MASTER DEVELOPMENT DRAINAGE PLAN UPDATE
FOR
FAIRLANE TECHNOLOGICAL PARK INTERQUEST SOUTH
(LOT 4, FAIRLANE TECHNOLOGY PARKING FILING NO. 2)

AND

FINAL DRAINAGE REPORT
FOR
LOTS 1 & 2 INTERQUEST FILING NO. 6

APRIL 2007

PREPARED FOR:
CORPORATE OFFICE PROPERTIES TRUST
102 S. TEJON STREET, SUITE 720
COLORADO SPRINGS, CO 80903

PREPARED BY:
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2190.00

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

MASTER DEVELOPMENT DRAINAGE PLAN UPDATE
FAIRLANE TECHNOLOGICAL PARK / INTERQUEST SOUTH
FINAL DRAINAGE REPORT – INTERQUEST FILING NO. 7

DRAINAGE REPORT STATEMENT

ENGINEER'S STATEMENT:
The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.

Kyle R. Campbell, Colorado P.E. #29794 — Date: 11.5.07

DEVELOPER'S STATEMENT:
I, the developer, have read and will comply with all of the requirements specified in this drainage report and plan.

Business Name: **CORPORATE OFFICE PROPERTIES TRUST**

By:

Title: V.P. - ASSET MANAGEMENT

Address: **102 S. TEJON STREET, SUITE 720**

COLORADO SPRINGS, CO 80903

CITY OF COLORADO SPRINGS:
Filed in accordance with Section 7.7.906 of the Code of the City of Colorado Springs, 2001 as amended.

City Engineer — Date: Nov 7, 2007

Conditions:

MASTER DEVELOPMENT DRAINAGE PLAN UPDATE
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MASTER DEVELOPMENT DRAINAGE PLAN UPDATE
FOR FAIRLANE TECHNOLOGICAL PARK INTERQUEST SOUTH
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FOR LOTS 1 & 2 INTERQUEST FILING NO. 6

PURPOSE

The purpose of this Master Development Drainage Plan (MDDP) and the Final Drainage Report (FDR) is to identify the existing drainage patterns and present improvements to safely route runoff both on-site and tributary flows through the site to existing identified improved and unimproved facilities.

GENERAL DESCRIPTION

The Fairlane Technological Park is a phased office campus and light industrial development located in northeast Colorado Springs. The development is located on the land that lies east and west of State Highway 83, between Old Ranch Road and Interquest Parkway, bounded by Interstate 25 to the west and Kettle Creek Road to the east. The development lies between the Black Squirrel and Kettle Creek basins. The watershed has been noted as the 'Elkhorn Basin' in past drainage basin planning studies. The development has been and will continue to be constructed in phases. The total acreage subject to master planning covers approximately 665 acres but the main area of focus is the existing Lot 4, Fairlane Technology Park Filing No. 2, which contains the main undeveloped area of the Interquest South Campus.

The property subject to development is located in portions of Sections 15, 16, 20, 21, 28 and 29 of Township 12 South, Range 66 West of the 6th Principal Meridian. The watershed contains both areas within the City and unincorporated El Paso County. The total watershed area covers 730 acres. The project site is shown on the Vicinity Map in the Appendix of this report.

The average soil condition reflects Hydrologic Group "A" (Blakeland and Columbine) and Hydrologic Group "B" (Stapleton) as determined by the "Soil Survey of El Paso County Area", prepared by the Soil Conservation Service (see map in the Appendix of this report).

The overall studied area is divided into two general sub-basins, referred to in this report as Elkhorn-1 (south area) and Elkhorn-2 (north area adjacent to Interquest Parkway). The runoff from the Elkhorn-1 drainage basin discharges into the existing 60" RCP culvert under I-25 and continues to flow west into the Monument Creek Drainage Basin across the U.S. Air Force Academy property. The runoff from the Elkhorn-2 drainage basin discharges into the existing 24" RCP culvert under I-25 and continues to flow west into the Monument Creek Drainage Basin across the U.S. Air Force Academy property.

The Master Development Drainage Plan Update (MDDP) portion of this report is divided into three different development conditions. The conditions analyzed were: Existing Condition, Fully Developed Condition and the Filing No. 6 & 7 Interim Condition. The Existing Condition development stage is how the basin functions as of the time of this report, April 2007. The Fully Developed Condition development stage is how the basin is projected to function once the overall tributary area is fully built. The Filing No. 6 & 7 Interim Condition is how the basin is projected to function once the Filing No. 6 & 7 phase of development is completed. Detailed descriptions, calculations and drainage maps describing these three conditions follow.

The 'Final Drainage Report Lots 1 & 2 Interquest Filing No. 6' portion of this report specifies smaller, more detailed drainage basins associated with this specific site within the basin. Drainage maps showing these basins, design points, and pipe runs are also found in the appendix. All future development on other Lots within Filing No. 6 will require a separate Final Drainage Report.

HYDROLOGY / HYDRAULICS

The hydrologic methods used to evaluate the drainage basin are consistent with the methods outlined in the City/County Storm Drainage Criteria manual. Topography for the site was compiled at a two-foot contour interval and a horizontal scale of one inch to 200-feet. This topography was used to verify the onsite sub-basin boundaries. Offsite sub-basin boundaries were determined using the above referenced reports, the City of Colorado Springs FIMS mapping base, and the USGS quadrangle maps for the area.

Peak discharges for Existing, Fully Built Out and Interim Conditions were determined for the 2, 5, 10, 25, 50 and 100-year recurrence intervals. The 24-hour storm duration was modeled using a SCS method Type IIA rainfall distribution. The software used to model the drainage system was PondPack 10.0. On-site storm facilities associated with Filing No. 6 are sized using the Modified Rational Method to provide a more conservative design for storm runoff interception and conveyance (See Final Drainage Report portion and Appendix). The SCS analysis is completed with the M.D.D.P. portion of this report to prove the functionality of the overall drainage basin and conformance of release rates to prior master drainage studies (i.e. D.B.P.S. and M.D.D.P.).

Curve numbers for the sub-watersheds used in this update are consistent with the values used in References 4, 5, 8, 9 & 10. The curve numbers for the existing developed sub-watersheds were determined by measurement of the impervious areas off of the topographic mapping referenced above. SCS curve numbers in this report are established as being a portion Cn=98 (impervious areas), Cn=61 (undeveloped areas) and Cn=69 (graded and grassed developed areas) per City of Colorado Springs Drainage Criteria Manual, Table 5-5. Times of concentrations in this report are calculated assuming an average overland flow of 100' for undeveloped areas and 50' for developed areas at 2% slope and the balance of the flow length as channel flow at 3% slope. This produces times of concentrations consistent with values in previous reports. The calculations for each basin are in the appendix of this report. These slopes are typical for the area subject to master planning.

Included in the Appendix of this report is a copy of a letter from the Air Force Academy to the City of Colorado Springs dated November 30, 2000. The letter addresses concerns from increased storm water runoff from upstream basins to the Academy property. The letter states: "Development occurring upstream of the Academy shall ensure that peak flows for the 2-, 5-, 10-, 50- and 100- year storm frequencies are not increased over historical "undeveloped" flows. Proposed development occurring upstream of the Academy must adequately demonstrate that storm water facilities (i.e., detention / retention ponds) will function in conjunction with other facilities within drainage and not increase peak flows for the frequencies listed above or adversely impact water quality". This criterion has been adhered to in this study for the subject site.

EXISTING CONDITIONS (M.D.D.P. ANALYSIS)

ELKHORN-1 DRAINAGE BASIN

Basin O1 (Q_5= 12.73 cfs, Q_{100}= 45.16 cfs) is 19.2 acres in area. Reference 10 cited that the construction of Powers Boulevard would reduce this basin from a historic area of 170 acres to 19 acres. Confirmation of this change was made at the time of this study. Runoff from this basin will sheet flow into an existing drainage channel south of Interquest Parkway. At this location, an existing 36" RCP (Conveyance 1) will accept both the 5-year and 100-year flows (Q_5= 10.55 cfs, Q_{100}= 42.84 cfs, Capacity = 94.32 cfs). Conveyance 1 routes the flows to Design Point 1.

Basin B2, the Pikes Peak Community College site, (Q_5= 65.46 cfs, Q_{100}= 184.43 cfs) is 62.7 acres. Runoff from this basin will sheet flow into the 'PCC' detention pond located at the southwest corner of the basin. The developed flows are detained by this pond to historic rates of (Q_5= 31.59 cfs, Q_{100}= 60.98 cfs), 100 year ponding elevation = 6762.40. The routed flows from the detention pond combine with Conveyance 1 at Design Point 1. At this location, an existing 48" RCP (Conveyance 2) will accept both the 5-year and 100-year flows (Q_5= 39.25 cfs, Q_{100}= 98.20 cfs, Capacity = 215.45 cfs). Conveyance 2 routes the flows to Design Point 2.

Basin B1, portions of the Interquest Parkway northern right-of-way and portions of the New Life Church and the Pikes Peak Community College, (Q_5= 8.42 cfs, Q_{100}= 32.02 cfs) is 15.1 acres in area. Runoff from this basin will sheet flow into an existing drainage channel and storm facilities north of Interquest Parkway flowing to the west to Design Point 2.

Basin O2B (Q_5= 4.42 cfs, Q_{100}= 37.46 cfs) is 32.2 acres in area. This basin has an overall drainage area of 44.6 acres, however as documented in previous studies and verify in the field, the existing 24" culvert under New Life Drive is undersized and approximately 12.4 acres bypasses the culvert and continues flowing in the existing drainage ditch west into the Elkhorn-3 drainage basin. The balance of the basin runoff (32.2 acres) will sheet flow into an existing drainage channel north of New Life Drive and pass through the existing 24" culvert. At this location, an existing grass lined swale (Conveyance 3) will accept both the 5-year and 100-year flows (Q_5= 1.57 cfs, Q_{100}= 24.70 cfs, Capacity = 169.57 cfs). Conveyance 3 routes the flows to Design Point 2.

Basin B3, the New Life Church site, (Q_5= 54.19 cfs, Q_{100}= 135.15 cfs) is 40.0 acres. Runoff from this basin will sheet flow into the 'NLC' detention pond located at the southwest corner of the basin. The developed flows are then detained by this pond to historic rates of (Q_5= 11.97 cfs, Q_{100}= 28.62 cfs), 100 year ponding elevation = 6740.21. The routed flows from the detention pond combine with Conveyance 2, Basin B1 and Conveyance 3 at Design Point 2. At this location, an existing 54" RCP (Conveyance 4) will accept both the 5-year and 100-year flows (Q_5= 58.56 cfs, Q_{100}= 168.19 cfs, Capacity = 268.18 cfs). Conveyance 4 routes the flows to Design Point 3.

Basin VP-1 (Q_5= 1.92 cfs, Q_{100}= 4.33 cfs), the western half of the Voyager Parkway right-of-way north of Interquest Parkway and south of New Life Drive, is 2.3 acres in area. This basin has an overall drainage area of 2.3 acres, however as documented in previous studies and verify in the field, the existing curb inlet located at the northwest corner of the intersection of Interquest Parkway and Voyager Drive is undersized and approximately 1.2 acres of drainage area flows by the inlet and continues flowing in the existing curb line / drainage swale along the northern edge of Interquest Parkway flow to the west into the Elkhorn-3 drainage basin (area north of Interquest right-of-way covered by the Marketplace MDDP). The balance of the basin runoff (1.1 acres) will sheet flow into the existing curb line / drainage swale along the northern edge of Interquest Parkway. At this location the captured drainage area is conveyed to Design Point 3 through the existing storm sewer system under Interquest Parkway.

Basin IP-1 (Q_5= 2.07 cfs, Q_{100}= 4.68 cfs), the northern half of the Interquest Parkway right-of-way west of Voyager Parkway and east of Federal Drive, is 2.5 acres in area. This basin has an overall drainage area of 2.5 acres, however as documented in previous studies and verify in the field, the existing curb inlet located at the northern curb of Interquest Parkway at the intersection of Federal Drive is undersized and approximately 1.3 acres of drainage area flows by the inlet and continues flowing in the existing curb line / drainage swale along the northern edge of Interquest Parkway flow into to the west into the Elkhorn-3 drainage basin flow re-enters the northerly swale since no curb & gutter is present along the northerly side of the street, west of Voyager Parkway. The balance of the basin runoff (1.2 acres) will sheet flow into the existing curb line / drainage swale along the northern edge of Interquest Parkway. At this location the captured drainage area is conveyed to Design Point 3 through the existing storm sewer system.

Basin IP-2 (Q_5= 4.30 cfs, Q_{100}= 9.74 cfs), the southern half of the Interquest Parkway right-of-way west of Voyager Parkway and east of Federal Drive, is 2.5 acres in area. At this location the captured drainage area is conveyed to Design Point 3 through the existing storm sewer system. From this point the captured flows from Basin VP-1, Basin IP-1, Basin IP-2 and Conveyance 4 are routed through and existing 54" RCP, Conveyance 5, accepting the both the 5-year and 100-year flows (Q_5= 58.56 cfs, Q_{100}= 181.40 cfs, Capacity = 278.09 cfs) to Design Point 4.

Basin B5 (Q_5= 6.04 cfs, Q_{100}= 22.25 cfs), a portion of Interquest Filing No. 3, is 11.4 acres in area. The basin is the beginning of the area subject to master planning. The basin is partially developed as the First United Bank at the southeast corner of Interquest Parkway and Federal Drive. As typical for the areas subject to master planning the SCS curve number is established as Cn=88 for developed Industrial areas and Cn=61 for undeveloped portions, per City of Colorado Springs Drainage Criteria Manual, Table 5-5, Type B soils. Basin B5 is typical of a partially developed basin and the curve number is calculated as the prorated amount of developed and undeveloped areas. For Basin B5, the curve number is 69.8. Curve numbers and times of concentrations for off-site areas, O1, B1, B2, B3, O2B are taken from previous studies. Calculated times of concentrations in this report are calculated assuming an average overland flow of 100' at 2% and the balance of the flow time as channel flow at 3%, producing times of concentrations comparable to previous reports. The calculations for each basin are in the appendix of this report. These slopes are typical for the area subject to master planning. Runoff from Basin B5 sheet flows across the basin to an existing storm sewer system at the First United Bank site and continues to Design Point 4. At this location, an existing 54" RCP (Conveyance 6) will accept both the 5-year and 100-year flows (Q_5= 64.45 cfs, Q_{100}= 203.36 cfs, Capacity = 278.09 cfs). Conveyance 6 routes the flows to Design Point 5.

Basin B6 (Q_5= 2.67 cfs, Q_{100}= 20.47 cfs), an undeveloped portion of Interquest Filing No. 3 is 18.4 acres in area. Runoff from Basin B6 sheet flows into/across the basin/site to an existing storm sewer system at the northeast corner of Federal Drive and Republic Drive and continues to Design Point 5.

Basin B7 (Q_5= 7.57 cfs, Q_{100}= 31.13 cfs), a partially developed portion of Interquest Filing No. 5 is 17.8 acres in area. Runoff from Basin B7 sheet flows into/across the basin/site to an existing storm sewer system at the southeast corner of Federal Drive and Republic Drive and continues to Design Point 5. At this location, an existing 60" RCP (Conveyance 7) will accept both the 5-year and 100-

year flows from Basin B6, Basin B7 and Conveyance 6, (Q_5= 74.49 cfs, Q_{100}= 254.17 cfs, Capacity = 383.63 cfs). Conveyance 7 routes the flows to Design Point 6.

Basin B9 (Q_5= 9.25 cfs, Q_{100}= 18.59 cfs), a developed portion of the Interquest development, the Residence Inn located at the southwest corner of Federal Drive and Republic Drive is 4.1 acres in area. Runoff from Basin B9 sheet flows across the hotel parking lot into a storm sewer system that continues to Design Point 6. At this location, an existing 20' grass lined swale (Conveyance 8) will accept both the 5-year and 100-year flows (Q_5= 71.44 cfs, Q_{100}= 254.44 cfs, Capacity = 476.05 cfs). Conveyance 8 routes the flows to Design Point 7, detention pond EK1A.

Basin A14 (Q_5= 5.12 cfs, Q_{100}= 39.80 cfs), an undeveloped area of the Interquest development, is 38.4 acres in area. Runoff from this basin sheet flows directly into the existing detention pond EK1A at Design Point 7.

Basin A6 (Q_5= 24.83 cfs, Q_{100}= 57.02 cfs), detention pond EK1A, is 14.7 acres in area. Runoff from this basin will sheet flow directly into the existing detention pond EK1A at Design Point 7.

Basin A1 (Q_5= 6.55 cfs, Q_{100}= 52.06 cfs), an undeveloped portion of Fairlane Technology Park, Filing No. 1 is 56.9 acres in area. Runoff from this basin sheet flows across the basin/site area to an existing grass lined swale flowing south on the east side of State Highway 83. At this location, the swale (Conveyance 9) will accept both the 5-year and 100-year flows (Q_5= 6.55 cfs, Q_{100}= 52.06 cfs, Capacity = 365.58 cfs). Conveyance 9 routes the flows to Design Point 8.

Basin A2 (Q_5= 11.58 cfs, Q_{100}= 92.62 cfs), an undeveloped portion of Fairlane Technology Park, Filing No. 1 is 108.0 acres in area. Runoff from this basin sheet flows across the site area to Design Point 8. At this location a concrete lined channel (Conveyance 10) will accept both the 5-year and 100-year cumulative flows of (Q_5= 15.34 cfs, Q_{100}= 137.05 cfs, Capacity = 1297.29 cfs). Conveyance 10 routes the flows to Design Point 9.

Basin C1 (Q_5= 4.02 cfs, Q_{100}= 30.71 cfs), is an undeveloped / unplatted area, not treated as part of the master planning area in previous studies is 27.5 acres in area. Runoff from this basin will sheet flow across the basin/site area to an existing storm sewer system (Conveyance 11) to Design Point 10.

Basin D1, the Quantum Development, Fairlane Technology Park Filing No. 4, (Q_5= 80.73 cfs, Q_{100}= 168.68 cfs) is 40.4 acres. Runoff from this basin will sheet flow into the detention pond EK1B located at the southwest corner of the basin. The developed flows are routed by this pond (Q_5= 65.96 cfs, Q_{100}= 123.14 cfs), 100 year ponding elevation = 6696.88. The routed flows from the detention pond combine with Conveyance 11 at Design Point 10. At this location, an existing 48" RCP (Conveyance 12) will accept both the 5-year flows (Q_5= 68.33 cfs, Q_{100}= 152.60 cfs, Capacity = 113.10 cfs), flows not accepted by the conveyance are passed through an existing grass lined swale to Design Point 9.

Basin A3 (Q_5= 34.56 cfs, Q_{100}= 85.49 cfs), is partially developed, Fairlane Technology Park, Filing No. 3 is 25.0 acres in area. Runoff from this basin will sheet flow across the basin/site area to Design Point 9. At this location a concrete lined channel (Conveyance 13) will accept both the 5-year and 100-year flows (Q_5= 97.45 cfs, Q_{100}= 310.36 cfs, Capacity = 2191.56 cfs). Conveyance 13 routes the flows to Design Point 11.

Basin A4 (Q_5= 3.38 cfs, Q_{100}= 10.09 cfs), is the northern half of the Old Ranch Road right-of-way west of State Highway 83 and a portion of the eastern half of the State Highway right-of-way north of Old Ranch Road and south of Federal Drive, is 4.0 acres in area. Runoff from this basin will sheet flow and shallow channel flow across the basin/site area to Design Point 11.

Basin A5 (Q_5= 5.67 cfs, Q_{100}= 16.61 cfs), is a partially developed site, the Ford Microelectronics development, a portion of the Fairlane Technology Park Filing No. 2, is 6.7 acres in area. Runoff from this basin will sheet flow across the basin/site area to Design Point 11. At this location a concrete lined channel (Conveyance 14) will accept both the 5-year and 100-year flows (Q_5= 106.23 cfs, Q_{100}= 333.27 cfs, Capacity = 1882.71 cfs). Conveyance 14 routes the flows to Design Point 7, the existing detention pond EK1A.

Basin A7 (Q_5= 26.33 cfs, Q_{100}= 70.36 cfs), is a partially developed site, the Lockheed Martin development, a portion of the Fairlane Technology Park Filing No. 2, is 25.1 acres in area. Runoff from this basin will sheet flow across the basin/site area to a grass lined channel (Conveyance 17)

will accept both the 5-year and 100-year flows (Q_5= 26.02 cfs, Q_{100}= 69.92 cfs, Capacity = 170.49 cfs). Conveyance 17 routes the flows to Design Point 7, the existing detention pond EK1A.

Basin A8 (Q_5= 30.97 cfs, Q_{100}= 75.24 cfs), is a partially developed site, the Lockheed Martin development, a portion of the Fairlane Technology Park Filing No. 2, is 24.7 acres in area. Runoff from this basin will sheet flow across the basin/site area to a 30" RCP storm sewer (Conveyance 15) will accept both the 5-year flows (Q_5= 30.97 cfs, Q_{100}= 75.24 cfs, Capacity = 56.24 cfs) flows not accepted by the conveyance are passed through the existing curbed roadway, Federal Drive to existing storm sewer inlets at Design Point 12. Conveyance 16 routes the flows to Design Point 7, the existing detention pond EK1A.

Basin A9 (Q_5= 40.67 cfs, Q_{100}= 93.39 cfs) is a completely developed site, a portion of the Interquest Filing No. 1 development, is 27.5 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 15 draining into a 48" RCP storm sewer (Conveyance 16) will accept both the 5-year and 100 year flows (Q_5= 71.33 cfs, Q_{100}= 168.10 cfs, Capacity = 248.78 cfs). Conveyance 16 routes the flows to Design Point 7, the existing detention pond EK1A.

In December 2006, the existing detention pond EK1A was field surveyed to establish accurate volumes and configuration of the existing outlet structure and emergency spillway. The existing condition model was analyzed using that as-built information, updating previous studies with information from other sources. The flows entering detention pond EK1A in the Existing Conditions (Design Point 7) are: (Q_5= 279.6 cfs, Q_{100}= 878.0 cfs). The restricted flows leaving the pond are: (Q_5= 19.0 cfs, Q_{100}= 132.5 cfs), 100 year ponding elevation = 6618.32, with a storage volume of 41.34 ac-ft. The existing emergency spillway elevation was measured to be 6619.60, above the 100-year ponding limits analyzed, with a length of about 85.0'. This gives a depth of flow within the spillway (100-yr design flood) of 2.2'. The crest of the existing pond berm is at an elevation of 6623.50. This provides an existing freeboard of only 1.7' (2.0' is the minimum per City criteria). The existing outlet structure has a 19.5" diameter orifice plate with an invert elevation of 6609.13, a concrete riser at elevation 6615.80 with inside dimensions of 6.9' x 9.3' with 16 clear openings (6.9' x 0.354' each). The orifice plate and riser discharge into an existing 54" RCP pipe at 0.83% slope. The 54" RCP pipe discharges through a baffle wall into a rip-rap lined channel,

Conveyance 18. The detention pond release rates are consistent with the capacity of downstream drainage systems and previous models of this pond.

