDCCA	B	TOP	C	TREATY	B/D
THREDCNDP	B	TINAMOU	C	TDCCA	B	TOP	C	TREBLO	B
THREDCOT	D	TINDAHAY	A	TDCCA	B	TOP	C	TREBLOC	B
THREEK	C	TINDAHAY, GRAVELLY	A	TDCCA	B	TOP	C	TREBOR	C
THREEFILE	B	TINE	B	TDCCA	B	TOP	C	TREKOR	D
THREETOP	C	TINEMAN	A	TDCCA	B	TOP	C	TREKOR, NONSTONY	C
THROCK	C	TINEMAN, WET	C	TDCCA	B	TOP	C	TREEN	D
THULSPAN	C	TINGEY	B	TDCCA	B	TOP	C	TREGO	C
THURZLAND	B	TINKER	C	TDCCA	B	TOP	C	TREHARNE	C
THURBERBIRD	D	TINM	D	TDCCA	B	TOP	C	TRELK	B
THURBER	D	TINMIN	A	TDCCA	B	TOP	C	TRELONA	B
	D	TINSLEY	A	TDCCA	B	TOP	C	TRENT	D
	D		A	TDCCA	B	TOP	C	TRENT	D

NOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS B/C INDICATES THE DRAINED/UNDRAINED SITUATION.
MODIFIERS SHOWN, E.G., BEDROCK SUBSTRATUM, REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.

