

DRAINAGE REPORT

Planned Centennial Boulevard  
Between the North Terminus of  
Existing Improved Centennial Boulevard  
and Vindicator Drive

City of Colorado Springs  
El Paso County, Colorado

June 3, 1983  
Revised June 30, 1983  
Revised July 27, 1983

RETURN TO:  
Land Development  
101 West Costilla, Suite 122  
Colorado Springs, CO 80903

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Prepared for:

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Existing Improved Centennial Boulevard  
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City of Colorado Springs  
El Paso County, Colorado  
June 3, 1983  
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Engineer's Statement:

"The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by the negligent acts, errors, or omissions on my part in preparing this report."

Seal

Stephen C. Behrens  
Signature and P.E. # 11447

Developer's Statement:

"The Developer and/or his representative has read and will comply with all the requirements specified in this drainage report and plan as filed by the City Engineer."



Ronald W. [Signature]  
Signature

Filed in accordance with Section  
15-3-906 of the Code of the City  
of Colorado Springs, 1980, as  
amended.

Arcy R. Hayes 10-6-83  
City Engineer Date

- subject to
- (1) Detailed design plans for street and drainage facilities
  - (2) Geometrics of street not ~~included~~ included
  - (3) Excludes comment 5 on page 11

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## DRAINAGE REPORT

Planned Centennial Boulevard Between the North Terminus of Existing Improved Centennial Boulevard and Vindicator Drive City of Colorado Springs, El Paso County, Colorado

### INTRODUCTION

This Drainage Report (1) summarizes anticipated future fully developed drainage conditions affecting planned Centennial Boulevard as identified and reported by others (References 1 and 2) and in addition (2) presents recommended conceptual design drainage facilities along planned Centennial Boulevard.

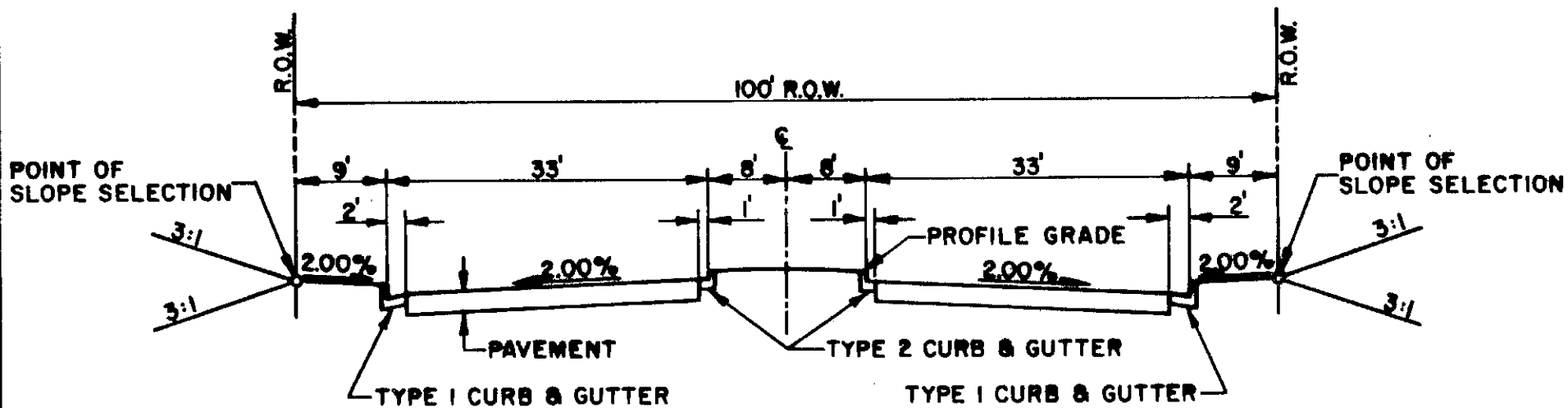
### SITE LOCATION AND DESCRIPTION

Centennial Boulevard is a planned four lane north-south major arterial roadway located in the northwest quadrant of the City of Colorado Springs, El Paso County, Colorado. A typical section of planned Centennial Boulevard is shown on Figure 1. The segment of Centennial Boulevard under investigation coincides with Wilson Road, an existing two lane rural (ditched) roadway between the north terminus of existing improved Centennial Boulevard on the south to Vindicator Drive on the north, for a distance of approximately 7,500 lineal feet as shown on Figure 2 and approximately described as follows:

From a point within the northwest quarter of Section 23, Township 13 south, range 67 west of the 6th Principal Meridian located approximately 1100 feet south and 1400 feet east of the northwest corner of Section 23 to a point within the southwest quarter of Section 11, Township 13 south, range 67 west of the 6th Principal Meridian located approximately 1,100 feet north and 1,350 feet south of the southwest corner of Section 11.

Planned Centennial Boulevard is considered a major arterial street. Minor arterial streets include Vindicator Drive, Flying "W" Ranch Road, and Wilson Road. Gravel Pit Road is a planned local street. Other planned street intersections include north commercial access, north and south Reed Ranch Access Roads, a Research and Development access north of Atherton Way, Atherton Way, Mule Deer Drive, a Research and Development access south of Mule Deer Drive and access to the Office Land Use at the approximate north terminus of existing improved Centennial Boulevard.

Existing and planned developments adjacent to this segment of Centennial Boulevard include Mountain Shadows to the west and Pinon Valley to the east.



**TYPICAL SECTION**  
**PLANNED CENTENNIAL BOULEVARD**

NO SCALE  
 FIGURE 1

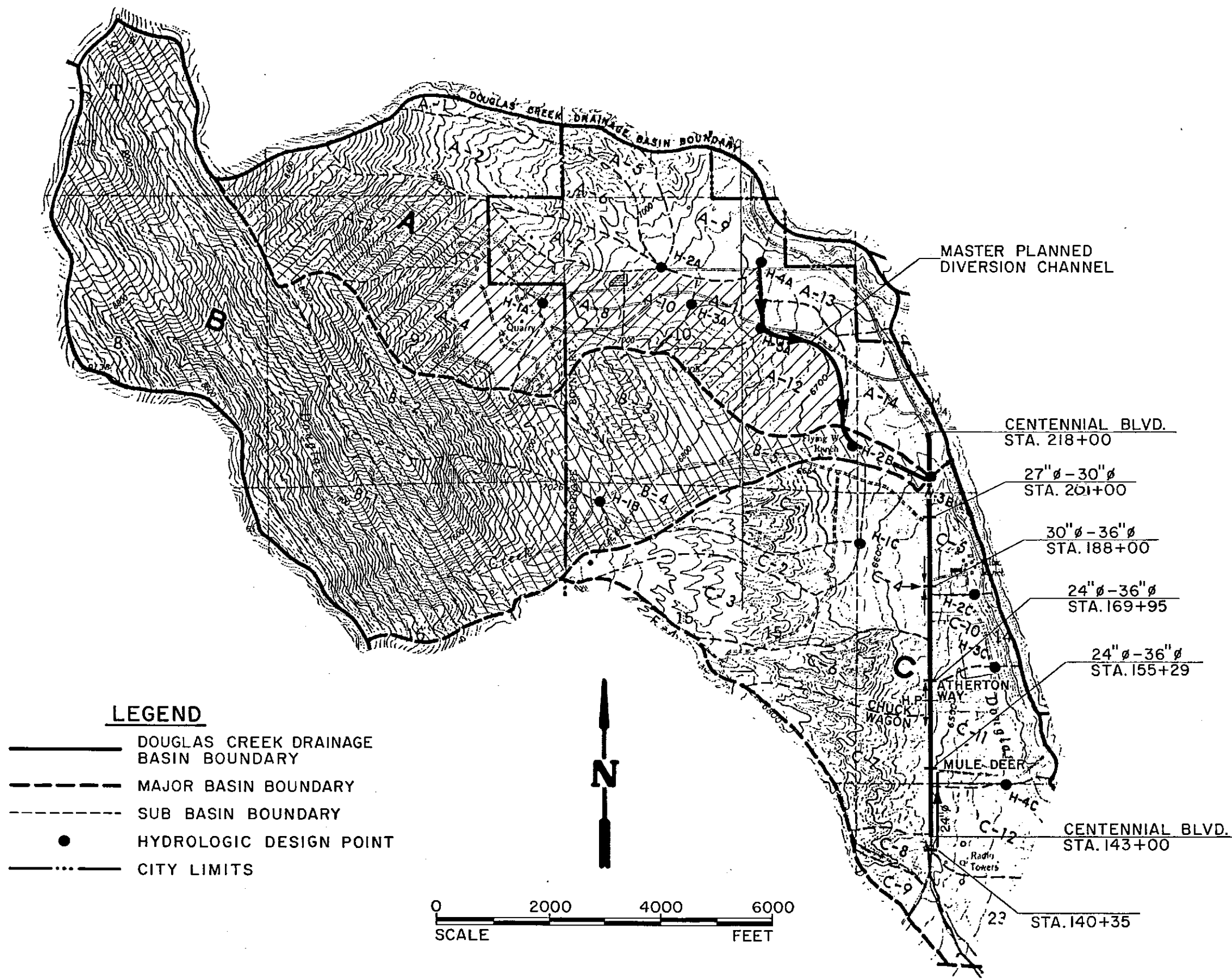
2.

DOUGLAS CREEK  
DRAINAGE BASIN  
CITY OF COLORADO SPRINGS  
EL PASO COUNTY, COLORADO

CENTENNIAL BOULEVARD  
PRELIMINARY DESIGN  
URS/NE S PROJECT NO. 3073

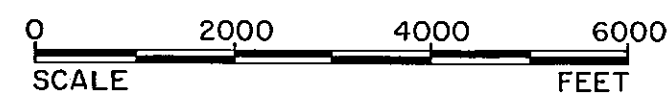
JUNE 3, 1983

FIGURE 2



**LEGEND**

- DOUGLAS CREEK DRAINAGE BASIN BOUNDARY
- - - - MAJOR BASIN BOUNDARY
- - - - SUB BASIN BOUNDARY
- HYDROLOGIC DESIGN POINT
- ..... CITY LIMITS

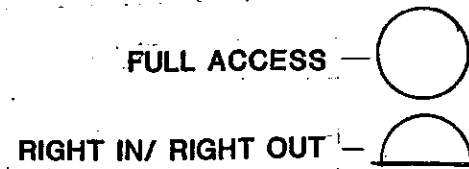


# CENTENNIAL BOULEVARD

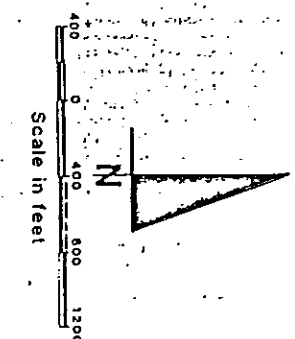
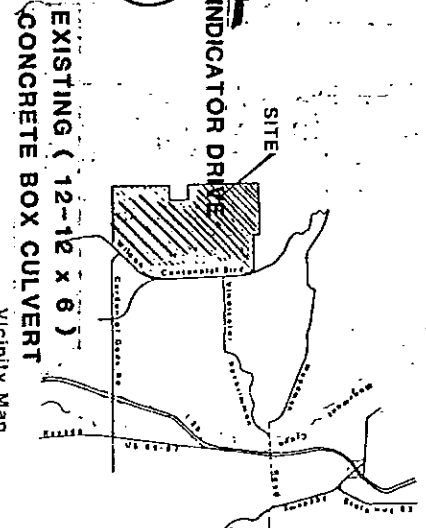
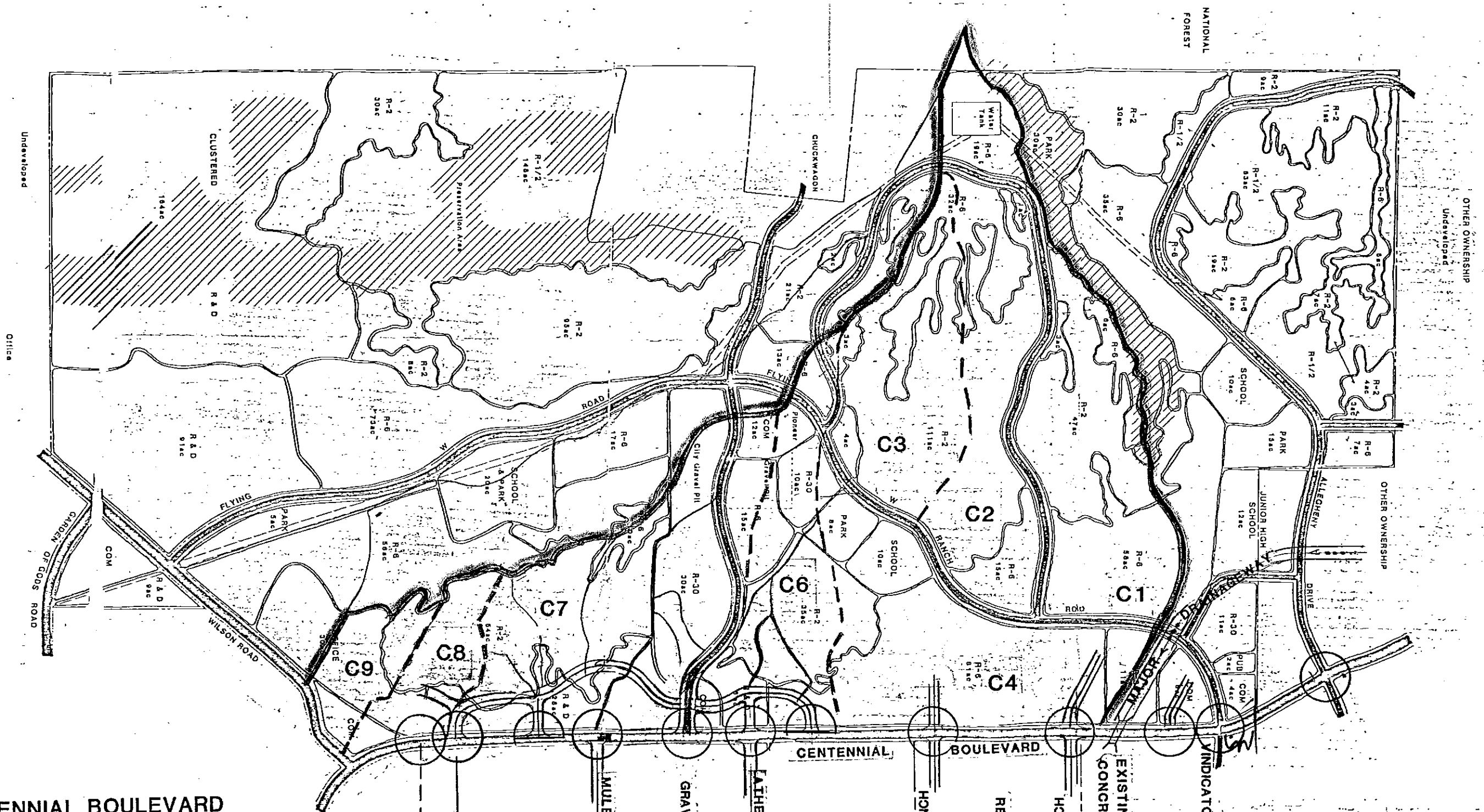
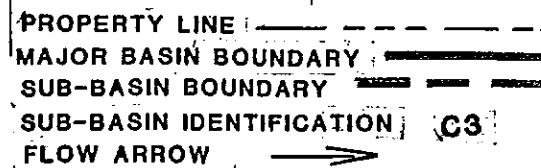
## DRAINAGE MASTERPLAN

CITY OF COLORADO SPRINGS, COLORADO

JUNE 30, 1983



### LEGEND





The main stem of the North Fork of Douglas Creek, the major drainageway in the vicinity of this segment of planned Centennial Boulevard, flows from west to east under planned Centennial Boulevard via an existing (12-12)X6 concrete box culvert (CBC) and then flows to the southeast on the east side of Centennial Boulevard.

#### DRAINAGE MASTER PLAN

Conceptual drainage improvements to accommodate expected peak storm runoff associated with future developed basin conditions have been masterplanned within the Douglas Creek Drainage Basin. (Reference 1).

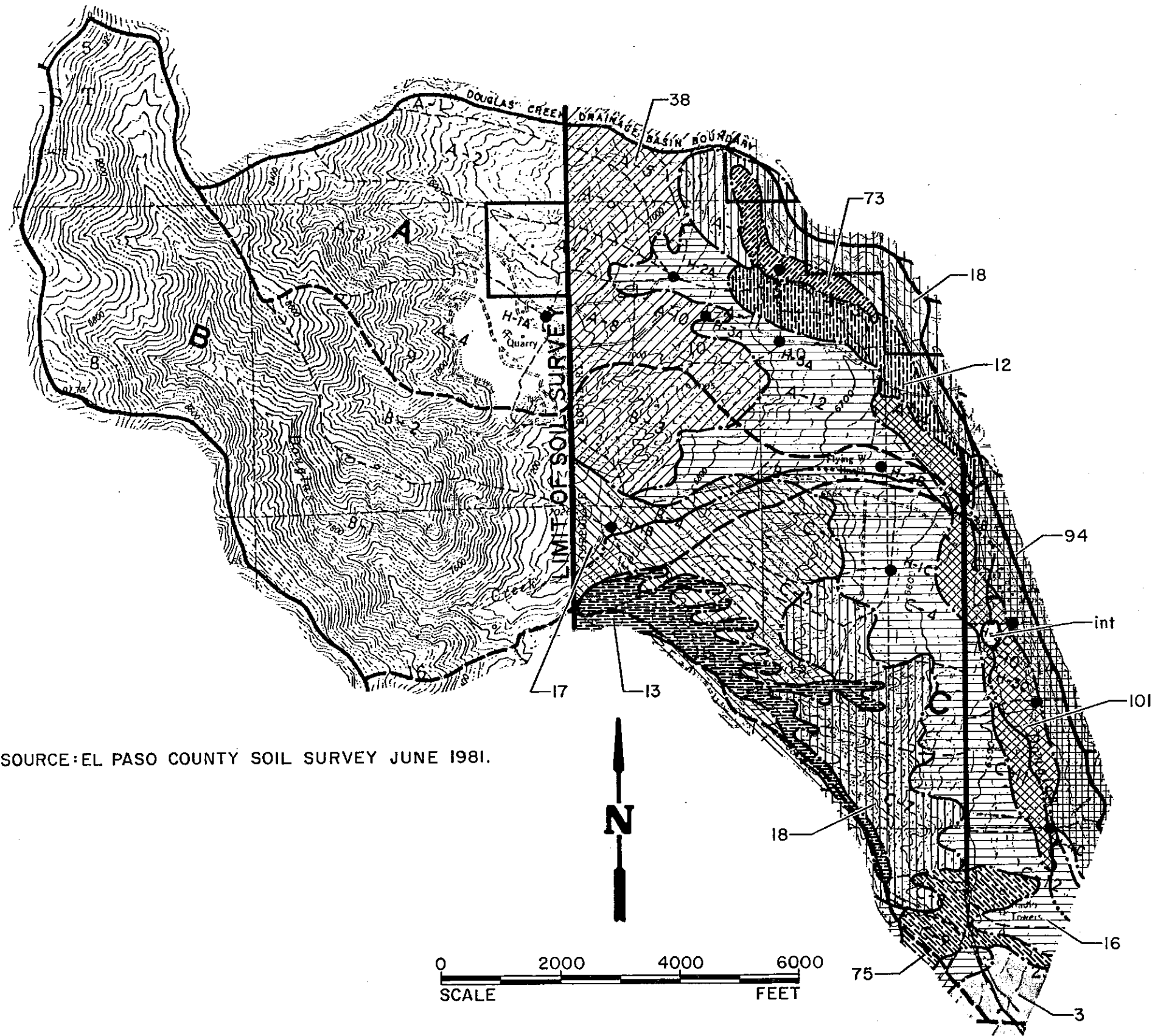
#### DRAINAGE BASINS

Drainage basin boundaries, sub-basins, and hydrologic design points identified in the Douglas Creek Drainage Basin Masterplanning Study of March 1981 (Reference 1) are shown in Figure 2.

#### SOILS

Soils in the vicinity of Centennial Boulevard consist of loamy ustic torrifluvents (#101), Chaseville Sandy Loam (#16), Chaseville Gravelly Sandy Loam (#17), Chaseville-Midway Complex (#18), all of which are classified as Type A hydrologic soils; Bresser Sandy Loam (#12), Bresser Sandy Loam (#13), both of which are classified as Type B hydrologic soils; and Razor Clay Loams (#73), and Razor Stoney Clay Loams (#74) both of which are classified as Type C hydrologic soils all as obtained from the El Paso County Colorado Soil Survey (Reference 4) and presented in Figure 3.

These soil types and hydrologic soil group classifications differ from preliminary soils information used in the Douglas Creek Drainage Basin Study (Reference 1) which were obtained prior to finalization of the El Paso County Soil Survey. Present soil and hydrologic soil group classifications in the vicinity of planned Centennial Boulevard appear to have somewhat less runoff potential than the preliminary soils classification used in the Douglas Creek Drainage Basin Masterplanning Study. Estimated storm runoff magnitudes presented in the Douglas Creek Drainage Basin Masterplanning Study are, however, believed to be acceptable for preliminary design in view of (1) the number of uncertainties associated with storm drainage including street layout and drainage facilities associated with future developments along planned Centennial Boulevard and (2) acceptance of the present Douglas Creek Drainage Basin Masterplanning Study by the City of Colorado Springs.