Basin A13 (Q_5= 2.12 cfs, Q_{100}= 16.10 cfs), is an undeveloped portion of the Interquest development, is 13.9 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale (Conveyance 19) will accept both the 5-year flows and 100-year flows (Q_5= 0.91 cfs, Q_{100}= 11.27 cfs, Capacity = 209.13 cfs). Conveyance 19 routes the flows to Design Point 14, the existing 60" RCP culvert under Interstate 25.

Basin A12 (Q_5= 4.38 cfs, Q_{100}= 33.67 cfs), is the undeveloped area of the eastern half of the Interstate 25 right-of-way development north of Old Ranch Road south of the Elkhorn-2 drainage basin, is 31.1 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale to Design Point 14, the existing 60" RCP culvert under Interstate 25. At Design Point 14 (Q_5= 19.92 cfs, Q_{100}= 137.18 cfs), Drainage Basins A12 and Conveyances 18 and 19 are combined at the entrance of the existing 60" RCP culvert. This point is analyzed as a detention pond to model the ponding at the culvert entrance. This pond is referred to as detention pond I25-1. The routed flows leaving the pond are: (Q_5= 19.90 cfs, Q_{100}= 135.83 cfs), 100 year ponding elevation = 6594.01. Previous studies have established a 100-year historic flow of 195 cfs for the basin at the inlet of the 60" culvert. The ponding elevation at this location is below the existing saddle elevation and the existing northbound lane elevation where the flows would divert across the Interstate or to the south into the Kettle Creek Basin. The depth of ponding at this point is 6594.0 – 6589.0 = 5.0', inline with the crown of the culvert. At elevation 6596.5 (7.5' head over the culvert invert) the drainage would begin to bypass the 60" culvert and flow to the south into the Kettle Creek Basin. The maximum flow entering the 60" culvert (point at which it diverts to the south) is calculated to be 211 cfs. The basin runoff in the existing condition is less than the historic flow for the basin at the 60" culvert and is safely conveyed to the receiving Air Force Academy drainage system downstream eventually draining into Monument Creek.

ELKHORN-2 DRAINAGE BASIN

Basin IP-3 (Q_5= 3.16 cfs, Q_{100}= 7.28 cfs), the northern half of the Interquest Parkway right-of-way east of Interstate 25 on-ramp and west of Federal Drive, is 2.0 acres in area. This basin has an

overall drainage area of 4.0 acres, however as documented in previous studies and verified in the field, the existing curb inlet located at the northern curb of Interquest Parkway at the intersection of Interquest Parkway and the I-25 northbound on-ramp is undersized and approximately 2.0 acres of drainage area flows by the inlet and continues flowing in the existing curb line / drainage swale along the northern edge of Interquest Parkway and flows to the west into the Elkhorn-3 drainage basin. The balance of the basin runoff 2.0 acres) will sheet flow into the existing curb line / drainage swale along the northern edge of Interquest Parkway. At this location the captured drainage area is conveyed to Design Point 15 through the existing storm sewer system.

Basin IP-4 (Q_5= 4.89 cfs, Q_{100}= 11.28 cfs), the southern half of the Interquest Parkway right-of-way west of Federal Drive and east of the Interstate 25 northbound off-ramp, is 3.1 acres in area. At this location the captured drainage area is conveyed to Design Point 15 through the existing storm sewer system. From this point the captured flows from Basin IP-3 and Basin IP-4 are routed through and existing 30" RCP, Conveyance 21, accepting the both the 5-year and 100-year flows (Q_5= 7.87 cfs, Q_{100}= 18.48 cfs, Capacity = 70.09 cfs) to Design Point 16. At this location a grass lined swale, Conveyance 22 accepts both the 5-year and 100-year flows (Q_5= 6.96 cfs, Q_{100}= 17.56 cfs, Capacity = 80.50 cfs) conveying to Design Point 17, the existing 24" RCP culvert crossing Interstate 25.

Basin E7 (Q_5= 9.69 cfs, Q_{100}= 76.64 cfs) is a undeveloped portion of the Interquest Development, is 79.2 acres in area. Runoff from this basin will sheet flow across the basin area to an existing shallow grass lined swale, Conveyance 20 accepting both the 5-year and 100-year flows (Q_5= 8.94 cfs, Q_{100}= 74.97 cfs, Capacity = 432.24 cfs). Conveyance 20 routes the flows to Design Point 17 the existing 24" RCP culvert crossing Interstate 25. Drainage Basin E5 and Conveyances 20 and 22 are combined at the entrance of the existing 24" RCP culvert, Design Point 17 (Q_5= 18.46 cfs, Q_{100}= 123.89 cfs). This point is analyzed as a detention pond to model the ponding at the culvert entrance. This pond is referred to as detention pond I25-2. The routed flows leaving the pond are: (Q_5= 8.59 cfs, Q_{100}= 29.33 cfs), 100 year ponding elevation = 6603.13. Previous studies have established a 100-year historic flow of 40 cfs for the basin at the inlet of the 24" culvert. The ponding elevation at this location is below the existing saddle elevation and the existing northbound lane elevation where the flows would divert across the Interstate or to the south into the Elkhorn-1 drainage basin previously discussed. The depth of ponding at this point is 6603.1 – 6599.0 = 4.1' 2.1' above the crown of the culvert. At elevation 6604.5 (5.5' head over the culvert invert) the drainage would

begin to bypass the 24" RCP culvert and flow to the south into the Elkhorn-1 Basin. The maximum flow entering the 24" culvert (point at which it diverts to the south) is calculated to be 36 cfs. The basin runoff in the existing condition is less than the historic flow for the basin at the 24" culvert and is safely conveyed to the receiving Air Force Academy drainage system downstream eventually draining into Monument Creek.

FULLY DEVELOPED CONDITIONS

ELKHORN-1 DRAINAGE BASIN

The description of the storm runoff from Basins O1, B2, B1, O2B, B3, VP-1, IP-1, and IP-2 remain the same as in the Existing Condition analysis (See above).

Basin B5 (Q_5= 24.08 cfs, Q_{100}= 49.57 cfs), a portion of Interquest Filing No. 3, is 11.4 acres in area. The basin is the beginning of the area subject to master planning. The basin is modeled as fully developed in the future. As typical for the areas subject to master planning the SCS curve number is established as Cn=88 for developed Industrial areas and Cn=61 for undeveloped portions, per City of Colorado Springs Drainage Criteria Manual, Table 5-5, Type B soils. Basin B5 is typical of a partially developed basin and the curve number is calculated as the prorated amount of developed and undeveloped areas. For Basin B5, the fully developed curve number is 88.0. Curve numbers and times of concentrations for off-site areas, O1, B1, B2, B3, O2B are taken from previous studies. Calculated times of concentrations in this report are calculated assuming an average overland flow of 50' (in developed areas) to 100' (in undeveloped areas) at 2% and the balance of the flow time as channel flow at 3%, producing times of concentrations comparable to previous reports. The calculation for each basin is in the appendix of this report. These slopes are typical for the area subject to master planning. Runoff from Basin B5 sheet flows into/across the basin/site to an existing storm sewer system at the First United Bank site and continues to Design Point 4. At this location, an existing 54" RCP (Conveyance 6) will accept both the 5-year and 100-year flows (Q_5= 75.51 cfs, Q_{100}= 224.99 cfs, Capacity = 278.09 cfs). Conveyance 6 routes the flows to Design Point 5.

Basin B6 (Q_5= 38.36 cfs, Q_{100}= 79.32 cfs), modeled as a future developed portion of Interquest Filing No. 3 is 18.4 acres in area. Runoff from Basin B6 sheet flows into/across the basin/site to an existing

storm sewer system at the northeast corner of Federal Drive and Republic Drive and continues to Design Point 5.

Basin B7 (Q_5= 37.42 cfs, Q_{100}= 77.19 cfs), modeled as a future developed portion of Interquest Filing No. 5 is 17.8 acres in area. Runoff from Basin B6 sheet flows across the basin/site to an existing storm sewer system at the southeast corner of Federal Drive and Republic Drive and continues to Design Point 5. At this location, an existing 60" RCP (Conveyance 7) will accept both the 5-year and 100-year flows from Basin B6, Basin B7 and Conveyance 6, (Q_5= 145.19 cfs, Q_{100}= 374.92 cfs, Capacity = 383.63 cfs). Conveyance 7 routes the flows to Design Point 25.

Basin B9 (Q_5= 9.25 cfs, Q_{100}= 18.59 cfs), a developed portion of the Interquest development, the Residence Inn development, located at the southwest corner of Federal Drive and Republic Drive is 4.1 acres in area. Runoff from Basin B9 sheet flows into/across the hotel parking lot into a storm sewer system that continues to Design Point 25.

Basin EK1A (Q_5= 8.15 cfs, Q_{100}= 16.36 cfs), modeled as a future developed portion of the Interquest development, located at the southwest corner of Federal Drive and Interquest Parkway is 3.6 acres in area. Runoff from Basin EK1A sheet flows across the drainage area into a storm sewer system to a location where a future 24" RCP (Conveyance 30), (minimum slope 0.5%) will accept both the 5-year and 100-year flows, (Q_5= 8.06 cfs, Q_{100}= 16.29 cfs, Capacity = 16.00 cfs). Conveyance 30 routes the flows to Design Point 25.

Basin EK1B (Q_5= 17.26 cfs, Q_{100}= 34.78 cfs), modeled as a future developed portion of the Interquest development, located at the northwest corner of Federal Drive and Republic Drive is 7.7 acres in area. Runoff from Basin EK1B sheet flows across the drainage area into a storm sewer system to a location, Design Point 25, (Q_5= 175.09 cfs, Q_{100}= 438.38 cfs) where a future 60" RCP, (min. slope 3.06%), (Conveyance 31) will accept both the 5-year and 100-year flows (Q_5= 175.09 cfs, Q_{100}= 438.38 cfs, Capacity = 455.56 cfs). Conveyance 31 routes the flows to Design Point 27.

Basin EK1C (Q_5= 10.89 cfs, Q_{100}= 21.84 cfs), modeled as a future developed portion of the Interquest development, located to the south of Interquest Parkway, to the west of Federal Drive and the east of a proposed roadway. Runoff from Basin EK1C sheet flows across the drainage area into a

storm sewer system to a location where a future 30" RCP, (min. slope 0.50%), (Conveyance 32) will accept both the 5-year and 100-year flows (Q_5= 10.70 cfs, Q_{100}= 21.68 cfs, Capacity = 29.00 cfs). Conveyance 32 routes the flows to Design Point 26.

Basin EK1D (Q_5= 17.95 cfs, Q_{100}= 36.68 cfs), modeled as a future developed portion of the Interquest development, located at the southeast corner of Interquest Parkway and a proposed roadway is 8.3 acres in area. Runoff from Basin EK1D sheet flows across the drainage area into a storm sewer system to a location, Design Point 26, (Q_5= 28.65 cfs, Q_{100}= 58.36 cfs) where a future 42" RCP, (min. slope 0.50%), (Conveyance 33) will accept both the 5-year and 100-year flows (Q_5= 28.40 cfs, Q_{100}= 58.12 cfs, Capacity = 71.14 cfs). Conveyance 33 routes the flows to Design Point 27.

Basin EK1E (Q_5= 26.87 cfs, Q_{100}= 54.89 cfs), modeled as a future developed portion of the Interquest development, located at the northeast corner of Republic Drive and a proposed roadway is 12.4 acres in area. Runoff from Basin EK1E sheet flows across the drainage area into a storm sewer system to a location, Design Point 27, (Q_5= 229.97 cfs, Q_{100}= 549.59 cfs) where a future 78" RCP, (min. slope 1.25%), (Conveyance 34) will accept both the 5-year and 100-year flows (Q_5= 229.97 cfs, Q_{100}= 549.59 cfs, Capacity = 586.13 cfs). Conveyance 34 routes the flows to Design Point 29.

Basin EK1F (Q_5= 11.97 cfs, Q_{100}= 24.43 cfs), modeled as a future developed portion of the Interquest development, located at the northwest corner of Federal Drive and a proposed roadway is 5.5 acres in area. Runoff from Basin EK1F sheet flows across the drainage area into a storm sewer system to a location where a future 30" RCP, (min. slope 0.50%), (Conveyance 35) will accept both the 5-year and 100-year flows (Q_5= 11.96 cfs, Q_{100}= 24.36 cfs, Capacity = 29.00 cfs). Conveyance 35 routes the flows to Design Point 28.

Basin EK1G (Q_5= 9.97 cfs, Q_{100}= 20.36 cfs), modeled as a future developed portion of the Interquest development, located between Republic Drive and a proposed roadway is 4.6 acres in area. Runoff from Basin EK1G sheet flows across the drainage area into a storm sewer system to a location, Design Point 28, (Q_5= 21.93 cfs, Q_{100}= 44.71 cfs) where a future 36" RCP, (min. slope 0.50%), (Conveyance 36) will accept both the 5-year and 100-year flows (Q_5= 21.86 cfs, Q_{100}= 44.68 cfs, Capacity = 47.16 cfs). Conveyance 36 routes the flows to Design Point 29.

Basin EK1H (Q_5= 23.44 cfs, Q_{100}= 47.96 cfs), modeled as a future developed portion of the Interquest development, located at the southeast corner of Republic Drive and a proposed roadway is 10.9 acres in area. Runoff from Basin EK1H sheet flows across the drainage area into a storm sewer system to a location, Design Point 29, (Q_5= 274.98 cfs, Q_{100}= 640.97 cfs) where a future 84" RCP, (min. slope 1.25%), (Conveyance 37) will accept both the 5-year and 100-year flows (Q_5= 274.98 cfs, Q_{100}= 640.97 cfs, Capacity = 714.20 cfs). Conveyance 37 routes the flows to Design Point 30.

Basin EK1I (Q_5= 13.38 cfs, Q_{100}= 26.85 cfs), modeled as a future developed portion of the Interquest development, located at the northwest corner of Federal Drive and a proposed roadway is 5.9 acres in area. Runoff from Basin EK1I sheet flows across the drainage area into a storm sewer system to a location, Design Point 30, (Q_5= 287.11 cfs, Q_{100}= 667.00 cfs) where a future 84" RCP, (min. slope 1.25%), (Conveyance 38) will accept both the 5-year and 100-year flows (Q_5= 287.11 cfs, Q_{100}= 667.00 cfs, Capacity = 714.20 cfs). Conveyance 38 routes the flows to Design Point 31.

Basin EK1J (Q_5= 5.05 cfs, Q_{100}= 10.10 cfs), modeled as a future developed portion of the Interquest development, is 2.2 acres in area. Runoff from Basin EK1J sheet flows across the drainage area into a storm sewer system to a location, Design Point 31, (Q_5= 291.36 cfs, Q_{100}= 676.51 cfs) where a future 84" RCP, (min. slope 1.25%), (Conveyance 39) will accept both the 5-year and 100-year flows (Q_5= 291.36 cfs, Q_{100}= 676.51 cfs, Capacity = 714.20 cfs). Conveyance 39 routes the flows to Design Point 7, detention pond EK1A.

Basin EK1K (Q_5= 10.76 cfs, Q_{100}= 21.68 cfs), modeled as a future developed portion of the Interquest development, located at the southwest corner of Republic Drive and a proposed roadway is 4.8 acres in area. Runoff from Basin EK1K sheet flows across the drainage area into a storm sewer system to a location where a future 30" RCP, (min. slope 0.50%), (Conveyance 40) will accept both the 5-year and 100-year flows (Q_5= 10.63 cfs, Q_{100}= 21.55 cfs, Capacity = 29.00 cfs). Conveyance 40 routes the flows to Design Point 32.

Basin EK1L (Q_5= 15.35 cfs, Q_{100}= 31.25 cfs), modeled as a future developed portion of the Interquest development, is 7.0 acres in area. Runoff from Basin EK1L sheet flows across the drainage area into a storm sewer system to a location, Design Point 32, (Q_5= 25.96 cfs, Q_{100}= 52.81

cfs) where a future 42" RCP, (min. slope 0.50%), (Conveyance 41) will accept both the 5-year and 100-year flows (Q_5= 25.93 cfs, Q_{100}= 52.66 cfs, Capacity = 71.14 cfs). Conveyance 41 routes the flows to Design Point 7, detention pond EK1A.

Basin A1 (Q_5= 104.23 cfs, Q_{100}= 221.48 cfs), modeled as a fully developed portion of Fairlane Technology Park, Filing No. 1 is 56.9 acres in area. Runoff from this basin will sheet flow across the basin/site area to a future concrete lined swale flowing south on the east side of State Highway 83. At this location, the swale (Conveyance 9) will accept both the 5-year and 100-year flows (Q_5= 102.76 cfs, Q_{100}= 219.81 cfs, Capacity = 1720.39 cfs). Conveyance 9 routes the flows to Design Point 8.

Basin A2 (Q_5= 188.27 cfs, Q_{100}= 403.06 cfs), modeled as a fully developed portion of Fairlane Technology Park, Filing No. 1 is 108.0 acres in area. Runoff from this basin will sheet flow across the basin/site area to Design Point 8, (Q_5= 291.03 cfs, Q_{100}= 622.87 cfs) At this location an existing concrete lined channel (Conveyance 10) will accept both the 5-year and 100-year flows (Q_5= 288.67 cfs, Q_{100}= 620.32 cfs, Capacity = 941.35 cfs). Conveyance 10 routes the flows to Design Point 9.

Basin C1 (Q_5= 6.74 cfs, Q_{100}= 53.92 cfs) is an undeveloped / unplatted area, not treated as part of the master planning area in previous studies is 62.5 acres in area. This area represents a new development that has diverted 35 acres from the Kettle Creek Basin into the Elkhorn-1 Basin recently. The additional area is modeled in the fully developed condition as well as the filing no. 6 & 7 conditions. Runoff from this basin will sheet flow across the basin/site area to an existing storm sewer system (Conveyance 11) to Design Point 10. Stormwater quality and detention measures for future development within this basin are planned to be provided within the development area. Developed flows from this basin are to be equal or less than the historic flows.

Basin D1, the Quantum Development, Fairlane Technology Park Filing No. 4, (Q_5= 80.73 cfs, Q_{100}= 168.68 cfs) is 40.4 acres. Runoff from this basin will continue to act as described in the Existing Conditions portion of this report. At this location, an existing 48" RCP (Conveyance 12) will accept the 5-year flows (Q_5= 68.34 cfs, Q_{100}= 171.89 cfs, Capacity = 113.10 cfs), flows not accepted by the conveyance are passed through an existing grass lined swale to Design Point 9.

Basin A3 (Q_5= 52.68 cfs, Q_{100}= 108.56 cfs), is modeled as fully developed, Fairlane Technology Park, Filing No. 3 is 25.0 acres in area. Runoff from this basin will sheet flow across the basin/site area to Design Point 9 (Q_5= 394.97 cfs, Q_{100}= 862.68 cfs). At this location a concrete lined channel (Conveyance 13) will accept both the 5-year and 100-year flows (Q_5= 393.32 cfs, Q_{100}= 860.97 cfs, Capacity = 2191.56 cfs). Conveyance 13 routes the flows to Design Point 11.

Basin A4 (Q_5= 3.38 cfs, Q_{100}= 10.09 cfs) acts as described in the Existing Conditions portion of this report.

Basin A5 (Q_5= 13.77 cfs, Q_{100}= 28.55 cfs), is modeled as fully developed, the Ford Microelectronics development, a portion of the Fairlane Technology Park Filing No. 2, is 6.7 acres in area. Runoff from this basin will sheet flow across the basin/site area to Design Point 11, (Q_5= 407.63 cfs, Q_{100}= 895.08 cfs) At this location a concrete lined channel (Conveyance 14) will accept both the 5-year and 100-year flows (Q_5= 407.25 cfs, Q_{100}= 893.80 cfs, Capacity = 1882.71 cfs). Conveyance 14 routes the flows to Design Point 7, the existing detention pond EK1A.

Basin A7 (Q_5= 49.61 cfs, Q_{100}= 103.86 cfs), is modeled as fully developed, the Lockheed Martin development, a portion of the Fairlane Technology Park Filing No. 2, is 25.1 acres in area. Runoff from this basin will sheet flow across the basin/site area to a future 48" RCP storm sewer system, Conveyance 43, will accept both the 5-year and 100-year flows (Q_5= 49.49 cfs, Q_{100}= 103.68 cfs, Capacity = 101.57 cfs, minimum slope 0.50%). Conveyance 43 routes the flows to Design Point 33.

Basin A8 (Q_5= 47.48 cfs, Q_{100}= 100.04 cfs), is modeled as fully developed, the Lockheed Martin development, a portion of the Fairlane Technology Park Filing No. 2, is 24.7 acres in area. Runoff from this basin will sheet flow across the basin/site area to a 30" RCP storm sewer, Conveyance 15, will accept the 5-year flows (Q_5= 47.48 cfs, Q_{100}= 100.04 cfs, Capacity = 64.85 cfs) flows not accepted by the conveyance are passed through the existing curbed roadway, Federal Drive to existing storm sewer inlets at Design Point 12.

Basin A9 (Q_5= 54.95 cfs, Q_{100}= 114.82 cfs), is modeled as fully developed, a portion of the Interquest Filing No. 1 development, is 27.5 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from

Conveyance 15 at Design Point 12 (Q_5= 102.40 cfs, Q_{100}= 214.67 cfs) draining into a future 54" RCP storm sewer, Conveyance 42, will accept both the 5-year and 100 year flows (Q_5= 102.40 cfs, Q_{100}= 214.67 cfs, Capacity = 278.09 cfs, minimum slope 2.00%). Conveyance 42 routes the flows to Design Point 33.

Basin EK1M (Q_5= 8.65 cfs, Q_{100}= 17.35 cfs), is modeled as future developed portion of the Interquest development, is 3.8 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 42 at Design Point 33 (Q_5= 158.51 cfs, Q_{100}= 333.48 cfs) draining into a future 54" RCP storm sewer, Conveyance 44, will accept both the 5-year and 100 year flows (Q_5= 158.51 cfs, Q_{100}= 333.48 cfs, Capacity = 351.76 cfs, minimum slope 3.20%). Conveyance 44 routes the flows to Design Point 34.