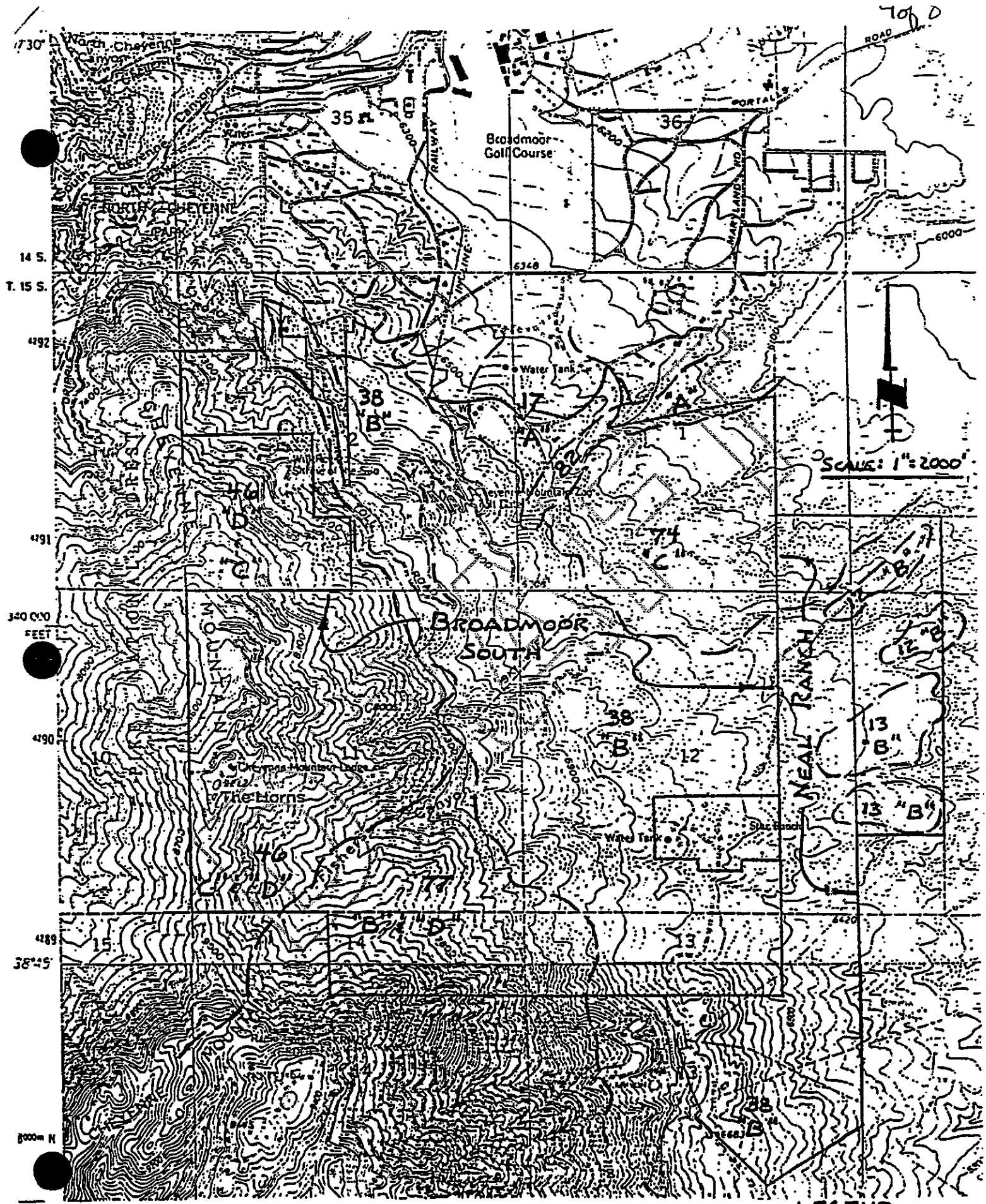
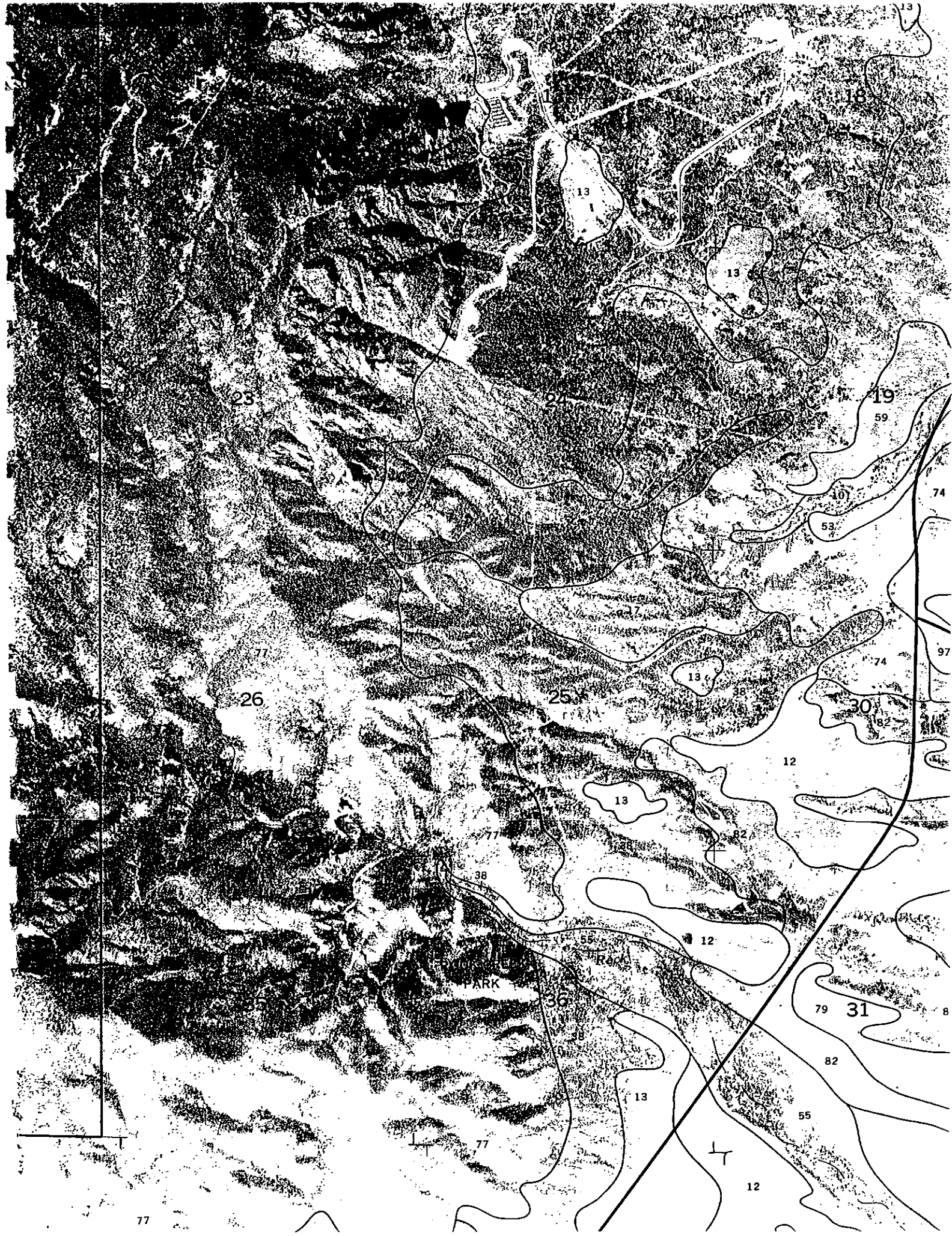