SOURCE: EL PASO COUNTY SOIL SURVEY JUNE 1981.

### SOIL SURVEY

SOURCE: EL PASO COUNTY SOIL SURVEY - JUNE 1981

SOIL ID #	NAME	HYDROLOGIC SOIL GROUP
12	BRESSER SANDY LOAM, 3-5%	B
13	BRESSER SANDY LOAM, 5-9%	B
16	CHASEVILLE SANDY LOAM, 1-8%	A
17	CHASEVILLE GRAVELLY SANDY LOAM, 8-40%	A
18	CHASEVILLE-MIDWAY COMPLEX	A
73	RAZOR CLAY LOAM	C
74	RAZOR STONEY CLAY LOAM	C
75	RAZOR MIDWAY COMPLEX	C
101	USTIC, TORRIFLUENTS, LOAMY	A
38	JARRE - TECOLOTE	B
3	ASCALON SANDY LOAM	B
94		

**CENTENNIAL BOULEVARD**

**PRELIMINARY DESIGN**

**URS/NE S PROJECT NO.3073**

**JUNE 3, 1983**

**FIGURE 3 (5)**

## DRAINAGE ASSUMPTIONS

Conceptual storm drainage improvements presented herein are based on the following assumptions:

1. Assume storm runoff from drainage masterplanning basin C-9 does not affect the segment of planned Centennial Boulevard presently under investigation due to 24 inch diameter storm sewer system in Centennial Boulevard and high point in Centennial Boulevard approximately 880 feet north of Wilson Road; all as shown on Centennial Boulevard plans Sta. 53+64.24 to Sta. 62+62.35 prepared by H. O. Kraettli and Sons and Centennial Boulevard plans Sta. 48+40.85 to Sta. 53+64.24 prepared by United Planning and Engineering.
2. Assume 5-year storm runoff generated from drainage masterplanning basin C-8 to be intercepted by existing 42 inch diameter cnp cross culvert at station 140+35+ and conveyed to planned 36 inch diameter Mule Deer Drive storm sewer system by 24 inch diameter rcp as indicated by Alternative B, Reference 5.
3. Assume that when drainage masterplanning basin C-8 is developed a storm sewer or open channel drainageway will be provided west of the existing cross culvert at station 140+35+ to intercept 5-year storm runoff.
4. Assume the planned Mule Deer Drive (Sta. 159+29+ east) storm sewer system would be extended to the west with a 24 inch diameter storm sewer stub-out and south to station 151+50 with a 36 inch diameter rcp storm sewer. Storm sewer inlets would be provided at the planned Right-in, Right-out R&D entrance at Station 149+20+ west. These inlets would be connected to the planned 36" diameter Mule Deer Drive storm sewer extension at Station 151+50+ by a 24 inch diameter rcp connector pipe. In addition, the planned 24 inch diameter rcp storm sewer associated with drainage masterplanning basin C-8 would be connected to the planned 36 inch diameter rcp Mule Deer Drive storm sewer at Sta. 151+50.
5. Assume Atherton Way masterplanned storm sewer will be extended to planned Gravel Pit Road as indicated in the "Douglas Creek Drainage Basin Masterplanning Study of March 1981."
6. The existing (12X12)X6 cbc cross culvert at Centennial Boulevard for the North Fork of Douglas Creek was not designed to accept storm runoff from drainage masterplanning basin C-1. Assume a high point would be created

in planned Flying W. Ranch Road south of the planned roadway crossing of the North Fork of Douglas Creek. This high point would prevent storm runoff associated with drainage masterplanning basin C-1 from entering the planned major channel on the west side of Centennial Boulevard.

7. Assume all storm runoff (5-year and 100-year) arriving at design points H-2A and H-4A (basins A-1 thru A-12) to be diverted to the planned major channel as implied in the Douglas Creek Drainage Basin Masterplanning Study dated March 1981.
8. Assume the difference between 100-year storm runoff from basin A-13 and masterplanned storm sewer capacity to carry over into Basin A-14 via Centennial Boulevard.
9. Assume all storm runoff (5-year and 100-year) from basins A-13 and A-14 to enter masterplanned channel at the existing (12-12)X6 concrete box culvert at the outlet of basin A-14 as implied in the Douglas Creek Drainage Basin Masterplanning Study dated March 1981.

#### DRAINAGE DESIGN CRITERIA

Storm drainage design criteria for planned Centennial Boulevard were obtained from (Reference 3), "Determination of Storm Runoff Criteria, City of Colorado Springs, Colorado, March 1977 as presently amended". A general drainage design criterion for this segment of planned Centennial Boulevard is that planned drainage facilities should accommodate phased buildout of future developments along the roadway.

##### A. DESIGN STORM(S)

City of Colorado Springs, Colorado drainage design criteria (Reference 3) presently specify the use of a 5-year 6-hour design storm of 2.1 inches for peak storm runoff up to 500 cubic feet per second; and the use of a 100-year, 6-hour design storm of 3.5 inches per hour for peak storm runoff greater than 500 cubic feet per second.

##### B. STORMWATER DETENTION

Present City of Colorado Springs, Colorado drainage criteria (Reference 3) do not require comparison of existing and anticipated future developed peak storm flows. Present City drainage criteria (Reference 3) generally favor upgrading existing outfall facilities to accept increased peak storm runoff associated with

development and discourage the construction of onsite storm water detention facilities without prior approval by the City Engineer.

The present City of Colorado Springs, Colorado drainage criteria (Reference 3) require a determination of appropriate peak discharges with routing and drainage facility design and in addition, require existing outfall facilities lacking the capacity to accept increased flow to be upgraded to accept such flows in proportion to the increased flow.

C. MAXIMUM PERMISSIBLE STREET FLOW CAPACITY

Maximum permissible street flow capacities obtained from Reference 3 are presented herein as Tables 1 and 2 .

STORM DRAINAGE METHODOLGY

City of Colorado Springs, Colorado storm drainage design criteria (Reference 3) permit the rational method of storm drainage analysis for areas encompassing less than 20 acres, and a U.S. Soil Conservation Service (SCS) graphical procedure modified by the City of Colorado Springs for drainage areas encompassing more than 20 acres.

A. RAINFALL

Rainfall intensity-duration-frequency curves obtained from Reference 3 are presented in Figure 4.

City of Colorado Springs 5-year and 100-year 6-hour rainfall amounts are 2.1 inches and 3.5 inches respectively.

B. RUNOFF COEFFICIENTS

Runoff coefficients, C, recommended for use in the Rational Method obtained from Reference 3 are presented as follows:

Park Areas	0.30
Low Residential (2 1/2 acre single-family lots)	0.40
Medium Residential (3 units per acre)	0.50
High Residential (multi-family areas)	0.70
Business and Commercial Areas	0.70
Light Industrial Areas	0.75
Heavy Industrial Areas	0.85
Road Pavements or Roofs	0.90

C. RUNOFF CURVE NUMBERS

Runoff curve numbers (CN) for use in the SCS method were obtained from the Douglas Creek Drainage Basin Masterplanning Study of March 1981 (Reference 1) and Table 3.

D. RUNOFF

Runoff for use in the SCS method was determined from the following equation:

$$Q = \frac{CN(P+2)^2 - 400(P+2 - 100/CN)}{CN(P-8) + 800}$$

Where Q=runoff in inches  
CN=runoff curve number  
P=rainfall in inches

E. TIME OF CONCENTRATION

Time of concentration (Tc) was determined as the sum of overland flow time and channel flow time. The overland flow path length was generally limited to 400 feet. Overland flow time in minutes (To) was estimated using the following equation:

$$T_o = \frac{1.8 (1.1 - C)L^{1/2}}{S^{1/3}}$$

Where To=overland flow time in minutes  
C=roughness coefficient assumed equal to rational method runoff coefficient  
L=length of overland flow path in feet  
S=slope of overland flow path in percent

Channel flow time (T) was estimated by dividing channel flow path length by average flow velocity.

F. PEAK STORM RUNOFF

Peak storm runoff for basins encompassing less than 20 acres was estimated by the rational method. Peak storm runoff for basins encompassing more than 20 acres was estimated by the SCS graphical method using CSM per inch (cfs per square mile per inch of runoff) obtained from Figure 5.

## CALCULATIONS

Hydrology calculations for each planned storm sewer system along Centennial Boulevard are presented in the Appendix.

## CONCEPTUAL DRAINAGE IMPROVEMENTS

Conceptual drainage improvements to accommodate computed design flows (5-year) along this segment of planned Centennial Boulevard are presented in Table 4 and Figures 6 thru 13. Conceptual pipe sizes presented in Table 3 are based on full pipe flow capacity at the given grade. These pipe sizes may require adjustment during final design based on detailed hydraulic calculations and utility data.

Peak storm runoff calculations indicate that storm sewer inlets are not required along the east side of this segment of planned Centennial Boulevard (except at Vindicator Drive) provided that storm runoff from the east side of planned Centennial Boulevard is intercepted and conveyed to the east by the planned Pinon Valley and Reed Ranch access roadways. That is the east side of planned Centennial Boulevard has adequate flow capacity to convey design (5-year) storm flows if periodically intercepted and conveyed to the east by planned access roadways.

## DRAINAGE AND BRIDGE FEES

The segment of planned Centennial Boulevard presently under investigation is within the Douglas Creek Drainage Basin (Basin Code 04) within which the 1983 Drainage Fee is \$3,463 per acre and the 1983 Bridge Fee is \$86 per acre. These Fees are based on the estimated cost of masterplanned drainage and bridge improvements presented in the Douglas Creek Drainage Basin Masterplanning Study (Reference 1) adjusted to present construction costs and the area of land within the basin subject to development.

The Douglas Creek Drainage Basin Masterplanning Study (Reference 1) indicates that the right-of-way associated with Wilson Road (planned Centennial Boulevard) is not subject to either Drainage or Bridge Fees and furthermore, that the party(s) responsible for constructing planned Centennial Boulevard may be eligible for Drainage and/or Bridge Fee Improvement Credits or Direct Reimbursement for drainage and bridge improvements constructed within public right-of-way. Present city procedures for obtaining Drainage Improvement Credits and Reimbursements are contained in a letter from the City of Colorado Springs Director of Public Works addressed, "To Whom It May Concern", dated July 25, 1979 (attached).

## CONCLUSIONS AND RECOMMENDATIONS

1. Whomsoever improves Centennial Boulevard in the vicinity of hydrologic design points H-2A and H-4A, should assure that improvements to divert 100-year storm runoff arriving at masterplanning hydrologic design points H-2A and H-4A to planned major channel are provided.
2. Whomsoever permanently improves Centennial Boulevard immediately north of the existing (12-12)X6 concrete box culvert should provide drainage improvements to divert the difference between 100-year peak storm runoff and the capacity of master planned storm sewer improvements into the major channel for the North Fork of Douglas Creek.
3. Detailed storm sewer hydraulic calculations should be made during final design.
4. Cost Estimate for the drainage facilities included in this preliminary design are shown in table 5. These facilities are included in the Douglas Creek Drainage Basin Master Planning Study and therefore should be eligible for Drainage and/or Bridge Fee Improvement Credit or Direct Reimbursement for improvements constructed within public right-of-way. Note should be taken of the major storm sewer extending from north of Vindicator to the existing concrete channel crossing approximately 940' south of Vindicator which is designated to be constructed by others. The majority of contributory flows to this storm sewer emminate from Oak Valley Ranch with minor Contributions from Mountain Shadows and the City property on the east side of Centennial. Final determination on construction responsibilities of this storm sewer should be made at the time of final design and implementation.
5. Mountain Shadows in association with the Reed Ranch will provide acceptable assurance to the City in the form of a letter of credit, that the facilities identified as the Reed Ranch system will be constructed and installed as indicated on the final design plans dated July 1, 1983.

*Excluded*



## REFERENCES

1. Douglas Creek  
Drainage Basin  
March 1981  
Prepared for: City of Colorado Springs  
Prepared by: Leigh Whitehead and Associates  
5 West Las Vegas  
Colorado Springs, Colorado 80903
2. Drainage Report for  
Pinon Valley, filings No. 2, 3 and 4  
January 4, 1983  
Prepared by: Berge-Brewer and Associates Inc.
3. Determination of Storm Runoff Criteria  
City of Colorado Springs, Colorado  
March 1977  
as amended
4. Soil Survey of El Paso County Area, Colorado  
Soil Conservation Service  
U.S. Department of Agriculture in cooperation with  
Colorado Agricultural Experiment Station  
June 1981
5. Drainage Report for  
Pinon Valley Industrial Park  
Filing No. 9  
Revised February 10, 1983  
Prepared by Berge-Brewer & Assoc., Inc.  
dba H. J. Kraettli & Sons

# TABLE 1

- Permissible Drainage Street Capacities with level ramp curbs\*

S %	34' Residential		36' Residential		40' Residential	
	FPS	CFS	FPS	CFS	FPS	CFS
0.5	2.85	11.7	2.77	11.6	2.62	11.2
1.0	4.03	16.6	3.92	16.4	3.70	15.8
1.5	4.93	20.3	4.80	20.1	4.54	19.3
2.0	5.69	23.5	5.54	23.2	5.24	22.3
2.5	6.37	26.2	6.20	25.9	5.86	24.9
3.0	6.97	28.7	6.79	28.4	6.42	27.3
3.5	7.53	31.0	7.33	30.7	6.93	29.5
4.0	8.05	33.2	7.84	32.8	7.41	31.5
4.5	8.54	35.2	8.31	34.8	7.86	33.4
5.0	9.00	37.1	8.76	36.7	8.28	35.2
5.5	9.44	38.9	9.19	38.5	8.69	37.0
6.0	9.86	40.6	9.60	40.2	9.07	38.6

\*Intermediate values may be obtained by arithmetic interpolation.

**TABLE 2** Permissible Drainage Street Capacities with 8" Vertical Curbs \*  
 8" Curb - Full Storm Water Capacity (with level curbs)

S	34' Residential		36' Residential		40' Residential		34' One-Way Art.		60' & 76' Arterial		S %
	FPS	CFS	FPS	CFS	FPS	CFS	FPS	CFS	FPS	CFS	
0.5	4.08	28.9	4.02	29.5	3.90	30.1		20.0		20.0	0.5
1.0	5.76	40.9	5.70	41.7	5.51	42.6		30.0		30.0	1.0
1.5	7.06	50.1	6.97	51.1	6.75	52.2	6.97	30.0	6.97	30.0	1.5
2.0	8.15	57.8	8.05	59.0	7.79	60.2	8.05	34.0	8.05	34.0	2.0
2.5	9.11	64.7	9.00	65.9	8.71	67.4	9.00	36.0	9.00	36.0	2.5
3.0	9.98	70.9	9.86	72.2	9.54	73.8	9.86	38.0	9.86	38.0	3.0
3.5	10.78	76.5	10.65	78.0	10.31	79.7	10.65	40.0	10.65	40.0	3.5
4.0	11.52	81.8	11.38	83.4	11.02	85.2	11.38	42.0	11.38	42.0	4.0
4.5	12.22	86.8	12.07	88.5	11.69	90.4	12.07	43.0	12.07	43.0	4.5
5.0	12.89	91.5	12.73	93.3	12.32	95.3	12.73	45.0	12.73	45.0	5.0
5.5	13.52	95.9	13.35	97.8	12.92	99.9	13.35	47.0	13.35	47.0	5.5
6.0	14.12	100.0	13.94	102.2	13.49	104.3	13.94	49.0	13.94	49.0	6.0

\* Values may be obtained by Arithmetic Interpolation

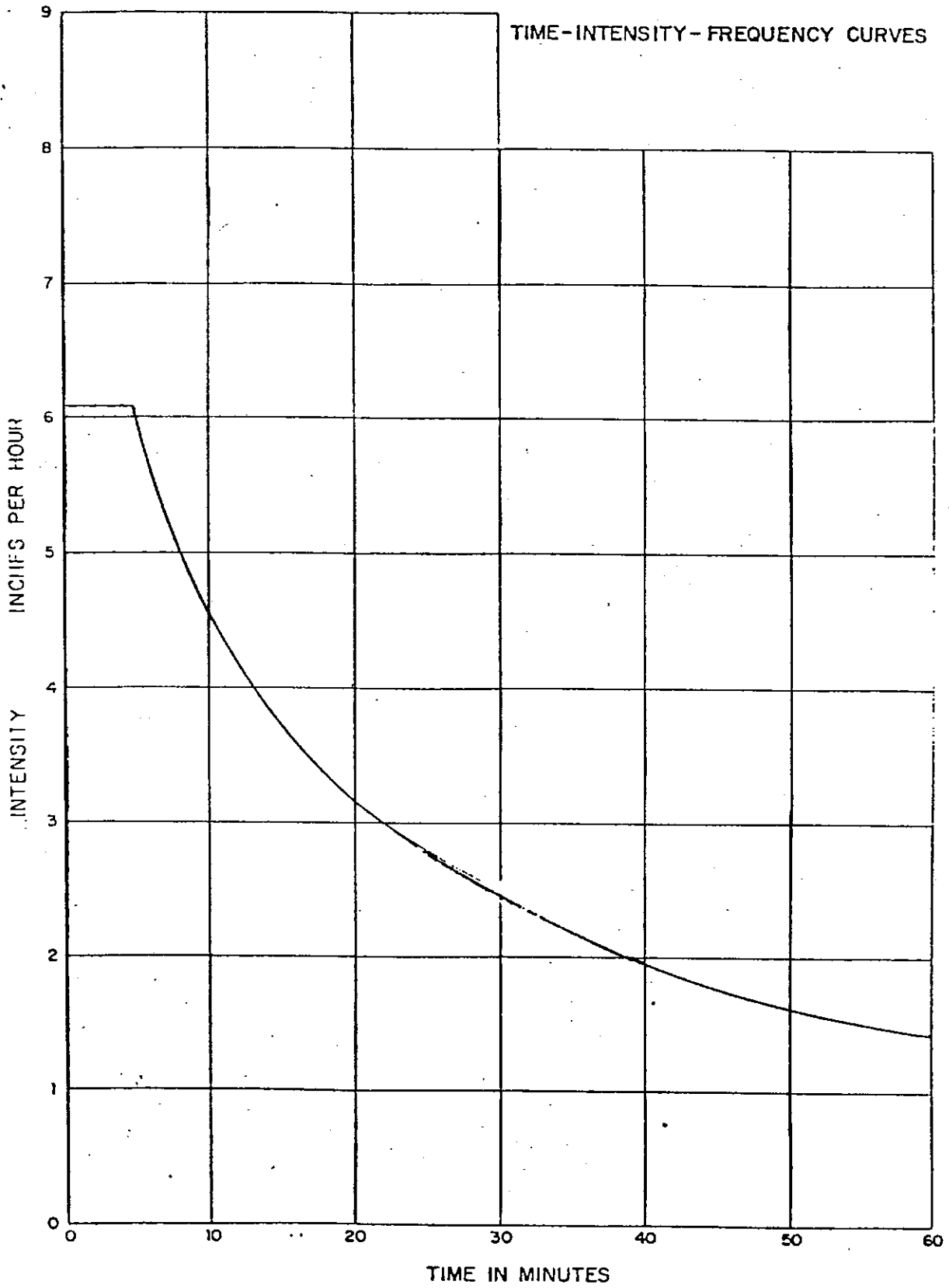
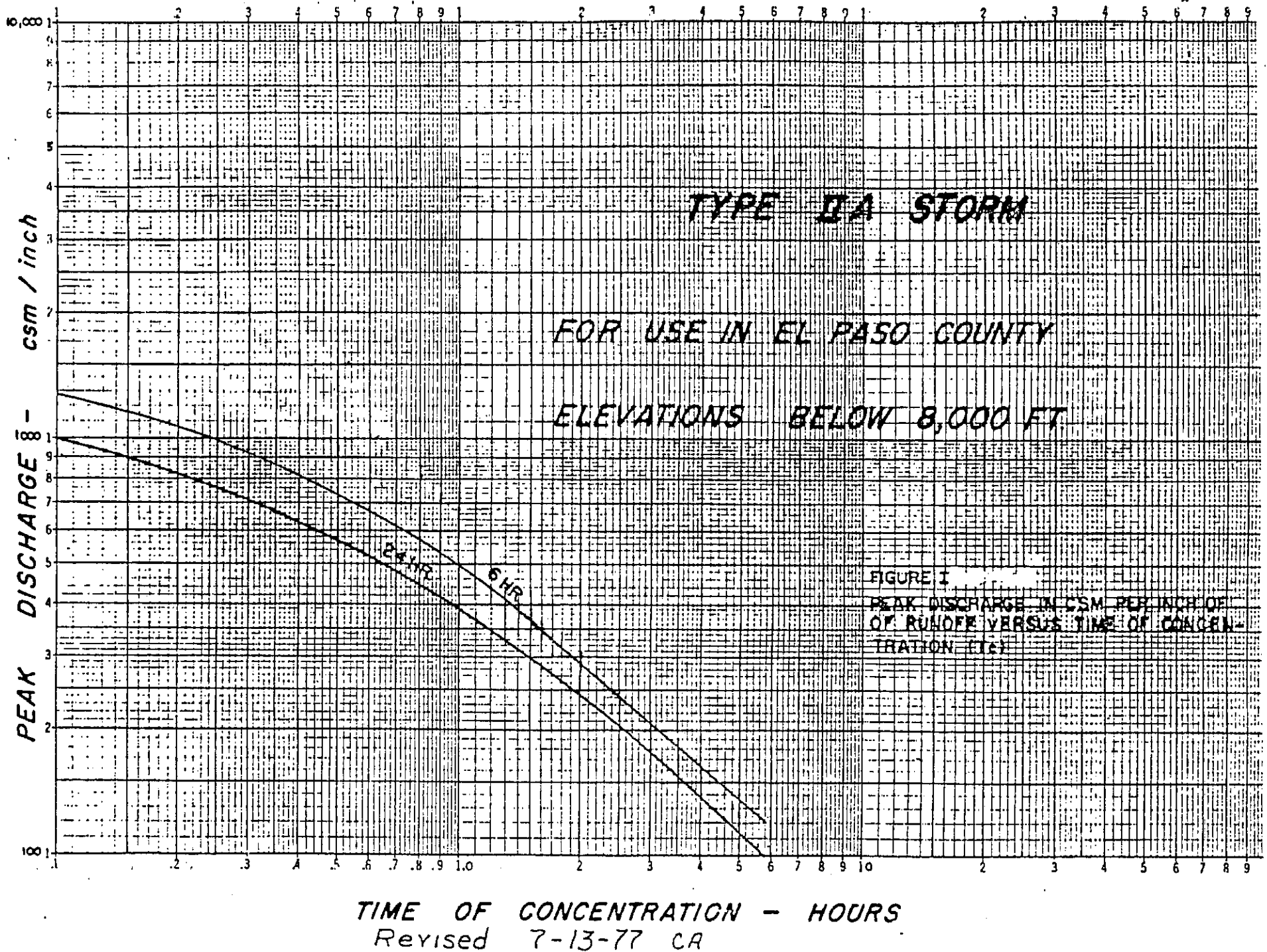


FIGURE 4



FIGURE 5  
17



URS NO. 3073 BY \_\_\_\_\_ DATE 6-2-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_  
CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD  
SUBJECT STORM DRAINAGE

**TABLE 4**  
CONCEPTUAL STORM DRAINAGE IMPROVEMENTS  
CENTENNIAL BOULEVARD STA 143+00 to 213+00

○ MULE DEER DRIVE STORM SEWER SYSTEM

○ STA 140+60 to 151+32 24"  $\phi$  rcp x 1,072 LF  
QDES = 15 CFS.

