

DRAINAGE REPORT
OF
CURR RESERVOIR & FISHERS CANYON
Tributary Areas to Colorado State Highway 115
Colorado Springs, Colorado
prepared for
GATES LAND COMPANY



DREXEL, BARRELL & CO.

ENGINEERS — SURVEYORS

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Prepared by:

DREXEL, BARRELL & CO.
March 5, 1980

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TRIBUTARY AREAS TO COLORADO STATE HIGHWAY 115
COLORADO SPRINGS, COLORADO

This report covers drainage areas containing about 3.8 square miles which are tributary to Curr Reservoir and Fishers Canyon to Colorado State Highway 115. This basin is located mostly in Section 5, R66W, T15S and Sections 1, 2, 11, 12, R67W, T15S, all in the 6th P.M., City of Colorado Springs, County of El Paso. More particularly, the area is located between Cheyenne Mountain and State Highway 115 and north and south of Star Ranch Road at its intersection with Highway 115.

The basin is located on an eastern facing slope, starting at a ridge and steeply sloping, in excess of fifty percent, for the first half mile and gradually flattening to approximately four percent slope at Highway 115. Trees exist at higher elevations with sparse grass prevalent at lower elevations, with some scrubs centering on major swales. The basin is tributary to two points at Highway 115. Curr Reservoir, the larger northern portion, drains about 2.5 square miles. Curr Dam and Reservoir, located directly west and adjacent to Highway 115, was constructed to its present configuration after 1971. The spillway drains to an existing 7'x7' box culvert under Highway 115. Fishers Canyon, the southern basin, contains about 1.3 square miles and has a fairly well defined swale throughout the length of the basin. The swale flattens about 500 feet west of the highway. No culvert exists under State Highway 115 at the base of Fishers Canyon.

Criteria for determining flows was taken from the Soils Conservation Service Manual entitled "Procedures for Determining Peak Flows in Colorado" including Technical Release No. 55. The six hour 100 year storm was used and values were obtained from "NOAA Atlas 2, Volume III" prepared for the S.C.S. (Soils Conservation Service). Culvert hydraulics were taken from "Hydraulic Engineering Circulars No. 13 and No. 5" prepared by the Federal Highway Administration. Colorado Springs uses a modified S.C.S. method as shown in the "City of Colorado Springs Runoff Criteria" manual.

Colorado State Highway 115 represents a barrier to drainage from both Curr Reservoir and Fishers Canyon. The existing 7'x7' box culvert is inadequate to pass the 100 year storm under the highway as required by the City of Colorado Springs. As previously indicated, the drainage is divided into two parts, the larger being Curr Reservoir. About 1700 c.f.s. from this area is historically tributary to the existing box culvert under Highway 115. The capacity of the existing box is limited to 300 c.f.s. when the highway overtops at a point approximately 800' south of the culvert. If Fishers Canyon flow, an additional 980 c.f.s., is considered, it is clear under historic conditions that the existing box culvert under State Highway 115 is inadequate.

Existing and proposed development has and will alter the drainage patterns of this area considerably. Curr Dam & Reservoir now has the capacity to store the 100 year storm for both existing and proposed development with the spillway outflow reduced to 500 c.f.s. Details can be found on this subject prepared by Hartzell, Pfeiffenberger & Associates, Inc. on April 21, 1971. Unless modifications are made, the existing 7'x7' box culvert at the base of the dam cannot handle this reduced 100 year developed flow. Development is proceeding along

Fishers Canyon increasing the need for a culvert under the highway at this point. The flow determination at this point varies by method. The S.C.S. method used in this report shows 1130 c.f.s. tributary to this point. Colorado Springs methodology and approved flow is 1355 c.f.s. The State of Colorado feels the S.C.S. method produces high results in this case and recommends a design discharge of 900 c.f.s. They also recommend a double 10'x6' box culvert be used at this point. At this report's design, flow of 1130 c.f.s. water will pond to a point 1.5' above the inside top of the culvert. The City of Colorado Springs approved flow causes ponding 3' above the inside top of the culvert. Thus it appears the culvert as recommended by the State Highway Department is adequate.

In summary, it is obvious that additional culvert capacity is necessary under Colorado State Highway 115 to prevent drainage overtopping the highway and to adhere to the City of Colorado Springs policies. Drainage calculations and data are included for your review.