FIGURE II - SOILS CLASSIFICATION MAP

LEGEND
 SCS MAP SYMBOL - 38
 HYDROLOGIC CLASS - "B"
 ASSUMED





Subject BRIDGES BROADMOOR - CROSS SECTIONS
LOCAL ROUTING

Project No. 24506

By SGS

Checked By SWR

Task No. 001

Date 12/1/97

Date 12/27/97

File No. _____

Sheet 1/2

OBJECTIVE: DEVELOP CROSS SECTION FOR ROUTED BASINS
GIVEN: SITE TOPS (1:2400)

A-A

LOB n	-	0.12
ROB n	-	0.12
CHANNEL n	-	0.08
EL ₀	-	7100'
EL ₁	-	6760'
LENN	-	1600'
S ₅	-	0.21



ROUTED 1B & 1C TANK 1D

B-B

LOB n	-	0.12
ROB n	-	0.12
CHANNEL n	-	0.08
EL ₀	-	6900'
EL ₁	-	6800'
LENN	-	700
S ₃	-	0.14



ROUTED 1A TANK 1D

C-C

LOB n	-	0.12
ROB n	-	0.12
CHANNEL n	-	0.08
EL ₀	-	6790'
EL ₁	-	6480'
LENN	-	4000'
S ₇	-	0.08



ROUTED 1A-1D TANK A

Subject Boulder's Brighton - CROSS SECTIONS
HCC-1 ROUTING

Project No. 24506

By Srs

Checked By

Task No. 201

Date 12/2/97

Date

File No.

Sheet 2/2

D-D

LOS π - 0.12
ROB π - 0.12
CHANNEL π - 0.08

EL₀ - 6760'
EL₁ - 6330'
LEAD - 6400'
SRS - 0.07



ROUTED 2 TRAW C

E-E

LOS π - 0.12
ROB π - 0.12
CHANNEL π - 0.08

EL₀ - 6690'
EL₁ - 6500'
LEAD - 7200'
SRS - 0.03



ROUTED 3 TRAW A

F-F

LOS π - 0.12
ROB π - 0.12
CHANNEL π - 0.08

EL₀ - 6650'
EL₁ - 6500'
LEAD - 2800'
SRS - 0.05



ROUTED 4 TRAW A

Subject Boulders Broadman

Project No. 24506

By SLD

Checked By SWR

Task No. 601

Date 11/18/97

Date 12/28/97

File No. _____

Sheet 1/2

OBJECTIVE:

ESTIMATE CAPACITIES OF EXISTING AND PROPOSED CONDUITS
WITHIN BOULDERS BROADMAN COMMUNITY MAP AREA

GIVEN:

REVIEW OF EXISTING 1:2400 TOPD & REPORTS

SOLUTION:

FOR PURPOSES OF THIS ANALYSIS, AND GIVEN LOCATIONS
OF CONDUITS, IT WAS ASSUMED THAT CONDUITS ARE
INLET CONTROLLED (WHOLE CONDUITS HAVE SLOPE > 1%)