1- manhole for 42"  $\phi$  CSP Arch  
1- manholes for 24"  $\phi$  rcp

○ STA 149+20  $\pm$  L 8' - D-10-R QDES = Q5 = 15 CFS  
24"  $\phi$  rcp connector line x 75 LF Q5 = 15 CFS  
1 manhole for 24"  $\phi$  rcp

○ STA 149+20 to 151+32 24"  $\phi$  rcp connector x 212 LF  
QDES = Q5 = 31 CFS

1- manhole for 36"  $\phi$  rcp

○ STA 151+32 10' D-10-R, S = 0.5%, QDES = 7.7 cfs  
36"  $\phi$  rcp x 90 LF

1- manhole for 36"  $\phi$  rcp

○ STA 151+32 to 155+29 36"  $\phi$  rcp x 397 LF  
QDES = 53.3 CFS

○ STA 155+29 14' inlet in sump QDES = 32.5 CFS  
24"  $\phi$  rcp stub out to west

1- manhole for 36"  $\phi$  rcp

24"  $\phi$  x 80 LF rcp

36"  $\phi$  x \_\_\_ LF rcp outfall



URS COMPANY

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URS NO. 3073 BY \_\_\_\_\_ DATE 6-2-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

TABLE 4 (CONTINUED)

ATHERTON WAY STORM SEWER SYSTEM (STA 169+95)

STA 169+75 6' D-10-R, QDES = Q5 = 10 CFS

STA 169+95 Manhole for 30"  $\phi$  rcp  
30"  $\phi$  rcp stub-out to west L.F.  
36"  $\phi$  rcp outfall QDES = 73 CFS

STA 169+75 to 169+95 18"  $\phi$  rcp connector

STA 169+75 to 170+15 24"  $\phi$  rcp connector  
QDES = 32 CFS

STA 170+15 to 176+00 24"  $\phi$  rcp connector  
QDES = 16 CFS

STA 173+10 1-manhole for 24"  $\phi$  rcp

STA 176+00 8', D-10-R in sump QDES = 16 CFS





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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BLVD

SUBJECT STORM DRAINAGE

TABLE 4 - (CONTINUED)

SOUTH MOUNTAIN SHADOWS ACCESS ROAD  
STORM SEWER SYSTEM (STA 188+00)

STA 188+00 2- manhole for 36"  $\phi$  rcp  
30"  $\phi$  rcp stub out to west x      LF  
QDES = 75 CFS.

36"  $\phi$  rcp outfall line x      LF.

STA 188+30 2- 8' INLET, D-10-R  
1- 24"  $\phi$  rcp x 30 LF, QDES = 12.7 CFS  
24"  $\phi$  rcp x 30 LF QDES = 6.6 CFS



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD  
SUBJECT STORM DRAINAGE

TABLE 4 (CONTINUED)

NORTH SHADOW MOUNTAIN ACCESS ROAD  
STORM SEWER SYSTEM (STA 201+00±)

STA 201+00 1-2 MANHOLE FOR 30"  $\phi$  rcp  
27"  $\phi$  rcp stub-out  
30"  $\phi$  rcp storm sewer outfall QDES=37 CFS

STA 201+00 to 201+30  
24"  $\phi$  rcp x 30 L.F. QDES = 12 CFS  
24"  $\phi$  rcp x 30 L.F. QDES = 7.3 CFS

STA 201+30± 8' inlet QDES = 12 CFS  
6' inlet QDES = 7.3 CFS

VINDICATOR DRIVE STORM SEWER

STA 205+40± to 218+00± 42"  $\phi$  RCP x 1250 L.F.  
3 MANHOLES FOR 42"  $\phi$

STA 215+00 2-8' D-10 R INLETS

STA 216+50 3-8' D-10 R INLETS

18"  $\phi$  RCP CONNECTOR X L.F.

24"  $\phi$  RCP CONNECTOR X L.F.

TABLE 5, ESTIMATED COST

7/29/83

1. MULE DEER DRIVE SYSTEM

36" RCP 360' @ \$60/LF = 21,600  
 24" RCP 385' @ \$35/LF = 13,475  
 18" RCP 80' @ \$25/LF = 2,000  
 8' D-10-R 1 @ \$1400 EA = 1,400  
 10' D-10-R 2 @ \$1600 EA = 3,200  
 Manholes 4 @ \$1700 EA = 6,800  
 48,475

2. ATHERTON WAY SYSTEM

36" RCP 85' @ \$60/LF = 5,100  
 30" RCP 35' @ \$45/LF = 1,575  
 18" RCP 120' @ \$25/LF = 3,000  
 6' D-10-R 2 @ \$1,200 = 2,400  
 10' D-10-R 1 @ \$1,600 = 1,600  
 Manholes 2 @ 1700 = 3,400  
 17,075

3. REED RANCH SYSTEM

36" RCP 246' @ \$60/LF = 14,760  
 30" RCP 202' @ \$45/LF = 9,090  
 18" RCP 128' @ \$25/LF = 3,200  
 15" RCP 128' @ \$20/LF = 2,560  
 10' D-10-R 2 @ \$1600 EA = 3,200  
 4' D-10-R 2 @ \$1200 EA = 2,400  
 Manhole (10.5') 1 @ \$1900 EA = 1,900  
 Manhole (10') 1 @ \$1900 EA = 1,900  
 Manhole (8.8') 1 @ \$1700 EA = 1,700  
 Manhole (8') 1 @ \$1700 EA = 1,700  
 42,410

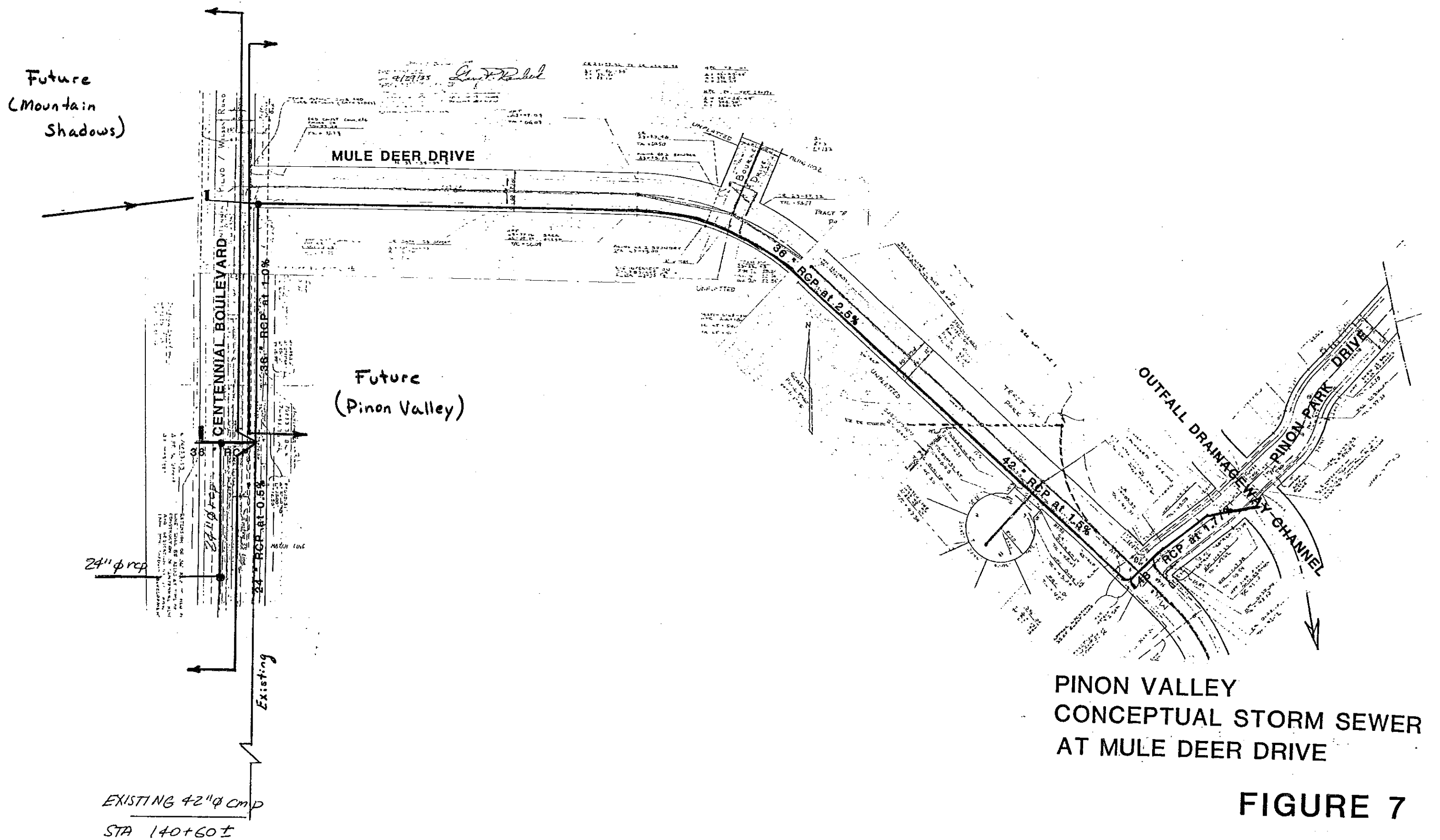
4. VINDICATOR / FLYING W RANCH ROAD SYSTEM

42" RCP 940' @ \$80/LF = 75,200  
 36" RCP 260' @ \$60/LF = 15,600  
 24" RCP 50' @ \$35/LF = 1,750  
 18" RCP 300' @ \$25/LF = 7,500  
 8' D-10-R 5 @ \$1400 EA = 7,000  
 Manhole 1 @ \$1700 EA = 1,700  
 108,750

Total Const. Cost 216,710  
 Contingencies (20%) 43,342  
 260,052  
 ENGR, LEGAL, ADMIN (10%) 26,005

# 286,057  
 SAY \$ 286,000 GRAND TOTAL



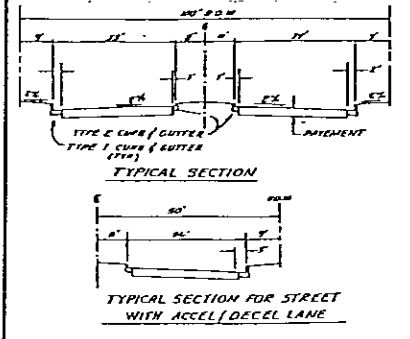
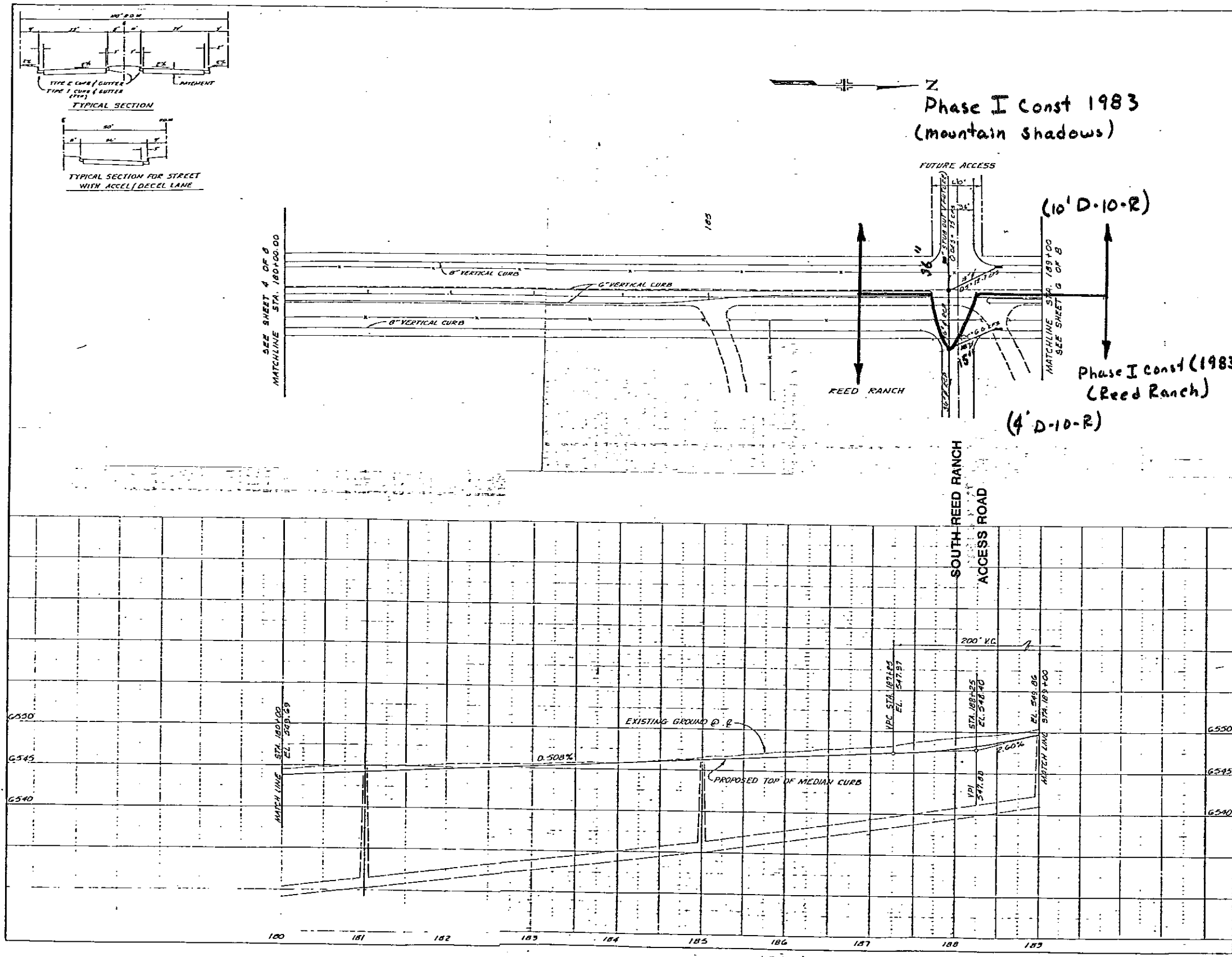


PINON VALLEY  
 CONCEPTUAL STORM SEWER  
 AT MULE DEER DRIVE

FIGURE 7







Phase I Const 1983  
(Mountain Shadows)

**DESIGN DATA**

CURB TYPE  WHEEN VALVE   
 MAT width \_\_\_\_\_ feet TRAFFIC INDEX \_\_\_\_\_  
 (Curb face to curb face)  
 SIDE WALKS \_\_\_\_\_  
 WIDTH \_\_\_\_\_ feet  
 LOCATION  attached to curb  
 detached, located \_\_\_\_\_ from P.V.E.  
 SUBBASE REQUIRED \_\_\_\_\_  
 B. SELECT \_\_\_\_\_ inches  
 B. P.V. RUN \_\_\_\_\_ inches

**STATEMENTS**

A. SEWER - Any changes or alterations affecting the grade, alignment, location, and depth of cover of sewers and appurtenances shown on this drawing shall be the responsibility of the developer.  
 B. WATER - This approval subject to the final street grade having a minimum cover of 5' (five feet) over the water main. Any changes shall be at the expense of the owner or developer.

**APPROVALS**

CITY ENGINEER \_\_\_\_\_ Date \_\_\_\_\_  
 Final Appr. \_\_\_\_\_ Date \_\_\_\_\_  
 WATER DEPT. BY \_\_\_\_\_ Date \_\_\_\_\_  
 SEWER DEPT. BY \_\_\_\_\_ Date \_\_\_\_\_