Respectfully submitted,

DREXEL, BARRELL & CO.



Barbara N. Weiss
P.E. 15471

1/1

Project Fishers Canyon, Curr Reservoir Job No E-1986

Client Gates By BNW Date

Summary of Flows - S.C.S.

Curr Reservoir historic 1683 cfs
developed 2080 cfs

Fishers Canyon historic 977 cfs
developed 1130 cfs

100
/

Project Fishers Canyon Job No E-1986

Client Gates By BNW Date 3-4-80

chk capacity of double 10' x 6' high box culvert
@ Q = 900 cfs $H_f = 6'$
@ Q = 1130 cfs $H_f = 1.25(6') = 7.5'$
@ Q = 1355 cfs $H_f = 1.5(6') = 9'$

Project: Curr Reservoir Job No: E-1986

Client: Gates By: BNW Date: Oct 19, 1979

Total area to state Hwy 115
(to Curr Reservoir)

Soil Group - C (85%) B (15%)

A - 1,600 Ac = 2.5 sq. mile

Historic condition

poor - good range land

$$CN = .85(80) + .15(70) = 78.5$$

determine $T_c = (11.8 L^3 / H)^{.385}$

H = 8880 - 7000' = 1800'	L = 3100'	$T_c = 2 \text{ min}$
H = 400'	L = 2400'	$T_c = 5 \text{ min}$
H = 400'	L = 3500'	$T_c = 7 \text{ min}$
H = 200'	L = 4000'	$T_c = 10 \text{ min}$
H = 80'	L = 2000'	$T_c = 15 \text{ min}$

$$T_c(\text{total}) = 49 \text{ min} = .82 \text{ hr}$$

P = 3.5 in (6 hr - 100 yr storm) (NOAA Atlas II)

Q = 1.53 (Tech. release #55)

$q_p = 440 \text{ csm/in}$ Type II storm (Tech. release #55)

$$q = q_p A Q$$

$$q = 440 (2.5) 1.53 = 1,683 \text{ cfs}$$

Project Curr Reservoir		Job No E-1986
Client Gates	By BNW	Date Feb 1980

Developed

30% 1/4 Acre lots $CN = .3 [.85(83) + .15(75)] = 24.5$

50% 1 Acre lots $CN = .5 [.85(79) + .15(68)] = 38.7$

20% exist. $CN = 78.5(.2) = 15.7$

$CN(\text{total}) = 78.9$

T_c - assume channels for tributary flow paved
for 4000' thru 1/4 Acre lots
@ 20 fps $\rightarrow 3.3 \text{ min}$ or $T_c = 37 \text{ min} = .62 \text{ hr}$

$Q = 1.6$ $q_p = 520 \text{ csm/hr}$

$q = q_p Q A$

$q = 520 (1.6) 2.5 = 2080 \text{ cfs}$

- EXIST BOX UNDER STATE HWY 115 - #1 el. 35 (approx)
size 7'x7'

\rightarrow Break-out point el 41 @ Star Ranch Rd @ Hwy 115
 \therefore capacity 300 cfs

\rightarrow berm to 42.5 el of exist. 115 @ culvert
capacity 420 cfs

Project Fisher Canyon Channel		Job No E-1986	
Client Gates [from Peak Flows in Co. SCS # 55, 100 yr, 6 hr storm]		By BMW	Date April 10, 1979

Total area to state Hwy 115 = 850 Ac

determine CN

- above el 7200 A = 360 Ac
- mostly I-1-EF + I-3-EF (rock outcrop & Cold Creek - Tolman complex & Cutler Broadmoor)
- 30% D 70% C - from SCS office C. Springs
- poor