GOVERNING DISCHARGE EQUATION:

$$Q = C_d A \sqrt{2gh}$$

- w/
- Q = DISCHARGE (CFS)
 - C_d = COEF. OF DISCHARGE
 - A = FLOW AREA (FT²)
 - g = 32.2 F/S²
 - h = HEAD @ INLET (FT)

OBJECTIVE:

ESTIMATE CURVE NUMBERS FOR IDENTIFIED BASINS

GIVEN:

SITE MAP w/ BASIN CHARACTERISTICS

SOLUTIONS:

<u>BASIN</u> <u>ID</u>	<u>(mi²)</u> <u>BASIN</u> <u>AREA</u>	<u>Soil</u> <u>Group</u>	<u>Curve</u> <u>Number</u>	
1A	0.023	B ↓	78	} ABOVE WOOD PLOTS
1B	0.231		78	
1C	0.037		78	
1D	0.050		78	
2	0.043		78	
3	0.687	B ↓	78	} BOULDER'S BROADMOOR COMM'Y (UNDEVELOPED)
4	0.090		78	
A	0.091		58	
B	0.018		58	
C	0.088	B ↓	58	} BOULDER'S BROADMOOR COMM'Y (DEVELOPED)
D	0.023		58	
E	0.010		58	
F	0.009		58	
G	0.034		58	
H	0.030		58	
A	"	"	72	} BOULDER'S BROADMOOR COMM'Y (DEVELOPED)
B	"		72	
C	"		72	
D	"		72	
E	"		72	
F	"		72	
G	"		72	
H	"		72	

Table 2-2d.—Runoff curve numbers for arid and semiarid rangelands¹

Cover description		Curve numbers for hydrologic soil group—			
Cover type	Hydrologic condition ²				
		A ³	B	C	D
Herbaceous—mixture of grass, weeds, and low-growing brush, with brush the minor element.	Poor		80	87	93
	Fair		71	81	89
	Good		62	74	85
Oak-aspen—mountain brush mixture of oak brush, aspen, mountain mahogany, bitter brush, maple, and other brush.	Poor		66	74	79
	Fair		48	57	63
	Good		30	41	48
Pinyon-juniper—pinyon, juniper, or both; grass understory.	Poor		75	85	89
	Fair		58	73	80
	Good		41	61	71
Sagebrush with grass understory.	Poor		67	80	85
	Fair		51	63	70
	Good		35	47	55
Desert shrub—major plants include saltbush, greasewood, creosotebush, blackbrush, bursage, palo verde, mesquite, and cactus.	Poor	63	77	85	88
	Fair	55	72	81	86
	Good	49	68	79	84

¹Average runoff condition, and $I_a = 0.2S$. For range in humid regions, use table 2-2c.

²*Poor*: <30% ground cover (litter, grass, and brush overstory).

Fair: 30 to 70% ground cover.

Good: >70% ground cover.

³Curve numbers for group A have been developed only for desert shrub.

TABLE 8.--BUILDING SITE DEVELOPMENT--Continued

Soil name and map symbol	Shallow excavations	Dwellings without basements	Dwellings with basements	Small commercial buildings	Local roads and streets
Elbeth: 127: Pring part-----	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.
Ellicott: 28-----	Severe: cutbanks cave, floods.	Severe: floods.	Severe: floods.	Severe: floods.	Severe: floods.
Fluvaquentic Haplaquolls: 29-----	Severe: wetness, floods.	Severe: wetness, floods.	Severe: wetness, floods.	Severe: wetness, floods.	Severe: wetness, floods, frost action.
Fort Collins: 30-----	Slight-----	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell, frost action.
31-----	Slight-----	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell.	Moderate: slope, low strength, shrink-swell.	Moderate: low strength, shrink-swell, frost action.
Fortwingate: 132: Fortwingate part-----	Severe: depth to rock, slope.	Severe: shrink-swell, slope.	Severe: depth to rock, shrink-swell, slope.	Severe: shrink-swell, slope.	Severe: shrink-swell, slope, low strength.
Rock outcrop part.					
Heldt: 33-----	Severe: too clayey.	Severe: shrink-swell, low strength.	Severe: shrink-swell, low strength.	Severe: shrink-swell, low strength.	Severe: shrink-swell, low strength.
Holderness: 34, 35-----	Moderate: too clayey.	Severe: shrink-swell.	Moderate: shrink-swell, low strength.	Severe: shrink-swell.	Severe: shrink-swell, low strength.
36-----	Moderate: too clayey, slope.	Severe: shrink-swell.	Moderate: slope, shrink-swell, low strength.	Severe: shrink-swell, slope.	Severe: shrink-swell, low strength.
Jarre: 37-----	Slight-----	Moderate: shrink-swell, low strength.	Slight-----	Moderate: slope, shrink-swell, low strength.	Moderate: frost action, shrink-swell, low strength.
138: Jarre part-----	Moderate: slope.	Moderate: slope, shrink-swell, low strength.	Moderate: slope.	Severe: slope.	Moderate: slope, shrink-swell, low strength.
Tecolote part--	Severe: slope, small stones.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.