**NOTES:**

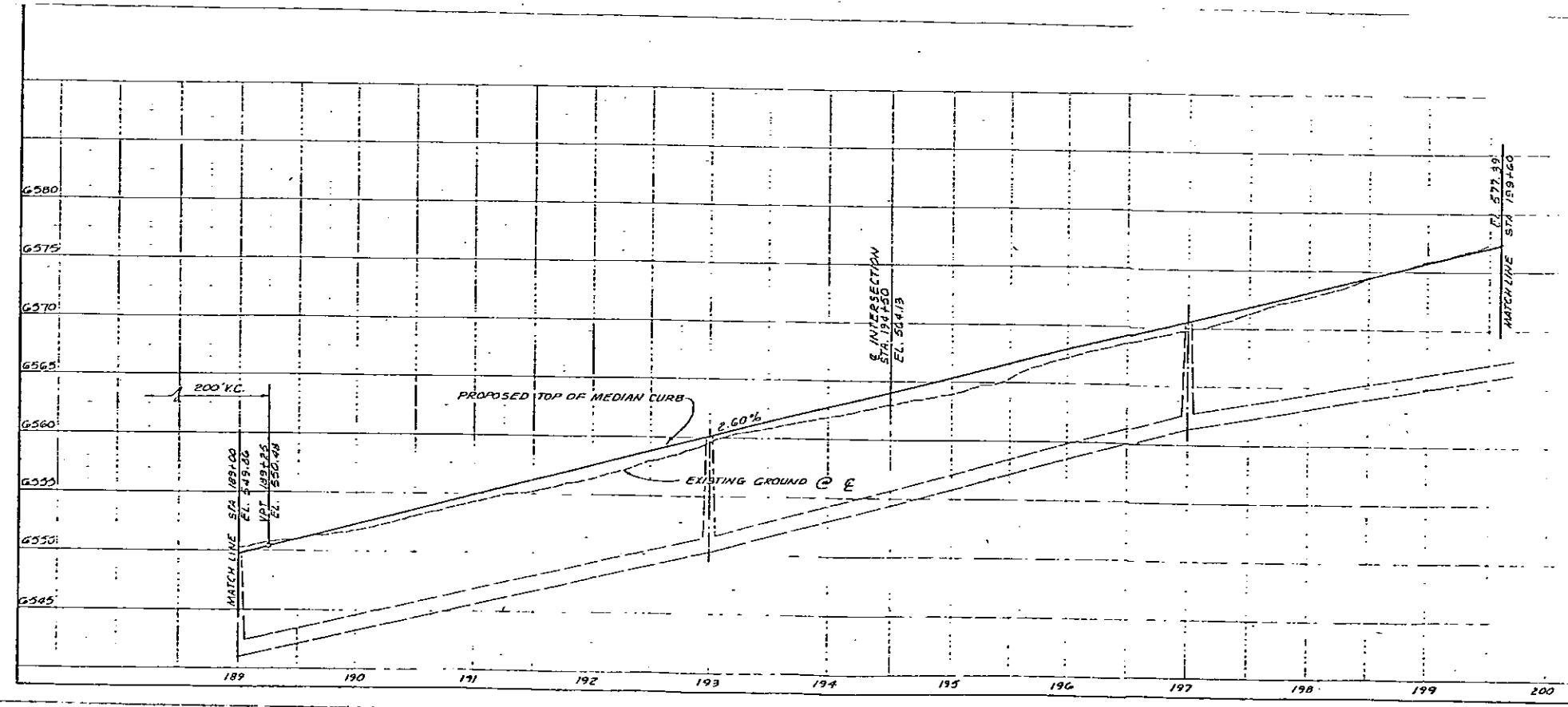
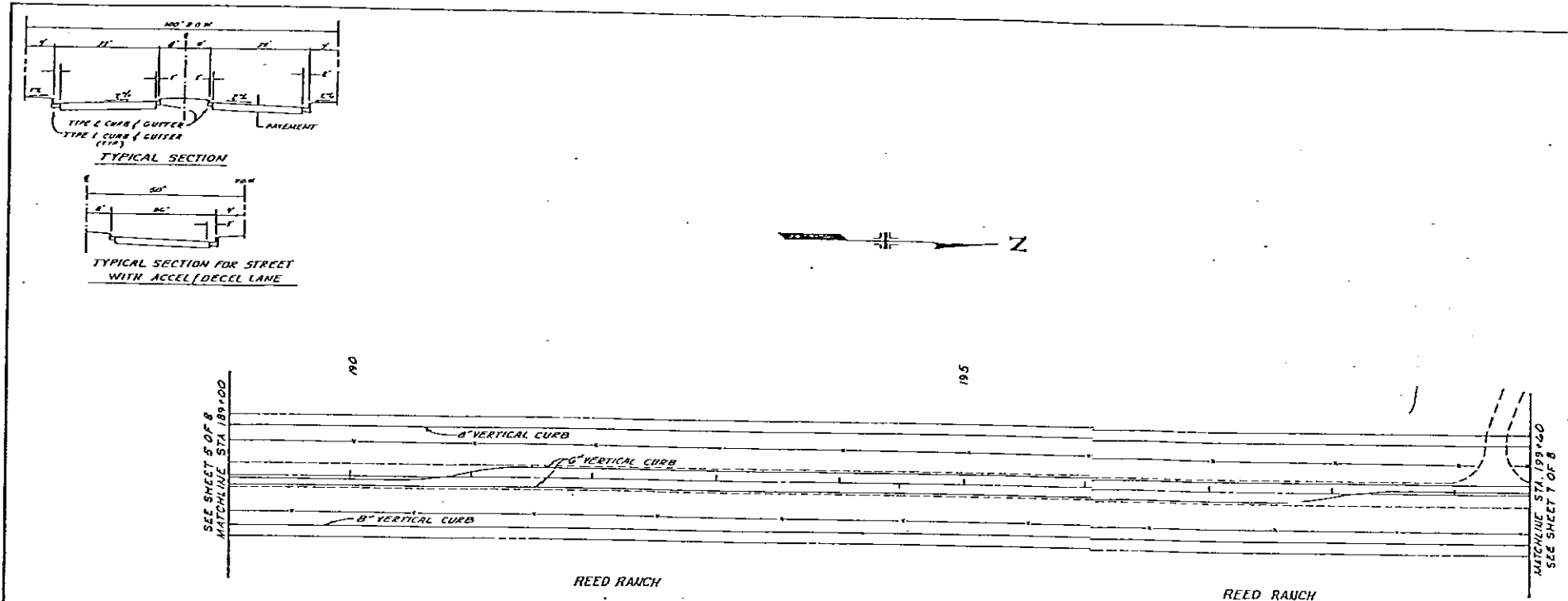
BENCH MARK \_\_\_\_\_  
 REVISIONS \_\_\_\_\_

ENGR. **URS COMPANY**

STREET NAME **CENTENNIAL BLVD**  
 FROM **180+00** TO **189+00**  
 SUBDIVISION \_\_\_\_\_  
 DRAWN BY **E.L.J.** DATE \_\_\_\_\_  
 CHECK BY **E.L.J.** DATE \_\_\_\_\_  
 JOB NO. **3076** SHEET **5** OF **8**

FIGURE 10





DEF. DATA  
 1" = 10' VERT. SCALE  
 1" = 100' HORIZ. SCALE  
 METRIC SCALE  
 1" = 10' MET. SCALE  
 1" = 100' MET. SCALE

WORK FACE TO CURB FACE  
 SIDE MARKS  
 WIDTH  
 HEIGHT  
 ATTACHED TO CURB  
 ATTACHED TO CURB

SUBBASE REQUIRED  
 SELECT  
 D.P.S. NO.

STATEMENTS  
 A SEWER - Any changes in elevations affecting the grade alignment, structure, and depth of sewer at manholes and appurtenances shown on this drawing are the responsibility of the sewer owner.  
 B WATER - This approval subject to the local street grade having a minimum cover of 2' at all times. Cover the water main. Any changes are to be the expense of the owner or developer.

By \_\_\_\_\_

APPROVALS  
 CITY ENGINEER  
 Rough-Cut Appr: \_\_\_\_\_ Date: \_\_\_\_\_  
 Final Appr: \_\_\_\_\_ Date: \_\_\_\_\_

WATER DEPT By: \_\_\_\_\_ Date: \_\_\_\_\_  
 SEWER DEPT By: \_\_\_\_\_ Date: \_\_\_\_\_

NOTES

BENCH MARK \_\_\_\_\_

REVISIONS

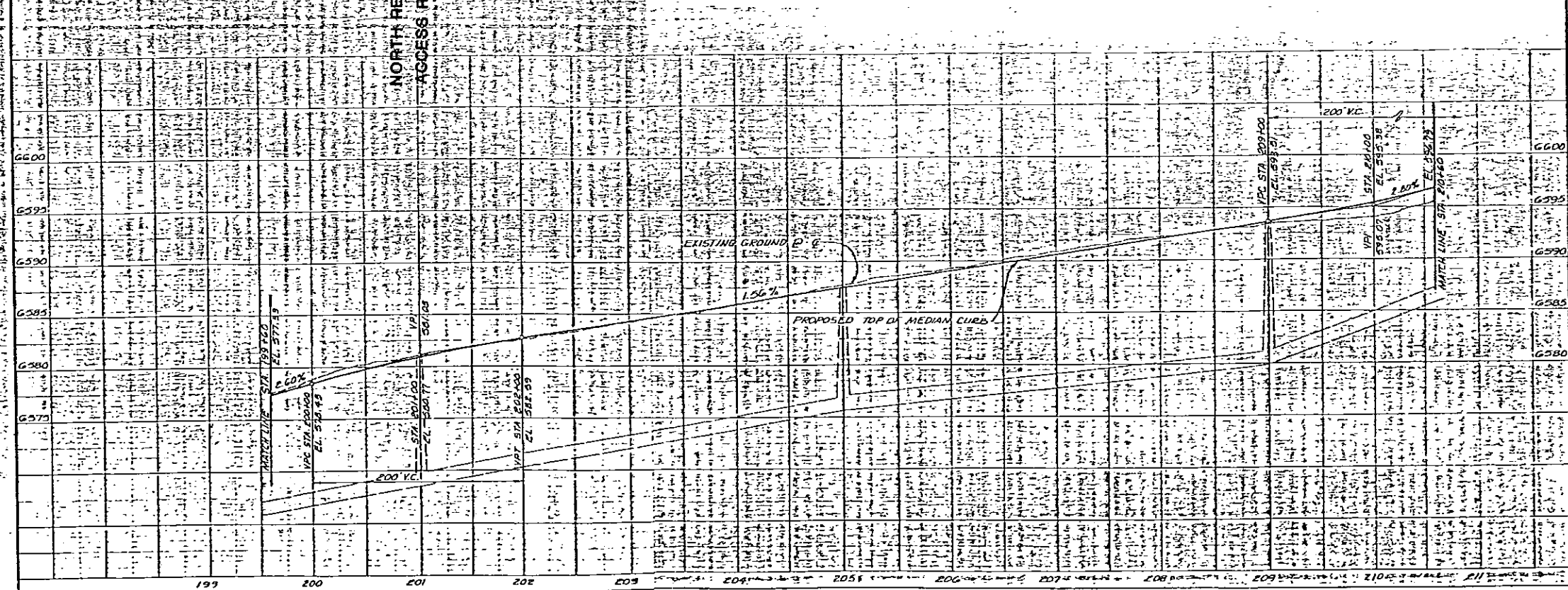
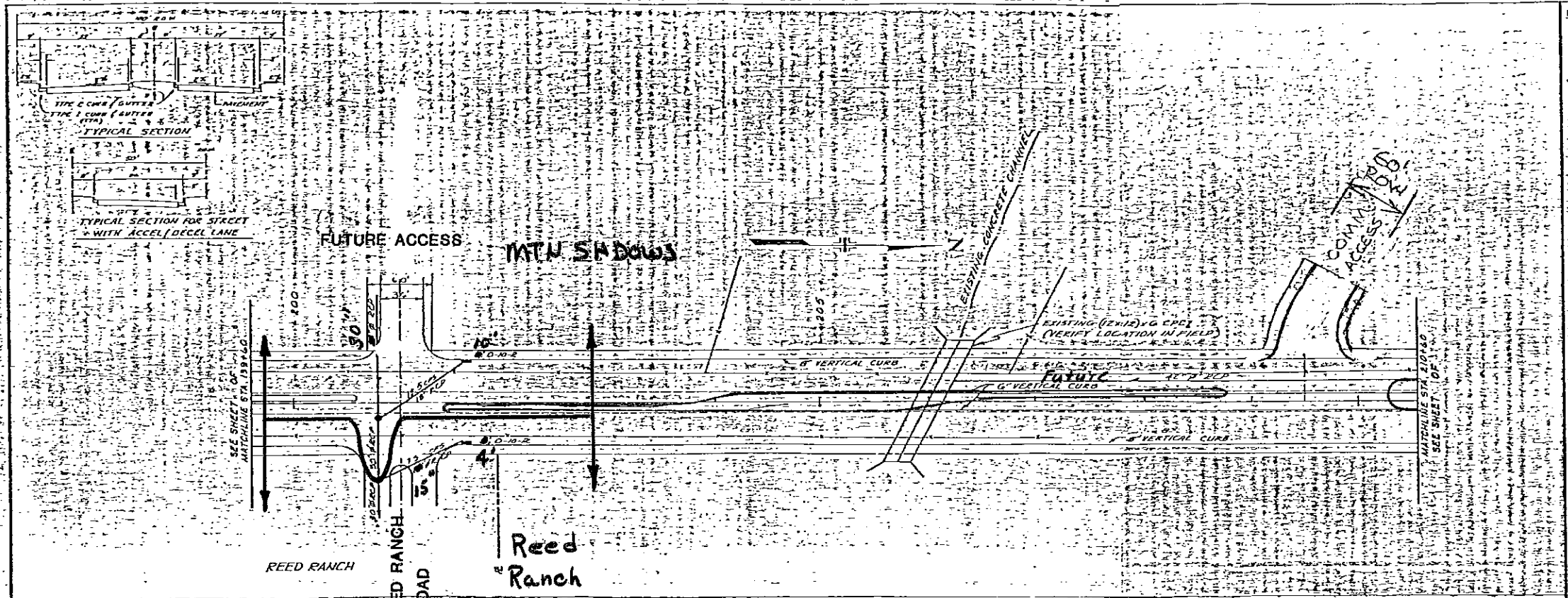
ENGR

STREET NAME CENTENNIAL BOULEVARD  
 FROM STA 189+00 TO STA 199+60

SUBDIVISION

DESIGN BY T.L.J. DATE  
 DRAWN BY D.S.N. DATE  
 JOB NO. 507B FILE NO. 6 B

FIGURE 11



**DESIGN DATA**

CURB TYPE:  **W/VEE VALUE**   
 MAX. W/VEE:  **TRAFFIC INDEX**   
 (ELEV. W/VEE IN FEET)

**SEWERS:**  
 WITH:  **W/VEE VALUE**   
 WITHOUT:  **TRAFFIC INDEX**   
 (ELEV. W/VEE IN FEET)

**STATEMENTS:**  
 A. SEWERS - Any changes or alterations affecting the grade, alignment, elevation, and depth of cover, etc. shown on the drawing shall be the responsibility of the developer.  
 B. WATER - The approval subject to the final street grade being a minimum of 5' (five feet) over the sewer main. Any changes shall be at the expense of the owner or developer.

**APPROVED:**  
 CITY ENGINEER: \_\_\_\_\_ Date: \_\_\_\_\_  
 WATER DEPT. BY: \_\_\_\_\_ Date: \_\_\_\_\_  
 SEWER DEPT. BY: \_\_\_\_\_ Date: \_\_\_\_\_

**NOTES:**

**BENCH MARK:**

**REVISIONS:**

**ENG.:**

**STREET NAME:** CENTENNIAL BOULEVARD  
 FROM STA 199+00 TO STA 210+00

**SCALE:** 1" = 10'

**DATE:** 11/11/03  
**BY:** D.S.H.  
**NO.:** 3073

FIGURE 12



**CITY OF COLORADO SPRINGS**  
DEPARTMENT OF PUBLIC WORKS • ADMINISTRATION (303) 471-6660 • ENGINEERING (303) 471-6606  
105 WEST COSTILLA • P.O. BOX 1575  
COLORADO SPRINGS, COLORADO 80901

July 25, 1979

TO WHOM IT MAY CONCERN:

RE: Procedures for Drainage Improvements Credits and Reimbursements

In order for a developer to obtain reimbursement of drainage costs or credit, certain procedures must be followed. The drainage ordinance, as amended, states that prior to proceeding with construction of major facilities, the developer must comply with the following:

- 1) Obtain through the subdivision process an approved drainage plan showing the facilities required and the estimated cost of those facilities, and indicate the drainage basin and fee for the acreage shown on the drainage plan. (Copies of the current per-acre fee can be obtained from the Public Works Department.) All drainage basins are identified and fees established each year. Letters of credit are to be posted with the City in accordance with the subdivision ordinance.
- 2) Obtain three (3) sealed bids for constructing the facilities. If the lowest bid exceeds the Engineer's estimated costs as indicated in the drainage report, the City Engineer must review and give approval (or disapproval) before proceeding.
- 3) If it is not possible to receive three bids, the developer must obtain the City Engineer's approval before proceeding with construction, and the cost and credits shall be determined at that time.
- 4) The developer proceeds with construction according to the approved plans and any changes during construction shall be discussed with the City Engineer.
- 5) Upon completion of the construction of facilities, the developer shall obtain a certification from a Colorado Registered Professional Engineer that the facilities inspected are constructed in accordance with the approved plans.
- 6) The developer then submits this certification along with records of the cost of construction, i.e., a copy of the bid, as-paid bills, and a request for inspection and reimbursement to the City Engineer.

7) The City Engineer will review the request, inspect the facilities and after verification of the costs, will place the request on the Drainage Board Agenda for formal review. The letter of credit for drainage facilities may be released at this point.

8) The Drainage Board meets on the third Thursday of each month and reviews the requests for reimbursement after hearing from both the City and the developer.

9) If the request is approved, the item is then forwarded to City Council for final approval of reimbursement of monies from the drainage fund account.

10) If the request is not approved, the Drainage Board will set the amount of reimbursement and developer has the right to appeal to City Council.

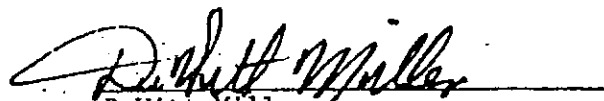
11) Reimbursements are made twice a year: on the first of June and the first of December. Items to be heard for June disbursement must be approved by the Drainage Board by April 1. Items to be heard for December disbursement must be approved by the Drainage Board no later than October 1.

12) If the Drainage Basin account has insufficient funds for reimbursement, the developer is placed on a priority list and is paid when revenue within the fund is sufficient. Partial reimbursements are made until the developer is fully reimbursed.

13) Any dispute over bidding or fees may be heard by the Drainage Board and City Council.

14) The developer should be aware that all drainage fees are finally computed to the time that bids are taken and not at the time of platting.

15) Fees on all basins are adjusted by City Council each year at their first meeting of the year.

  
DeWitt Miller  
Director of Public Works

RAM.

RAM/pjk



URS COMPANY

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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

## CALCULATIONS

- o MULE DEER DRIVE STORM SEWER SYSTEM (STA 155+29±)
- o ATHERTON WAY STORM SEWER SYSTEM (STA 169+95±)
- o SOUTH MOUNTAIN SHADOWS ACCESS ROAD STORM SEWER SYSTEM (STA 188+00±)
- o NORTH MOUNTAIN SHADOWS ACCESS ROAD STORM SEWER SYSTEM (STA 201+00±)
- o VINDICATOR DRIVE/MOUNTAIN SHADOWS STORM SEWER SYSTEM (STA 215+70±)



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - CONCEPTUAL DESIGN

MULE DEER DRIVE STORM SEWER SYSTEM

- o STA 140+60 to 151+32, 24"  $\phi$  RCP x 1072 LF  
QDES = QS = 15 CFS
- o STA 149+20 r 8' inlet in sump, QDES = QS = 15 CFS  
24"  $\phi$  rcp connector line x 75 LF QS = 15 CFS  
1- manhole for 24"  $\phi$  rcp
- o STA 149+20 to 151+32 915 LF  
24"  $\phi$  rcp connector x 212 LF  
1 manhole for 36"  $\phi$  rcp 20 LF  
QDES = QS = 46 CFS
- o STA 151+32 10-foot inlet on 0.5% QDES = QS = 7.7 CF  
36"  $\phi$  rcp x 90 LF  
1- manhole for 36"  $\phi$  RCP
- o STA 151+32 to 155+29 36"  $\phi$  rcp x 397 LF  
QDES = 53.3 CFS
- o STA 155+29 14-foot inlet in sump  
QDES = QS = 32.5 CFS  
24"  $\phi$  RCP stub out to west  
1- manhole for 36"  $\phi$  rcp  
24"  $\phi$  x 80 LF RCP  
36"  $\phi$  x \_\_\_\_\_ LF RCP OUTFALL



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

North side of planned R-6 R&D Entrance on the West side of planned Centennial Boulevard (STA 140+50)

Assume storm runoff from drainage masterplanning basin C-9 does not affect this segment of planned Centennial Boulevard.

Assume 5-year storm runoff associated with drainage masterplanning basin C-8 to be intercepted by existing cross culvert (STA 140+35) and conveyed to planned Mule Deer Drive storm sewer system.

o DRAINAGE AREA

C-8 21 acres, CN = 73,  $T_c = 0.10$

west side of planned Centennial Boulevard

$(950 \times 50) / 43,560 = 1.1$  acres, CN = 98, C = 0.9

planned R-6 entrance from the west

3.2 acres, CN = 72, C = 0.4

o TIME OF CONCENTRATION

Assume same time of concentration as masterplanning basin C-8 as obtained from Douglas Creek Drainage Basin Masterplanning Study of March 1981

$T_c = 0.113$  hours



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD  
STA 140+50 WEST SIDE OF PLANNED CENTENNIAL BLVD (CONT.)