Basin EK1N (Q_5= 25.22 cfs, Q_{100}= 51.48 cfs), is modeled as future developed portion of the Interquest development, is 11.6 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 44 at Design Point 34 (Q_5= 183.30 cfs, Q_{100}= 384.00 cfs) draining into a future 54" RCP storm sewer, Conveyance 45, will accept both the 5-year and 100 year flows (Q_5= 183.30 cfs, Q_{100}= 384.00 cfs, Capacity = 351.76 cfs, minimum slope 3.20%). Conveyance 45 routes the flows to Design Point 7, detention pond EK1A.

Basin A6 (Q_5= 24.83cfs, Q_{100}= 57.02 cfs), detention pond EK1A, is 14.7 acres in area. Runoff from this basin will sheet flow directly into the existing detention pond EK1A at Design Point 7, (Q_5= 891.81 cfs, Q_{100}= 1999.54 cfs).

The storm runoff entering detention ponds EK1A in the Fully Developed Conditions (Design Point 7) are: (Q_5= 891.8 cfs, Q_{100}= 1999.5 cfs). The restricted flows leaving the pond with the proposed storm water quality outlet box are: (Q_5= 70.5 cfs, Q_{100}= 166.3 cfs), 100 year ponding elevation = 6623.00, with a storage volume of 86.17 ac-ft to the 100-yr water surface elevation. The current City of Colorado Springs Drainage Criteria states that detention ponds must be designed to contain the 100-yr storm runoff (as described) plus an additional volume of half of the required storm water quality volume. The required storm water quality volume is calculated at 8.68 ac.-ft. (50% = 4.34 ac.ft.). The proposed emergency spillway will be raised to an elevation of 6623.50, instead of the

typical 100-yr W.S.E. This provides an additional pond volume of 5.24 ac.-ft., adhering to the drainage criteria.

The State of Colorado classifies a detention pond as a "Jurisdictional Dam" as a "dam which impounds water above the elevation of the natural surface of the ground creating a reservoir with a capacity of more than 100 acre-feet, or creates a reservoir with a surface area in excess of 20 acres at the high water line, or exceeds 10 feet in height measured vertically from the elevation of the lowest point of the natural surface of the ground where that point occurs along the longitudinal centerline of the dam up to the flowline crest of the emergency spillway. For reservoirs created by excavation, or where the invert of the outlet conduit is placed below the surface of the natural ground at its lowest point beneath the dam, the jurisdictional height shall be measured from the invert of the outlet at the longitudinal centerline of the dam, which ever is greatest." This pond has a capacity of 85.37 acre-feet, a high water line area of 10.36 acres, and only 9.5' in height from lowest natural ground point at spillway (conservatively estimated at 6614.00). However, the outlet has been constructed below the surface of the 'natural ground', therefore the jurisdictional height is measured from the invert of the outlet pipe to the height of the spillway (6623.50 – 6608.60 = 14.9'). Therefore, this pond will be classified as a "Jurisdiction Dam/Pond". Classic Consulting has been in contact with William McCormick, State of Colorado Engineer and he agrees that this pond will become a Jurisdictional Pond and be subject to plan approval by the State of Colorado. A preliminary examination of the 54" outlet pipe/structure has taken place by Classic Consulting and Bill McCormick. It has been determined that this outlet is in good condition to use with a few modifications per the State criteria. These modifications, and all final pond improvement plans, will be issued as construction documents with both the City of Colorado Springs and State of Colorado Engineers' approval.

The existing spillway was constructed at an elevation of 6619.60 and length of 85.0'. This gave a depth of design flood flow of 2.2' through the emergency spillway. Decreasing this allowable depth of water through the spillway, a proposed spillway is designed at a length of 275.00' and elevation of 6623.50. This will convey the entire 100-yr developed flows (2000 cfs) in case of a complete pond outlet failure with a depth of only 1.77'.

Adding 2.0' freeboard to the depth of emergency flow (City of Colorado Springs minimum criteria) gives a minimum crest elevation of 6623.50' + 1.8' + 2.0' = 6627.30. The existing detention pond

will be re-graded to achieve the volumes and elevations described. See in the appendix of this report a drawing showing a preliminary plan for the fully developed pond improvements at pond EK-1A.

The existing two-staged outlet structure will be modified to achieve the desired storm water quality and outlet release. Currently the outlet structure consists of a separate 18" orifice structure in front of an existing riser box with trash rack. The riser box is in very good condition and will remain to serve as the main pond outlet. The 18" orifice structure will be removed and replaced with the proposed water quality orifice structure with orifice plate at the same location. The water quality volume required for the developed areas in Basins EK1A, EK1B, EK1C, EK1D, EK1E, EK1F, EK1G, EK1H, EK1I, EK1J, EK1K, EK1L, EK1M, EK1N, A1, A2, A3 (6.25 ac.), B5, B6 and B7 (305.20 acres of future development) is 8.679 acre-feet. The rooftop storm runoff for the future developments is exempt from this requirement since sediment trap features are proposed to intercept the roof drain flows prior to connection to the storm system. To obtain the required water quality volume, a top of orifice plate elevation of 6613.80 is needed. The proposed top of orifice plate is at 6614.00, providing 9.74 ac.-ft. of water quality volume, and the top of the outlet riser box is at 6615.80, providing even more water quality volume.

The water quality orifice is planned to be a metal plate with an invert of 6609.00 and a top elevation 6614.00. The orifice plate will have 3 columns and 15 rows of 1.50" diameter holes. The proposed concrete riser has inside dimensions of 3.0' x 3.0' and will contain an open top with trash rack. The combination of the existing riser box and proposed new SWQ box adequately reduces the outfall runoff to the existing 54" pipe to allowable rates. The 54" RCP pipe discharges through a baffle wall into a rip-rap lined channel, Conveyance 18. Detailed construction plans would be prepared and approved by the City and State prior to any reconstruction of the detention pond. Routed flows from detention pond EK1A drain into an existing grass / rip-rap lined swale, Conveyance 18, accepting both the 5-year and 100 year flows (Q_5= 87.90 cfs, Q_{100}= 164.6 cfs, Capacity = 934.14 cfs). The maximum allowable release from this pond (Q_5= 126 cfs, Q_{100}= 170 cfs) was quantified within the original MDDP for Fairlane Technology Park Interquest South by Kiowa Engineering. The pond EK-1A, with proposed improvements, releases storm water at rates less than the allowable discharge.

Basin EK1O (Q_5= 0.37 cfs, Q_{100}= 2.37 cfs), is an undeveloped portion of the Interquest development, is 1.5 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale to Design Point 14, the existing 60" RCP culvert under Interstate 25.

Basin A12 (Q_5= 4.38 cfs, Q_{100}= 33.67 cfs), is the undeveloped area of the eastern half of the Interstate 25 right-of-way development north of Old Ranch Road south of the Elkhorn-2 drainage basin, is 31.1 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale to Design Point 14, the existing 60" RCP culvert under Interstate 25. At Design Point 14 (Q_5= 88.20 cfs, Q_{100}= 167.02 cfs), Drainage Basins A12 and EK1O and Conveyance 18 are combined at the entrance of the existing 60" RCP culvert. This point is analyzed as a detention pond to model the ponding at the culvert entrance. This pond is referred to as detention pond I25-1. The routed flows leaving the pond are: (Q_5= 88.20 cfs, Q_{100}= 167.02 cfs), 100 year ponding elevation = 6594.76. Previous studies have established a 100-year historic flow of 195 cfs for the basin at the inlet of the 60" culvert. The ponding elevation at this location is below the existing saddle elevation and the existing northbound lane elevation where the flows would divert across the Interstate or to the south into the Kettle Creek Basin. The depth of ponding at this point is 6594.76 – 6589.0 = 5.76' 0.76' above the crown of the culvert. At elevation 6596.5 (7.5' head over the culvert invert) the drainage would begin to bypass the 60" culvert and flow to the south into the Kettle Creek Basin. The maximum flow entering the 60" culvert (point at which it diverts to the south) is calculated to be 211 cfs. The basin runoff in the fully developed condition is modeled to be less than the historic flow for the basin at the 60" culvert and would be safely conveyed to the receiving Air Force Academy drainage system downstream eventually draining into Monument Creek.

ELKHORN-2 DRAINAGE BASIN

Basin IP-3 (Q_5= 3.16 cfs, Q_{100}= 7.28 cfs), continue to DP-15 as described in the Existing Conditions portion of the report (See above).

Basin IP-4 (Q_5= 4.89 cfs, Q_{100}= 11.28 cfs), the southern half of the Interquest Parkway right-of-way west of Federal Drive and east of the Interstate 25 northbound off-ramp, is 3.1 acres in area. At this location the captured drainage area is conveyed to Design Point 15 through the existing storm sewer

system. From this point the captured flows from Basin IP-3 and Basin IP-4 are planned to be routed through a future 30" RCP, Conveyance 46, accepting the both the 5-year and 100-year flows (Q_5= 8.05 cfs, Q_{100}= 18.56 cfs, Capacity = 29.00 cfs, minimum slope 0.50%) to Design Point 35.

Basin EK2A (Q_5= 22.88 cfs, Q_{100}= 47.71 cfs), is modeled as a fully developed portion of the Interquest development, is 11.1 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 46 at Design Point 35 (Q_5= 30.88 cfs, Q_{100}= 66.17 cfs) draining into a future 42" RCP storm sewer, Conveyance 47, will accept both the 5-year and 100 year flows (Q_5= 30.88 cfs, Q_{100}= 66.17 cfs, Capacity = 71.14 cfs, minimum slope 0.50%). Conveyance 47 routes the flows to Design Point 37, detention pond EK2A.

Basin EK2B (Q_5= 11.87 cfs, Q_{100}= 24.19 cfs), is modeled as a fully developed portion of the Interquest development, is 5.4 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system draining into a future 30" RCP storm sewer, Conveyance 48, will accept both the 5-year and 100 year flows (Q_5= 11.87 cfs, Q_{100}= 24.19 cfs, Capacity = 29.00 cfs, minimum slope 0.50%). Conveyance 48 routes the flows to Design Point 36.

Basin EK2C (Q_5= 21.32 cfs, Q_{100}= 44.30 cfs), is modeled as a fully developed portion of the Interquest development, is 10.2 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system draining into a future 42" RCP storm sewer, Conveyance 49, will accept both the 5-year and 100 year flows (Q_5= 33.19 cfs, Q_{100}= 68.49 cfs, Capacity = 71.14 cfs, minimum slope 0.50%). Conveyance 49 routes the flows to Design Point 37, detention pond EK2A.

Basin EK2D (Q_5= 12.85 cfs, Q_{100}= 29.50 cfs), is modeled as a fully developed portion of the Interquest development, is 7.6 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system draining into detention pond EK2A at Design Point 37 (Q_5= 76.62 cfs, Q_{100}= 164.15 cfs).

Runoff from this Basin EK2D and Conveyances 47 and 47 combine at Design Point 27, Detention pond EK2A. The developed flows are routed by this pond (Q_5= 0.68 cfs, Q_{100}= 8.81 cfs), 100 year

ponding elevation = 6625.04 with a 100 year storage volume of 6.24 acre-feet. The water quality volume required for the developed areas in Basins EK2A, EK2B, EK2C and EK2D (30.20 acres of future development) is 0.859 acre-feet. The water quality orifice is planned to be a metal plate attached to a 4' x 4' grated open top concrete riser, with an invert of 6619.0 and a top elevation 6624.75. The orifice plate will have two rows, with 7 circular orifices with a diameter of 1.000 inch each, from elevation 6619.0 to 6621.3. The detention pond emergency spillway crest elevation would be 6625.1, above the 100-year ponding elevation, with a width of 50', with a flow depth of 1.0'. Adding 2' freeboard to the depth of emergency flow would yield a future minimum crest elevation of 6625.1' + 1.0' + 2.0' = 6628.1. Detailed construction plans will be prepared and approved by the City prior to any construction of the detention pond.

The detention pond would release into an existing grass lined swale, Conveyance 20 accepting both the 5-year and 100-year flows (Q_5= 0.68 cfs, Q_{100}= 8.81 cfs, Capacity = 432.24 cfs). Conveyance 20 routes the flows to Design Point 17 the existing 24" RCP culvert crossing Interstate 25.

Basin EK2E (Q_5= 0.66 cfs, Q_{100}= 4.32 cfs), is an undeveloped portion of the Interquest development, is 2.6 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale to Design Point 17, the existing 24" RCP culvert under Interstate 25.

Basin E5 (Q_5= 4.53 cfs, Q_{100}= 35.43 cfs), the undeveloped eastern half of the Interstate 25 right-of-way north of the Elkhorn-1 drainage basin and south of Interquest Parkway is 34.3 acres in area. Runoff from this basin will sheet flow across the basin area and combined with Basin EK2E and Conveyance 20 at Design Point 17, the existing 24" RCP culvert under Interstate 25 (Q_5= 5.27 cfs, Q_{100}= 37.35 cfs). This point is analyzed as a detention pond to model the ponding at the culvert entrance. This pond is referred to as detention pond I25-2. The routed flows leaving the pond are: (Q_5= 0.91 cfs, Q_{100}= 16.11 cfs), 100 year ponding elevation = 6601.22. Previous studies have established a 100-year historic flow of 40 cfs for the basin at the inlet of the 24" culvert. The ponding elevation at this location is below the existing saddle elevation and the existing northbound lane elevation where the flows would divert across the Interstate or to the south into the Elkhorn-1 drainage basin previously discussed. The depth of ponding at this point is 6601.2 – 6599.0 = 2.2' 0.2' above the crown of the culvert. At elevation 6604.5 (5.5' head over the culvert invert) the drainage would begin to bypass the 24" RCP culvert and flow to the south into the Elkhorn-1 Basin.

The maximum flow entering the 24" culvert (point at which it diverts to the south) is calculated to be 36 cfs. The basin runoff in the existing condition is less than the historic flow for the basin at the 24" culvert and is safely conveyed to the receiving Air Force Academy drainage system downstream eventually draining into Monument Creek.

FILING NO. 6 & 7 INTERIM CONDITIONS

ELKHORN-1 DRAINAGE BASIN

The description of the storm runoff from Basins O1, B2, B1, O2B, B3, VP-1, IP-1, and IP-2 remain the same as in the Existing Condition analysis (See above).

Basin B5 (Q_5= 6.04 cfs, Q_{100}= 22.25 cfs), a portion of Interquest Filing No. 3, is 11.4 acres in area. Runoff from Basin B5 sheet flows into/across the basin/site to an existing storm sewer system at the First United Bank site and continues to Design Point 4. At this location, an existing 54" RCP (Conveyance 6) will accept both the 5-year and 100-year flows (Q_5= 64.45 cfs, Q_{100}= 203.36 cfs, Capacity = 278.09 cfs). Conveyance 6 routes the flows to Design Point 5.

Basin B6 (Q_5= 6.04 cfs, Q_{100}= 22.25 cfs) is 18.4 acres in area. Runoff from Basin B6 sheet flows into/across the basin/site to an existing storm sewer system at the northeast corner of Federal Drive and Republic Drive and continues to Design Point 5.

Basin B7 (Q_5= 7.57 cfs, Q_{100}= 31.13 cfs) is 17.8 acres in area. Runoff from Basin B6 sheet flows across the basin/site to an existing storm sewer system at the southeast corner of Federal Drive and Republic Drive and continues to Design Point 5. At this location, an existing 60" RCP (Conveyance 7) will accept both the 5-year and 100-year flows from Basin B6, Basin B7 and Conveyance 6, (Q_5= 74.53 cfs, Q_{100}= 254.85 cfs, Capacity = 383.63 cfs). Conveyance 7 routes the flows to Design Point 25.

Basin B9 (Q_5= 9.25 cfs, Q_{100}= 18.59 cfs), a developed portion of the Interquest development, the Residence Inn development, located at the southwest corner of Federal Drive and Republic Drive is 4.1 acres in area. Runoff from Basin B9 sheet flows into/across the hotel parking lot into a storm sewer system that continues to Design Point 25.

Basin EK1Q (Q_5= 14.01 cfs, Q_{100}= 46.75 cfs), modeled as a future partially (overlot grading, extension of Republic Drive and a portion of Allegiance Drive) developed portion of the Interquest development, located at the northwest corner of Federal Drive and Republic Parkway is 23.8 acres in area. Runoff from Basin EK1Q sheet flows across the drainage area into a storm sewer system to Design Point 27 where a future 78" RCP (Conveyance 34), (minimum slope 1.25%) will accept both the 5-year and 100-year flows, (Q_5= 111.36 cfs, Q_{100}= 358.73 cfs, Capacity = 473.47 cfs). Conveyance 34 routes the flows to Design Point 29.

Basin EK1C (Q_5= 10.89 cfs, Q_{100}= 21.84 cfs), modeled as a future developed portion of the Interquest development, located to the south of Interquest Parkway, to the west of Federal Drive and the east of a proposed roadway. Runoff from Basin EK1C sheet flows across the drainage area into a storm sewer system to a location where a future 30" RCP, (min. slope 0.50%), (Conveyance 32) will accept both the 5-year and 100-year flows (Q_5= 10.49 cfs, Q_{100}= 21.43 cfs, Capacity = 29.00 cfs). Conveyance 32 routes the flows to Design Point 26.

Basin EK1D (Q_5= 17.95 cfs, Q_{100}= 36.68 cfs), modeled as a future developed portion of the Interquest development, located at the southeast corner of Interquest Parkway and a proposed roadway is 8.3 acres in area. Runoff from Basin EK1D sheet flows across the drainage area into a storm sewer system to a location, Design Point 26, (Q_5= 28.44 cfs, Q_{100}= 58.08 cfs) where a future 42" RCP, (min. slope 0.50%), (Conveyance 33) will accept both the 5-year and 100-year flows (Q_5= 28.14 cfs, Q_{100}= 57.74 cfs, Capacity = 71.14 cfs). Conveyance 33 routes the flows to Design Point 27.

Basin EK1R (Q_5= 4.17 cfs, Q_{100}= 25.37 cfs), modeled as a future partially developed (grading of a portion of Allegiance Drive) portion of the Interquest development, located between Federal Drive, Republic Drive and Allegiance Drive is 20.2 acres in area. Runoff from Basin EK1R sheet flows across the drainage area into a storm sewer system to a location, Design Point 29, (Q_5= 115.12 cfs, Q_{100}= 383.38 cfs) where a future 84" RCP, (min. slope 1.25%), (Conveyance 37) will accept both the 5-year and 100-year flows (Q_5= 115.12 cfs, Q_{100}= 383.38 cfs, Capacity = 714.20 cfs). Conveyance 37 routes the flows to Design Point 7.

Basin EK1S (Q_5= 3.05 cfs, Q_{100}= 11.78 cfs), modeled as a future developed portion of the Interquest development, located at the northeast corner of Federal Drive and Allegiance Drive is 6.2 acres in area. Runoff from Basin EK1S sheet flows across the drainage area into a storm sewer system to a Design Point 7.

Basin A1 (Q_5= 14.97 cfs, Q_{100}= 73.26 cfs), modeled as a partially developed portion of Fairlane Technology Park, Filing No. 1 is 56.9 acres in area. Basin A1 is a portion of the current development named Colorado Crossing. This basin has approximately 11 acres being developed of the 56.9 acres overall. Runoff from this basin will sheet flow across the basin/site area to a future concrete lined swale flowing south on the east side of State Highway 83. At this location, the swale (Conveyance 9) will accept both the 5-year and 100-year flows (Q_5= 12.35 cfs, Q_{100}= 67.91 cfs, Capacity = 365.58 cfs). Conveyance 9 routes the flows to Design Point 8.

Basin A2 (Q_5= 16.85 cfs, Q_{100}= 107.29 cfs), modeled as a partially developed portion of Fairlane Technology Park, Filing No. 1 is 108.0 acres in area. Basin A2 is a portion of the current development named Colorado Crossing. This basin has approximately 8 acres being developed of the 108 acres overall. Runoff from this basin will sheet flow across the basin/site area to Design Point 8, (Q_5= 28.74 cfs, Q_{100}= 174.40 cfs) At this location an existing concrete lined channel (Conveyance 10) will accept both the 5-year and 100-year flows (Q_5= 28.06 cfs, Q_{100}= 172.86 cfs, Capacity = 1297.29 cfs). Conveyance 10 routes the flows to Design Point 9.

Basin C1 (Q_5= 6.74 cfs, Q_{100}= 53.92 cfs), an undeveloped / unplatted area, not treated as part of the master planning area in previous studies is 62.5 acres in area. This area represents a new development that has diverted 35 acres from the Kettle Creek Basin into the Elkhorn-1 Basin recently. The additional area is modeled in the fully developed condition as well as the Filing No. 6 & 7 Condition. Runoff from this basin will sheet flow across the basin/site area to an existing storm sewer system (Conveyance 11) to Design Point 10. Stormwater quality and detention measures for future development within this basin are planned to be provided within the development area. Developed flows from this basin are to be equal or less than the historic flows.

Basin D1, the Quantum Development, Fairlane Technology Park Filing No. 4, acts as described in the Existing Conditions.

Basin A3 & Basin A4 are the same as described in the Existing Conditions of this report. Runoff from these basins will sheet flow across the basin/site area to Design Point 11, (Q_5= 110.74 cfs, Q_{100}= 387.77 cfs) At this location a concrete lined channel (Conveyance 14) will accept both the 5-year and 100-year flows (Q_5= 110.59 cfs, Q_{100}= 387.67 cfs, Capacity = 1882.71 cfs). Conveyance 14 routes the flows to Design Point 7, the existing detention pond EK1A.

Basin A7 (Q_5= 26.33 cfs, Q_{100}= 70.36 cfs), is modeled as partially developed, the Lockheed Martin development, a portion of the Fairlane Technology Park Filing No. 2, is 18.8 acres in area. Runoff from this basin will sheet flow across the basin/site area to a future 48" RCP storm sewer system, Conveyance 43, will accept both the 5-year and 100-year flows (Q_5= 26.33 cfs, Q_{100}= 70.36 cfs, Capacity = 101.57 cfs, minimum slope 0.50%). Conveyance 43 routes the flows to Design Point 33.

Basin A8 (Q_5= 30.97 cfs, Q_{100}= 75.24 cfs), is modeled as partially developed, the Lockheed Martin development, a portion of the Fairlane Technology Park Filing No. 2, is 24.7 acres in area. Runoff from this basin will sheet flow across the basin/site area to a 30" RCP storm sewer, Conveyance 15, will accept the 5-year flows (Q_5= 30.97 cfs, Q_{100}= 75.24 cfs, Capacity = 56.24 cfs) flows not accepted by the conveyance are passed through the existing curbed roadway, Federal Drive to existing storm sewer inlets at Design Point 12.