See footnote at end of table.

TABLE 8.--BUILDING SITE DEVELOPMENT--Continued

Soil name and map symbol	Shallow excavations	Dwellings without basements	Dwellings with basements	Small commercial buildings	Local roads and streets
Razor: 74-----	Moderate: too clayey, large stones.	Severe: shrink-swell.	Severe: shrink-swell.	Severe: shrink-swell, slope.	Severe: shrink-swell, low strength.
175: Razor part-----	Moderate: depth to rock, too clayey.	Severe: shrink-swell, low strength.	Severe: shrink-swell, low strength.	Severe: slope, shrink-swell, low strength.	Severe: shrink-swell, low strength.
Midway part-----	Severe: too clayey, depth to rock.	Severe: shrink-swell, low strength.	Severe: shrink-swell, low strength, depth to rock.	Severe: slope, shrink-swell, low strength.	Severe: shrink-swell, low strength.
Rizoza: 176: Rizoza part-----	Severe: depth to rock, slope.	Severe: depth to rock, slope.	Severe: depth to rock, slope.	Severe: depth to rock, slope.	Severe: slope, depth to rock.
Neville part-----	Moderate: slope.	Moderate: slope, low strength, shrink-swell.	Moderate: slope, low strength, shrink-swell.	Severe: slope.	Severe: frost action.
Rock outcrop: 177: Rock outcrop part.					
Coldcreek part-----	Severe: slope, small stones, cutbanks cave.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.
Tolman part-----	Severe: slope, depth to rock.	Severe: slope, depth to rock.	Severe: slope, depth to rock.	Severe: slope, depth to rock.	Severe: slope, depth to rock.
Sampson: 78-----	Slight-----	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell.	Moderate: low strength, frost action, shrink-swell.
Satanta: 79, 80-----	Slight-----	Moderate: shrink-swell, low strength.	Moderate: shrink-swell, low strength.	Moderate: shrink-swell, low strength.	Moderate: shrink-swell, low strength.
181: Satanta part-----	Slight-----	Moderate: shrink-swell, low strength.	Moderate: shrink-swell, low strength.	Moderate: shrink-swell, low strength.	Moderate: shrink-swell, low strength.
Neville part-----	Slight-----	Moderate: low strength, shrink-swell.	Moderate: low strength, shrink-swell.	Moderate: slope, low strength, shrink-swell.	Severe: frost action.
Schamber: 182: Schamber part-----	Severe: slope, cutbanks cave.	Severe: slope.	Severe: slope.	Severe: slope.	Severe: slope.
Razor part-----	Moderate: depth to rock, too clayey.	Severe: shrink-swell, low strength.	Severe: shrink-swell, low strength.	Severe: slope, shrink-swell, low strength.	Severe: shrink-swell, low strength.

See footnote at end of table.