- o CSM per inch ( $T_c = 0.11$  hr) = 1250
- o COMPOSITE CN

$$\begin{aligned} 21 \times 73 &= 1533 \\ 1.6 \times 981 &= 108 \\ \hline 3.2 \times 72 &= 230 \end{aligned}$$

$$25.3 \quad 1871$$

$$\bar{CN} = 1871 / 25.3 = 74$$

- o RUNOFF

$$\begin{aligned} Q_5 (CN = 74) &= 0.40 \text{ inches} \\ Q_{100} (CN = 74) &= 1.24 \text{ inches} \end{aligned}$$

- o PEAK RUNOFF

$$\begin{aligned} 5\text{-year} &= \frac{25.3}{640} \times 1250 \times 0.40 = 19.8 \text{ CFS} \\ &\quad - 15. (C-8, 24" \phi) \\ &\quad \hline &\quad 4.8 \text{ CFS} \end{aligned}$$

$$\begin{aligned} 100\text{-year} &= \frac{25.3}{640} \times 1250 \times 1.24 = 61.3 \\ &\quad - 15 (C-8, 24" \phi) \\ &\quad \hline &\quad 46.3 \text{ CFS} \end{aligned}$$

- o EQUIVALENT AREA CARRY OVER

$$5\text{-year} = \frac{4.8 \times 640}{1250 \times 0.40} = 6.1 \text{ acres, CN} = 73.5$$

$$100\text{-year} = \frac{46.3 \times 640}{1250 \times 1.24} = 19.1 \text{ acres, CN} = 73.5$$

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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

SOUTH SIDE OF PLANNED R-6 ENTRANCE STATION 149+20  
ON THE WEST SIDE OF PLANNED CENTENNIAL BOULEVARD

o DRAINAGE AREA

West of planned Centennial Boulevard

4.82 acres, CN = 88

West half of planned Centennial Boulevard

$(800 \times 50) / 43560 = 0.92$  acres CN = 98

o COMPOSITE CN

	<u>5-year</u>	<u>100-year</u>
	$4.8 \times 88 = 423$	$4.8 \times 88 = 423$
	$1 \times 98 = 98$	$1 \times 98 = 98$
CARRY-OVER FROM STATION 140+50 →	$6.1 \times 73.5 = 449$	$19.1 \times 73.5 = 1404$
	<u>11.9</u>	<u>24.9</u>
	$1970 / 11.9 = \underline{\underline{81.5}}$	$1925 / 24.9 = \underline{\underline{77.3}}$

o RUNOFF

$Q_5 (CN = 81.5) = 0.69$

$Q_{100} (CN = 77.3) = 1.45$

o TRAVEL TIME BETWEEN 140+50 TO 149+20

$\frac{800}{5 \times 60 \times 60} = 0.044$

o TIME OF CONCENTRATION

$T_c = 0.113 + 0.044 = 0.157$  hrs

o CSM per inch ( $T_c = 0.157$  hrs) = 1150



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

SOUTH SIDE OF STATION 149+20 WEST SIDE OF CENTENNIAL BLVD

0 PEAK STORM RUNOFF INCLUDING CARRY OVER FROM BASIN C-8

$$5\text{-year} = \frac{11.9}{640} \times 1150 \times 0.69 = 14.8 \text{ CFS.}$$

$$100\text{-year} = \frac{24.9}{640} \times 1150 \times 1.45 = 64.9 \text{ CFS.}$$

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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

PLANNED STORM SEWER INLET TO PICK UP RUNOFF FROM SOUTHERN PORTION OF DRAINAGE MASTER PLANNING BASIN C-7 WEST OF PLANNED R/D LOOP ROAD DIRECTLY ACROSS FROM PLANNED MULTI DEER DRIVE

○ DRAINAGE AREA

C-7 110 acres

- 11.1

- 4.8

- 16.3

- 5.06

- 6.3

- 8.7

57.75 acres CN=72

○ TIME OF CONCENTRATION

6880

6500

$$\frac{380}{3000} = 12.67\%$$

$$T_c = \frac{(11.9 \times (\frac{3000}{5280})^3)^{0.385}}{380} = 0.137 \text{ hrs}$$

○ CSM per inch = 1180

○ RUNOFF

$$Q_5 (CN=72) = 0.34 \text{ inches}$$

$$Q_{100} (CN=72) = 1.12 \text{ inches}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

PLANNED STORM SEWER INLET TO INTERCEPT STORM RUNOFF FROM THE SOUTHERN PORTION OF DRAINAGE MASTERPLANNING BASIN C-7 WEST OF PLANNED R&D LOOP ROAD DIRECTLY ACROSS FROM PLANNED MULE DEER DRIVE (CONTINUED)

o PEAK STORM RUNOFF

$$5\text{-year} = \frac{57.75}{640} \times 1180 \times 0.34 = 36.2 \text{ CFS}$$

$$100\text{-year} = \frac{57.75}{640} \times 1180 \times 1.12 = 119.3 \text{ CFS}$$

o ASSUME STORM SEWER INLET INTERCEPTS 5-YEAR PEAK STORM RUNOFF OF 36.2 CFS

o EQUIVALENT AREA ASSOCIATED WITH 100-YEAR CARRY OVER

$$A_{\text{EQUIV. 100-year}} = \frac{(119.3 - 36.2) \times 640}{1180 \times 1.12} = 40.24 \text{ Acres}$$

CN = 72



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

PLANNED STORM SEWER INLET TO INTERCEPT  
STORM RUNOFF FROM THE SOUTHERN PORTION OF  
DRAINAGE MASTER PLANNING BASIN C-7 WEST  
OF PLANNED R & D LOOP ROAD.

o DRAINAGE AREA

17.9 acres

16.3 acres

24.2 Ac CN = 72

o RUNOFF

$$Q_5 (CN=72) = 0.34 \text{ inches}$$

$$Q_{100} (CN=72) = 1.12 \text{ inches}$$

o TIME OF CONCENTRATION

$$\begin{array}{r} 6780 \\ - 6520 \\ \hline \end{array}$$

$$260/2100 = 12.4\%$$

$$T_c = \left( \frac{11.9 \left( \frac{2100}{5280} \right)^3}{260} \right)^{0.385} = 0.105 \text{ hrs}$$

o CSM per inch = 1260

o PEAK STORM RUNOFF

$$5\text{-year} = \frac{24.2}{640} \times 1260 \times 0.34 = 16.2$$

$$100\text{-year} = \frac{24.2}{640} \times 1260 \times 1.12 = 53.4 \text{ CFS}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

STATION 155+2.9 MULE DEER DRIVE

o TOTAL FLOW FROM NORTH AND SOUTH

\* EXCLUDING STORM RUNOFF FROM CHUCK WAGON ROAD

	AREA (acres)	CN	CNxA
FROM THE NORTH	70	72	5040
FROM THE SOUTH	61	72.3	4,410
	<u>131</u>		<u>9450</u>

Composite CN =  $9450 / 131 = 72.14$

RUNOFF

$Q5 (CN=72.14) = 0.34$  inches

$Q100 (CN=72.14) = 1.13$  inches

TIME OF CONCENTRATION 0.10 hours

CSM per inch = 1260

PEAK STORM RUNOFF

5-year =  $\frac{131}{640} \times 1260 \times 0.34 = 88$  cfs

100-year =  $\frac{131}{640} \times 1260 \times 1.13 = 291.4$  cfs

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 CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD  
 SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD.

STATION 155+29 MULE DEER DRIVE FROM THE SOUTH

- o SOUTHERN PORTION OF MASTER PLANNING BASIN C-7  
AND ALL OF MASTER PLANNING BASIN C-8

BASIN	AREA (acres)	CN	CN x A
C-7 (SOUTH)	40	72	2880
C-8	<u>21</u>	73	<u>1533</u>
	61		4413

o Composite CN =  $4413 / 61 = 72.34$

- o RUNOFF

$Q_5 (CN = 72.3) = 0.34$  inches  
 $Q_{100} (CN = 72.3) = 1.14$  inches

- o TIME OF CONCENTRATION

SOUTHERN PORTION OF C-7

6780

6490

$290 / 2600 = 11.1 \%$

$T_c (\text{natural}) = 0.129$

adjustment factor for 25% length modified = 0.83

adjustment factor for 25% impervious = 0.83

$T_c (\text{developed}) = 0.83 \times 0.83 \times 0.129 = 0.089$

ASSUME  $T_c = 0.1$  hours

- o CSMA per inch = 1260





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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

STATION 155+29 FROM THE SOUTH (CONTINUED)

o PEAK STORM RUNOFF

$$5\text{-year} = \frac{61}{640} \times 1260 \times 0.34 = 41 \text{ CFS}$$

$$100\text{-year} = \frac{61}{640} \times 1260 \times 1.14 = 137 \text{ CFS}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

SOUTH SIDE OF MULE DEER DRIVE STATION (155+29)  
EXTENDED FROM THE SOUTH ON THE WEST

○ DRAINAGE AREA

5.06 Acres C = 0.7

$(600 \times 50) / 43560 = 0.69$  acres, C = 0.9

○ Assume  $T_c = 16$  minutes

○ RAINFALL INTENSITIES

15 = 3.5 inches per hour

100 =  $1.9 \times 3.5 = 6.7$  inches per hour

○ PEAK STORM RUNOFF

5-year =  $5.75 \times 0.72 \times 3.5 = 14.5$  CFS

100-year =  $5.75 \times 0.72 \times 6.7 = 27.7$  CFS

\* Provide an inlet on the west side of planned Centennial Boulevard between station 149+20 and 155+29. Assume this inlet intercepts half storm runoff or  $14.5 / 2 = 7.3$  CFS



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CLIENT BRIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

CHUCKWAGON DRIVE STATION ON THE WEST SIDE  
OF PLANNED CENTENNIAL BOULEVARD

o DRAINAGE AREA

5.5 Acres

6.3 Acres

1.8 Acres

8.7 Acres

$$\frac{(1100 \times 50)}{43,560} = 1.26 \text{ Acres}$$

23.6 Acres, CN = 74

o RUNOFF

$Q_5 (CN=74) = 0.40 \text{ inches}$

$Q_{100} (CN=74) = 1.24 \text{ inches}$

o TIME OF CONCENTRATION

6700

6510

$$\frac{190}{2250} = 8.4\%$$

$$T_c = \left( \frac{11.9 \times \left( \frac{2250}{5280} \right)^3}{190} \right)^{0.385} = 0.128 \text{ hours}$$

o CSM per inch = 1220

o PEAK STORM RUNOFF

5-year =  $\frac{23.6}{640} \times 1220 \times 0.40 = 18 \text{ CFS}$

100-year =  $\frac{23.6}{640} \times 1220 \times 1.24 = 55.8 \text{ CFS}$

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 CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD  
 SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLVD

MULE DEER DRIVE (STA 155+29) ON THE EAST SIDE OF PLANNED CENTENNIAL BOULEVARD FROM THE SOUTH

● AREA

STA 155+29

STA 132+35

2,294 L.F

50

114,700 / 43,560 = 2.63 acres

● Time of Concentration

assume overland flow for first 400 feet

$$T_0 = \frac{1.8 (1.1 - C) D^{1/2}}{S^{1/3}}$$

$$= \frac{1.8 (1.1 - 0.9) 400^2}{0.5^{1/3}} = 9.07$$

assume average flow velocity of 4 feet per second

$$T_T = \frac{(2294 - 400)}{4 \times 60} = 9.4 \text{ minutes}$$

$$T_c = T_0 + T_T = 9.07 + 9.4 = 18.47 \text{ min}$$

● RAIN FALL INTENSITIES

$$i_5 = 3.3 \text{ inches per hour}$$

$$i_5 = 1.9 \times i_5 = 6.27 \text{ inches per hour}$$

● PEAK STORM RUNOFF

$$5\text{-year} = 2.63 \times 0.9 \times 3.3 = 7.8 \text{ CFS}$$

$$100\text{-year} = 2.63 \times 0.9 \times 6.27 = 14.8 \text{ CFS}$$

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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLVD

MULE DEER DRIVE (STA 155+29) ON THE EAST SIDE OF PLANNED CENTENNIAL BOULEVARD FROM THE NORTH

● AREA

$$\begin{array}{r} 1200 \\ \times 50 \\ \hline 60,000 / 43560 = 1.4 \text{ acres} \end{array}$$

● TIME OF CONCENTRATION

OVERLAND FLOW

$$T_o = \frac{1.8(1.1 - 0.9) 400^{1/2}}{4^{1/2}} = 4.5 \text{ min}$$

channel-flow

$$T_T = \frac{(1200 - 400)}{5 \times 60} = 2.7 \text{ min}$$

$$T_c = T_o + T_T = 4.5 + 2.7 = 7.2 \text{ min}$$

● RAINFALL INTENSITY

$$i_5 = 5.2 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 9.9 = \dots \text{ inches per hour}$$

● PEAK STORM RUNOFF

$$5\text{-year} = 1.4 \times 0.9 \times 5.2 = 6.6 \text{ cfs}$$

$$100\text{-year} = 1.4 \times 0.9 \times 9.9 = 12.5 \text{ cfs}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE -

ATHERTON WAY STORM SEWER SYSTEM

STA 169+75 16' INLET, QDES = QS = 10 CFS

STA 169+95 MANHOLE for 30" Ø RCP  
30" Ø RCP STUB-OUT TO WEST X L.F.  
36" Ø RCP OUTFALL QDES = QS = 73 CFS.

STA 169+75 to STA 169+95 24" Ø RCP connector

STA 170+15 8' INLET, QDES = QS = 10 CFS.

STA 169+75 to 170+15 24" Ø RCP connector  
QDES = QS = 32 CFS.

STA 170+15 to 176+00 24" Ø RCP connector  
QDES = QS = 16 CFS

STA 173+10 manhole for 24" Ø RCP



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLYD.

DRAINAGE MASTERPLANNING BASIN A-B ON THE WEST SIDE OF PLANNED CENTENNIAL BOULEVARD

o AREA

$$(2400 \times 50) / 43560 = 2.8 \text{ acres}$$

Note: the area to the west drains to channel away from planned Centennial Boulevard  $C = 0.9$

o TIME OF CONCENTRATION

OVERLAND FLOW

$$T_o = \frac{1.8 (1.1 - 0.9) 400^{1/2}}{3.3^{1/3}} = 4.8 \text{ minutes}$$

CHANNEL FLOW TIME

$$T_T = \frac{(2400 - 400)}{5 \times 60} = 6.7 \text{ minutes}$$

$$T_c = T_o + T_T = 4.8 + 6.7 = 11.5 \text{ minutes}$$

o RAINFALL INTENSITY

$$i_5 = 4.25 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 4.25 = 8.1 \text{ inches per hour}$$

o PEAK STORM RUNOFF

$$5\text{-YEAR} = 2.8 \times 0.9 \times 4.25 = 10.71 \text{ CFS}$$

$$100\text{-YEAR} = 2.8 \times 0.9 \times 8.1 = 20.41 \text{ CFS}$$

o MASTERPLANNED STORM SEWER CAPACITY

10' and 8' inlet



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

STATION 169 + 95 ATHERTON WAY

\* Including drainage area tributary to Chuck Wagon Road  
Masterplanning Basin C-6

o D.A. = 82 acres

o CN = 78

o RUNOFF:

$$Q5 (CN=78) = 0.54$$

$$Q100 (CN=78) = 1.50$$

o  $T_c = 0.212$  hours

o CSM per inch = 1055

o Peak storm runoff

$$5\text{-year} = 73 \text{ cfs}$$

$$100\text{-year} = 203 \text{ cfs}$$





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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

SOUTHWEST CORNER OF ATHERTON WAY AND CENTENNIAL BOULEVARD FROM THE SOUTH (STA. 169+95)

o DRAINAGE AREA

$$3.03 \text{ Acres} \quad C = 0.7$$

$$(350 \times 50) / 43,560 = 0.40 \text{ Acres}, \quad C = 0.9$$

$$I.D.A. = 3.43 \text{ acres}$$

o Assume  $T_c = 10$  min

o RAINFALL INTENSITIES

$$i_5 = 4.5 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 4.5 = 8.55 \text{ inches per hour}$$

o PEAK STORM RUNOFF

$$5\text{-year} = 3.43 \times 0.63 \times 4.5 = 9.72 \text{ CFS.}$$

$$100\text{-year} = 3.43 \times 0.63 \times 8.55 = 18.5 \text{ CFS.}$$



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Northwest corner of Atherton Way and Centennial  
Boulevard from the north (station 169+95.)

○ DRAINAGE AREA

17.8 ACRES, CN = 78

4.02 Acres, CN = 98

$(1800 \times 50) / 43,560 = 2.1 \text{ ACRES}$  CN = 98

D.A = 23.9 acres

○ COMPOSITE RUNOFF NUMBER =  $19.50 / 23.9 = 81.5$

○ RUNOFF

Q5 (CN = 81.5) = 0.69 inches

Q100 (CN = 81.5) = 1.75 inches

○ TIME OF CONCENTRATION

6815

6510

$305 / 2650 = 11.5\%$

$T_c = 0.129 \text{ hours}$

○ CSM per inch = 1200

○ PEAK RUNOFF

5-YEAR =  $\frac{23.9}{640} \times 1200 \times 0.69 = 30.9 \text{ CFS}$

100-year =  $\frac{23.9}{640} \times 1200 \times 1.75 = 78.4 \text{ CFS}$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

SOUTH WEST CORNER OF PLANNED COMMERCIAL  
 R-IN, R-OUT ENTRANCE (STATION 176+00)

o DRAINAGE AREA

17.8 acres, CN = 78

$(1200 \times 50) / 43560 = 1.39$  acres, CN = 98

o RUNOFF

$Q_5$  (CN = 79.4) = 0.80 inches

$Q_{100}$  (CN = 79.4) = 1.59 inches

o TIME OF CONCENTRATION

$$\begin{array}{r} 6815 \\ -6550 \\ \hline \end{array}$$

$265 / 2050 = 12.9 \%$

$T_c = 0.101$  hours

o CSM per inch = 1260

o PEAK RUNOFF

5-year =  $\frac{17.8}{640} \times 1260 \times 0.6 = 21.0$  CFS

100-year =  $\frac{17.8}{640} \times 1260 \times 1.59 = 55.7$  CFS

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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BOULEVARD

ATHERTON WAY (STATION 169+95) ON THE EAST SIDE OF PLANNED CENTENNIAL BOULEVARD FROM THE NORTH

● AREA

1800

50

$$90,000 / 43,560 = 2.1 \text{ acres}$$

● Time of Concentration

overland flow

$$T_o = \frac{1.9 (1.1 - 0.9) 400^{1/2}}{3.3^{1/3}} = 4.9 \text{ minutes}$$

channel flow

$$T_f = \frac{1800 - 400}{5 \times 60} = 4.7 \text{ minutes}$$

$$T_c = T_o + T_f = 4.9 + 4.7 = 9.6 \text{ minutes}$$

● RAINFALL INTENSITIES

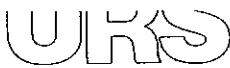
$$i_5 = 4.65 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 4.65 = 8.84 \text{ inches per hour}$$

● PEAK STORM RUNOFF

$$5\text{-year} = 2.1 \times 0.9 \times 4.65 = 8.8 \text{ CFS}$$

$$100\text{-year} = 2.1 \times 0.9 \times 8.84 = 16.7 \text{ CFS}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

SOUTH MOUNTAIN SHADOWS ACCESS ROAD

STORM SEWER IMPROVEMENTS (STATION 188+00)

- STATION 188+00 MANHOLE FOR 36"Ø RCP
  - 30" Ø RCP STUB-OUT TO WEST X 10 LF.
  - QDES = Q5 = 75 CFS
  - 36"Ø RCP OUTFALL LINE X \_\_\_\_\_ LF.
- STATION 188+30 8' INLET
  - 18"Ø RCP X 30 LF
  - QDES = Q5 = 12.7 CFS.

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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD

NORTHWEST CORNER OF PLANNED ACCESS ROAD  
TO MOUNTAIN SHADOWS R-6 AREA (STA 188+00)  
FROM THE NORTH.

o DRAINAGE AREA

$$(1250 \times 50) / 43560 = 1.43 \text{ acres}, \quad C = 0.9$$

$$(1250 \times 100) / 43560 = 2.86 \text{ acres}, \quad C = 0.5$$

$$D.A. = 4.29 \text{ ac} \quad \bar{C} = 0.63$$

o TIME OF CONCENTRATION

$$\begin{array}{r} 6580 \\ -6550 \\ \hline \end{array}$$

$$30 / 1250 = 2.4\%$$

$$T_0 = \frac{1.8(1.1 - 0.9) 400^{1/2}}{2.4^{1/3}} = 5.4 \text{ minutes}$$

$$T_T = \frac{(1250 - 400)}{4 \times 60} = 3.5 \text{ minutes}$$

$$T_c = 5.4 + 3.5 = 8.9 \text{ minutes}$$

o RAINFALL INTENSITIES

$$i_5 = 4.7 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 4.7 = 8.93 \text{ inches per hour}$$

o PEAK STORM RUNOFF

$$5\text{-YEAR} = 4.29 \times 0.63 \times 4.7 = 12.7 \text{ CFS}$$

$$100\text{-YEAR} = 4.29 \times 0.63 \times 8.93 = 24.1 \text{ CFS}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLVD.

SOUTH ACCESS ROAD TO REED RANCH (STA 188+00)  
ON THE EAST SIDE OF PLANNED CENTENNIAL BOULEVARD  
FROM THE NORTH.

● AREA

$$\begin{array}{r} 1300 \\ \times 50 \\ \hline \end{array}$$

$$65000 / 43560 = 1.5 \text{ Acres}$$

● TIME OF CONCENTRATION

OVERLAND FLOW

$$T_o = \frac{1.8(1.1 - 0.9) 400^{\frac{1}{2}}}{2.6^{\frac{1}{3}}} = 5.2 \text{ minutes}$$

Channel Flow

$$T_c = \frac{(1300 - 400)}{5 \times 60} = 3 \text{ minutes}$$

$$T_c = T_o + T_f = 8.2 \text{ minutes}$$

● RAINFALL INTENSITIES

$$i_5 = 4.9 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 4.9 = 9.31 \text{ inches per hour}$$

● PEAK STORM RUNOFF

$$5\text{-year} = 1.5 \times 0.9 \times 4.9 = 6.6 \text{ CFS}$$

$$100\text{-year} = 1.5 \times 0.9 \times 9.31 = 12.6 \text{ CFS}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BOULEVARD

STATION 188+00

planned access road to Mountain Shadows Road on the west and South entrance to Reed Ranch on the east.

o MASTER PLANNING BASINS C-2, C-3 AND SOUTHERN PORTION OF C-4

BASIN	AREA (acres)	CN	CN <sub>XA</sub>	T <sub>c</sub> (HRS)
C-2	75	60	4,500	0.269
C-3	94	60	5,400	0.212
C-4	(93-16.5)	86	6,579	0.208
	245.5		16,479	

o Composite CN =  $16,479 / 245.5 = 67.12$

RUNOFF

Q<sub>5</sub> (CN=67.12) = 0.21

Q<sub>100</sub> (CN=67.12) = 0.86

o FLOW TRAVEL TIME BETWEEN OUTLET OF C-2 AND STATION 188+00

Access Road

6633

6552

$\frac{81}{1300} = 6.3\%$

Average flow velocity = 14 ft/sec

$\frac{1300}{14 \times 60 \times 60} = 0.026$  hours

o Time of Concentration =  $0.269 + 0.026 = 0.295$  hours

o CSM per inch = 930





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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BLVD.