Basin A9 (Q_5= 40.67 cfs, Q_{100}= 93.39 cfs), is modeled as a partially developed, a portion of the Interquest Filing No. 1 development, is 27.5 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 15 at Design Point 12 (Q_5= 71.50 cfs, Q_{100}= 168.43 cfs) draining into a future 54" RCP storm sewer, Conveyance 42, will accept both the 5-year and 100 year flows (Q_5= 71.50 cfs, Q_{100}= 168.43 cfs, Capacity = 278.09 cfs, minimum slope 2.00%). Conveyance 42 routes the flows to Design Point 33.

Basin EK1M (Q_5= 8.65 cfs, Q_{100}= 17.35 cfs), is modeled as future developed portion of the Interquest development, is 3.8 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 42 at Design Point 33 (Q_5= 100.64 cfs, Q_{100}= 246.26 cfs) draining into a future 54" RCP storm sewer,

Conveyance 44, will accept both the 5-year and 100 year flows (Q_5= 100.64 cfs, Q_{100}= 246.26 cfs, Capacity = 351.76 cfs, minimum slope 3.20%). Conveyance 44 routes the flows to Design Point 34.

Basin EK1N (Q_5= 25.22 cfs, Q_{100}= 51.48 cfs), is modeled as future developed portion of the Interquest development, is 11.6 acres in area. Runoff from this basin will sheet flow across the basin/site area into an on-site storm sewer system and combine with flows from Conveyance 44 at Design Point 34 (Q_5= 121.22 cfs, Q_{100}= 291.99 cfs) draining into a future 54" RCP storm sewer, Conveyance 45, will accept both the 5-year and 100 year flows (Q_5= 121.22 cfs, Q_{100}= 291.99 cfs, Capacity = 351.76 cfs, minimum slope 3.20%). Conveyance 45 routes the flows to Design Point 7, detention pond EK1A.

Basin A6 (Q_5= 24.83 cfs, Q_{100}= 57.02 cfs), detention pond EK1A, is 14.7 acres in area. Runoff from this basin will sheet flow directly into the existing detention pond EK1A at Design Point 7, (Q_5= 357.91 cfs, Q_{100}= 1058.41 cfs).

The outlet structure for the existing detention pond EK1A is to be removed and replaced with the storm water quality structure described in the Fully Developed Conditions. Pond improvements, including increase of the spillway elevation, crest of pond berm, installation of fore bays and micropool, will be completed with the development of Interquest Filing 6 & 7.

The flows entering detention pond EK1A in the Interim Conditions (Design Point 7) are: (Q_5= 357.9 cfs, Q_{100}= 1058.4 cfs). The restricted flows leaving the pond thru the proposed storm water quality outlet box are: (Q_5= 28.6 cfs, Q_{100}= 128.0 cfs), 100 year ponding elevation = 6619.37, with a storage volume of 51.16 ac-ft. The emergency spillway elevation is proposed to be increase to 6623.50, far above the Interim 100 year ponding elevation. The Fully Developed outlet structure, including orifice plate and holes, adequately conveys the Interim design flows to a rate less than the Fully Developed release rate for all storm event levels. Constant reevaluation of the pond and outlet box as development within the entire tributary area occurs will be required within future drainage reports. Possible outlet box modifications may be necessary if released flows exceed the allowable rates. Detailed construction plans would be prepared and approved by the City prior to any reconstruction of the detention pond. Routed flows from detention pond EK-1A drain into an existing grass / rip-

rap lined swale, Conveyance 18, accepting both the 5-year and 100 year flows (Q_5= 28.6 cfs, Q_{100}= 128.0 cfs, Capacity = 934.14 cfs,)

Basin EK1T (Q_5= 2.41 cfs, Q_{100}= 16.18 cfs), is an undeveloped portion of the Interquest development, is 13.5 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale to Design Point 14, the existing 60" RCP culvert under Interstate 25.

Basin A12 (Q_5= 4.38 cfs, Q_{100}= 33.67 cfs), is the undeveloped area of the eastern half of the Interstate 25 right-of-way development north of Old Ranch Road south of the Elkhorn-2 drainage basin, is 31.1 acres in area. Runoff from this basin will sheet flow across the basin/site area to a shallow grass lined swale to Design Point 14, the existing 60" RCP culvert under Interstate 25. At Design Point 14 (Q_5= 29.50 cfs, Q_{100}= 133.05 cfs), Drainage Basins A12 and EK1O and Conveyance 18 are combined at the entrance of the existing 60" RCP culvert. This point is analyzed as a detention pond to model the ponding at the culvert entrance. This pond is referred to as detention pond I25-1. The routed flows leaving the pond are: (Q_5= 29.47 cfs, Q_{100}= 132.77 cfs), 100 year ponding elevation = 6593.93. Previous studies have established a 100-year historic flow of 195 cfs for the basin at the inlet of the 60" culvert. The ponding elevation at this location is below the existing saddle elevation and the existing northbound lane elevation where the flows would divert across the Interstate or to the south into the Kettle Creek Basin. The depth of ponding at this point is 6593.93 – 6589.0 = 4.93' inline with the crown of the culvert. At elevation 6596.5 (7.5' head over the culvert invert) the drainage would begin to bypass the 60" culvert and flow to the south into the Kettle Creek Basin. The maximum flow entering the 60" culvert (point at which it diverts to the south) is calculated to be 211 cfs. The basin runoff in the fully developed condition is modeled to be less than the historic flow for the basin at the 60" culvert and would be safely conveyed to the receiving Air Force Academy drainage system downstream eventually draining into Monument Creek.

ELKHORN-2 DRAINAGE BASIN

Basin IP-3 & Basin IP-4 act as described in the Existing Conditions section of this report (see above).

Basin EK2H (Q_5= 5.96 cfs, Q_{100}= 41.55 cfs) is an undeveloped portion of the Interquest Development, is 39.3 acres in area. Runoff from this basin will sheet flow across the basin area to an existing shallow grass lined swale, Conveyance 20 accepting both the 5-year and 100-year flows (Q_5= 5.32 cfs, Q_{100}= 40.23 cfs, Capacity = 432.24 cfs). Conveyance 20 routes the flows to Design Point 17 the existing 24" RCP culvert crossing Interstate 25. Drainage Basin EK2H and Conveyances 20 and 22 are combined at the entrance of the existing 24" RCP culvert, Design Point 17 (Q_5= 15.28 cfs, Q_{100}= 90.65 cfs). This point is analyzed as a detention pond to model the ponding at the culvert entrance. This pond is referred to as detention pond I25-2. The routed flows leaving the pond are: (Q_5= 7.38 cfs, Q_{100}= 25.87 cfs), 100 year ponding elevation = 6602.51. Previous studies have established a 100-year historic flow of 40 cfs for the basin at the inlet of the 24" culvert. The ponding elevation at this location is below the existing saddle elevation and the existing northbound lane elevation where the flows would divert across the Interstate or to the south into the Elkhorn-1 drainage basin previously discussed. The depth of ponding at this point is 6602.5 – 6599.0 = 3.5' 1.5' above the crown of the culvert. At elevation 6604.5 (5.5' head over the culvert invert) the drainage would begin to bypass the 24" RCP culvert and flow to the south into the Elkhorn-1 Basin. The maximum flow entering the 24" culvert (point at which it diverts to the south) is calculated to be 36 cfs. The basin runoff in the existing condition is less than the historic flow for the basin at the 24" culvert and is safely conveyed to the receiving Air Force Academy drainage system downstream eventually draining into Monument Creek.

LOTS 1 & 2 – INTERQUEST FILING NO. 6 FINAL DRAINAGE REPORT

Based upon the existing and developed flow conditions outlined in the MDDP portion of this report, the following is a description of the individual developed basins, flows, and required storm facilities associated with Lots 1 & 2 of Interquest Filing No. 6. A large amount of existing developed area produces runoff tributary to the proposed site via an existing storm main that currently releases flows onto the proposed site as well as surface flows entering the site from the existing Federal Drive. Two drainage maps are provided for this portion of the report. Sheet 1 shows the larger scale basin and

existing tributary flows to the site, whereas Sheet 2 shows the more detailed drainage features of Lots 1 & 2 of Interquest Filing No. 6.

Design Points FA-1 thru FA-9 (Basins A8.2.1 thru A9.0.6):

The existing development of Lots 1-3, Fairlane Technology Park Filing No. 2, was studied in the "Ford Aerospace Filing No. 1 Preliminary and Final Drainage Report," dated October 9, 1990 by URS Consultants, Inc. In order to get an accurate quantity of storm runoff within the existing storm pipe and within Federal Drive, these 'upstream' design points and basins were modeled within this report.

Design Point FA-1: The runoff from the existing parking lot and building area of Basin A8.2.4 is intercepted by multiple inlets conveyed to the grass swale at FA-1. These flows run north then west to Design Point FA-2.

Design Point FA-2: The storm runoff from the all of existing Lot 2 (Basins A8.2.5, A8.2.3, FA-1) as well as from a portion of Federal Drive that drains into Lot 2 (Basin A8.2.8) collect at an existing depressed area (FA-2) east of the access road to Fairlane Lot 3. The quantity of runoff at this point is Q_5 = 35 cfs and Q_{100} = 69 cfs. An existing 24" storm pipe conveys the majority of these flows under the access road to FA-3. The runoff that overtops this existing depressed area, remains on the south of Federal Drive and still reaches FA-3.

Design Point FA-3: The storm runoff from FA-2, Basins A8.2.1 & A8.2.2, and existing Federal Drive Basin A8.2.7 collect at this existing 'ponding' area labeled FA-3. An existing 30" x 19" elliptical concrete pipe conveys a portion of the collected runoff to the west; releasing flows onto proposed Filing 6. A significant portion of this runoff overtops the exiting 'ponding' area and sheet flows onto the proposed site. See Design Point 22 for a description of the proposed conveyance and interception of these off-site flows.

Design Point FA-5: The runoff from the existing portion of Voyager Pkwy. and Federal Drive (Basin A9.0.5.) is split into multiple directions. This report models the runoff from this Basin as assumed in the previously approved report. A portion of this runoff continues to the south down

Voyager Parkway, approximately 7.8 cfs in the 10-yr storm event. The remaining runoff enters Lot 1 of Fairlane Filing 2 within an existing drive aisle and routes to Design Point FA-6.

Design Point FA-6: The runoff from Basin A9.0.4 and FA-5 collect at an existing inlet within the parking lot of Lot 1. The intercepted runoff is conveyed within an existing pipe to the grass swale/ponding area (FA-7). A portion of the runoff overtops this inlet and sheet flows into Federal Drive, to Design Point FA-8.

Design Point FA-7: The storm runoff collected at FA-6 and Basin A9.0.3 collect at this existing 'ponding' area labeled FA-7. An existing 30" RCP (Pipe 2) conveys the entire collected runoff to the west; toward the proposed site and regional detention pond EK-1.

Design Point FA-8: The storm runoff overtopping at FA-6 and Federal Drive Basin A9.0.6 collect at this existing sump inlet. The inlet was modeled according to the previously approved report assumptions. An existing 30" RCP conveys the entire collected runoff to the west; toward the proposed site and regional detention pond EK-1. Pipe 3 represents the existing 30" storm main after the interception of flows at this inlet/design point and the connection of the existing lateral picking up the entire runoff from Basin A9.0.2. Flows within this main are Q_5 = 40 cfs and Q_{100} = 65 cfs. The remainder of the runoff (Q_5 = 4 cfs, Q_{100} = 22 cfs) not collected by this sump inlet continues to the west as surface flow on Federal Drive to FA-9.

Design Point FA-9: Represents an existing 8.0' at-grade inlet that receives runoff from FA-8 as well as Basins A9.0.1 and EX-AA. Basin EX-AA is currently undeveloped and is assumed to sheet flow onto Federal Drive. This inlet only intercepts flows of approximately Q_5 = 4 cfs and Q_{100} = 10 cfs. The remaining flow continues within the curb and gutter of Federal to Design Point 10a. Pipe 4 represents the existing 30" main with the addition of the intercepted flows at this point (Q_5 = 45 cfs, Q_{100} = 74 cfs).

Design Points IF-1 thru IF-3 (Basins EX-D1 thru EX-OS3):

The existing development of Lots 1-2 of Interquest Filing No. 1B, and Lot 1 of Interquest Filing No. 1A, was studied in the "Interquest Filing No. 1 Preliminary and Final Drainage Plan and Report," revised January 2000 by Rockwell-Minchow Consultants, Inc. In order to get an accurate quantity of

storm runoff within the existing storm pipe and within Federal Drive, these 'upstream' design points and basins were modeled within this report. Time of concentrations of these basins was taken directly from the approved report. 'C values' were increased slightly to provide a more conservative estimate of tributary runoff.

Design Point IF-1: The runoff from the existing parking lot and building area of Basin EX-D6 and the runoff from Basin EX-OS3 is intercepted by an existing 8.0' sump inlet. These flows are conveyed to the south and west toward Design Point IF-2 by an existing 18" storm pipe.

Design Point IF-2: The runoff from the existing parking lot of Basin EX-D2 is collected by an existing 10.0' sump inlet. This runoff combines with that from IF-1 and is conveyed to the west within a 30" existing storm pipe (Pipe 5).

Design Point IF-3: The runoff from the existing parking lot and building area of Basin EX-D4 is collected by an existing 12.0' sump inlet. This runoff combines with that from IF-2 as well as the intercepted flow from Basin EX-D1. Pipe 6 (existing 30" RCP, Q_5 = 44 cfs, Q_{100} = 83 cfs) conveys all of this runoff to the existing 14' sump inlet within Federal Drive (DP-10a).

Design Points 10a/10b thru 28 (Proposed Lots 1 & 2 of Interquest Filing No. 6):

Design Point 10a: An existing 14.0' sump inlet is located at this point on the north side of Federal Drive, across from the center entrance into the Filing No. 6 site. This inlet was installed with the Interquest Filing No. 1 development. An existing 10.0' sump inlet sits adjacent to this inlet (See DP-10b). The runoff to the existing 14' inlet consists of the 'flow-by' from Design Point FA-9 and runoff from Basin EX-D3 (Q_5 = 12 cfs, Q_{100} = 35 cfs). The capacity of the north half of Federal Drive, east of DP-10a and prior to the proposed site entrance at DP-11 is calculated at 20 cfs with the approach road slope of 1.60%. Therefore, 15 cfs of the 100-yr storm runoff will overtop the crown of the road and enter the site as surface flow at DP-11. The capacity of the north half of Federal, just prior to the existing 14' inlet is 17.5 cfs. Therefore, 2.5 cfs on the 100-yr storm runoff overtops the crown prior to the sump inlet and enters the site at DP-12.

The inlet intercepts the remainder of the flow of DP-10a, as well as 9 cfs in the 5-year and 3.5 cfs in the 100-yr of inlet overtopping flow from DP-10b. The capacity of this existing 14' inlet is 21 cfs,

which is the amount intercepted by the inlet in both the 5-year and 100-year storm events. An existing storm main combines these intercepted flows from those of Pipe 6.

Design Point 10b: An existing 10.0' sump inlet is located at this point on the north side of Federal Drive, across from the center entrance into the Filing No. 6 site. This inlet was installed with the original construction of Federal Drive. An existing 14.0' sump inlet sits adjacent to this inlet (Sec DP-10a). The runoff to the existing 10' inlet consists of a majority of the Interquest Filing No. 1A site (Basins EX-OS2, EX-D7, EX-D5, & EX-D8). The runoff from these areas (Q_5 = 29 cfs, Q_{100} = 56 cfs) flows across Lot 1 of Interquest Filing No. 1, into Federal Drive, and south to the main low point. The capacity of the half of Federal Drive, north of DP-10a and prior to the proposed site entrance at DP-13 is calculated at 32 cfs with the approach road slope of 4.00%. Therefore, 24 cfs of the 100-yr storm runoff will overtop the crown of the road and enter the site as surface flow at DP-13. The capacity of the half of Federal, just prior to the existing 10' inlet is 26.2 cfs. Therefore, 5.8 cfs on the 100-yr storm runoff overtops the crown prior to the sump inlet and enters the site at DP-12.

The inlet intercepts the remainder of the flow of DP-10b, with 9 cfs in the 5-year and 3.5 cfs in the 100-yr of inlet overtopping the inlet and reaching DP-10a. The capacity of this existing 10' inlet is 17 cfs, which is the amount intercepted by the inlet in both the 5-year and 100-year storm events. The existing 10.0' inlet at the south side of Federal is to be removed to allow for the access roads into the proposed site. With this removal, some minor storm reconfiguration will be required within the street. Pipe 7, existing 30"/proposed 36" RCP, represents the combination of flows from Pipe 6 and these two sump inlets (Q_5 = 79 cfs, Q_{100} = 118 cfs). Pipe 8, a proposed 54" RCP, combines the intercepted flows from Pipe 6 with those from Pipe 4 (discussed at DP-FA-9). The quantity of runoff within this pipe is Q_5 = 113 cfs and Q_{100} = 175 cfs.

Design Point 11: Consists of the overtopping flow in the 100 year condition of DP-10a (15 cfs) and the existing Federal Drive Basin A8.2.6. These flows (Q_5 = 1 cfs, Q_{100} = 17 cfs) enter the site at this proposed site access road and sheet flow to the inlet at Design Point 16.

Design Point 12: Consists of the overtopping flow in the 5-year and 100-year condition of DP-10a & DP-10b and the existing Federal Drive Basins EX-D10 & EX-D11. These flows (Q_5 = 4cfs, Q_{100} = 16 cfs) enter the site at this proposed site access road and sheet flow to the inlet at Design Point 15.

Design Point 13: Consists of the overtopping flow in the 100-year condition of DP-10b (24 cfs) and the existing Federal Drive Basin EX-D9. These flows (Q_5 = 3 cfs, Q_{100} = 30 cfs) enter the site at this proposed site access road and sheet flow all the way down the access road to the inlet at Design Point 28. The curb line accepting these flows adjacent to the proposed building will be 8" vertical curb to handle the entire 100-year flow.

Design Point 14: Basin C consists of parking lot and landscape islands east of Building 2. A proposed 6.0' sump inlet will intercept this runoff (Q_5 = 4 cfs, Q_{100} = 7 cfs) and convey them to the south in an 18" RCP storm (Pipe 11). Area drains for the small landscape areas will be installed and designed with final construction drawings.

Design Point 15: Basin J consists of parking lot and landscape islands between the proposed Building 1 and Federal Drive. The runoff described at Design Point 12 is conveyed to this sump inlet within the drive aisle. A proposed 14.0' sump inlet will intercept this runoff (Q_5 = 6 cfs, Q_{100} = 19 cfs) and convey them to the north in a 24" RCP storm (Pipe 10, Q_5 = 8 cfs, Q_{100} = 38 cfs). Pipe 10 also contains the runoff collected at Design Point 16 and connects to the 54" RCP main (Pipe 12) coming from Federal Drive toward the Detention Pond. The runoff within this 54" main at this point is Q_5 = 123 cfs and Q_{100} = 216 cfs.

Design Point 16: Basin I consists of 0.81 acres of parking lot and landscape areas east of Building 1. The runoff described at Design Point 11 is conveyed to this sump inlet within the drive aisle. A proposed 16.0' sump inlet will intercept this runoff (Q_5 = 3 cfs, Q_{100} = 21 cfs) and convey them to the north in a 24" RCP storm (Pipe 9) to DP-15.

Design Point 17: The roof drain systems for both Hybrid building within the proposed site are separate systems that surround the building and collect at a StormCeptor Water Quality facility. This StormCeptor for Building 2 is at DP-17 and collects flows of Q_5 = 6 cfs and Q_{100} = 11 cfs. An 18" RCP (Pipe 13) conveys the outfall runoff from the StormCeptor to the 54" main (Pipe 14, Q_5 = 127 cfs, Q_{100} = 224 cfs).

Design Point 18: Basins D and M both produce runoff that is conveyed down the access drive between the proposed buildings to this 6.0' sump inlet. The proposed 6.0' sump inlet will intercept this runoff (Q_5 = 5 cfs, Q_{100} = 9 cfs) and convey them to the south in an 18" main (Pipe 15) to a storm manhole. This manhole combines the 54" main (Pipe 14) with Pipe 15 and outfalls in a 60" RCP toward the pond. The quantity of runoff within this 60" Pipe 16 is Q_5 = 127 cfs and Q_{100} = 224 cfs.

Design Point 19: Basin G consists of 0.82 acres of parking lot and landscape areas west of Building 2. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 3 cfs, Q_{100} = 7 cfs) and convey them to the south in an 18" RCP storm (Pipe 17) toward DP-20.

Design Point 20: Basin H consists of parking lot and landscape islands west of Building 2. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 3 cfs, Q_{100} = 6 cfs) and convey them to the south in an 18" RCP storm (Pipe 18, Q_5 = 6 cfs, Q_{100} = 12 cfs). Pipe 10 also contains the runoff collected at Design Point 19 and connects to the 60" RCP main (Pipe 20).

Design Point 21: Basin N consists of parking lot and landscape areas west of Building 1. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 3 cfs, Q_{100} = 6 cfs) and convey them to the north in an 18" RCP storm (Pipe 19). Pipe 19 connects to the 60" RCP main (Pipe 20) coming from Federal Drive toward the Detention Pond. The runoff within this 60" main that releases into the Detention Facility is Q_5 = 136 cfs and Q_{100} = 241 cfs.

Design Point 22: Basin O consists of 0.69 acres of parking lot and landscape areas east of Building 1. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 3 cfs, Q_{100} = 5 cfs) and convey them to storm main coming from Design Point FA-3. Currently the 30"x18" elliptical RCP does not adequately convey the complete developed flows and overtopping occurs in large storm events. A proposed 42" RCP (Pipe 21) is to be installed to replace this existing pipe and adequately intercept the entire off-site runoff to FA-3 (Q_5 = 54 cfs, Q_{100} = 106 cfs). Pipe 22 represents the runoff within the 42" pipe after the connection of DP-22.

Design Point 23: Basin P produces runoff that is conveyed down the parking stalls toward Building 1. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 4 cfs, Q_{100} = 7 cfs) and convey them to

the 42" main (Pipe 23, Q_5 = 58 cfs, Q_{100} = 113 cfs). This main runs south, combines with the runoff from DP-24, and connects to a proposed storm manhole.