TABLE 11.--WATER MANAGEMENT

"Seepage," and some of the other terms that describe restrictive soil features are defined in the Glossary. Absence of an entry means soil was not evaluated]

Soil name and map symbol	Pond reservoir areas	Embankments, dikes, and levees	Drainage	Irrigation	Terraces and diversions	Grassed waterways
lamosa: 1-----	Seepage, excess humus.	Low strength, compressible, piping.	Slope, floods, poor outlets.	Slope, wetness, floods.	Wetness, piping.	Wetness.
scalon: 2-----	Seepage, slope.	Piping, low strength.	Slope-----	Slope, erodes easily.	Erodes easily, piping.	Erodes easily.
3-----	Seepage, slope.	Piping, low strength.	Slope-----	Slope, erodes easily.	Erodes easily, piping, slope.	Erodes easily, slope.
Badland: 4.						
ajjou: 5, 6, 7-----	Seepage, slope.	Piping-----	Slope-----	Slope, droughty, erodes easily.	Erodes easily, piping.	Erodes easily, droughty.
lakeland: 8-----	Seepage, slope.	Piping-----	Slope-----	Erodes easily, slope, droughty.	Erodes easily, piping.	Slope, erodes easily, droughty.
19: Blakeland part-	Seepage, slope.	Piping-----	Slope-----	Erodes easily, slope, droughty.	Erodes easily, piping.	Slope, erodes easily, droughty.
Fluvaquentic Haplaquolls part-----	---	---	Wetness-----	Wetness-----	Wetness-----	Wetness.
lendon: 10-----	Seepage-----	Seepage, piping.	Not needed-----	Seepage-----	Not needed-----	Favorable.
resser: 11-----	Seepage-----	Piping-----	Slope-----	Slope, erodes easily.	Erodes easily, piping.	Erodes easily.
12-----	Seepage, slope.	Piping-----	Slope-----	Slope, erodes easily.	Erodes easily, piping.	Slope, erodes easily.
13-----	Seepage, slope.	Piping-----	Slope-----	Slope, erodes easily.	Slope, erodes easily, piping.	Slope, erodes easily.
russett: 14-----	Slope, seepage.	Low strength, hard to pack, piping.	Slope-----	Slope-----	Piping-----	Favorable.
15-----	Slope, seepage.	Low strength, hard to pack, piping.	Slope-----	Slope-----	Piping-----	Slope.
haseville: 16-----	Seepage, slope.	Erodes easily, piping, seepage.	Cutbanks cave	Slope, droughty.	Erodes easily, piping.	Slope, erodes easily, droughty.
17-----	Seepage, slope.	Erodes easily, piping, seepage.	Slope, outbanks cave.	Slope, droughty.	Slope, erodes easily, piping.	Slope, erodes easily, droughty.

See footnote at end of table.