STATION 188+00 (CONTINUED)

o PEAK STORM RUNOFF

$$5\text{-year} = \frac{245.5}{640} \times 930 \times 0.21 = 75 \text{ CFS.}$$

$$100\text{-year} = \frac{245.5}{640} \times 930 \times 0.86 = 307 \text{ CFS.}$$



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

NORTH MOUNTAIN SHADOWS ACCESS ROAD (STA 201+00)  
STORM SEWER SYSTEM

- o STA 201+00 MANHOLE FOR 36"Ø RCP  
27"Ø RCP STUB OUT TO WEST X 10 L.F.  
30"Ø RCP STORM SEWER OUTFALL  
QDES = QS = 37 CFS
- o STA 201+00 to STA 201+30  
24"Ø RCP x 30 L.F.  
QDES = QS = 7.3 CFS.  
24"Ø RCP x 30 L.F. QDES = 12 CFS
- o STA 201+30 6' Inlet QDES = QS = 7.3 CFS  
8' Inlet QDES = QS = 12 CFS



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CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

NORTHWEST CORNER, NORTH MOUNTAIN SHADOWS  
ACCESS ROAD AND PLANNED CENTENNIAL BOULEVARD

○ DRAINAGE AREA

$$1.09 \text{ acres } C = 0.7$$

$$(600 \times 50) / 43,560 = 0.69 \text{ acres, } C = 0.90$$

○ TIME OF CONCENTRATION

$$T_c = \frac{1.8 (1.1 - 0.9) 600^{1/2}}{1.56^{1/3}} = 7.60 \text{ minutes}$$

○ RAINFALL INTENSITIES

$$i_5 = 5.05 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 5.05 = 9.6 \text{ inches per hour}$$

○ PEAK STORM RUNOFF

$$5\text{-YEAR} = 1.8 \times 1.38 \times 5.05 = 12.5 \text{ CFS}$$

$$100\text{-YEAR} = 1.8 \times 1.38 \times 9.6 = 23.9 \text{ CFS}$$

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLYD.

NORTH REED RANCH ACCESS ROAD (STA 201+00) ON THE EAST SIDE OF PLANNED CENTENNIAL BOULEVARD FROM THE NORTH

● AREA

$$\begin{array}{r} 1500 \\ \times 50 \\ \hline 75,000 / 43,560 = 1.72 \text{ acres} \end{array}$$

● TIME OF CONCENTRATION

OVERLAND FLOW

$$T_0 = \frac{1.8(1.1 - 0.9)400^{1/2}}{1.6^{1/3}} = 6.2 \text{ minutes}$$

CHANNEL FLOW

$$\frac{(1500 - 400)}{5 \times 60} = 3.7 \text{ minutes}$$

$$T_c = T_0 + T_T = 6.2 + 3.7 = 9.9 \text{ minutes}$$

● RAINFALL INTENSITIES

$$i_5 = 4.7 \text{ inches per hour}$$

$$i_{100} = 1.9 \times 4.7 = 8.9 \text{ inches per hour}$$

● PEAK STORM RUNOFF

$$5\text{-year} = 1.72 \times 0.9 \times 4.7 = 7.3 \text{ CFS.}$$

$$100\text{-year} = 1.72 \times 0.9 \times 8.9 = 13.8 \text{ CFS}$$

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARDSUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BOULEVARD

Assume all storm runoff (5-year and 100-year) arriving at masterplanning hydrologic design point H-3B to be intercepted by the North Fork of Douglas Creek as implied in the Douglas Creek Drainage Basin Masterplanning study of March 1981.

STATION 201+00 - planned access road to Mountain Shadows to the west and north access road to Reed Ranch to the east (full intersection)

### MASTERPLANNING BASIN C-1

- o D.A. = 87 Acres
- o CN = 68
- o RUNOFF
  - o  $Q_5 = 0.23$  inches
  - o  $Q_{100} = 0.90$  inches
- o  $T_c = 0.192$  hours
- o CSM per inch = 1080
- o Peak storm runoff

$$5\text{-year} = \frac{87}{640} \times 1080 \times 0.23 = 34 \text{ CFS.}$$

$$100\text{-year} = \frac{87}{640} \times 1080 \times 0.90 = 133 \text{ CFS.}$$

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_  
 CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD  
 SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BOULEVARD

STATION 201+00 (continued)

NORTHERN PORTION OF MASTERPLANNING BASIN C4

o D.A. =  $\frac{1200 \times 600}{43,560} = 16.52$  Acres

o Type A soil ((16) chaseville sand; loam)

o Assume 5 to 6 units per acres

o CN = 68

o COMPOSITE CN =  $\frac{87 \times 68 + 16.5 \times 68}{(87 + 16.5)} = 68$

FLOW TRAVEL TIME BETWEEN OUTLET OF MASTER PLANNING BASIN C-1 AND STATION 201+00

MOUNTAIN SHADOWS

$$\begin{array}{r} 6634 \\ 6621 \\ \hline 13/1000 = 1.3\% \end{array}$$

ACCESS ROAD

$$\begin{array}{r} 6621 \\ 6580 \\ \hline 41/1100 = 3.7\% \end{array}$$

ASSUME AVERAGE FLOW VELOCITY = 11 ft/sec

$$\frac{2100}{11 \times 60 \times 60} = \underline{0.053 \text{ hours}}$$

o  $T_c = 0.192 + 0.053 = 0.245$

o CSM per inch = 1000



URS COMPANY

Mailing Address:

Denver, Colorado 80206

3955 East Exposition Avenue • Suite 300 • Denver, Colorado 80209 • 303/744-1861

PAGE 3 OF 3

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - WEST SIDE OF PLANNED CENTENNIAL BOULEVARD

STATION 201+00 (CONTINUED)

o PEAK STORM RUNOFF

$$5\text{-year} = \frac{(87+16.5)}{640} \times 1000 \times 0.23 = 37 \text{ CFS}$$

$$100\text{-year} = \frac{(87+16.5)}{640} \times 1000 \times 0.90 = 145 \text{ CFS}$$

o



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PAGE \_\_\_\_\_ OF \_\_\_\_\_

URS NO. 3073 BY \_\_\_\_\_ DATE 6-2-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE

INTERSECTION OF VINDICATOR DRIVE (STA 215+70±)  
AND PLANNED CENTENNIAL BOULEVARD STORM  
SEWER SYSTEM





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PAGE \_\_\_\_\_ OF \_\_\_\_\_

URS NO. 3073 BY \_\_\_\_\_ DATE 6-2-93 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_  
CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD  
SUBJECT STORM DRAINAGE

NORTHEAST CORNER OF VINDICATOR DRIVE AND  
PLANNED CENTENNIAL BOULEVARD FROM THE NORTH



URS COMPANY

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3955 East Exposition Avenue • Suite 300 • Denver, Colorado 80209 • 303/744-1861

PAGE 1 OF 2

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLVD

EAST PORTION OF DRAINAGE MASTERPLANNING BASIN A-13

o DRAINAGE AREA

$7.25 \text{ Sq In @ } 1" = 400 = 26.7 \text{ acres} \quad C = 0.40$

$(2400' \times 50') / 43560 = 2.8 \text{ acres} \quad C = 0.90$

DA = 29.5 acres

o TIME of Concentration

overland-flow

6910

6764

$146 / 750 = 19.5\%$

channel flow along planned Centennial Boulevard

6764

6685

$79 / 2400 = 3.3\%$

$T_T = \frac{2400}{5 \times 60} = 8 \text{ minutes}$

$T_c = T_0 + T_T = 12.8 + 8 = 20.8 \text{ minutes}$

o RAINFALL INTENSITIES

$15 = 3.1 \text{ inches per hour}$

$1100 = 1.9 \times 3.1 = 5.9 \text{ inches per hour}$

o PEAK STORM RUNOFF

5-year =  $29.5 \times 0.45 \times 3.1 = 41.2 \text{ cfs}$

100-year =  $29.5 \times 0.45 \times 5.9 = 78.3 \text{ cfs}$

o MASTER PLANNED STORM SEWER IMPROVEMENT

8' and 10' inlet  $C = 3.3\%$



URS COMPANY

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PAGE 2 OF 2

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-87 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLVD

EAST PORTION OF DRAINAGE MASTER PLANNING BASIN A-13 (CONTINUED)

- o Estimated storm carry-over at the outlet of Basin A-13 on the east side of planned Centennial Boulevard

$$10' \text{ Inlet at } 3\% \quad Q = 15.4$$

$$8' \text{ Inlet at } 3\% \quad Q = \frac{14.8}{30.2}$$

$$5\text{-year } 41.2 - 30.2 = 11 \text{ cfs Carry over}$$

$$100\text{-year } 78.3 - 30.2 = 48.1 \text{ cfs Carry over}$$

- o Estimated street flow capacity (half section) 60' and 76' arterial at 3% =  $38/2 = 19$  cfs.

- o EQUIVALENT CxA ASSOCIATED WITH ESTIMATED STREET FLOW CARRY OVER BEYOND OUTLET OF BASIN A-13

$$CxA_{5\text{-year}} = \frac{11}{3.1} = 8.46$$

$$CxA_{100\text{-year}} = \frac{48.1}{5.9} = 8.15$$



URS COMPANY

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PAGE \_\_\_\_\_ OF \_\_\_\_\_

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEYARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLVD

EAST PORTION OF DRAINAGE MASTER PLANNING BASIN A-14  
NORTH OF VINDICATOR DRIVE / MOUNTAIN SHADOWS

o AREA

2.45 Sq In at 1"=400 = 9.0 Acres      C = 0.4 (low density res)

(2050 x 50) / 43,560 = 2.4 Acres      C = 0.9

11.4 Acres       $\hat{C} = 0.51$

o TIME OF CONCENTRATION

overland flow

$\frac{6735}{6685} \cdot \frac{50}{220} = 23.0\%$

$T_o = \frac{1.8(1.1 - 0.4) 220^{1/2}}{220^{1/3}} = 3.1 \text{ minutes}$

channel flow along east side of planned Centennial Blvd.

$\frac{6610}{75} / 1000 = 7.5\%$

$T_f = \frac{1000}{5 \times 60} = 3.3 \text{ minutes}$

$T_c = T_o + T_f = 3.1 + 3.3 = 6.4 \text{ minutes}$

o RAINFALL INTENSITIES

15 = 5.4 inches per hour

100 = 1.9 x 5.4 = 10.26 inches per hour

o PEAK STORM RUNOFF

5-YEAR = 11.4 x 0.51 x 5.4 = 31.4 CFS

100-YEAR = 11.4 x 0.51 x 10.26 = 59.7 CFS

\* EXCLUSIVE OF CARRY OVERS FROM A-13



URS COMPANY

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PAGE \_\_\_\_\_ OF \_\_\_\_\_

URS NO. 3073 BY \_\_\_\_\_ DATE 5-28-83 CHECKED BY \_\_\_\_\_ DATE \_\_\_\_\_

CLIENT RIDGE DEVELOPMENT PROJECT CENTENNIAL BOULEVARD

SUBJECT STORM DRAINAGE - EAST SIDE OF PLANNED CENTENNIAL BLYD

EAST PORTION OF DRAINAGE MASTER PLANNING : 1  
BASIN A-14 NORTH OF YINDICATOR DRNE/MOUNTAIN SHADOWS  
(CONTINUED)

- o MASTERPLANNED STORM SEWER IMPROVEMENTS  
2- 8' inlets, S = 7.5% ,  $Q = 13.6 \text{ CFS} \times 2 = 27.2 \text{ CFS}$   
CAP
- o ESTIMATED PARTIAL SUB-BASIN CARRY OVER EXCLUSIVE OF  
CARRY OVER FROM BASIN A-13  
5-year =  $31.4 - 27.2 = 4.2 \text{ CFS}$   
100-year =  $59.7 - 27.2 = 32.5 \text{ CFS}$

Item Three

Request for credits for construction of drainage facilities within Cottonwood Creek Basin in Briarwood Village Filing No. 1 for the Briargate Development Group.

Mr. Mike Mallon represented the item for the developer and stated his disagreement with the staff's recommendation. After discussing the differences the Board heard a motion by Rick Simpson to approve the staff's recommendation for credits in the amount of \$455.50. The motion was seconded by Mr. Weber. Vote was 4-0 in favor of the motion.

X Item Four

Request to establish partial credits within Douglas Creek for facilities built to date in Mountain Shadows for the Ridge Development Co. Mr. Whitehead excused himself from the meeting and Roland Obering represented the item for the developer.

Mr. Obering stated his objections to the staff's recommendation. The City Engineer stated that one of the items listed in the staff's recommendation could be withdrawn and given credit for, in the amount of \$975.00; this could be added to the request.

Other items of disagreement were discussed and after considerable discussion on those, Mr. Weber moved to approve the staff's recommendation with the exception that the developer be allowed credit for barricades at \$222.50, general overhead and profit for \$266.50, pipe patching for \$975.00; to allow no more than 10% for engineering costs; and that the 1982 fees for subdivisions currently platted be deducted from the total credit allowed as adjusted. The motion was seconded by Mr. Simpson. Vote was 3-0 in favor of the motion.

The revised total credit as adjusted will be \$239,827.48.

Item Ten was heard next as Mr. Whitehead had a conflict and remained excused from the meeting.

Item Ten

Request of Stewart R. Scott, Inc. to appeal City Engineer's position regarding reimbursement for a portion of Monument Creek within the 2400 Wood Avenue Subdivision.

Mr. Obering, representing Stewart R. Scott, presented his position that Monument Creek should be eligible for reimbursement from the Miscellaneous Basin Fund if his client was being asked to construct a portion of Monument Creek.

continued ...

INTER-OFFICE MEMORANDUM  
ENGINEERING

To: Bev

From: Gary

Date: 4-1-82

- For: \_\_\_\_\_ Response  
\_\_\_\_\_ Your project file  
\_\_\_\_\_ Your information  
\_\_\_\_\_ Your signature and return  
\_\_\_\_\_ Urgent  
\_\_\_\_\_ Report back  
\_\_\_\_\_ Route and return

Review and prepare for  
Drainage Board Agenda of  
4/15/82.

Note that the requested  
engineering fee of \$45,869  
exceeds the 10% allowed  
by City Ordinance.

Signature \_\_\_\_\_

APRIL 15, 1982 *Approved \$ 239,827.48*

DRAINAGE BOARD ITEM

<sup>4</sup>  
ITEM #3 Request to ESTABLISH CREDITS for partial construction of drainage facilities within DOUGLAS CREEK BASIN in Mountain Shadows Filing No. 1 Ridge Development Company - Developer.

Request as submitted by the developer is as follows:

Storm Sewer System (per low bid)	\$ 45,451.05
Box Culvert - Wilson Rd. \$32,762.52 (per low bid)	
24" Water Anchors @ Box 1,930.00	34,692.52
Park Channel, Box Transitions (per low bid)	131,134.08
60" Concrete Pipe	10,322.33
Total	<u>\$221,599.98</u> ✓
Additional Engineering (16.9%) <i>LWH</i>	37,606.89
Additional Engineering (3.7%) <i>JN</i>	8,262.50
TOTAL CREDITS REQUESTED	<u>\$267,469.37</u>

Staff Recommendation:

Storm Sewer System	\$ 45,451.05
Box Culvert & Anchors	34,692.52 ✗
Barricades (not reimbursable)	- 222.50 ✗
General overhead @ 15%	- 266.50 ✗
Park Channel, Box Transitions	131,134.08
Temporary Outfall Channel	- 1,400.00
150 cu.yds 18" - 24" Riprap (Below Box Culvert @ Wilson Road)	- 2,175.00
50 l.f. reinforced Patch 60" R.C.P	- 975.00 ✗
60" Concrete Pipe	10,322.33
	<u>\$216,560.98</u>
Additional 10% Engineering	21,656.09
	<u>238,217.07</u>

*( 218024.98 )*  
*21802.50*  
*239,827.48*

Drainage fees will be calculated at time of final completion of drainage facilities.

5087.

~~216,560.98~~  
~~1,400~~  
~~215,160.98~~

~~216,560.98~~  
~~8,575~~  
~~220,135.98~~  
~~220,136.0~~  
~~242,149.58~~

~~220,135.98~~  
~~22.50~~  
~~220,358.48~~  
~~266.50~~  
~~220,624.98~~  
~~975~~  
~~220,599.98~~

~~216,560.98~~  
~~222.50~~  
~~266.50~~  
~~975~~  
~~229,025.98~~

~~225,063.98~~

~~219.4~~

~~218,024.98~~  
~~21,802.50~~  
~~239,827.48~~  
~~21,948.90~~  
~~21~~



# LEIGH WHITEHEAD & ASSOCIATES

CONSULTING ENGINEERS AND SURVEYORS  
 5 WEST LAS VEGAS • PHONE 636-5179

COLORADO SPRINGS, COLORADO 80903

March 29, 1982

City of Colorado Springs  
 Department of Public Works  
 Engineering Division  
 30 S. Nevada Avenue  
 Colorado Springs, CO 80903

Re: Mountain Shadows Filing No. 1  
 Drainage Construction Costs  
 Douglas Creek Basin

LW Project No. 80089-DRG

ATTN: Bob Martin  
 Public Works Administrator

Gentlemen:

Ridge Development Company, Ltd., owner/developer of record of Mountain Shadows Filing No. 1 does hereby submit to the City of Colorado Springs the following summary of costs incurred for the construction of certain approved drainage facilities within the Douglas Creek Drainage Basin. Reference the following approved drainage reports and plans which included or affected facilities in this summary:

1. Mountain Shadows Filing No. 1 and Addenda No. 1 and No. 2 (September 12, 1980, October 8, 1980, June 24, 1981).
2. Mountain Shadows Filing No. 2 and Supplement (November 5, 1980, April 3, 1981).
3. Garden of the Gods Road Realignment (July 28, 1981).

The facilities in place and approved have been summarized in a letter to Public Works dated March 19, 1982, a copy of which is attached. Generally, the facilities in place to date will convey storm runoff to the East side of Wilson Road with the exception of the proposed diversion in Filing No. 1. The diversion will be constructed as part of the ROLM development. The purpose of this letter is to document and have on record with the City an amount expended to date on drainage facilities prior to April 1, 1982.

The following is a summary of the Contractor, amounts for various work, and a general description of the work. Copies of all invoices and checks are attached as are the bids received for the work.

<u>CONTRACTOR</u>	<u>DESCRIPTION</u>	<u>AMOUNT</u>
Pate Const. Co. (2 invoices)	Storm Sewer Systems	\$ 45,451.05
T.L. Printz Const. (3 invoices)	Box Culvert-Wilson Rd.	\$ 32,762.52
Frazer Const. Co. (1 invoice)	24" Water Anchors @ Box	\$ 1,930.00
M & M Cement Contractors, Inc. (2 invoices)	Park Channel, Box Transitions 60" Pipe	\$131,134.08
TOTAL DRAINAGE CONSTRUCTION COSTS.....		\$221,599.98

*Calderon Consultants*  
**RECEIVED**  
 PUBLIC WORKS  
 COLORADO SPRINGS, COLO.

MAR 30 1982

AM 7 8 9 10 11 12 1 2 3 4 5 6 PM

Letter: City of Colorado Springs  
Re: Mountain Shadoes Filing No. 1  
Drainage Construction Costs  
Douglas Creek Basin  
LW Project No.

March 29, 1982  
Page 2

The engineering fees relative to the preparation of the approved Filing No. 1 drainage report and plans and addenda, and final design of facilities and inspection of the constructed facilities incurred by the developer are \$45,869.39 all of which have been paid to date. This figure includes \$37,606.89 in fees paid to Leigh Whitehead & Associates and \$ 8,262.50 paid to Jerry Novak who was on the developer's staff and lending technical assistance throughout the project. The fees to date represent engineering work on all of the facilities included in the approved report and plan, some of which will be constructed at a later date.

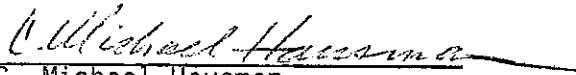
The total costs incurred to date are <sup>267,469.37</sup>~~\$267,429.37~~. All in place facilities have been installed in accordance with the approved drainage report and plans, final design plans, and the City of Colorado Springs Standards and Specifications. The facilities have been inspected and accepted by the Consultant and the City of Colorado Springs subject to the one (1) year warranty period. This correspondence will document bona fide drainage costs to date for Mountain Shadows Filing No. 1.

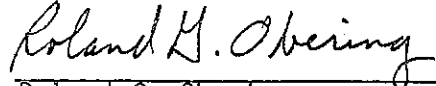
Copies of paid invoices, project bids, and other pertinent records are attached. Additional information will be made available at the City's request. If there are any questions please contact either of the undersigned.