$CN = 30(83) + 70(77) = 80$ - historic & proposed = CN

- to a point approx 500' W of 115 A = 270 Ac

- X-16-F (Jarre Geoculote complex) B
- poor range land

CN = 79 historic

CN = (assume 1/2 Acre lots) 73

- lower 220 Acres
- CSE Razor stony clay loam - C
- poor range land

CN = 86 historic

$CN = (\text{assume } 1/4 \text{ acre lots}) 86(86\%) + (\text{commercial}) 94(14\%) = 87$

determine T_c

to el 9300	Z min			
to el 7200	H = 2100'	L = 5700'	$T_c = 9 \text{ min}$	Figure 5-5
to el 6500	H = 700	L = 5400'	$T_c = 13 \text{ min}$	"
to el 6200	H = 300'	L = 3000'	$T_c = 9 \text{ min}$	
*to el 6000	H = 200	L = 3100	$T_c = 10$	
*to el 5950	H = 50'	L = 1700	$T_c = 9 \text{ min}$	

T_c Historic Total = 52 min

* Historic only

proposed to el 5950 L = 4800 @ 20 fps $T_c = 4 \text{ min}$

T_c proposed Total = 37 min

Project Fisher Canyon Channel		Job No E-1986
Client Gates	By brw	Date April 10, 1979

rev'd to Tech #55
March - 80

CN

$$CN \text{ historic} = \frac{360}{850} (80) + \frac{270}{850} (79) + \frac{220}{850} (86) = 81$$

$$CN \text{ (proposed)} = \frac{360}{850} (80) + \frac{270}{850} (73) + \frac{220}{850} (87) = 80$$

$$q_p = q_p A Q$$

historic $q_p = 430 \text{ csm/in}$ (Technical Release #55)
 $Q = 1.71 \text{ in}$ (for $P = 3.5 \text{ in C Springs}$)

$$q_p = 430 \left(\frac{850}{640} \right) 1.71 = 977 \text{ cfs}$$

proposed $q_p = 520 \text{ csm/in}$ (Technical Release #55)
 $Q = 1.64$ (for $P = 3.5 \text{ in C Springs}$)

$$q_p = 520 \left(\frac{850}{640} \right) 1.64 = 1130 \text{ cfs}$$

E-1986

Steve Buchberger
Hydraulics Unit
Colorado Dept of Highways
4201 E. Arkansas Avenue
Denver, CO 80222
Sept. 25, 1979

RECEIVED

SEP 27 1979

DREXEL, BARRELL & CO.

Barbara Weiss
Drexel, Barrell & CO

Dear Barbara -

Per your request, enclosed please find a copy of the letter to District Engineer, Mr. Watt Harris, which documents the drainage structure recommended by the Hydraulics Unit for the Fishers Canyon Crossing. Although we acknowledge that your estimated design discharge is based on accepted procedure, we feel that the Soil Conservation Service method gives results which are somewhat high - especially when considering the streamflow history for this area. Consequently, we feel a design discharge of 900 cfs is more reasonable.

If you have any questions, please contact me.

Sincerely,

Steven G. Buchberger

ENCLOSURE

DIVISION OF HIGHWAYS
STATE OF COLORADO
4201 E. Arkansas Ave.
DENVER, COLORADO 80222

District 2 Miscellaneous
SH 115 [Formerly D-AD 16(1)]
South of Colorado Springs

Design File 09402

May 23, 1979

TO: H. W. HARRIS

FROM: D. L. VERNON

SUBJECT: REVIEW OF DRAINAGE STRUCTURE FOR FISHERS CANYON

On Thursday, April 5, 1979, a meeting between Gary Haynes (office of City Engineer - Colorado Springs), Barbara Weiss (Drexel, Barrell and Company), and Steven Buchberger (CDOH) was held to discuss and to resolve the difference between design discharges for Fishers Canyon at SH 115. Following the meeting, the Hydraulics Unit reviewed its hydrologic analysis for Fishers Canyon.

Results of our re-evaluation again show that our estimated design flow is less than the design discharge proposed by Drexel, Barrell and Company. However, recognizing the uncertainty involved in such an analysis and in order to provide an additional margin of safety, we recommend that a DBL 10' x 6' CBC be used at the site rather than the DBL 8' x 6' CBC referenced in our letter to you dated March 7, 1979.

This structure will pass ⁹⁰⁰900 cfs and, thereby, will decrease substantially any risk of flooding SH 115 at the drainage crossing of Fishers Canyon. The cost of the DBL 10' x 6' is approximately \$550 per linear foot.

A field survey is needed before final design can be completed.