Design Point 24: Basin Q consists of 0.61 acres of parking lot and landscape areas east of Building 1. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 2 cfs, Q_{100} = 4 cfs) and convey them to the west, combing with Pipe 23. The storm manhole releases the runoff in a 48" RCP west, toward the pond. The quantity of runoff within this 48" Pipe 24 is Q_5 = 57 cfs and Q_{100} = 112 cfs.

Design Point 25: The roof drain systems for both Hybrid building within the proposed site are separate systems that surround the building and collect at a StormCeptor Water Quality facility. This StormCeptor for Building 1 is at DP-25 and collects flows of Q_5 = 8 cfs and Q_{100} = 16 cfs. A 24" RCP (Pipe 25) conveys the outfall runoff from the StormCeptor to the 48" main (Pipe 26, Q_5 = 62 cfs, Q_{100} = 121 cfs).

Design Point 26: Basin R consists of 0.62 acres of parking lot and landscape areas west of Building 1. A proposed 4.0' sump inlet will intercept this runoff (Q_5 = 3 cfs, Q_{100} = 5 cfs) and convey them to the south in an 18" RCP storm (Pipe 27) toward DP-27.

Design Point 27: Basin S consists of parking lot, drive aisle, and landscape areas south of Building 1. A proposed 6.0' sump inlet will intercept this runoff (Q_5 = 5 cfs, Q_{100} = 10 cfs) and convey them to the south in an 18" RCP storm (Pipe 28, Q_5 = 8 cfs, Q_{100} = 15 cfs). Pipe 28 also contains the runoff collected at Design Point 26 and connects to the 48" RCP main (Pipe 29) at proposed storm manhole. The runoff within this 48" main that releases into the Detention Facility is Q_5 = 67 cfs and Q_{100} = 130 cfs.

Design Point 28: Basin A consists of 0.74 acres of parking lot, drive aisle, and landscape areas north of Building 2. The runoff described at Design Point 13 is conveyed to this sump inlet within the drive aisle that runs along the north side of Building 2. A proposed 18.0' sump inlet will intercept this runoff (Q_5 = 5 cfs, Q_{100} = 34 cfs) and convey them to the north in a 24" RCP storm to the 84" storm main that is planned to be built concurrently or prior to this storm system. Please see MDDP and Final Drainage Report for Interquest Filing No. 7 for more details of this 84" storm system.

Comparison of MDDP to FDR Calculations (Lots 1 & 2 of Interquest Filing No. 6):

The M.D.D.P. portion of this report quantifies the runoff within the 60" storm main from Federal Drive to Pond EK-1 as Q_5 = 183 cfs and Q_{100} = 384 cfs. This quantity includes the developed runoff from the proposed lots of Interquest Filing No. 6 and was found using the S.C.S. drainage analysis method.

The Final Drainage Report portion of the report has this runoff described in more detail as is it conveyed through the site and into the pond. The same area as within the M.D.D.P. actually releases at three different locations described within the FDR text. Pipe 20 is the 60" main from Federal Drive to the pond and contains runoff of Q_5 = 136 cfs and Q_{100} = 241 cfs. Pipe 29 is the 48" main that conveys the runoff from the existing developments south of Federal Drive and the proposed basins along the south of Filing No. 6. This pipe contains runoff of Q_5 = 67 cfs and Q_{100} = 130 cfs. DP-28 releases its intercepted runoff to the 84" main at the north of the detention pond. The total storm runoff into the pond thru the site using the Modified Rational Method is Q_5 = 208 cfs and Q_{100} = 405 cfs. These numbers are in accordance with on another and values using the Modified Rational Method are always slightly higher than with the S.C.S. method.

STORM WATER QUALITY CALCULATIONS

Storm water quality calculations were performed in accordance with the City of Colorado Springs Drainage Criteria Volume 2. The SCS method was used to estimate storm water runoff anticipated from design storms 2, 5, 10, 25, 50 and 100-year recurrence interval. Detailed discussion of each basin and the facilities proposed are included in the Fully Developed Conditions section and the Filing No. 6 & 7 Interim Conditions section.

CONSTRUCTION COST OPINION

Lots 1 & 2 Interquest Filing No. 6 Public Drainage Facilities (Non-Reimbursable)

ITEM	DESCRIPTION	QUANTITY	UNIT COST	COST
1.	24" RCP Storm Drain	25 LF	$36/LF	$ 900.00
2.	42" RCP Storm Drain	60 LF	$105/LF	$ 6,300.00
3.	54" RCP Storm Drain	540 LF	$150/LF	$ 81,000.00
4.	TYPE I MH	1 EACH	$8,000/EA	$ 8,000.00
SUB-TOTAL				**$ 96,200.00**
15% ENGINEERING & CONTINGENCIES				**$ 14,430.00**
TOTAL				**$ 110,630.00**

Classic Consulting Engineers & Surveyors cannot and does not guarantee that the construction cost will not vary from these opinions of probable construction costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular.

DRAINAGE AND BRIDGE FEES (Filing No. 6)

All portions of the area proposed to be platted as Filing No. 6 are within the Elkhorn Drainage Basin therefore no fees are due.

EROSION CONTROL PLAN

The City of Colorado Springs Drainage Criteria Manual specifies an Erosion Control Plan and associated cost estimate be submitted with the Final Drainage Report. We respectfully request that the Erosion Control Plan and cost estimate be submitted in conjunction with the Grading Plan and construction assurances posted prior to obtaining a grading permit.

FLOODPLAIN STATEMENT

There are no portions of this site that lie within a 100-year floodplain as depicted in the Colorado Springs Flood Insurance Study, prepared by the Federal Emergency Management Agency (FEMA) as shown on FIRM Panel 08041C0506 F, dated March 17, 1997. The FIRM Panel is shown in the Appendix of this report.

CONCLUSION

Based upon the existing, fully developed and interim buildout scenarios described in this report no adverse impact to downstream or adjacent properties is anticipated. The use of regional detention facilities and regional stormwater quality help to mitigate the impacts of developed flows. Subsequent Final Drainage Reports (beyond Lots 1 & 2 of Interquest Filing No. 6) will continue to monitor the hydraulic characteristics of the proposed storm facilities to ensure Air Force Academy historic flow rates are being adhered to. It is proposed that the required improvements to the EK-1A Detention Facility be made in conjunction with the development of Interquest South Filing No. 6 and 7 or other upstream development tributary to this facility. At this time, the majority owners of undeveloped land in the basin are COPT (31.77%) and Jannie Richardson (60.26%). It is proposed that these two developers proportionally split the last of the improvements.

PREPARED BY:

Classic Consulting, Engineers & Surveyors, LLC

Matthew A. Larson, E.I.
Project Engineer

Kyle R. Campbell, P.E.
Division Manager

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REFERENCES

1. Soil Survey for El Paso County, Colorado, dated June 1981.

2. "City of Colorado Springs/El Paso County Drainage Criteria Manual", prepared by City of Colorado Springs, El Paso County, dated May 1987, revised 1996.

3. "Flood Insurance Studies for Colorado Springs, and El Paso County, Colorado", prepared by the Federal Emergency Management Agency (FEMA), revised March, 1997.

4. Master Development Drainage Plan, Fairlane Technological Park prepared by URS Consultants, Inc., dated October 1993, revised January 1994.

5. Master Development Drainage Plan Update, Fairlane Technological Park prepared by Ayres Associates, Inc., dated November 1997.

6. Drainage Memorandum for Pikes Peak Community College North Campus, prepared by URS Consultants, Inc., dated October 1996.

7. Drainage Addendum No. 1 for the New Life Church Filing No. 2, prepared by Haynes and Associates, Ltd., dated May 1996.

8. Final Hydraulic Report, Interstate 25 and Fairlane Parkway Interchange, Phase 1, prepared by DMJM, Inc., dated August 1998

9. Final Hydraulic Report, Interstate 25, Interquest Parkway/S. H. 83 Relocation, prepared by DMJM, Inc., dated March 1999.

10. Master Development Drainage Plan Update, Fairlane Technological Park, prepared by Kiowa Engineering Corporation, dated November 27, 2000 revised February 8, 2001.

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APPENDIX

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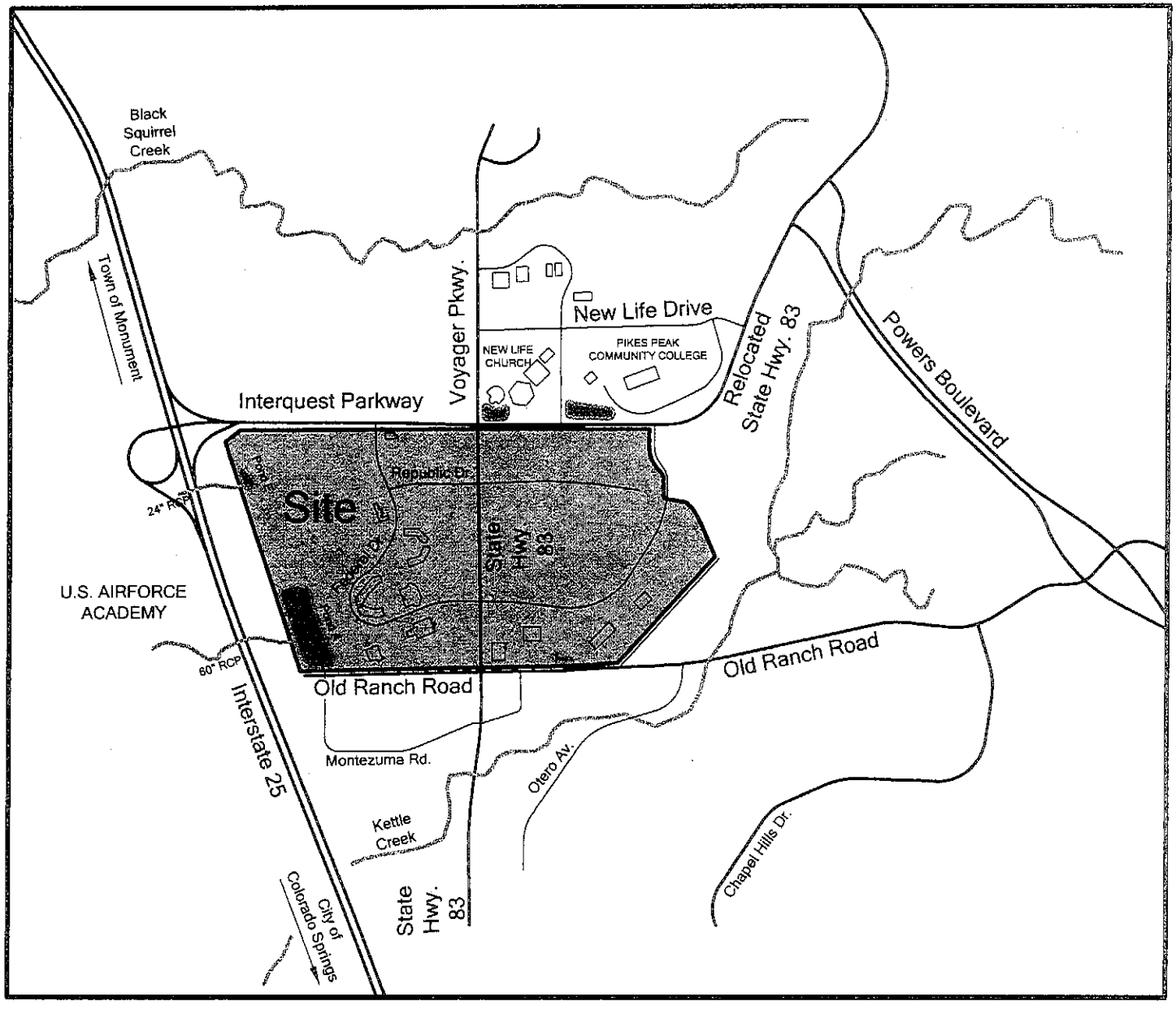
VICINITY MAP



Vicinity Map

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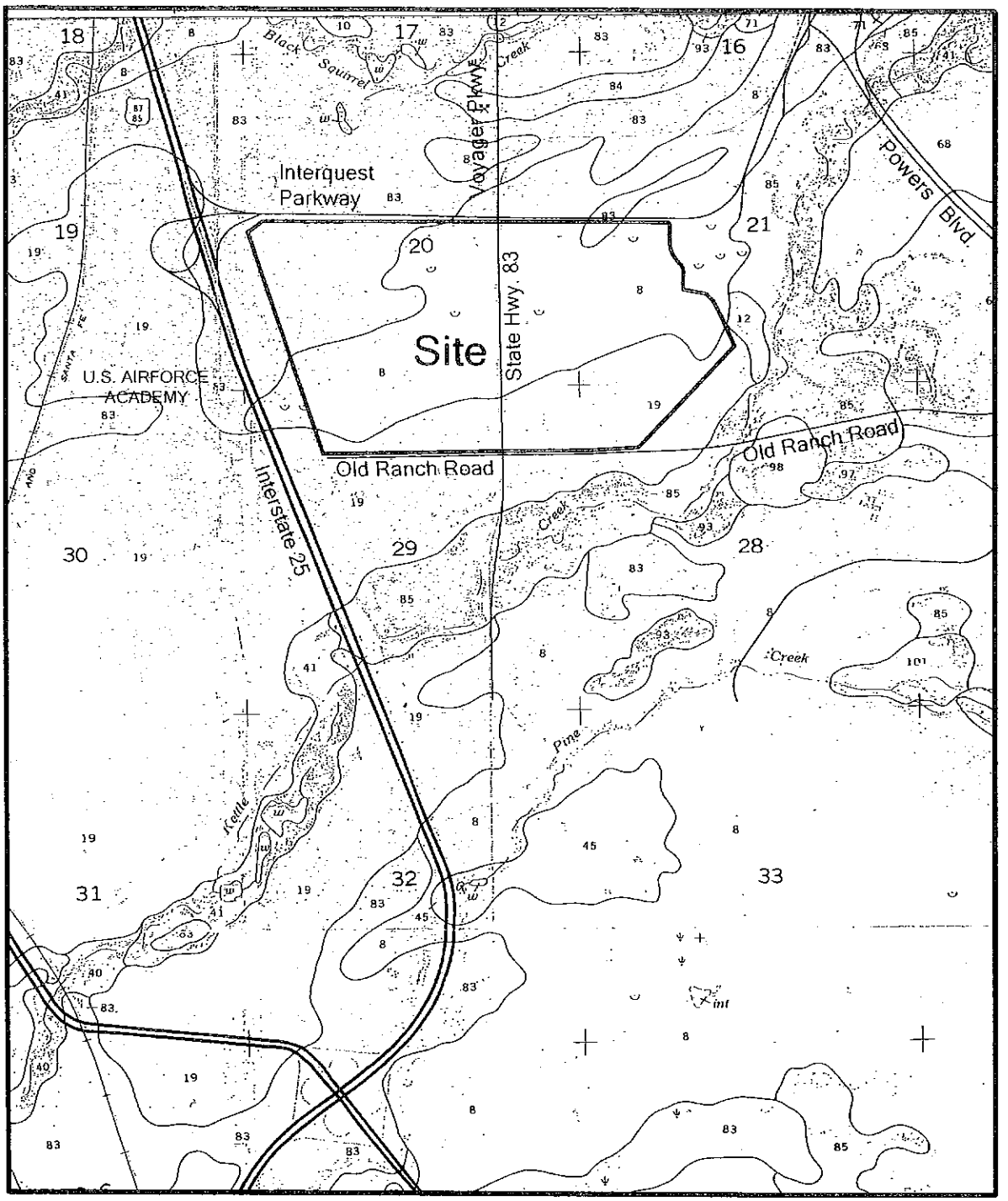
SOILS MAP (S.C.S SURVEY)



Key

8	Blakeland	Hydrologic Group A
19	Columbine	Hydrologic Group A
83	Stapleton	Hydrologic Group B
85	Stapleton - Bernal	Hydrologic Groups B & D

2400' 1200' 0 2400' 4800'

SCALE: 1" = 2400'

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Soils Map

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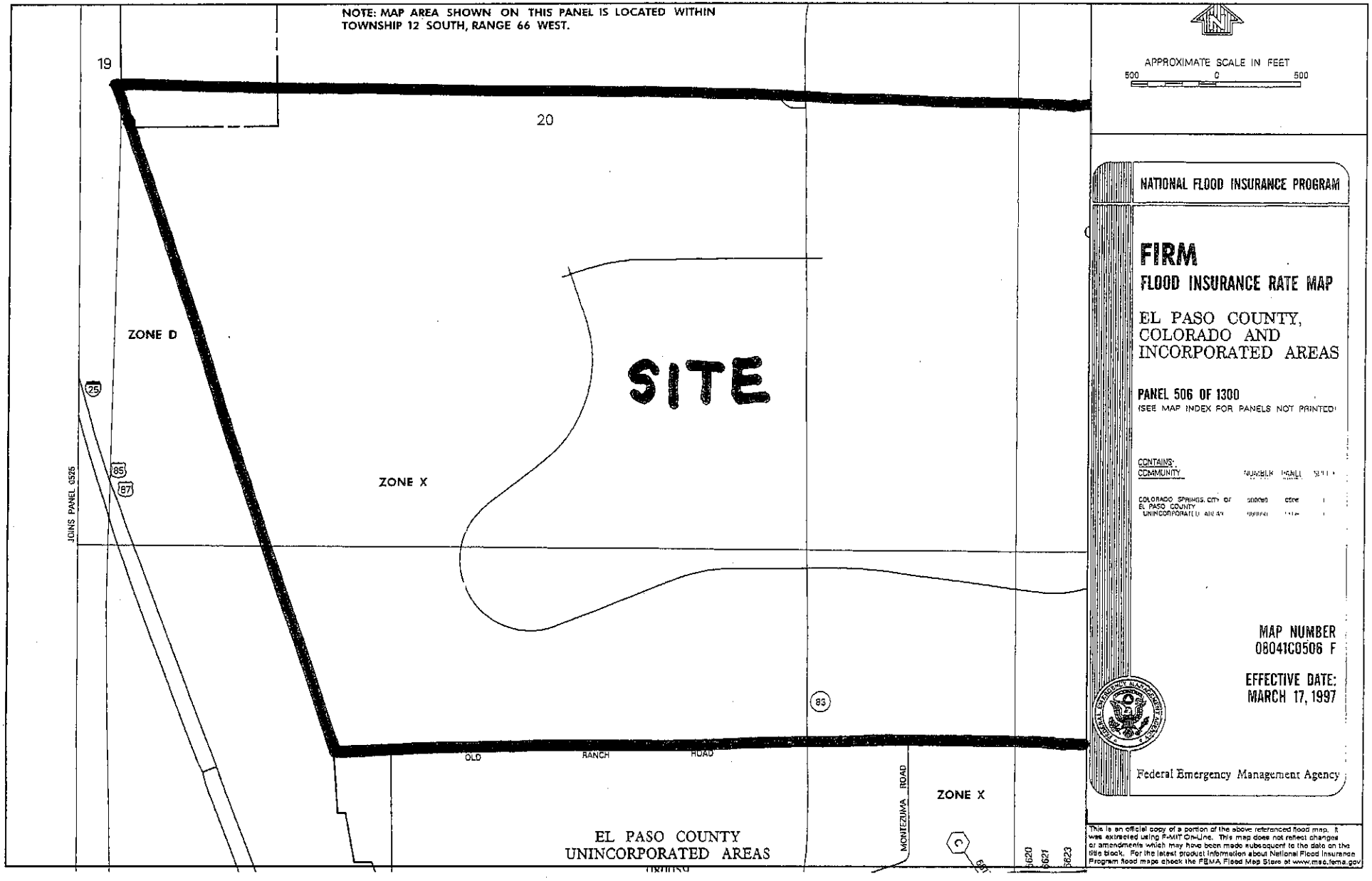
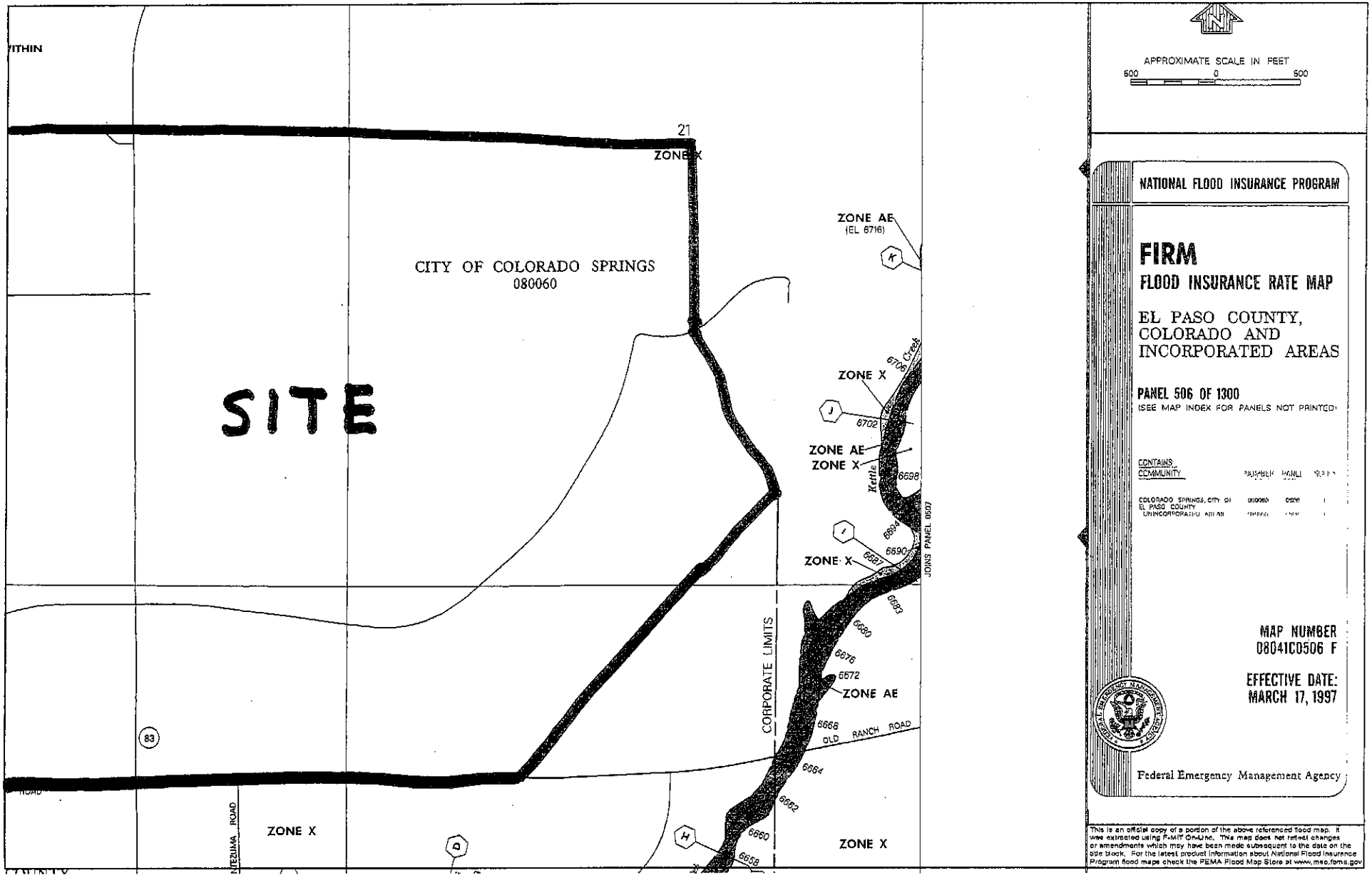
F.E.M.A. MAP