Very truly yours,

RIDGE DEVELOPMENT CO. LTD.

LEIGH WHITEHEAD & ASSOCIATES

  
C. Michael Hausman  
President

  
Roland G. Obering  
P.E. & L.S.

RGO/pg  
Enclosures

Ridge Development Co., Ltd.  
P.O. BOX 7711  
COLORADO SPRINGS, CO 80933

(303) 594-0227

MEMO TO: Roland Obering  
Leigh Whitehead Engineers

FROM: Mike Hausman  
Ridge Development Co., Ltd.

DATE: March 30, 1982

RE: Engineering services of Jerry Novak  
regarding the storm drainage reports  
and the subsequent construction of  
storm drainage improvements for  
Mountain Shadows Filing No. 1

While Jerry Novak was employed by Ridge Development, he performed many engineering functions, including an extensive amount of research and calculation work for the drainage reports which your firm prepared for the City, and also an extensive amount of supervisory work as well as inspections of the actual construction of these drainage facilities.

We have logged his time for these engineering services, and have billed the hours at \$25.00 per hour. The following figures are a summary for these services (we have a detailed breakdown by date, time, and service performed in our files if you need further documentation):

Engineering services relating to the preparation of the Drainage Reports:	\$ 3,650.00
Engineering services relating to the construction of Drainage improvements:	<u>\$ 4,612.50</u>
TOTAL .....	\$ 8,262.50

MH/pg

*Mike Hausman*



CITY OF COLORADO SPRINGS

The "America the Beautiful" City

DEPARTMENT OF PUBLIC WORKS CITY ENGINEERING INSPECTIONS (303) 578-6782

105 WEST COSTILLA P.O. BOX 1575  
COLORADO SPRINGS, COLORADO 80901

March 22, 1982

Ridge Development Company  
P. O. Box 7711  
Colorado Springs, CO 80933

ATTN: C. MICHAEL HAUSMAN

RE: MOUNTAIN SHADOWS PARK DRAINAGE CHANNEL FILING #1

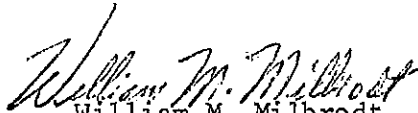
Dear Mr. Hausman,

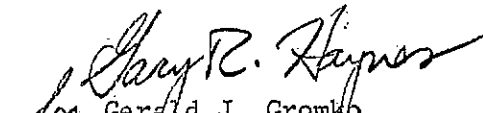
PRELIMINARY INSPECTION

- A. Box Culvert on Wilson Road - O.K.
- B. Two each four foot D.10.R's - O.K.
- C. Concrete lined channel - Good
- D. Headwall between the concrete channel and the 60 inch R.C.P.-Good
- E. 60 inch R.C.P. - Good
- F. One each six foot D.10.R. - O.K.

The one year warranty may begin as of March 19, 1982. If you have any questions, please contact Bill Milbrodt at 578-6782.

Sincerely,

  
William M. Milbrodt  
Engineering Inspector

  
Gerald J. Gromko  
City Engineer

WMM:GJG:sp

cc: Bev Dustin-Land Development  
John Sentena-Chief of Inspections  
Dick Ernster-Asst. Street Supt.

Ridge Development Co., Ltd.  
P.O. BOX 7711  
COLORADO SPRINGS, CO 80933

(303) 594-0227

JOHN  
INSP  
FILE

March 12, 1982

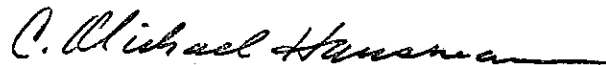
Mr. Gerald Gromko,  
City Engineer  
Public Works Division  
105 West Costilla, P.O. Box 1575  
Colorado Springs, CO. 80901

Dear Mr. Gromko:

The work on Mountain Shadows Park Drainage Channel in Filing #1 will be completed during the work week of March 15th, 1982. We would like to have the City of Colorado Springs inspect the above mentioned project on Friday March 19, 1982.

Please let us know when on the 19th the inspection will take place as we would like to make our engineers aware of the time for the inspection.

Sincerely,



C. Michael Hausman

CMH:sn

cc: Leigh Whitehead and Associates  
cc: DeWitt Miller  
cc: Gil Martnez, M & M Cement

RECEIVED

MAR 15 1982  
10:15  
PUBLIC WORKS  
ENGINEERING



NATIONAL ASSOCIATION OF HOME BUILDERS

# LEIGH WHITEHEAD & ASSOCIATES

CONSULTING ENGINEERS AND SURVEYORS

5 WEST LAS VEGAS • PHONE 636-5179

COLORADO SPRINGS, COLORADO 80903

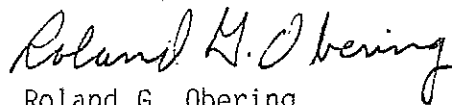
March 29, 1982

Re: Engineering Fees - Drainage  
Mountain Shadows Filing No. 1  
LW Project No. 80089-DRG

To Whom It May Concern:

The firm of Leigh Whitehead and Associates, Consultant to the developer of Mountain Shadows Filing No. 1 has accumulated engineering fees relative to preparation of drainage reports and plans, final design plans, construction staking and inspection and contract administration for the subdivision. These fees total \$37,606.89. All fees except the March invoice have been paid in full by Ridge Development Company, Ltd. All fees were related directly to the approved drainage facilities. Approximately 45% of the facilities have been constructed at this time.

Very truly yours,  
LEIGH WHITEHEAD & ASSOCIATES



Roland G. Obering  
P.E. & L.S.

RG0/pg

COPY

# LEIGH WHITEHEAD & ASSOCIATES

CONSULTING ENGINEERS AND SURVEYORS  
5 WEST LAS VEGAS • PHONE 638-5179

COLORADO SPRINGS, COLORADO 80903

March 19, 1982

City of Colorado Springs  
Department of Public Works  
Engineering Division-Gary Haynes  
P.O. Box 1575  
Colorado Springs, CO 80901

Re: Mountain Shadows  
Filing No. 1  
Drainage Facility Construction  
LW Project No. 80089-Drg.

Gentlemen:

The construction of certain portions of the Drainage Facilities required for Mountain Shadows Filing No. 1 has been completed and inspected by the Engineer. The purpose of this letter is to summarize the completed construction and established dates for beginning the required one (1) year guarantee period.

The facilities that are currently in place and included are:

- 1) Inlets and culverts within the 100 foot Wilson Road right-of-way at its intersection with future Garden of the Gods Road realigned and Mountain Shadows Road. (Pate Construction Co., Inc. - Completed and Accepted September 30, 1981.)
- 2) Box Culvert and attached Inlets at Mountain Shadows Road and Wilson Road. (T.L. Printz Construction, Co. - Completed and Accepted November 24, 1981.)
- 3) Park Channel, Box Culvert Transitions, Twin 60" RCP Extensions and Inlet. (M & M Cement Contractors, Inc. - Completed and accepted March 19, 1982.)

The facilities that are in place will convey runoff from Filing No. 1 to the Southeast side of 100 foot Wilson Road right-of-way. This does not include a proposed diversion system which will be required between the Filing No. 1 and Filing No. 2 boundary. The diversion system construction is being planned as part of the development of Filing No. 1 by the ROLM Corporation.

All facilities that are now in place have been constructed in accordance with the approved Drainage Report and Plan and the detailed plans and specifications as prepared by Leigh Whitehead and Associates. It is recommended that the facilities be accepted by the City of Colorado Springs as part of their drainage system subject to the required one (1) year guarantee periods for the various segments of construction included in this summary.

The field inspection records of the Engineer together with quality control test results are all available at the City's request. A summary of all construction costs will be submitted under separate cover establishing exact amounts for

Letter: City of Colorado Springs  
Department of Public Works  
Engineering Division  
Re: Mountain Shadows Filing No. 1  
Drainage Facility Construction

March 19, 1982  
Page 2

credit against drainage fees within the Douglas Creek Drainage Basin.

If there are any questions concerning acceptance of these facilities, please contact the undersigned.

Very truly yours,  
LEIGH WHITEHEAD & ASSOCIATES

*Roland G. Obering*

Roland G. Obering  
P.E. & L.S.

RG0/pg

cc: Ridge Development Co., Ltd.  
ATTN: Mike Hausman



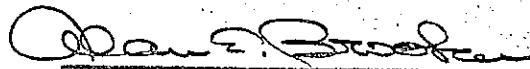
BIDDING SCHEDULE FOR MOUNTAIN SHADOWS

PARK DRAINAGE CHANNEL

ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1.	39,556 S.F.	Complete reinforced concrete channel including 6 berms	\$ 3.25	\$ 128,557.00
2.	3 each	50 L.F. Channel transition	\$ 10,870.00	\$ 32,610.00
3.	288 L.F.	60" R.C.P. Install only	\$ 21.00	\$ 6,048.00
4.	1 each	17' X 9' X 12" Head Wall	\$ 1,700.00	\$ 1,700.00
5.	1 each	6' Catch Basin	\$ 1,615.00	\$ 1,615.00
6.	12 L.F.	18" R.C.P.	\$ 22.00	\$ 264.00
7.	1 each	Temporary Outfall Channel	\$ 3,600.00	\$ 3,600.00
8.	150 cubic yards	18" - 24" Riprap	\$ 30.00	\$ 4,500.00
9.	50 L.F.	Reinforced Patch - 60" R.C.P.	\$ 22.00	\$ 1,100.00
Total				\$ 180,194.00

Dated this 30th day of November, 1981.

Mel-Ro Construction, Inc.  
Company

  
By Alan E. Brooker

President  
Title

The accompanying list of instructions and clarifications are acknowledged as a part of the construction specifications and requirements.

Addendum Number 1 acknowledged.

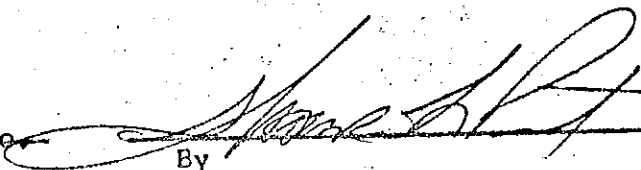
BIDDING SCHEDULE FOR MOUNTAIN SHADOWS  
PARK DRAINAGE CHANNEL

ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1.	39,556 S.F.	Complete reinforced concrete channel including 6 berms	\$2.91/S.F.	\$115,046.00
2.	3 each	50 L.F. Channel transition	\$12,400/each	37,200.00
3.	288 L.F.	60" R.C.P. Install only	\$23 /L.F.	6,624.00
4.	1 each	17' X 9' X 12" Head Wall		4,200.00
5.	1 each	6' Catch Basin		1,983.00
6.	12 L.F.	18" R.C.P.	\$25 /L.F.	300.00
7.	1 each	Temporary Outfall Channel		2,500.00
8.	150 cubic yards	18" - 24" Riprap	\$26.66/C.Y.	4,000.00
9.	50 L.F.	Reinforced Patch - 60" R.C.P.	\$26.50/L.F.	1,325.00
Total				\$173,178.00

Dated this 30th day of November, 1981.

~~T. L. Printz Construction Co.~~  
Company

By

  
President  
Title

The accompanying list of instructions and clarifications are acknowledged as a part of the construction specifications and requirements.

# CARDER CONCRETE PRODUCTS CO.

8311 WEST CARDER COURT  
LITTLETON, COLORADO 80125  
PHONE 794-6303

## QUOTATION

Date November 11, 1981

Ridge Development  
P. O. Box 7711  
Colorado Springs, CO 80933

Project

Letting Date

Attention: Jerry Novak

RE: RCP seconds for Storm Drain

We are pleased to quote the following reinforced concrete #2 R.C.P.

288 LF 60" #2 R.C.P. CL A-25 (3) 34.00 LF

319 FT 1 1/4" Kent Seal .66 FT

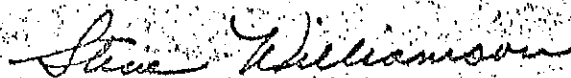
Weight of 60" R.C.P. 1,473 per ft X 16' = 23,568 LB

Price quoted delivered to jobsite as close to the line of ditch as our trucks can travel under their own power.

Pipe in 16' lengths to be unloaded by contractor.

TERMS: NET THIRTY (30) DAYS

CARDER CONCRETE PRODUCTS CO.



Steve Williamson  
576-9266

Accepted this 17 day of NOV.

1981, subject to and including all of said terms and conditions on the reverse side.

Purchase Order No. \_\_\_\_\_

RIDGE DEV. CO. LTD. Buyer

By Jerry Novak

CITY OF COLORADO SPRINGS

DEPARTMENT OF PUBLIC WORKS • ADMINISTRATION (303) 471-6660 • ENGINEERING (303) 471-6606  
108 WEST COSTILLA • P.O. BOX 1575  
COLORADO SPRINGS, COLORADO 80901

November 18, 1981

Ridge Development Company  
P.O. Box 7711  
Colorado Springs, CO 80933

Attn: Mr. Jerry Novak

Re: RCP Seconds for Mountain Shadows Development

Dear Jerry:

After review of your request to use RCP seconds in the park drainage system adjacent to Mountain Shadows Road, the City Engineering Division has decided to allow the use of same in accordance with the ASTM C-76 requirements. We further stipulate the following items:

1. We require the certification of a registered professional engineer concerning the soundness of the design and certification that the installation was in accordance with City Specifications.
2. The pipe will be used only in the park at the locations shown on your design drawings.
3. The joints will be grouted and imperfections patched in accordance with the specifications presented by your engineer.

We are allowing the use of this pipe material for this installation only, and this permission is not to be used for any other installations within the City of Colorado Springs. We feel that the special circumstances presented on this project were justification for this special consideration.

Please keep us informed of the progress of this installation so that we can have an Inspector present.

By carbon copy of this letter to your Engineer, Roland Obering of Leigh Whitehead & Associates, Inc., I am asking you to investigate the use of epoxy grout for better bonding under paragraph R of your specifications.

Sincerely,

*Gerald J. Gromko*  
Gerald J. Gromko  
City Engineer

cc: DeWitt Miller, Director of Public Works  
Gary Haynes, Assistant City Engineer  
Roland Obering, Leigh Whitehead & Associates, Inc.

*60" Concrete pipe  
per Candel's Bill # 10, 322.33*

*D.R.H.*

# CARDER CONCRETE PRODUCTS COMPANY

8311 WEST CARDER COURT - (NORTHWEST DOUGLAS COUNTY)  
LITTLETON, COLORADO 80125 (303) 794-6303

DATE
1/30/82
CUST. ORDER NO.
0008735

RIDGE DEVELOPMENT CO.  
P.O. BOX 7711  
COLORADO SPRINGS, CO 80933

*Chill*  
*1/22*  
*1-13-82*  
*LD*

PAGE 1

DATE	INVOICE NUMBER	ITEM DESCRIPTION	AMOUNT	
01/25/82	010450	INVOICE	10,322.33	
FINANCE CHARGE IS		COMPUTED BY A PERIODIC RATE OF 1.50 % PER		
MONTH WHICH IS AN		ANNUAL PERCENTAGE RATE OF 18.00 % APPLIED		
TO THE PREVIOUS BALANCE AFTER DEDUCTING CURRENT MONTHS		PAYMENTS AND CREDITS APPEARING ON THIS STATEMENT. MINIMUM		
FINANCE CHARGE IS 0.50				
CURRENT	30 DAYS PAST DUE	60 DAYS PAST DUE	90 DAYS PAST DUE	TOTAL DUE
10,322.33	0.00	0.00	0.00	10,322.33

50017-1

M & M Cement Contractors, Inc.  
 1195 Valley Street  
 COLORADO SPRINGS, COLORADO 80915

Phone 596-7159

Ridge Development Co., LTD  
 PO Box 7711  
 Colo. Spgs., Colo. 80933

REVISED BILLING

DATE	January 22, 1982
NUMBER	Mountain Shadow Channel

TERMS:

PLEASE DETACH AND RETURN WITH YOUR REMITTANCE

DATE	CHARGES AND CREDITS	BALANCE	
	BALANCE FORWARD		
	Rebar	\$ 10,705	36
	Concrete placed	929	25
	60' RCP Installed (90%)	5,400	00
	Excavation (channel)	9,000	00
		<u>\$ 26,034</u>	<u>61</u>

M & M CEMENT CONTRACTORS, INC.

*Thank You*

PAY LAST AMOUNT  
 IN THIS COLUMN

M & M Cement Contractors, Inc.  
 1195 Valley Street 5530 E. Pike Peak  
 COLORADO SPRINGS, COLORADO 80915 80916

DATE	March 23, 1982
NUMBER	Mtn. Shadow Drainage

Phone 596-7159

Ridge Development  
 PO ~~KXXXX~~ 7711  
 Colo. Spgs., Colo. 80933

TERMS:

PLEASE DETACH AND RETURN WITH YOUR REMITTANCE

DATE	CHARGES AND CREDITS	BALANCE	
	BALANCE FORWARD		
	Final Footage on Channel		
Item #1.	Reinforced Channel including berms: 39,890.2 SF @ \$2.58 SF	\$102,916	72
2.	50 LF Trans. 3 each @ \$4817.44	14,452	32
3.	60" RCP Installed 288 LF @ \$20.83 LF	5,999.	04
4.	Headwall 1 each @ \$1400.00 each	1,400	00
5.	6' Catch Basin 1 each @ \$1600.00 each	1,600	00
6.	18" RCP 12 LF @ \$18.00 LF	216	00
7.	Tem. Outfall Channel 1 each @ \$1,400 00	1,400	00
8.	18" -24" Riprap 150 CY @ \$14.50 SY	2,175	00
9.	Reinforced Patch-60' RCP 50 LF @ \$19.50 LF	975	00
		\$131,134	08
	Less previous payment	(26,034	61)
		\$105,099	47

M & M CEMENT CONTRACTORS, INC.

*Thank You*

PAY LAST AMOUNT  
 IN THIS COLUMN

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Materials for drainage channel Mountain Shadows Filing #1  
Labor for drainage channel Mountain Shadows Filing #1:  
Concrete placed \$ 929.25  
60' RCP installed 5400.00  
\$6329.25

Nº 1925

REPUBLIC NATIONAL BANK 42-56  
PUEBLO, COLORADO 1070

DATE Feb 3 1982

PAY THE SUM 6329 DOLLARS 25 CTS Dollars \$ 6329.25

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

TO THE ORDER OF M & M Cement Contractors, Inc.  
1195 Valley St  
Colorado Springs Colorado 80915

RIDGE DEVELOPMENT CO., LTD.

*C. Michael Hansen*

⑆107000563⑆ 08 712 20⑈

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Pipe Delivered to Mountain Shadows  
Colorado Springs, Colorado

\$10,322.33

Nº 1922

REPUBLIC NATIONAL BANK 42-56  
PUEBLO, COLORADO 1070

DATE Feb 3 1982

PAY THE SUM 10322 DOLLARS 33 CTS Dollars \$ 10,322.33

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

TO THE ORDER OF Carder Concrete Products  
8511 West Carder Court  
Littleton Colorado 80125

RIDGE DEVELOPMENT CO., LTD.

*C. Michael Hansen*

⑆107000563⑆ 08 712 20⑈

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Work performed and materials for the drainage channel  
in Mountain Shadows Filing #1, Colorado Springs Colorado

\$10,705.36

Nº 1924

REPUBLIC NATIONAL BANK 42-56  
PUEBLO, COLORADO 1070

DATE Feb 3 1982

PAY THE SUM 10705 DOLLARS 36 CTS Dollars \$ 10,705.36

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

TO THE ORDER OF M & M Cement Contractors, Inc. and  
Transit Mix Concrete Co.  
1195 Valley Street  
Colorado Springs Colorado 80915

RIDGE DEVELOPMENT CO., LTD.

*C. Michael Hansen*

⑆107000563⑆ 08 712 20⑈

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Work performed and materials for the drainage channel  
in Mountain Shadows Filing #1, Colorado Springs, Colo.

\$9,000.00

Nº 1923

REPUBLIC NATIONAL BANK 42-56  
PUEBLO, COLORADO 1070

DATE Feb 3 1982

PAY THE SUM 9000 DOLLARS 00 CTS Dollars \$ 9,000.00

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

TO THE ORDER OF M & M Cement Contractors, Inc. and  
Harr Ditching Company  
1195 Valley Street  
Colorado Springs Colorado 80915

RIDGE DEVELOPMENT CO., LTD.



DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Materials and Labor for park drainage channel  
Mountain Shadows - Filing #1

No 1971

REPUBLIC NATIONAL BANK  
PUEBLO, COLORADO

42-56  
1070

DATE Mar, 29, 1982

THE SUM 48531.00

Dollars \$ 48,531.21

PAY

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

RIDGE DEVELOPMENT CO., LTD.

TO THE ORDER OF

M & M Cement Contractors Inc,  
5536 East Pikes Peak  
Colorado Springs, CO 80916

BY *C. Michael Hansen*

⑆107000563⑆ 08 712 21⑈

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

M & M Cement Contractors Inc, & Transit Mix Concrete Co,  
"Materials & Labor for park drainage Channel"  
Mountain Shadows - Filing #1

No 1972

REPUBLIC NATIONAL BANK  
PUEBLO, COLORADO

42-56  
1070

DATE Mar, 29, 1982

THE SUM 49468.26

Dollars \$ 49,468.26

PAY

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

RIDGE DEVELOPMENT CO., LTD.