Exhibit A-1, continued: Hydrologic soil groups for United States soils

BOLFA	C	BORGEAU	B	BRACEVILLE	C	BREY	C	BROKENHORN	D
BOLICKER	B	BORGES	D	BRACKEN	B	BREYER	C	BROLLIAR	D
BOLINA	D	BORIANA	D	BRACKETT	C	BREWLESS	C	BROMER	C
BOLIVAR	B	BORKY	C	BRAD	D	BREWSTER	D	BROWDE	B
BOLLING	C	BORNSTEDT	B	BRADDOCK	C	BREWTON	C	BROWDE	B
BOLINA	B	BORO	D	BRADEN	B	BRIBUTTE	D	BROWNAUGH	B
BOLIVAR	C	BOROEY	C	BRADENTON	B/D	BRICKEL	C	BRONCHO	B
BOLIVAR	D	BORREGO	D	BRADENTON, FLOODED	D	BRICKMILL	C	BRONCHO, LOAMY	A
BOLIVAR	C	BORREGUERO	C	BRADER	C	BRICKTON	C	SUBSTRATUM	
BOLIVAR	D	BORSKI	B	BRADSHAW	B	BRICO	C	BROWNELL	B
BOLIVAR	B	BORTH	C	BRADSON	B	BRIDGE	C	BROWNSON	B
BOLIVAR	C	BORUP	B/D	BRADWAY	D	BRIDGE	C	BRONTE	C
BOLIVAR	B	BORVANT	D	BRADY	R	BRIDGEHAMPTON	C	BROOKE	D
BOLIVAR	D	BOSANKO	D	BRADYVILLE	C	BRIDGEPORT	B	BROOKFIELD	B
BOLIVAR	B	BOSCO	B	BRAFFITS	B	BRIDGER	D	BROOKINGS	B
BOLIVAR	A	BOSKET	B	BRAGG	C	BRIDGESON	D	BROOKLYN	C/D
BOLIVAR	B	BOSLER	B	BRAMAH	B	BRIDGESON, DRAINED	C	BROOKMAN	D
BOLIVAR	D	BOSO	D	BRAILSFORD	C	BRIDGET	E	BROOKSHIRE	C
BOLIVAR	D	BOSQUE	D	BRAINERD	C	BRIDGEWATER	B	BROOKSIDE	C
BOLIVAR	D	BOSSBURG	B	BRAILLER	D	BRIEDWELL	B	BROOKSTON	B/D
BOLIVAR	D	BOSSBURG, DRAINED	C	BRAM	C	BRIF	B	BROOKSTON, STONY	D
BOLIVAR	C	BOSTON	C	BRAMARD	E	BRIFER	D	BROOKSVILLE	D
BOLIVAR	D	BOSTRUM	D	BRAMLETT	C	BRIGGS	A	BROOME	B
BOLIVAR	B	BOSTWICK	B	BRAMWELL	C	BRIGGSDALE	C	BROPHY	A/D
BOLIVAR	C	BOSVILLE	C	BRANCH	B	BRIGGSVILLE	C	BROSE	D
BOLIVAR	B	BOSWELL	D	BRANCOFT	C	BRIGHTON	E/D	BROSELEY	B
BOLIVAR	C	BOSWORTH	D	BRANO	D	BRIGHTWOOD	B	BROSS	B
BOLIVAR	A	BOTELLA	B	BRANDENBURG	A	BRILEY	B	BROUGHTON	D
BOLIVAR	C	BOTHWELL	B	BRANDON	B	BRILL	B	BROVARD	C
BOLIVAR	A	BOTHWI	C	BRANGYVINE	C	BRILLIANT	B	BROWER	B
BOLIVAR	B	BOTON	B	BRANFORD	B	BRIMFIELD	C/D	BROWNBEAR	C
BOLIVAR	D	BOUQUIN	C	BRANHAM	C	BRIMLEY	E	BROWNDELL	D
BOLIVAR	D	BOUQUIN	C	BRANSCOMB	B	BRINSTONE	D	BROWNELL	B
BOLIVAR	D	BOULDER	B	BRANTFORD	B	BRINGAR	E	BROWNFIELD	A
BOLIVAR	A	BOULDER LAKE	D	BRANTLEY	C	BRINGEE	B	BROWNLEE	B
BOLIVAR	C	BOULDER LAKE	D	BRANTON	D	BRINKER	C	BROWNRIFF	D
BOLIVAR	B	BOULDER POINT	E	BRASHEAR	C	BRINKERT	C	BROWNSCREEK	C
BOLIVAR	B	BOULDERCREEK	B	BRASSFIELD	B	BRINKERTON	D	BROWNSCREEK	B
BOLIVAR	B	BOULDIN	B	BRATTON	B	BRINNUM	D	BROWNSDALE	C
BOLIVAR	A	BOULFLAT	C	BRAY	C	BRINNUM, DRAINED	C	BROWNSTO	B