NOTE: MAP AREA SHOWN ON THIS PANEL IS LOCATED WITHIN TOWNSHIP 12 SOUTH, RANGE 66 WEST.
19
20
ZONE D
SITE
ZONE X
JOINS PANEL 0525
25
85
87
83
OLD RANCH ROAD
MONTEZUMA ROAD
ZONE X
EL PASO COUNTY
UNINCORPORATED AREAS
5620
5621
5623
APPROXIMATE SCALE IN FEET
500 0 500
NATIONAL FLOOD INSURANCE PROGRAM
FIRM
FLOOD INSURANCE RATE MAP
EL PASO COUNTY, COLORADO AND INCORPORATED AREAS
PANEL 506 OF 1300
(SEE MAP INDEX FOR PANELS NOT PRINTED)
CONTAINS:
COMMUNITY
COLORADO SPRINGS, CITY OF
EL PASO COUNTY
UNINCORPORATED AREAS
MAP NUMBER
08041C0506 F
EFFECTIVE DATE:
MARCH 17, 1997
Federal Emergency Management Agency
This is an official copy of a portion of the above referenced flood map. It was extracted using FMIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov


ITHIN
21
ZONE X
CITY OF COLORADO SPRINGS
080060
SITE
ZONE AE
(EL 6716)
K
6706 Creek
ZONE X
J
6702
ZONE AE
ZONE X
Kettle
6698
6694
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ZONE X
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JOINS PANEL 0507
6683
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ZONE AE
CORPORATE LIMITS
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MONTEZUMA ROAD
83
APPROXIMATE SCALE IN FEET
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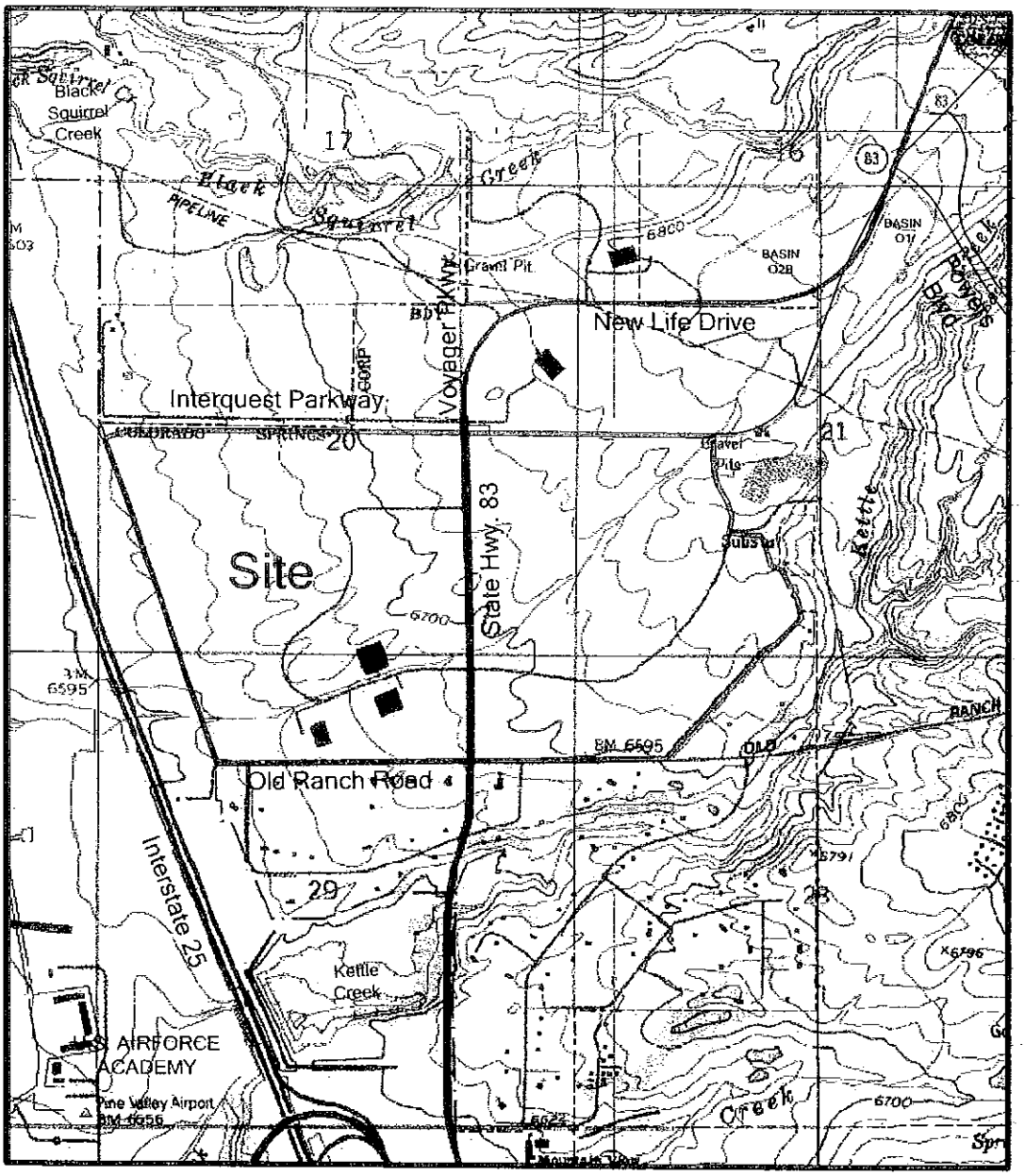
U.S.G.S. MAP



U.S.G.S. Map

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MISCELLANEOUS APPENDICES

DEPARTMENT OF THE AIR FORCE

10TH CIVIL ENGINEER GROUP
USAF ACADEMY COLORADO

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PUBLIC WORKS

30 NOV 2000

Mr. Thomas M. Mitchell
Deputy Civil Enginner
8120 Edgerton Drive, Suite 40
USAF Academy CO 80840-2400

Mr. David S. Zelenok
Group Support Manager
City of Colorado Springs
PO Box 1575, Mail Code 450
Colorado Springs CO 80901-1575

Dear Mr. Zelenok

Increased storm water flow from development along tributaries to Monument Creek has the potential to impact the riparian environment and infrastructure on the Academy and downstream neighbors' property. We have tried to get developers to voluntarily detain increased flows to the Academy and limit their discharge to historical levels. This has been unsuccessful. Therefore, to maintain the health of the Monument Creek system, we ask that the city enforce the following criteria for development occurring upstream of the Academy to ensure that developed flows do not adversely affect water quality, riparian habitat or our infrastructure.

> Development occurring upstream of the Academy shall ensure that peak flows for the 2-, 5-, 10-, 50- and 100-year storm frequencies are not increased over historical "undeveloped" flows. Proposed development occurring upstream of the Academy must adequately demonstrate that storm water facilities (i.e., detention/retention ponds) will function in conjunction with other facilities within the drainage and not increase peak flows for the frequencies listed above or adversely impact water quality.

We believe that these requirements are consistent with Colorado case law (Docheff v. City of Broomfield, 623 P.2d 69 (Colo. App1980), Hankins v. Borland, 163 Colo. 575 (1967); City of Boulder v. Boulder and White Rock Ditch and Reservoir Co., 73 Colo. 462 (1926)) and with the Academy's Conservation and Management Plan and agreement with the US Fish and Wildlife Service for the Preble's meadow jumping mouse (signed on 6 June 2000 by the Academy's Superintendent and the US Fish and Wildlife Service, Region 6 Director).

Please feel free to contact me or have your staff contact Josh Kellar at 333-6709 if there are any questions regarding this material.

Sincerely

THOMAS M. MITCHELL

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TABLE 5-5
RUNOFF CURVE NUMBERS FOR HYDROLOGIC SOIL COVER COMPLEXES - URBAN AND SUBURBAN CONDITIONS 1/
(Antecedent Moisture Condition II)
(From: U.S. Dept. of Agriculture, Soil Conservation Service, 1977)

Land Use		Average % Impervious 3/	Hydrologic Soil Group A	B	C	D
Open spaces, lawns, parks, golf courses, cemeteries, etc.						
	Good condition: grass cover on 75% or more of the area		39*	61	74	80
	Fair condition: grass cover on 50% to 75% of the area		49*	69	79	84
Commercial and Business areas (85% Impervious)			89*	92	94	95
Industrial Districts 72% Impervious)			81*	88	91	93
Residential: 2/						
	Acres per Dwelling Unit					
	1/8 acre or less	65	77*	85	90	92
	1/4 acre	38	61*	75	83	87
	1/3 acre	30	57*	72	81	86
	1/2 acre	25	54*	70	80	85
	1 acre	20	51*	68	79	84
Paved parking lots, roofs, driveways, etc.			98	98	98	98
Streets and Roads:						
	paved with curbs and storm sewers		98	98	98	98
	gravel		76*	85	89	91
	dirt		72*	82	87	89

1/ For a more detailed description of agricultural land use curve numbers, refer to the National Engineering Handbook (U.S. Dept. of Agriculture, Soil Conservation Service, 1972).
2/ Curve numbers are computed assuming the runoff from the house and driveway is directed towards the street with a minimum of roof water directed to lawns where additional infiltration could occur.
3/ The remaining pervious areas (lawn) are considered to be in good pasture condition for these curve numbers.

* Not to be used wherever overlot grading or filling is to occur.

TABLE 5-1

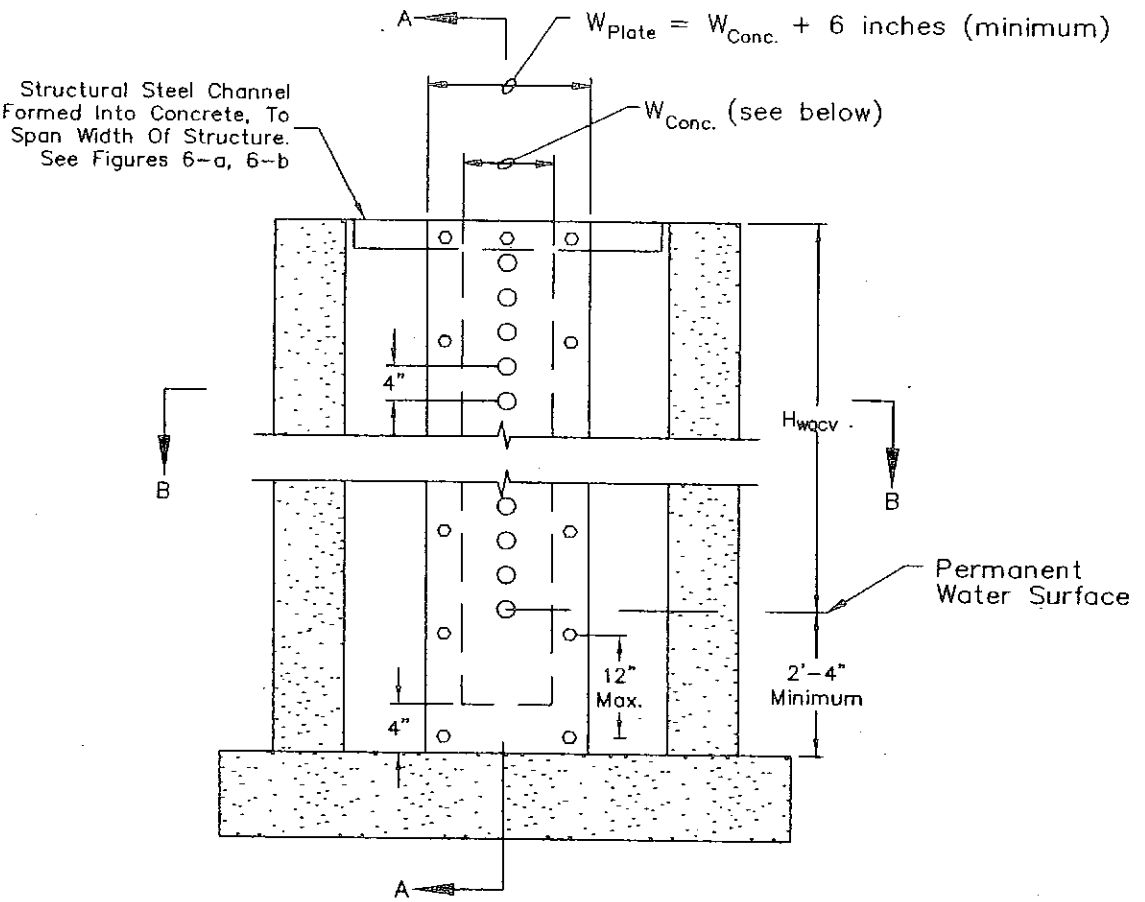
RECOMMENDED AVERAGE RUNOFF COEFFICIENTS AND PERCENT IMPERVIOUS

LAND USE OR SURFACE CHARACTERISTICS	PERCENT IMPERVIOUS	"C" FREQUENCY 10 A&B*	10 C&D*	100 A&B*	100 C&D*
Business					
Commercial Areas	95	0.90	0.90	0.90	0.90
Neighborhood Areas	70	0.75	0.75	0.80	0.80
Residential					
1/8 Acre or less	65	0.60	0.70	0.70	0.80
1/4 Acre	40	0.50	0.60	0.60	0.70
1/3 Acre	30	0.40	0.50	0.55	0.60
1/2 Acre	25	0.35	0.45	0.45	0.55
1 Acre	20	0.30	0.40	0.40	0.50
Industrial					
Light Areas	80	0.70	0.70	0.80	0.80
Heavy Areas	90	0.80	0.80	0.90	0.90
Parks and Cemeteries	7	0.30	0.35	0.55	0.60
Playgrounds	13	0.30	0.35	0.60	0.65
Railroad Yard Areas	40	0.50	0.55	0.60	0.65
Undeveloped Areas					
Historic Flow Analysis- Greenbelts, Agricultural	2	0.15	0.25	0.20	0.30
Pasture/Meadow	0	0.25	0.30	0.35	0.45
Forest	0	0.10	0.15	0.15	0.20
Exposed Rock	100	0.90	0.90	0.95	0.95
Offsite Flow Analysis (when land use not defined)	45	0.55	0.60	0.65	0.70
Streets					
Paved	100	0.90	0.90	0.95	0.95
Gravel	80	0.80	0.80	0.85	0.85
Drive and Walks	100	0.90	0.90	0.95	0.95
Roofs	90	0.90	0.90	0.95	0.95
Lawns	0	0.25	0.30	0.35	0.45

* Hydrologic Soil Group

9/30/90

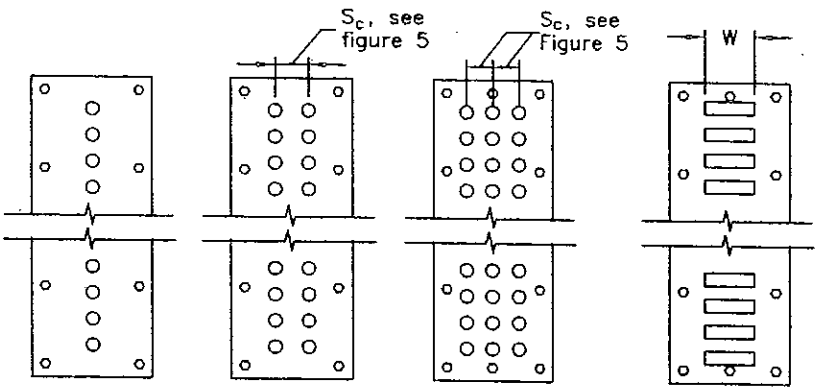
Orifice Perforation Details



Circular Openings: $W_{Conc.}$ Obtained From Table 6a-1

Rectangular Openings: $W_{Conc.}$ = (Width of Rectangular Perforation W) + 12"

Rectangular Openings: $W_{Opening}$ (see Figure 6-b) Obtained From Table 6b-1



Example Perforation Patterns

Note: The goal in designing the outlet is to minimize the number of columns of perforations that will drain the WQCV in the desired time. Do not, however, increase the diameter of circular perforations or the height of the rectangular perforations beyond 2 inches. Use the allowed perforation shapes and configurations shown above along with Figure 5 to determine the pattern that provides an area per row closest to that required without exceeding it.

Urban Drainage and Flood Control District Drainage Criteria Manual (V.3) File: Details.dwg	Figure 4 Orifice Details for Draining WQCV

Orifice Plate Perforation Sizing

Circular Perforation Sizing

Chart may be applied to orifice plate or vertical pipe outlet.

Hole Dia (in) *	Hole Dia (in)	Min. S_c (in)	Area per Row (sq in)		
			n=1	n=2	n=3
1/4	0.250	1	0.05	0.10	0.15
5/16	0.313	2	0.08	0.15	0.23
3/8	0.375	2	0.11	0.22	0.33
7/16	0.438	2	0.15	0.30	0.45
1/2	0.500	2	0.20	0.39	0.59
9/16	0.563	3	0.25	0.50	0.75
5/8	0.625	3	0.31	0.61	0.92
11/16	0.688	3	0.37	0.74	1.11
3/4	0.750	3	0.44	0.88	1.33
13/16	0.813	3	0.52	1.04	1.56
7/8	0.875	3	0.60	1.20	1.80
15/16	0.938	3	0.69	1.38	2.07
1	1.000	4	0.79	1.57	2.36
1 1/16	1.063	4	0.89	1.77	2.66
1 1/8	1.125	4	0.99	1.99	2.98
1 3/16	1.188	4	1.11	2.22	3.32
1 1/4	1.250	4	1.23	2.45	3.68
1 5/16	1.313	4	1.35	2.71	4.06
1 3/8	1.375	4	1.48	2.97	4.45
1 7/16	1.438	4	1.62	3.25	4.87
1 1/2	1.500	4	1.77	3.53	5.30
1 9/16	1.563	4	1.92	3.83	5.75
1 5/8	1.625	4	2.07	4.15	6.22
1 11/16	1.688	4	2.24	4.47	6.71
1 3/4	1.750	4	2.41	4.81	7.22
1 13/16	1.813	4	2.58	5.16	7.74
1 7/8	1.875	4	2.76	5.52	8.28
1 15/16	1.938	4	2.95	5.90	8.84
2	2.000	4	3.14	6.28	9.42
n = Number of columns of perforations					
Minimum steel plate thickness			1/4 "	5/16 "	3/8 "

* Designer may interpolate to the nearest 32nd inch to better match the required area, if desired.

Rectangular Perforation Sizing

Only one column of rectangular perforations allowed.

Rectangular Height = 2 inches

$$\text{Rectangular Width (inches)} = \frac{\text{Required Area per Row (sq in)}}{2"}$$

Rectangular Hole Width	Min. Steel Thickness
5"	1/4 "
6"	1/4 "
7"	5/32 "
8"	5/16 "
9"	11/32 "
10"	3/8 "
>10"	1/2 "

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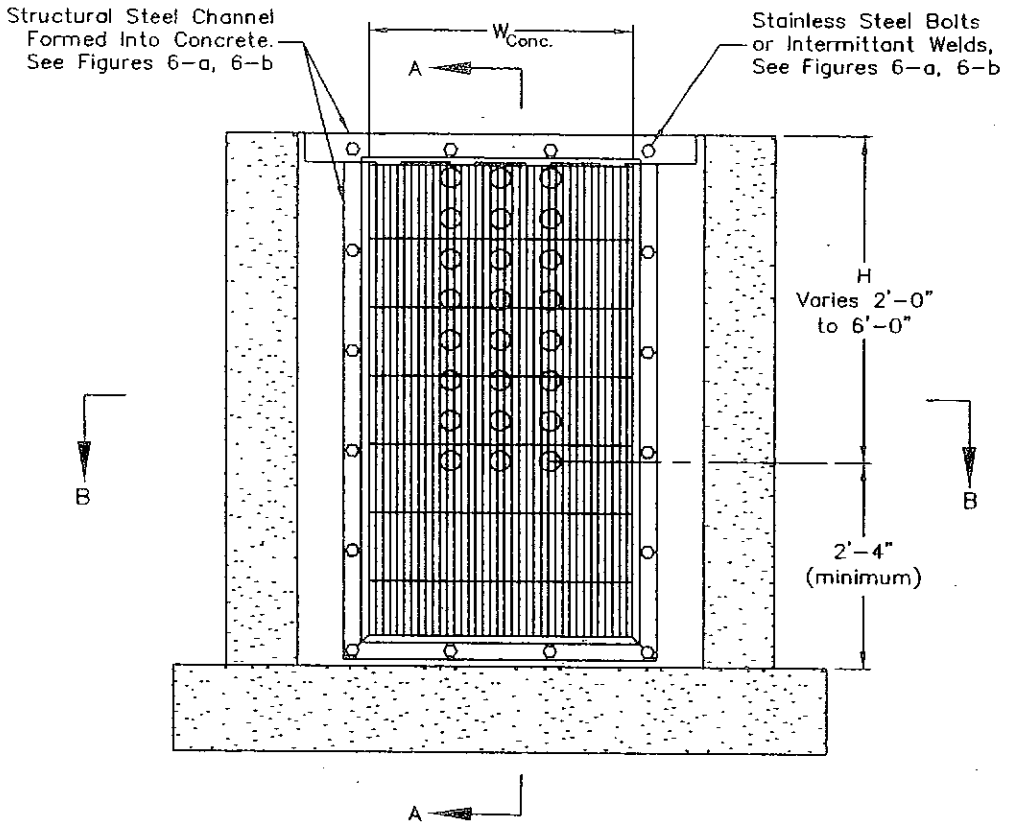
Figure 5

WQCV Outlet Orifice Perforation Sizing

Note: Vertical WQCV Trash Racks are shown in Figures 6, 6–a, and 6–b for suggested standardized outlet design. Adverse–Slope Trash Rack design may be used for non–standardized designs, but must meet minimum design criteria.