D. L. VERNON
Staff Design Engineer

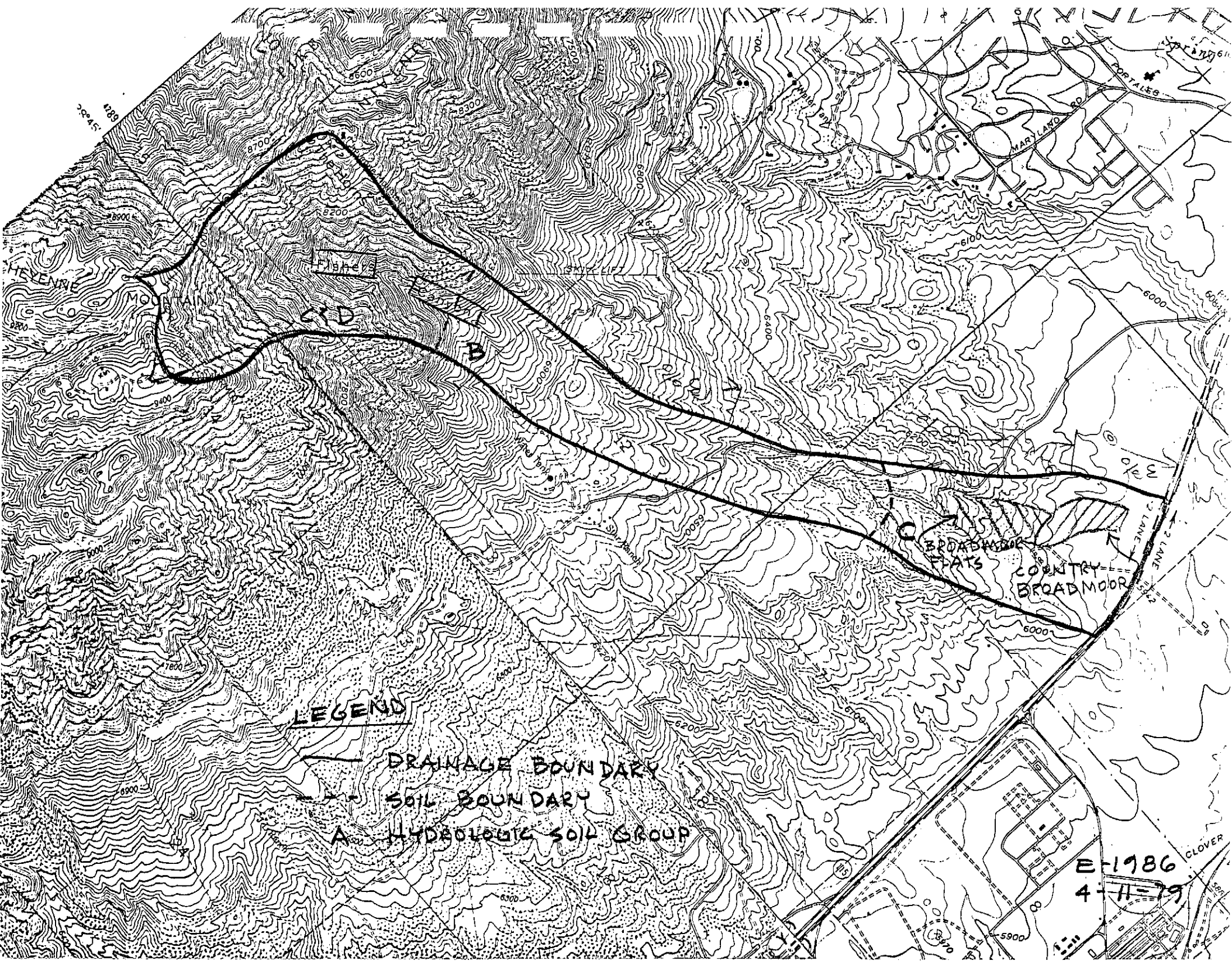
By

D. H. HEIDERSTADT
Coordinating Engineer

DHHn1

cc: Vernon/Kasenga
D H Heiderstadt
J L Pim
R Q Brown

[Handwritten signatures and initials: a large checkmark, "VED", and "Sops"]



LEGEND

- DRAINAGE BOUNDARY
- - - SOIL BOUNDARY
- A HYDROLOGIC SOIL GROUP

E-1986
4-11-79