BOLIVAR	A	BOUNCER	D	BRAYNE	D	BRIONES	B	BROWNSVILLE	C
BOLIVAR	C/D	BOUNDARY	B	BRAXLEY	D	BRIOS	A	BROWNTON	C/C
BOLIVAR	C	BOURBON	B	BRAXTON	C	BRISBANE	B	BRUKOM	B
BOLIVAR	D	BOURNE	C	BRAY	D	BRISCO	B	BRUKOM	B
BOLIVAR	D	BOUSIC	D	BRAYTON	C	BRISCOT	D	BRUKOM	B
BOLIVAR	D	BOV	D	BRAZILTON	D	BRISCOY, DRAINED	C	BRUCE	B/D
BOLIVAR	B	BOVBAC	C	BRAZITO, THICK	B	BRISKY	D	BRUELLA	B
BOLIVAR	C	BOVBELLS	B	BRAZITO, THICK	B	BRISTON	D	BRUELLA, HARD	C
BOLIVAR	C	BOVDISH	C	SURFACE		BRITTO	D	SUBSTRATUM	
BOLIVAR	D	BOVOLE	B	BRAZITO, THICK	C	BRITTON	D	BRUFFY	B
BOLIVAR	C	BOVDON	D	SURFACE		BRITNATER	B	BRUMEL	B
BOLIVAR	C	BOVDRE	C	SALINE-ALKALI		BRDAD	C	BRUMIN	B
BOLIVAR	D	BOVEN	C	BRAZON	C	BROAD CANYON	B	BRUMIN	B
BOLIVAR	B	BOVENS	C	BRAZORIA	D	BROADALBIN	C	BRUMBAUGH	C
BOLIVAR	D	BOVES	B	BRECKENRIDGE	B/D	BROADAX	E	BRUNCAN	D
BOLIVAR	C	BOVIE	B	BRECKNOCK	E	BROADBROOK	C	BRUNDAGE	D
BOLIVAR	B	BOVLAKE	C	BRECKSVILLE	C	BROADHEAD	C	BRUNEEL	D
BOLIVAR	B	BOVLUS	E	BREECE	B	BROADHURST	D	BRUNELDA	D
BOLIVAR	D	BOVMAN	C	BREGAR	D	BROADHOOR	C	BRUND	A
BOLIVAR	D	BOVMANSVILLE	B/D	BREIEN	E	BROADUS	B	BRUNSVICK	B
BOLIVAR	A	BOVNS	C	BREK	B	BROADWELL	B	BRUNZELL	B
BOLIVAR	B	BOVSTRING	A/D	BREMER	C	BROBETT	C	BRUSHCREEK	C
BOLIVAR	B	BOVSTRO	C	BREMER, SANDY	E	BROCK	D	BRUSHCREEK	B
BOLIVAR	C	BOVLOER	C	SUBSTRATUM		BROCKEY	C	BRUSSELS	C
BOLIVAR	C	BOVFORD	C	BREMO	C	BROCKEY	C	BRUSSETT	B
BOLIVAR	C	BOVVILLE	C	BREMS	A	BROCKGULCH	C	BRUSSETT	B
BOLIVAR	D	BOVWELL	D	BRENDA	B	BROCKLISS	B	BRYAN	A
BOLIVAR	C	BOV	B	BRENDA	C	BROCKMAN	C	BRYAN	B
BOLIVAR	C	BOVCE	D	BRENNAM	C	BROCKO	B	BRYARLY	D
BOLIVAR	D	BOVD	D	BRENNAM	B	BROCKPORT	D	BRYCAN	B
BOLIVAR	D	BOVY	B	BRENNER	D	BROCKROAD	C	BRYCE	D
BOLIVAR	A/D	BOVET	B	BRENT	D	BROCKSBURG	B	BRYMAN	B
BOLIVAR	C	BOVETT	B	BRENTON	E	BROCKTON	D	BRYSTAL	B
BOLIVAR	C	BOVKIN	D	BRENTSVILLE	C	BROCKWAY	C	BRYSTAL	B
BOLIVAR	C	BOVLE	D	BRENTWOOD	B	BROCKWELL	B	SUB	C
BOLIVAR	D	BOVSAG	D	BRESSA	B	BROCKWELL	B	SUB	C
BOLIVAR	D	BOVSEN	B	BRESSER	B	BRODALE	C	SUB	C
BOLIVAR	B	BOZE	D	BREVAR	B	BRODY	C	SUB	C
BOLIVAR	B	BOZEMAN	B	BREVAR	B	BROE	B	SUB	C
BOLIVAR	B	BORDER	D	BREVATOR	C	BROGAN	B	SUB	C
BOLIVAR	D	BRACE	C	BREVORT	C	BROGDON	B/D	SUB	C

NOTES: TWO HYDROLOGIC SOIL GROUPS SUCH AS B/C INDICATES THE DRAINED/UNDRAINED SITUATION. MODIFIERS SHOWN. E.G., BEDROCK SUBSTRATUM. REFER TO A SPECIFIC SOIL SERIES PHASE FOUND IN SOIL MAP LEGEND.