TO THE ORDER OF

M & M Cement Contractors Inc, and  
Transit Mix Concrete Co,  
5536 East Pikes Peak  
Colorado Springs, CO. 80916

BY *C. Michael Hansen*

⑆107000563⑆ 08 712 21⑈

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

"Materials and Labor for park drainage Channel"  
Mountain Shadows - Filing #1

No 1973

REPUBLIC NATIONAL BANK  
PUEBLO, COLORADO

42-56  
1070

DATE Mar, 29, 1982

THE SUM 7100.00

Dollars \$ 7,100.00

PAY

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

RIDGE DEVELOPMENT CO., LTD.

TO THE ORDER OF

M & M Cement Contractors Inc, and  
Murr Ditching Company  
5536 East Pikes Peak  
Colorado Springs, CO 80916

BY *C. Michael Hansen*

⑆107000563⑆ 08 712 21⑈

MTN SHADOWS FILING No. 1  
CITY COPY

- 1) BIDS
- 2) INVOICES
- 3) CHECK STUBS.

M & M CEMENT CONTR.  
PARK CHAN, TRANSITIONS,  
AND 60" RLP.

OK ←

BIDDING SCHEDULE FOR MOUNTAIN SHADOWS

PARK DRAINAGE CHANNEL

ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1.	39,556 S.F.	Complete reinforced concrete channel including 6 berms	2.58	102,054.48
2.	3 each	50 L.F. Channel transition	4,817.44	14,452.32
3.	288 L.F.	60" R.C.P. Install only	20.83	5,999.04
4.	1 each	17' X 9' X 12" Head Wall	1,400.00	1,400.00
5.	1 each	6' Catch Basin	1,600.00	1,600.00
6.	12 L.F.	18" R.C.P.	18.00	216.00
7.	1 each	Temporary Outfall Channel	1,400.00	1,400.00
8.	150 cubic yards	18" - 24" Riprap	14.50	2,175.00
9.	50 L.F.	Reinforced Patch - 60" R.C.P.	19.50	975.00
Total				130,271.84

Dated this 30<sup>TH</sup> day of NOVEMBER, 1981.

M & M CEMENT CONTRACTORS      Gilbert Martinez      President  
 Company                                      By                                      Title

The accompanying list of instructions and clarifications are acknowledged as a part of the construction specifications and requirements.

MTN. SHAD. FILE #1

CITY COPY.

- 1) BIDS
- 2) INVOICES
- 3) CHECK STUBS.

PATE'S WORK

STORM SEWER SYSTEMS.

OK ←

BIDDING SCHEDULE FOR WILSON ROAD - MOUNTAIN SHADOWS  
DRAINAGE IMPROVEMENTS

Schedule A				
ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1	70 L.F.	60" R.C.P.	\$ 103.37	\$ 7,235.90
2	2 each	60" X 45° R.C.P. bends	178.91	357.82
3	40 L.F.	42" R.C.P.	65.88	2,635.20
4	93 L.F.	36" R.C.P.	40.44	3,760.92
5	88 L.F.	30" R.C.P.	29.92	2,632.96
6	252 L.F.	27" R.C.P.	28.51	7,184.52
7	143 L.F.	21" R.C.P.	22.29	3,187.47
8	130 L.F.	18" R.C.P.	19.76	2,568.80
9	1 each	27" to 18" R.C.P. Reducer	90.50	90.50
10	3 each	8' Catch Basins	1,870.00	5,610.00
11	2 each	6' Catch Basins	1,650.00	3,300.00
12	3 each	4' Catch Basins	1,210.00	3,630.00
13	1 each	Junction Box		5000
			Total	\$ 42,194.09
Schedule B				
14	1 each	Double 5' x 6' R.C.B.C.		
14 ALT.	1 each	Single 5' x 12' R.C.B.C.		

JIC  
SW

Dated this 9<sup>th</sup> day of SEPTEMBER, 1981.

PATE CONSTRUCTION Co., INC  
Company

By Pat E. Pate Title

BID

BIDDING SCHEDULE FOR WILSON ROAD - MOUNTAIN SHADOWS  
DRAINAGE IMPROVEMENTS

Schedule A				
ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1	70 L.F.	60" R.C.P.	121.95	8,536.50
2	2 each	60" X 45° R.C.P. bends	637.88	1,275.76
3	40 L.F.	42" R.C.P.	59.50	2,380.00
4	93 L.F.	36" R.C.P.	41.95	3,901.35
5	88 L.F.	30" R.C.P.	31.95	2,811.60
6	252 L.F.	27" R.C.P.	29.50	7,434.00
7	143 L.F.	21" R.C.P.	24.95	3,567.85
8	130 L.F.	18" R.C.P.	21.95	2,853.50
9	1 each	27" to 18" R.C.P. Reducer	292.45	292.45
10	3 each	8' Catch Basins	2,160.00	6,480.00
11	2 each	6' Catch Basins	1,890.00	3,780.00
12	3 each	4' Catch Basins	1,350.00	4,050.00
13	1 each	Junction Box	Sub-Total	\$47,363.01 <i>of</i>
			Total	
Schedule B				
14	1 each	Double 5' x 6' R.C.B.C.		

Dated this 9th day of September, 1981.

BABCOCK & BABCOCK CONSTRUCTION, INC.  
Company

By

Title

*Eric Babcock*  
President

BIDDING SCHEDULE FOR WILSON ROAD - MOUNTAIN SHADOWS  
DRAINAGE IMPROVEMENTS

Schedule A				
ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1	70 L.F.	60" R.C.P.	131 <sup>00</sup>	9170 <sup>00</sup>
2	2 each	60" X 45° R.C.P. bends	750 <sup>00</sup>	1500 <sup>00</sup>
3	40 L.F.	42" R.C.P.	76 <sup>00</sup>	3040 <sup>00</sup>
4	93 L.F.	36" R.C.P.	53 <sup>00</sup>	4929 <sup>00</sup>
5	88 L.F.	30" R.C.P.	47 <sup>00</sup>	4136 <sup>00</sup>
6	252 L.F.	27" R.C.P.	39 <sup>00</sup>	9828 <sup>00</sup>
7	143 L.F.	21" R.C.P.	34 <sup>00</sup>	4862 <sup>00</sup>
8	130 L.F.	18" R.C.P.	26 <sup>40</sup>	3432 <sup>00</sup>
9	1 each	27" to 18" R.C.P. Reducer	400 <sup>00</sup>	400 <sup>00</sup>
10	3 each	8' Catch Basins	2000 <sup>00</sup>	6000 <sup>00</sup>
11	2 each	6' Catch Basins	1740 <sup>00</sup>	3480 <sup>00</sup>
12	3 each	4' Catch Basins	1400 <sup>00</sup>	4200 <sup>00</sup>
13	1 each	Junction Box	NO Bid	—
			Total	54,977
Schedule B				
14	1 each	Double 5' x 6' R.C.B.C.	NO Bid	—

Dated this 9 day of SEP, 1981.

Schmidt-King Construction Company By [Signature] Title Estimator





# INVOICE

Page of 2

## Pate Construction Co., Inc.

1937 Aspen Circle • Pueblo, Colorado 81006 • Telephone 544-8132

ORDER TO

SHIPPED TO

Ridge Development

MOUNTAIN SHADOWS DRAINAGE

Box 7711

IMPROVEMENTS-Colo. Springs

Colorado Springs, Co. 80933

Job 81-13

DATE	DATE SHIPPED	SHIPPED VIA	QUANTITY ORDERED	PRICE	TOTALS	TAXES
30/81						
QUANTITY	DESCRIPTION				PRICE	TOTAL
LF	60" RCP				103.37/lf	7,752.75
EA	60" X 45 Degree Bends				178.91/ea	357.82
LF	42" RCP				65.88/lf	2,964.60
5 LF	36" RCP				40.44/lf	3,942.90
LF	30" RCP				29.99/lf	2,692.80
2 LF	27" RCP				28.51/lf	7,184.52
0 LF	21" RCP				22.24/lf	2,891.20
3.5 LF	18" RCP				19.76/lf	2,143.96
EA	27" X 18" Reducer				90.50/ea	90.50

NY-874 © The Drawing Board Inc., Box 505 Dallas, Texas

THANK YOU

INVOICE

Page 2 of 2

**Pate Construction Co., Inc.**

1937 Aspen Circle • Pueblo, Colorado 81005 • Telephone 544-8132

SOLD TO

Ridge Development

SHIPPED TO

Mountain Shadows Drainage

Job 81-13

DATE	DATE SHIPPED	SHIPPED VIA	YOUR ORDER NO.	QTY	TERMS	INVOICE NO.
9/30/81						
QTY	UNIT	DESCRIPTION	PRICE	AMOUNT		
3	EA	8' D10R CB3	1870.00/ea	5,610		
2	EA	6' D10R C.B.	1650.00/ea	3,300		
1	EA	4' D10R D.B.	1210.00/ea	1,210		
1	EA	Junction Box	5000.00/ea	5,000		
TOTAL AMOUNT DUE:				<u>\$45,141</u>		

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Job 81-13 Mountain Shadows Drainage

No 1822

REPUBLIC NATIONAL BANK  
PUEBLO, COLORADO

42-56  
1070

DATE Oct. 28, 1981

AY

THE SUM 45 141 05 00

Dollars \$45,141.05\*\*

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

THE  
DER

Pate Construction Co., Inc.  
1937 Aspen Circle  
Pueblo, Colorado 81006

RIDGE DEVELOPMENT CO., LTD.

BY *U. Michael Hansen*

⑆107000563⑆ 08 712 21⑆

# Pate Construction Co., Inc.

1937 Aspen Circle • Pueblo, Colorado 81006 • Telephone 544-8132

**SOLD TO**

**SHIPPED TO**

Ridge Development Co., Inc.

1951 Pueblo Blvd.

Pueblo, Co. 81005

Attention: Jerry Novak

DATE	DATE SHIPPED	SHIPPED BY	PUR. ORDER NO.	QTY	PRICE	AMOUNT
8/31/81						
45 LF	21" CMP					7.75/1F \$348.75
<i>Return</i>						
<i>Reduction to \$310.00 OK Bill Papper</i>						

Form NV-874 ©The Drawing Board Inc., Box 505 Dallas, Texas

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Invoice for 40 LF 21" CMP

NO 1792

NATIONAL BANK  
PUEBLO, COLORADO

\$2.76  
1375

DATE Sept. 25, 19 81

AY THE SUM OF \_\_\_\_\_

Dollars \$ 310.00

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

THE  
DER

Pate Construction Co., Inc.  
1937 Aspen Circle  
Pueblo, Colorado 81006

RIDGE DEVELOPMENT CO., LTD.

NOT NEGOTIABLE

BY \_\_\_\_\_

81010005831 CB 712 20

MTN. SHAD. FILING No. 1

CITY COPY

- 1) BIDS
- 2) INVOICES
- 3) CHECK STUBS

T. L. PRINTE  
BOX CULVERT-WILSON RD.

OK ✓



BIDDING SCHEDULE FOR WILSON ROAD - MOUNTAIN SHADOWS  
DRAINAGE IMPROVEMENTS

Schedule A				
ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1	70 L.F.	60" R.C.P.		
2	2 each	60" X 45° R.C.P. bends		
3	40 L.F.	42" R.C.P.		
4	93 L.F.	36" R.C.P.		
5	88 L.F.	30" R.C.P.		
6	252 L.F.	27" R.C.P.		
7	143 L.F.	21" R.C.P.		
8	130 L.F.	18" R.C.P.		
9	1 each	27" to 18" R.C.P. Reducer		
10	3 each	8' Catch Basins		
11	2 each	6' Catch Basins		
12	3 each	4' Catch Basins		
13	1 each	Junction Box		
			Total	
Schedule B				
14	1 each	Double 5' x 6' R.C.B.C.	34,790 <sup>00</sup>	34,790 <sup>00</sup>
14 ALT.	1 each	Single 5' x 12' R.C.B.C.	35,725 <sup>00</sup>	35,725 <sup>00</sup>

Dated this 9<sup>th</sup> day of SEPT., 1981.

MEL-RO CONSTRUCTION INC. Alan P. Bueker  
 Company By President Title

BIDDING SCHEDULE FOR WILSON ROAD - MOUNTAIN SHADOWS  
DRAINAGE IMPROVEMENTS

Schedule A				
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8	130 L.F.	18" R.C.P.		
9	1 each	27" to 18" R.C.P. Reducer		
10	3 each	8' Catch Basins		
11	2 each	6' Catch Basins		
12	3 each	4' Catch Basins		
13	1 each	Junction Box		
			Total	
Schedule B				
14	1 each	Double 5' x 6' R.C.B.C.		52,636 <sup>00</sup>
14 ALT.	1 each	Single 5' x 12' R.C.B.C.		56,795 <sup>00</sup>

Dated this 9 day of November, 1981.

BYERLY & Byerly Inc.  
Company

By A. J. Byerly Title PRESIDENT  
ALLEN BYERLY



M & M

BIDDING SCHEDULE FOR WILSON ROAD - MOUNTAIN SHADOWS  
DRAINAGE IMPROVEMENTS

Schedule A				
ITEM #	QUANTITIES & UNIT	DESCRIPTION	UNIT PRICE	BID AMOUNT
1	70 L.F.	60" R.C.P.		
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5	88 L.F.	30" R.C.P.		
6	252 L.F.	27" R.C.P.		
7	143 L.F.	21" R.C.P.		
8	130 L.F.	18" R.C.P.		
9	1 each	27" to 18" R.C.P. Reducer		
10	3 each	8' Catch Basins	1,575.00	4,725.00 ✓
11	2 each	6' Catch Basins	1,350.00	2,700.00 ✓
12	3 each	4' Catch Basins	1,100.00	3,300.00 ✓
13	1 each	Junction Box		
			Total	10,725.00 ✓
Schedule B				
14	1 each	Double 5' x 6' R.C.B.C.	326.31	31,000.00 (ok)
14 ALT.	1 each	Single 5' x 12' R.C.B.C.		

Dated this 9 day of September, 1981.

M&M CEMENT CONTRACTORS, INC.  
Company

Gilbert Martinez President  
By \_\_\_\_\_ Title

# T.L. PRINTZ CONSTRUCTION CO.

P.O. BOX 1025 1114 ERIE PUEBLO, COLORADO 81002 PHONE 545-6710

November 24, 1981

Ridge Development Co., Ltd.  
 Attn: Jerry Kovak  
 P. O. Box 7711  
 Colorado Springs, Colorado 80933

## STATEMENT

Contract Amount		\$28,419.00
Add: Change Order #1		<u>1,260.00</u>
		\$29,679.00
Less: Payment 11/13/81		<u>25,226.00</u>
Project Complete, Balance Due		\$ 4,453.00
Add: Extra rebar labor (Per agreement with Jerry Kovak)	\$479.00	
Increased carpentry labor plus extra labor at construction joints	<u>332.00</u> ?	<u>910.00</u>
TOTAL DUE (Contract)		<u>\$ 5,263.00</u> <i>hundred</i>

\*NOTE: Additional excavation and compaction will be billed cost plus 15% as per Jerry Kovak.

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Mountain Shadows Filing #1 Storm Sower \$5263.00

No 1868

REPUBLIC NATIONAL BANK  
 PUEBLO, COLORADO 42-55  
1070

DATE Dec 21 1981

THE SUM 5263 00 00

Dollars \$ 5263.00

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

T. L. Printz Construction Co.  
 P O Box 1025  
 Pueblo, Colorado 81002

RIDGE DEVELOPMENT CO., LTD.

# T.L. PRINTZ CONSTRUCTION CO.

P.O. BOX 1025 1114 ERIE PUEBLO, COLORADO 81002. PHONE 545-6710

December 3, 1981

Ridge Development Co., Ltd.  
 Attn: Jerry Novak  
 P. O. Box 7711  
 Colorado Springs, Colorado 80933

## STATEMENT

RE: Box Culvert - Wilson RD.  
 Extra excavation and compaction for  
 building culvert in two sections  
 after back fill by water department.  
 Per agreement with Jerry Novak.

1.) Excavation Labor (Dace):		\$ 414.00	
2.) Transportation of Machine:			
Janitel:	\$123.75		
Weickers:	<u>134.70</u>	258.45	
3.) Backfill, 910 Cat & operator:		280.31	
4.) Charge for tamping:			
Machines rental:	\$138.00		
2 Laborers 16 hrs. each			
32 hours @ \$10.88	<u>348.16</u>	486.16	
5.) Extra compaction tests:	\$ 26.00		
Extra concrete tests:	<u>90.00</u>	116.00	
6.) Form lumber that had to be buried:		<del>45.60</del>	
7.) Midwest Barricade: (We feel we could have been done 10 days earlier.)			
B & L 12 for 10 days @ \$ .60	\$ 72.00		
Stands 6 for 10 days @ .65	39.00		
Type III 4 for 10 days @ 1.25	50.00		
48" Signs 11 for 10 days @ .50	55.00		
Stand 1 for 10 days @ .65	<u>6.50</u>	222.50	
8.) General Overhead @ 15%		<del>275.38</del> 266.55	
9.) Dace for Ridge Development 5 hrs. @ \$46		\$2,095.30	
		<u>230.00</u>	
		<del>60,395.80</del> 2273.52	

*OK*  
*Room*  
*Stewart*  
*Jerry Novak*  
*12/3/81*

*266.55*  
*2043.97*  
*PPV*  
*TALIS*  
*J.N.*

TOTAL BALANCE DUE FOR OTHER WORK:

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Wilson Road \$2273.52

SPRING NATIONAL BANK  
 PUEBLO, COLORADO

DATE DEC 3 1981

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY ENDORSED

T.L. Printz Construction Co.  
 P.O. Box 1025  
 Pueblo, Colorado 81002

NOT NEGOTIABLE

# T.L. PRINTZ CONSTRUCTION CO.

P.O. BOX 1025 1114 ERIE PUEBLO, COLORADO 81002 PHONE 545-6710

November 3, 1981

Ridge Development Co., Ltd.  
 Attn: Jerry Novak  
 P. O. Box 7711  
 Colorado Springs, Colorado 80933

## STATEMENT

Base Bid - Original Contract		\$28,050.00
*Deduct: original excavation in base bid		<u>(1,600.00)</u>
		\$27,250.00
Add: Excavation Costs to Date		
Weicker Invoice #S15765	\$ 155.75	
Weicker Invoice #P17145	172.15	
Elice Brothers Statement-		
September 30, excavate culvert - 7 hrs.	322.00	
October 1, excavate culvert - 7 hrs.	322.00	
Layout, shoot grade -14 hrs.	149.00	
Contractors overhead	<u>57.10</u>	
		<u>1,169.00</u>
New Contract Amount		\$28,419.00
Add: Change Order #1		<u>1,260.00</u>
		\$29,679.00
Less: Retainage to be held until handrails are installed and deck concrete comes up to strength		<u>(4,453.00) - 15%</u>
Total due on or before November 10, 1981		<u>\$25,226.00</u>

\*Note: All additional excavation and compaction will be billed cost plus 15% as per Jerry Novak.

*OK 10/19/81*  
*J.N.*  
*OK*  
*MS #1*

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS

Statement of Nov. 3, 1981 work in Colorado Springs

NO 1838

FEDERAL NATIONAL BANK  
 PUEBLO, COLORADO

42-38  
 1579

DATE Nov. 13, 1981

PAY THE SUM OF \$25,226.00 Dollars \$ 25,226.00

THIS CHECK WILL NOT BE PAID UNLESS PROPERLY DATED AND ENDORSED

EDGE DEVELOPMENT CO. LTD.

TO THE ORDER OF

T. L. Printz Construction Co.  
 P. O. Box 1025  
 Pueblo, Colorado 81002



# FRAZEE CONSTRUCTION COMPANY

SEWER and WATER MAINS

OFFICE - 392-3443

855 East Las Vegas  
P.O. Box 5308

Colorado Springs  
Colorado 80931

Date 10/22/81  
Name Ridge Development Co.  
Address P.O. Box 7711  
Co Springs, Co 80933

DATE	ITEMS	BALANCE
	Wilson Rd - Install concrete collar encasements on 24" water main 2 EA @ 965 <sup>00</sup>	\$ 1930 00
	OK	

DESCRIPTION OF PROPERTY, WORK AND/OR MATERIALS  
Concrete Collar on 24 inch water mains

No 1835

AMERICAN NATIONAL BANK  
PUEBLA, COLORADO  
DATE 11/9 1981

PAY Two Thousand Nine Hundred 00/100 Dollars \$ 2900 00  
THIS CHECK WILL NOT BE PAID UNLESS PROPERLY ENDORSED

TO THE ORDER OF  
Frazee Construction Co.  
P.O. Box 5308  
Colo. Springs, Colo. 80931

RIDGE DEVELOPMENT CO., LTD.

NOT NEGOTIABLE

⑆107080583⑆ CB 912 20

	TOTAL	\$ 1930 00
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