Elevation

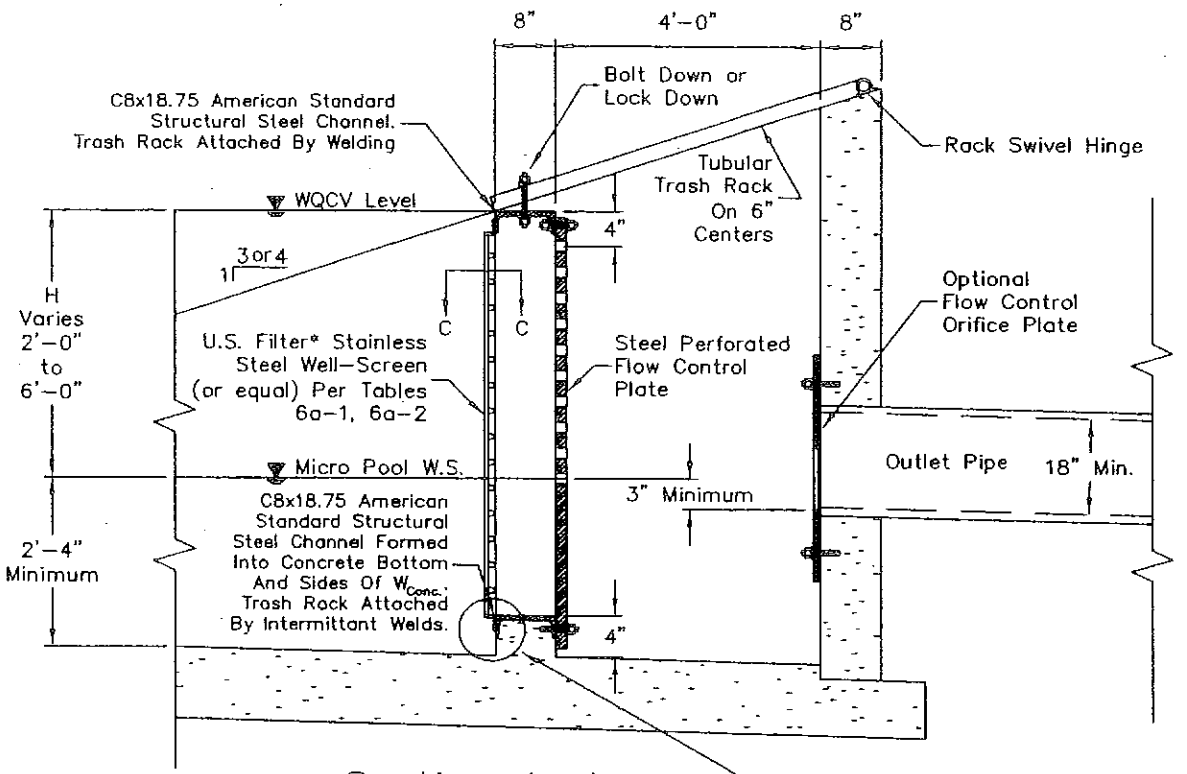
WQCV Trash Racks:

1. Well–screen trash racks shall be stainless steel and shall be attached by intermittant welds along the edge of the mounting frame.
2. Bar grate trash racks shall be aluminum and shall be bolted using stainless steel hardware.
3. Trash Rack widths are for specified trash rack material. Finer well–screen or mesh size than specified is acceptable, however, trash rack dimensions need to be adjusted for materials having a different open area/gross area ratio (R value)
4. Structural design of trash rack shall be based on full hydrostatic head with zero head downstream of the rack.

Overflow Trash Racks:

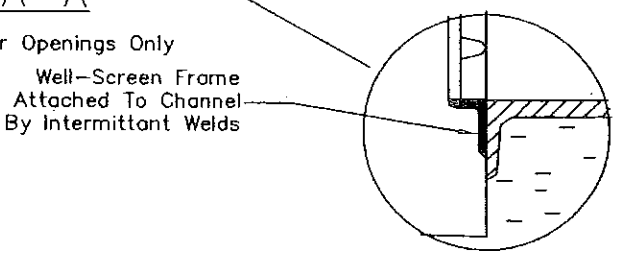
1. All trash racks shall be mounted using stainless steel hardware and provided with hinged and lockable or boltable access panels.
2. Trash racks shall be stainless steel, aluminum, or steel. Steel trash racks shall be hot dip galvanized and may be hot powder painted after galvanizing.
3. Trash Racks shall be designed such that the diagonal dimension of each opening is smaller than the diameter of the outlet pipe.
4. Structural design of trash rack shall be based on full hydrostatic head with zero head downstream of the rack.

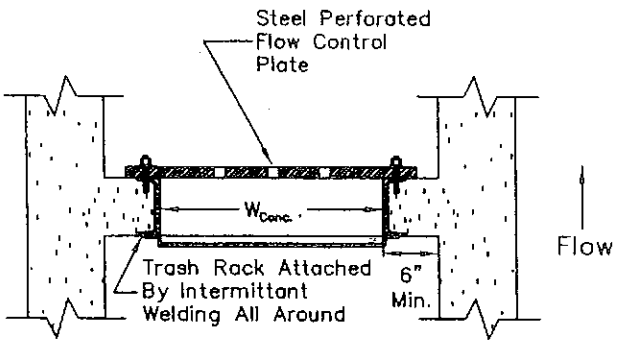
Urban Drainage and Flood Control District Drainage Criteria Manual (V.3) File: Details.dwg	Figure 6 Suggested WQCV Outlet Standardized Trash Rack Design



Section A-A

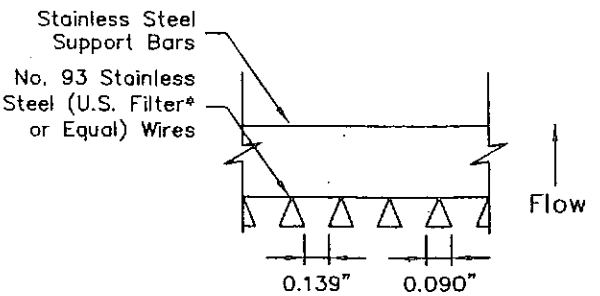
From Figure 6, Circular Openings Only





Section B-B – Plan View

From Figure 6, Circular Openings Only



Section C-C

From Figure 6, Circular Openings Only

R Value = (net open area)/(gross rack area)
= 0.60

Limits for this Standardized Design:

1. All outlet plate openings are circular.
2. Maximum diameter of opening = 2 inches.

*U.S. Filter, St. Paul, Minnesota, USA

Urban Drainage and Flood Control District Drainage Criteria Manual (V.3) File: Details.dwg	Figure 6-a Suggested Standardardized Trash Rack and Outlet Design For WQCV Outlets With Circular Openings

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

HYDROLOGIC / HYDRAULIC CALCULATIONS
EXISTING CONDITIONS

COMPOSITE C_N VALUES - EXISTING CONDITIONS

BASIN (label)	BASIN AREA (ac)	LAND USE	SUB-AREA LAND USE 1		AREA (AC)	LAND USE	SUB-AREA LAND USE 2		AREA (AC)	TOTAL BASIN AREA (ac)	TOTAL BASIN AREA (sq mi)	WEIGHTED C_N
			ESTIMATED PERCENT IMPERIOUS	ESTIMATED			ESTIMATED PERCENT IMPERIOUS	ESTIMATED C_N				
02B	32.2	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	32.2	0.05031	60.0
O1	19.2	REPORT	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	19.2	0.03000	67.8
B1	15.1	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	15.1	0.02359	68.0
B2	62.7	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	62.7	0.09797	75.0
B3	40.0	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	40.0	0.06250	80.0
B5	11.4	UNDEV.	0	61	7.7	ROADS	100	88	3.7	11.4	0.01781	69.8
B6	18.4	UNDEV.	0	61	18.4	ROADS	100	88	0.0	18.4	0.02875	61.0
B7	17.8	UNDEV.	0	61	13.5	ROADS	100	88	4.3	17.8	0.02781	67.5
B9	4.1	UNDEV.	0	61	0.0	ROADS	100	88	4.1	4.1	0.00641	88.0
A1	56.9	UNDEV.	0	61	56.9	ROADS	100	88	0.0	56.9	0.08891	61.0
A2	108.0	UNDEV.	0	61	108.0	ROADS	100	88	0.0	108.0	0.16875	61.0
A3	25.0	UNDEV.	0	61	7.5	ROADS	100	88	17.5	25.0	0.03906	79.9
A4	4.0	UNDEV.	0	61	2.0	ROADS	100	88	2.0	4.0	0.00625	74.5
A5	6.7	UNDEV.	0	61	5.8	ROADS	100	88	0.9	6.7	0.01047	64.6
A6	14.7	UNDEV.	0	61	3.5	ROADS	100	88	11.2	14.7	0.02297	81.6
A7	25.1	UNDEV.	0	61	6.0	ROADS	100	88	19.1	25.1	0.03922	81.5
A8	24.7	UNDEV.	0	61	4.5	ROADS	100	88	20.2	24.7	0.03859	83.1
A9	27.5	UNDEV.	0	61	2.8	ROADS	100	88	24.7	27.5	0.04297	85.3
A12	31.1	UNDEV.	0	61	31.1	ROADS	100	88	0.0	31.1	0.04859	61.0
A13	13.9	UNDEV.	0	61	13.9	ROADS	100	88	0.0	13.9	0.02172	61.0
A14	38.4	UNDEV.	0	61	38.4	ROADS	100	88	0.0	38.4	0.06000	61.0
C1	27.5	UNDEV.	0	61	27.5	ROADS	100	88	0.0	27.5	0.04297	61.0
D1	40.4	UNDEV.	0	61	0.0	ROADS	100	88	40.4	40.4	0.06313	88.0
E5	34.3	UNDEV.	0	61	34.3	ROADS	100	88	0.0	34.3	0.05359	61.0
E7	79.2	UNDEV.	0	61	79.2	ROADS	100	88	0.0	79.2	0.12375	61.0
VP-1	1.1	UNDEV.	0	61	0.2	ROADS	100	88	0.9	1.1	0.00172	82.6
IP-1	1.2	UNDEV.	0	61	0.2	ROADS	100	88	1.0	1.2	0.00188	82.6
IP-2	2.5	UNDEV.	0	61	0.5	ROADS	100	88	2.0	2.5	0.00391	82.6
IP-3	2.0	UNDEV.	0	61	0.4	ROADS	100	88	1.6	2.0	0.00313	82.6
IP-4	3.1	UNDEV.	0	61	0.6	ROADS	100	88	2.5	3.1	0.00484	82.6

TIME OF CONCENTRATION

BASIN	Cn	C(5)	Length (ft)	OVERLAND Height (ft)	Tc (min)	STREET / CHANNEL FLOW Length (ft)	Velocity (fps)	Tc (min)	Tc TOTAL (min)	Tc TOTAL (hr)	Lagtime TOTAL (hr)
02B	60.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	17.0000	0.2833	0.1700
O1	67.8	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	8.0000	0.1333	0.0800
B1	68.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	12.6000	0.2100	0.1260
B2	75.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	11.4000	0.1900	0.1140
B3	80.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	13.8000	0.2300	0.1380
B5	69.8	0.25	100	2	12.6	1375	3.0	7.6	20.2839	0.3381	0.2028
B6	61.0	0.25	100	2	12.6	1525	3.0	8.5	21.1173	0.3520	0.2112
B7	67.5	0.25	100	2	12.6	1425	3.0	7.9	20.5617	0.3427	0.2056
B9	88.0	0.75	100	2	5.2	500	3.0	2.8	7.9846	0.1331	0.0798
A1	61.0	0.25	100	2	12.6	2900	3.0	16.1	28.7562	0.4793	0.2876
A2	61.0	0.25	100	2	12.6	3400	3.0	18.9	31.5339	0.5256	0.3153
A3	79.9	0.75	100	2	5.2	1400	3.0	7.8	12.9846	0.2164	0.1298
A4	74.5	0.75	100	2	5.2	2500	3.0	13.9	19.0957	0.3183	0.1910
A5	64.6	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
A6	81.6	0.75	100	2	5.2	600	3.0	3.3	8.5401	0.1423	0.0854
A7	81.5	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
A8	83.1	0.25	100	2	12.6	2400	3.0	13.3	25.9784	0.4330	0.2598
A9	85.3	0.25	100	2	12.6	2000	3.0	11.1	23.7562	0.3959	0.2376
A12	61.0	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
A13	61.0	0.25	100	2	12.6	1000	3.0	5.6	18.2006	0.3033	0.1820
A14	61.0	0.25	100	2	12.6	2000	3.0	11.1	23.7562	0.3959	0.2376
C1	61.0	0.25	100	2	12.6	1500	3.0	8.3	20.9784	0.3496	0.2098
D1	88.0	0.75	100	2	5.2	2000	3.0	11.1	16.3179	0.2720	0.1632
E5	61.0	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
E7	61.0	0.25	100	2	12.6	2200	3.0	12.2	24.8673	0.4145	0.2487
VP-1	82.6	0.75	50	2	2.9	1200	3.0	6.7	9.5956	0.1599	0.0960
IP-1	82.6	0.75	50	2	2.9	1350	3.0	7.5	10.4290	0.1738	0.1043
IP-2	82.6	0.75	50	2	2.9	1350	3.0	7.5	10.4290	0.1738	0.1043
IP-3	82.6	0.75	50	2	2.9	1900	3.0	10.6	13.4845	0.2247	0.1348
IP-4	82.6	0.75	50	2	2.9	1900	3.0	10.6	13.4845	0.2247	0.1348

PREV. = DETERMINED FROM PREVIOUS REPORTS

BASIN SUMMARY - EXISTING CONDITIONS

BASIN (label)	TOTAL BASIN AREA (acres)	TOTAL BASIN AREA (sq mi)	WEIGHTED CN	TOTAL LAG TIME (hours)	TOTAL Tc (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 10 Yr. (cfs)	Q 25 Yr. (cfs)	Q 50 Yr. (cfs)	Q 100 Yr. (cfs)
02B	32.2	0.0503	60.0	0.1700	0.2833	0.25	4.42	9.72	20.10	28.39	37.46
O1	19.2	0.0300	67.8	0.0800	0.1333	4.73	12.73	19.06	29.60	37.17	45.16
B1	15.1	0.0236	68.0	0.1260	0.2100	2.95	8.42	12.92	20.51	26.11	32.02
B2	62.7	0.0980	75.0	0.1140	0.1900	33.20	65.46	89.55	128.36	155.96	184.43
B3	40.0	0.0625	80.0	0.1380	0.2300	31.11	54.19	71.14	97.89	116.35	135.15
B5	11.4	0.0178	69.8	0.2028	0.3381	2.31	6.04	9.10	14.37	18.20	22.25
B6	18.4	0.0288	61.0	0.2112	0.3520	0.21	2.67	5.54	11.14	15.58	20.47
B7	17.8	0.0278	67.5	0.2056	0.3427	2.49	7.57	11.92	19.51	25.14	31.13
B9	4.1	0.0064	88.0	0.0798	0.1331	6.22	9.25	11.31	14.42	16.50	18.59
A1	56.9	0.0889	61.0	0.2876	0.4793	0.61	6.55	13.65	27.90	39.29	52.06
A2	108.0	0.1688	61.0	0.3153	0.5256	1.15	11.58	24.15	49.44	69.84	92.62
A3	25.0	0.0391	79.9	0.1298	0.2164	19.83	34.56	45.24	62.06	73.64	85.49
A4	4.0	0.0063	74.5	0.1910	0.3183	1.65	3.38	4.72	6.92	8.47	10.09
A5	6.7	0.0105	64.6	0.2209	0.3682	2.83	5.67	7.84	11.43	13.98	16.61
A6	14.7	0.0230	81.6	0.0854	0.1423	15.15	24.83	31.72	42.41	49.67	57.02
A7	25.1	0.0392	81.5	0.2209	0.3682	14.32	26.33	35.34	49.80	59.91	70.36
A8	24.7	0.0386	83.1	0.2598	0.4330	18.22	30.97	40.16	54.82	64.93	75.24
A9	27.5	0.0430	85.3	0.2376	0.3959	25.01	40.67	51.88	69.34	81.26	93.39
A12	31.1	0.0486	61.0	0.2209	0.3682	0.35	4.38	9.09	18.32	25.61	33.67
A13	13.9	0.0217	61.0	0.1820	0.3033	0.17	2.26	4.64	9.24	12.82	16.77
A14	38.4	0.0600	61.0	0.2376	0.3959	0.42	5.12	10.64	21.54	30.18	39.80
C1	27.5	0.0430	61.0	0.2098	0.3496	0.31	4.02	8.32	16.72	23.39	30.71
D1	40.4	0.0631	88.0	0.1632	0.2720	53.05	80.73	99.82	129.16	148.88	168.68
E5	34.3	0.0536	61.0	0.2209	0.3682	0.38	4.53	9.49	19.29	26.99	35.43
E7	79.2	0.1238	61.0	0.2487	0.4145	0.86	9.69	20.24	40.79	57.82	76.64
VP-1	1.1	0.0017	82.6	0.0960	0.1599	1.19	1.92	2.43	3.24	3.78	4.33
IP-1	1.2	0.0019	82.6	0.1043	0.1738	1.27	2.07	2.63	3.49	4.08	4.68
IP-2	2.5	0.0039	82.6	0.1043	0.1738	2.65	4.30	5.47	7.27	8.49	9.74
IP-3	2.0	0.0031	82.6	0.1348	0.2247	1.91	3.16	4.04	5.40	6.34	7.28
IP-4	3.1	0.0048	82.6	0.1348	0.2247	2.97	4.89	6.26	8.37	9.82	11.28

CONVEYANCE CHART - EXISTING CONDITIONS

Conveyance Label	Type	Length ft.	Bottom Width ft.	Slope %	Side Slope H to 1	Manning's N value	Available Depth ft.	Q 5 Yr. cfs	Q 100 Yr. cfs	Q Capacity cfs
C1	36" RCP	2650	3.0	0.0200	N/A	0.013	N/A	10.55	42.84	94.32
C2	48" RCP	1375	4.0	0.0225	N/A	0.013	N/A	39.25	98.20	215.45
C3	GRASSLINED SWALE	3475	6.0	0.0230	3.0	0.035	2.000	1.57	24.70	169.57
C4	54" RCP	1670	4.5	0.0186	N/A	0.013	N/A	55.36	168.19	268.18
C5	54" RCP	450	4.5	0.0200	N/A	0.013	N/A	58.56	181.40	278.09
C6	54" RCP	550	4.5	0.0200	N/A	0.013	N/A	64.45	203.36	278.09
C7	60" RCP	600	5.0	0.0217	N/A	0.013	N/A	74.49	254.17	383.63
C8	GRASSLINED SWALE	1600	20.0	0.0250	3.0	0.035	2.000	71.44	254.44	476.05
C9	GRASSLINED SWALE	1800	6.0	0.0167	3.0	0.035	3.000	4.93	47.25	365.58
C10	CONCRETE CHANNEL	1100	6.0	0.0050	1.5	0.015	5.000	15.34	137.05	1297.29
C11	42" RCP	2200	3.5	0.0177	N/A	0.013	N/A	3.43	29.46	133.85
C12	48" RCP	1050	4.0	0.0062	N/A	0.013	N/A	68.33	152.60	113.10
C13	CONCRETE CHANNEL	1750	6.0	0.0271	1.5	0.015	5.000	97.45	310.36	2191.56
C14	CONCRETE CHANNEL	600	6.0	0.0200	1.5	0.015	5.000	106.23	333.27	1882.71
C15	30" RCP	800	2.5	0.0188	N/A	0.013	N/A	30.97	75.24	56.24
C16	48" RCP	500	4.0	0.0300	N/A	0.013	N/A	71.33	168.10	248.78
C17	GRASSLINED SWALE	520	6.0	0.0200	3.0	0.035	2.000	26.02	69.92	170.49
C18	GRASS / RIPRAP SWALE	700	20.0	0.0257	2.0	0.035	3.000	18.97	132.48	934.14
C19	GRASSLINED SWALE	1850	10.0	0.0189	10.0	0.035	1.500	0.93	11.74	209.13
C20	GRASSLINED SWALE	750	10.0	0.0227	10.0	0.035	2.000	8.94	74.97	432.24
C21	30" RCP	650	2.5	0.0292	N/A	0.013	N/A	7.87	18.48	70.09
C22	GRASSLINED SWALE	1150	6.0	0.0182	2.0	0.035	1.500	6.96	17.56	80.50

DESIGN POINTS - EXISTING CONDITIONS

Design Point (label)	Q 2 Yr. Q (cfs) Ponding (ft)	Q 5 Yr. Q (cfs) Ponding (ft)	Q 10 Yr. Q (cfs) Ponding (ft)	Q 25 Yr. Q (cfs) Ponding (ft)	Q 50 Yr. Q (cfs) Ponding (ft)	Q 100 Yr. Q (cfs) Ponding (ft)
PCC IN	33.20	65.46	89.55	128.36	155.96	184.43
PCC OUT	14.72 / 6758.26	31.59 / 6759.71	39.19 / 6760.42	49.69 / 6761.47	56.38 / 6762.09	60.98 / 6762.40
1	17.26	39.67	53.49	72.12	85.95	98.98
NLC IN	31.11	54.19	71.14	97.89	116.35	135.15
NLC OUT	5.01 / 6737.07	11.97 / 6737.79	17.07 / 6738.38	23.49 / 6739.25	26.23 / 6739.71	28.62 / 6740.21
2	22.93	56.08	79.77	116.78	143.70	169.78
3	23.78	58.66	84.60	124.29	153.19	181.69
4	25.89	64.51	93.58	138.51	171.10	203.65
5	28.41	74.53	110.84	168.83	211.43	254.85
6	29.33	76.61	11430.00	174.03	217.80	262.29
EK1B IN	53.05	80.73	99.82	129.16	148.88	168.68
EK1B OUT	42.25 / 6692.84	65.96 / 6693.83	79.25 / 6694.46	100.04 / 6695.50	112.94 / 6696.20	123.14 / 6696.88
10	42.30	68.33	85.89	115.27	135.02	152.60
8	1.61	15.82	34.40	72.53	103.60	138.56
9	58.52	98.35	131.38	199.90	254.15	311.03
11	61.97	106.44	142.96	215.92	273.19	333.29
12	43.06	71.50	91.85	123.85	145.96	168.43
7	142.52	279.60	393.16	587.34	730.63	878.03
EK1A IN	142.52	279.60	393.16	587.34	730.63	878.03
EK1A OUT	15.27 / 6613.35	18.97 / 6614.97	24.55 / 6616.04	59.83 / 6617.04	94.08 / 6617.69	132.49 / 6618.32
13	15.27	18.97	24.55	59.83	94.08	132.49
14	15.61	19.92	26.53	62.30	97.45	137.18
I25-1 IN	15.61	19.92	26.53	62.30	97.45	137.18
I25-1 OUT	15.60 / 6590.44	19.90 / 6590.64	25.44 / 6590.88	62.15 / 6592.09	96.64 / 6593.02	135.83 / 6594.01
18	15.60	19.90	25.44	62.15	96.64	135.83
15	4.88	8.05	10.29	13.78	16.16	18.56
16	4.71	7.87	10.12	13.64	16.04	18.48
17	4.36	18.46	35.45	68.25	94.34	123.89
I25-2 IN	4.36	18.46	35.46	68.25	94.34	123.89
I25-2 OUT	3.96 / 6599.94	8.59 / 6600.47	14.56 / 6601.07	23.46 / 6602.13	26.42 / 6602.60	29.33 / 6603.13
19	3.96	8.59	14.56	23.46	26.42	29.33

JOB NAME: Interquest South
JOB NUMBER: 2190.00
DATE: 04/05/07
CALCULATED BY: JHB

EK-1A Detention Pond - As-built Volumes

POND SIZING WITH PONDPACK EQUATION:
INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6609.13	-	-	acres
6610.00	8,202	0.19	acres
6612.00	88,550	2.03	acres
6614.00	236,677	5.43	acres
6616.00	337,152	7.74	acres
6618.00	386,093	8.86	acres
6620.00	420,700	9.66	acres
6622.00	454,570	10.44	acres
6624.00	490,000	11.25	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

							CUMMULATIVE VOLUME:
0.05	AC-FT	from	6,609	to	6,610		0.05
1.87	AC-FT	from	6,610	to	6,612		1.93
7.12	AC-FT	from	6,612	to	6,614		9.05
12.97	AC-FT	from	6,614	to	6,616		22.02
16.42	AC-FT	from	6,616	to	6,618		38.45
18.33	AC-FT	from	6,618	to	6,620		56.78
19.89	AC-FT	from	6,620	to	6,622		76.67
21.46	AC-FT	from	6,622	to	6,624		98.13
-	AC-FT	from	6,624	to	-		98.13
-	AC-FT	from	-	to	-		98.13
-	AC-FT	from	-	to	-		98.13

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

VOLUME = 98.13 AC-FT

APPROXIMATE SURFACE AREA REQUIREMENT

POND DEPTH (FT)	POND VOLUME AC-FT		CF	SURFACE AREA (SF)
4	98.13	=	######	1,068,628
6	98.13	=	######	712,419
8	98.13	=	######	534,314
10	98.13	=	######	427,451

JOB NAME: Interquest South
JOB NUMBER: 2190.00
DATE: 04/05/07
CALCULATED BY: JHB

I25-1 Detention Pond

POND SIZING WITH PONDPACK EQUATION:
INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6589.00	-	-	acres
6590.00	1,990	0.05	acres
6592.00	13,413	0.31	acres
6594.00	34,818	0.80	acres
6596.00	59,418	1.36	acres
6597.00	82,403	1.89	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

							CUMMULATIVE VOLUME:
0.02	AC-FT	from	6,589	to	6,590		0.02
0.31	AC-FT	from	6,590	to	6,592		0.33
1.06	AC-FT	from	6,592	to	6,594		1.38
2.12	AC-FT	from	6,594	to	6,596		3.50
1.60	AC-FT	from	6,596	to	6,597		5.11
-	AC-FT	from	6,597	to	-		5.11
-	AC-FT	from	-	to	-		5.11
-	AC-FT	from	-	to	-		5.11
-	AC-FT	from	-	to	-		5.11
-	AC-FT	from	-	to	-		5.11
-	AC-FT	from	-	to	-		5.11

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

VOLUME = 5.11 AC-FT

APPROXIMATE SURFACE AREA REQUIREMENT

POND DEPTH (FT)	POND VOLUME AC-FT		CF	SURFACE AREA (SF)
4	5.11	=	######	55,609
6	5.11	=	######	37,073
8	5.11	=	######	27,804
10	5.11	=	######	22,244

JOB NAME:	Interquest South
JOB NUMBER:	2190.00
DATE:	04/05/07
CALCULATED BY:	JHB

I25-2 Detention Pond

POND SIZING WITH PONDPACK EQUATION:

INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6599.00	-	-	acres
6600.00	5,265	0.12	acres
6602.00	61,149	1.40	acres
6603.00	88,446	2.03	acres
6603.50	118,000	2.71	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

						CUMMULATIVE VOLUME:
0.04	AC-FT	from	6,599	to	6,600	0.04
1.28	AC-FT	from	6,600	to	6,602	1.32
1.69	AC-FT	from	6,602	to	6,603	3.01
1.17	AC-FT	from	6,603	to	6,604	4.18
-	AC-FT	from	6,604	to	-	4.18
-	AC-FT	from	-	to	-	4.18
-	AC-FT	from	-	to	-	4.18
-	AC-FT	from	-	to	-	4.18
-	AC-FT	from	-	to	-	4.18
-	AC-FT	from	-	to	-	4.18
-	AC-FT	from	-	to	-	4.18

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

VOLUME = [redacted] AC-FT

APPROXIMATE SURFACE AREA REQUIREMENT

POND DEPTH (FT)	POND VOLUME AC-FT		CF	SURFACE AREA (SF)
4	4.18	=	######	45,492
6	4.18	=	######	30,328
8	4.18	=	######	22,746
10	4.18	=	######	18,197

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ENGINEERS & SURVEYORS

POND PACK OUTPUT
EXISTING CONDITIONS

MASTER DESIGN STORM SUMMARY

Network Storm Collection: C Springs

Return Event	Total Depth in	Rainfall Type	RNF ID
2	2.0000	Synthetic Curve	TYPEIIA 24HR
5	2.6000	Synthetic Curve	TYPEIIA 24HR
10	3.0000	Synthetic Curve	TYPEIIA 24HR
25	3.6000	Synthetic Curve	TYPEIIA 24HR
50	4.0000	Synthetic Curve	TYPEIIA 24HR
100	4.4000	Synthetic Curve	TYPEIIA 24HR

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A1	AREA	2	.347		6.6466	.61		
A1	AREA	5	1.073		6.2625	6.55		
A1	AREA	10	1.731		6.2291	13.65		
A1	AREA	25	2.932		6.2124	27.90		
A1	AREA	50	3.853		6.2124	39.29		
A1	AREA	100	4.855		6.1957	52.06		
A12	AREA	2	.190		6.5965	.35		
A12	AREA	5	.586		6.1790	4.38		
A12	AREA	10	.946		6.1623	9.09		
A12	AREA	25	1.602		6.1456	18.32		
A12	AREA	50	2.106		6.1456	25.61		
A12	AREA	100	2.654		6.1289	33.67		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A13	AREA	2	.085		6.1957	.17		
A13	AREA	5	.262		6.1456	2.26		
A13	AREA	10	.423		6.1289	4.64		
A13	AREA	25	.716		6.1122	9.24		
A13	AREA	50	.941		6.1122	12.82		
A13	AREA	100	1.186		6.0955	16.77		
A14	AREA	2	.234		6.5965	.42		
A14	AREA	5	.724		6.1957	5.12		
A14	AREA	10	1.168		6.1790	10.64		
A14	AREA	25	1.979		6.1623	21.54		
A14	AREA	50	2.600		6.1623	30.18		
A14	AREA	100	3.277		6.1456	39.80		
A2	AREA	2	.658		6.6633	1.15		
A2	AREA	5	2.037		6.2959	11.58		
A2	AREA	10	3.286		6.2625	24.15		
A2	AREA	25	5.565		6.2458	49.44		
A2	AREA	50	7.312		6.2291	69.84		
A2	AREA	100	9.216		6.2291	92.62		
A3	AREA	2	1.172		6.0621	19.83		
A3	AREA	5	1.997		6.0454	34.56		
A3	AREA	10	2.604		6.0454	45.24		
A3	AREA	25	3.575		6.0454	62.06		
A3	AREA	50	4.254		6.0454	73.64		
A3	AREA	100	4.951		6.0287	85.49		
A4	AREA	2	.127		6.1122	1.65		
A4	AREA	5	.237		6.0955	3.38		
A4	AREA	10	.320		6.0955	4.72		
A4	AREA	25	.458		6.0955	6.92		
A4	AREA	50	.556		6.0955	8.47		
A4	AREA	100	.657		6.0788	10.09		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A5	AREA	2	.231		6.1456	2.83		
A5	AREA	5	.422		6.1289	5.67		
A5	AREA	10	.567		6.1122	7.84		
A5	AREA	25	.803		6.1122	11.43		
A5	AREA	50	.971		6.1122	13.98		
A5	AREA	100	1.145		6.1122	16.61		
A6	AREA	2	.795		6.0287	15.15		
A6	AREA	5	1.313		6.0120	24.83		
A6	AREA	10	1.689		6.0120	31.72		
A6	AREA	25	2.285		6.0120	42.41		
A6	AREA	50	2.699		6.0120	49.67		
A6	AREA	100	3.122		6.0120	57.02		
A7	AREA	2	1.093		6.1289	14.32		
A7	AREA	5	1.893		6.1122	26.33		
A7	AREA	10	2.486		6.1122	35.34		
A7	AREA	25	3.439		6.1122	49.80		
A7	AREA	50	4.107		6.1122	59.91		
A7	AREA	100	4.796		6.0955	70.36		
A8	AREA	2	1.431		6.1456	18.22		
A8	AREA	5	2.330		6.1456	30.97		
A8	AREA	10	2.978		6.1289	40.16		
A8	AREA	25	3.999		6.1289	54.82		
A8	AREA	50	4.706		6.1289	64.93		
A8	AREA	100	5.428		6.1289	75.24		
A9	AREA	2	1.822		6.1289	25.01		
A9	AREA	5	2.884		6.1122	40.67		
A9	AREA	10	3.640		6.1122	51.88		
A9	AREA	25	4.821		6.1122	69.34		
A9	AREA	50	5.632		6.1122	81.26		
A9	AREA	100	6.458		6.0955	93.39		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
B1	AREA	2	.245		6.0788	2.95		
B1	AREA	5	.544		6.0621	8.42		
B1	AREA	10	.788		6.0621	12.92		
B1	AREA	25	1.208		6.0621	20.51		
B1	AREA	50	1.516		6.0454	26.11		
B1	AREA	100	1.844		6.0454	32.02		
B2	AREA	2	1.990		6.0454	33.20		
B2	AREA	5	3.708		6.0454	65.46		
B2	AREA	10	5.020		6.0454	89.55		
B2	AREA	25	7.174		6.0454	128.36		
B2	AREA	50	8.708		6.0287	155.96		
B2	AREA	100	10.305		6.0287	184.43		
B3	AREA	2	1.875		6.0621	31.11		
B3	AREA	5	3.196		6.0621	54.19		
B3	AREA	10	4.167		6.0454	71.14		
B3	AREA	25	5.720		6.0454	97.89		
B3	AREA	50	6.806		6.0454	116.35		
B3	AREA	100	7.922		6.0454	135.15		
B5	AREA	2	.229		6.1456	2.31		
B5	AREA	5	.479		6.1289	6.04		
B5	AREA	10	.679		6.1122	9.10		
B5	AREA	25	1.017		6.1122	14.37		
B5	AREA	50	1.263		6.1122	18.20		
B5	AREA	100	1.523		6.0955	22.25		
B6	AREA	2	.112		6.2625	.21		
B6	AREA	5	.347		6.1623	2.67		
B6	AREA	10	.560		6.1456	5.54		
B6	AREA	25	.948		6.1456	11.14		
B6	AREA	50	1.246		6.1289	15.58		
B6	AREA	100	1.570		6.1289	20.47		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
B7	AREA	2	.288		6.1623	2.49		
B7	AREA	5	.641		6.1289	7.57		
B7	AREA	10	.929		6.1289	11.92		
B7	AREA	25	1.424		6.1122	19.51		
B7	AREA	50	1.787		6.1122	25.14		
B7	AREA	100	2.173		6.1122	31.13		
B9	AREA	2	.330		6.0120	6.22		
B9	AREA	5	.501		6.0120	9.25		
B9	AREA	10	.621		6.0120	11.31		
B9	AREA	25	.806		6.0120	14.42		
B9	AREA	50	.932		6.0120	16.50		
B9	AREA	100	1.060		5.9953	18.59		
C1	AREA	2	.168		6.2458	.31		
C1	AREA	5	.519		6.1623	4.02		
C1	AREA	10	.837		6.1456	8.32		
C1	AREA	25	1.417		6.1289	16.72		
C1	AREA	50	1.862		6.1289	23.39		
C1	AREA	100	2.347		6.1289	30.71		
D1	AREA	2	3.250		6.0621	53.05		
D1	AREA	5	4.940		6.0621	80.73		
D1	AREA	10	6.121		6.0454	99.82		
D1	AREA	25	7.946		6.0454	129.16		
D1	AREA	50	9.187		6.0454	148.88		
D1	AREA	100	10.444		6.0454	168.68		
DP 1	JCT	2	2.301		6.1456	17.26		
DP 1	JCT	5	4.400		6.1122	39.67		
DP 1	JCT	10	6.023		6.0788	53.49		
DP 1	JCT	25	8.710		6.0788	72.12		
DP 1	JCT	50	10.636		6.0788	85.95		
DP 1	JCT	100	12.650		6.0621	98.98		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 10	JCT	2	3.407		6.1456	42.30		
DP 10	JCT	5	5.449		6.1456	68.33		
DP 10	JCT	10	6.948		6.1623	85.89		
DP 10	JCT	25	9.352		6.1623	115.27		
DP 10	JCT	50	11.039		6.1623	135.02		
DP 10	JCT	100	12.781		6.1623	152.60		
DP 11	JCT	2	5.941		6.1289	61.97		
DP 11	JCT	5	11.214		6.1289	106.44		
DP 11	JCT	10	15.456		6.1456	142.96		
DP 11	JCT	25	22.684		6.1623	215.92		
DP 11	JCT	50	27.983		6.1623	273.19		
DP 11	JCT	100	33.604		6.1790	333.29		
DP 14	JCT	2	17.408		8.0661	15.61		
DP 14	JCT	5	32.307		6.6132	19.92		
DP 14	JCT	10	44.022	R	6.2291	26.53		
DP 14	JCT	25	63.644	R	7.8490	62.30		
DP 14	JCT	50	77.635	R	7.3981	97.45		
DP 14	JCT	100	92.277	R	7.2311	137.18		
*DP 18	JCT	2	17.408		8.1496	15.60		
*DP 18	JCT	5	32.308		8.1162	19.90		
*DP 18	JCT	10	44.022	R	8.6673	25.44		
*DP 18	JCT	25	63.641	R	8.0661	62.15		
*DP 18	JCT	50	77.632	R	7.6152	96.64		
*DP 18	JCT	100	92.276	R	7.4315	135.83		
DP 2	JCT	2	4.583		6.1790	22.93		
DP 2	JCT	5	8.681		6.1289	56.08		
DP 2	JCT	10	11.871		6.1122	79.77		
DP 2	JCT	25	17.180		6.0955	116.78		
DP 2	JCT	50	21.001		6.0955	143.70		
DP 2	JCT	100	25.007		6.0788	169.78		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 3	JCT	2	4.860		6.1957	23.78		
DP 3	JCT	5	9.134		6.1456	58.66		
DP 3	JCT	10	12.449		6.1122	84.60		
DP 3	JCT	25	17.957		6.1122	124.29		
DP 3	JCT	50	21.916		6.0955	153.19		
DP 3	JCT	100	26.062		6.0955	181.69		
DP 4	JCT	2	5.089		6.1957	25.89		
DP 4	JCT	5	9.612		6.1456	64.51		
DP 4	JCT	10	13.128		6.1289	93.58		
DP 4	JCT	25	18.974		6.1122	138.51		
DP 4	JCT	50	23.179		6.1122	171.10		
DP 4	JCT	100	27.585		6.0955	203.65		
DP 5	JCT	2	5.490		6.1957	28.41		
DP 5	JCT	5	10.600		6.1623	74.53		
DP 5	JCT	10	14.617		6.1289	110.84		
DP 5	JCT	25	21.346		6.1289	168.83		
DP 5	JCT	50	26.212		6.1122	211.43		
DP 5	JCT	100	31.328		6.1122	254.85		
DP 6	JCT	2	5.819		6.1957	29.33		
DP 6	JCT	5	11.102		6.1623	76.61		
DP 6	JCT	10	15.238		6.1289	114.30		
DP 6	JCT	25	22.152		6.1122	174.03		
DP 6	JCT	50	27.144		6.1122	217.80		
DP 6	JCT	100	32.388		6.1122	262.29		
DP 7	JCT	2	17.134		6.1456	142.52		
DP 7	JCT	5	31.460		6.1623	279.60		
DP 7	JCT	10	42.654		6.1456	393.16		
DP 7	JCT	25	61.359		6.1456	587.34		
DP 7	JCT	50	74.870		6.1456	730.63		
DP 7	JCT	100	89.072		6.1289	878.03		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 8	JCT	2	1.004		6.7468	1.61		
DP 8	JCT	5	3.109		6.3293	15.82		
DP 8	JCT	10	5.017		6.2959	34.40		
DP 8	JCT	25	8.496		6.2625	72.53		
DP 8	JCT	50	11.164		6.2625	103.60		
DP 8	JCT	100	14.070		6.2458	138.56		
DP 9	JCT	2	5.583		6.0955	58.52		
DP 9	JCT	5	10.555		6.0955	98.35		
DP 9	JCT	10	14.569		6.1289	131.38		
DP 9	JCT	25	21.424		6.1623	199.90		
DP 9	JCT	50	26.457		6.1623	254.15		
DP 9	JCT	100	31.802		6.1790	311.03		
DP12	JCT	2	3.253		6.1456	43.06		
DP12	JCT	5	5.214		6.1289	71.50		
DP12	JCT	10	6.617		6.1289	91.85		
DP12	JCT	25	8.820		6.1122	123.85		
DP12	JCT	50	10.338		6.1122	145.96		
DP12	JCT	100	11.886		6.1122	168.43		
DP13	JCT	2	17.134		8.1997	15.27		
DP13	JCT	5	31.460		8.5504	18.97		
DP13	JCT	10	42.654		8.6673	24.55		
DP13	JCT	25	61.331	R	7.8490	59.83		
DP13	JCT	50	74.597	R	7.4148	94.08		
DP13	JCT	100	88.448	R	7.2812	132.49		
EK1A IN	POND	2	17.134		6.1456	142.52		
EK1A IN	POND	5	31.460		6.1623	279.60		
EK1A IN	POND	10	42.654		6.1456	393.16		
EK1A IN	POND	25	61.359		6.1456	587.34		
EK1A IN	POND	50	74.870		6.1456	730.63		
EK1A IN	POND	100	89.072		6.1289	878.03		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK1A	OUT POND	2	17.134		8.1997	15.27	6613.35	6.738
EK1A	OUT POND	5	31.460		8.5504	18.97	6614.97	15.351
EK1A	OUT POND	10	42.654		8.6673	24.55	6616.04	22.377
EK1A	OUT POND	25	61.331	R	7.8490	59.83	6617.04	30.564
EK1A	OUT POND	50	74.597	R	7.4148	94.08	6617.69	35.939
EK1A	OUT POND	100	88.448	R	7.2812	132.49	6618.32	41.336
EK1B	IN POND	2	3.250		6.0621	53.05		
EK1B	IN POND	5	4.940		6.0621	80.73		
EK1B	IN POND	10	6.121		6.0454	99.82		
EK1B	IN POND	25	7.946		6.0454	129.16		
EK1B	IN POND	50	9.187		6.0454	148.88		
EK1B	IN POND	100	10.444		6.0454	168.68		
EK1B	OUT POND	2	3.240		6.1456	42.25	6692.84	.553
EK1B	OUT POND	5	4.930		6.1456	65.96	6693.83	.850
EK1B	OUT POND	10	6.111		6.1456	79.25	6694.46	1.080
EK1B	OUT POND	25	7.935		6.1456	100.04	6695.50	1.486
EK1B	OUT POND	50	9.177		6.1456	112.94	6696.20	1.778
EK1B	OUT POND	100	10.434		6.1623	123.14	6696.88	2.118
I25-1	IN POND	2	17.408		8.0661	15.61		
I25-1	IN POND	5	32.307		6.6132	19.92		
I25-1	IN POND	10	44.022	R	6.2291	26.53		
I25-1	IN POND	25	63.644	R	7.8490	62.30		
I25-1	IN POND	50	77.635	R	7.3981	97.45		
I25-1	IN POND	100	92.277	R	7.2311	137.18		
I25-1	OUT POND	2	17.408		8.1496	15.60	6590.44	.089
I25-1	OUT POND	5	32.308		8.1162	19.90	6590.64	.120
I25-1	OUT POND	10	44.022	R	8.6673	25.44	6590.88	.156
I25-1	OUT POND	25	63.641	R	8.0661	62.15	6592.09	.375
I25-1	OUT POND	50	77.632	R	7.6152	96.64	6593.02	.864
I25-1	OUT POND	100	92.276	R	7.4315	135.83	6594.01	1.386

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
IP-1		AREA	2	.070		6.0287	1.27		
IP-1		AREA	5	.113		6.0287	2.07		
IP-1		AREA	10	.145		6.0287	2.63		
IP-1		AREA	25	.194		6.0287	3.49		
IP-1		AREA	50	.229		6.0120	4.08		
IP-1		AREA	100	.264		6.0120	4.68		
IP-2		AREA	2	.145		6.0287	2.65		
IP-2		AREA	5	.236		6.0287	4.30		
IP-2		AREA	10	.301		6.0287	5.47		
IP-2		AREA	25	.405		6.0287	7.27		
IP-2		AREA	50	.476		6.0120	8.49		
IP-2		AREA	100	.549		6.0120	9.74		
NLC	IN	POND	2	1.875		6.0621	31.11		
NLC	IN	POND	5	3.196		6.0621	54.19		
NLC	IN	POND	10	4.167		6.0454	71.14		
NLC	IN	POND	25	5.720		6.0454	97.89		
NLC	IN	POND	50	6.806		6.0454	116.35		
NLC	IN	POND	100	7.922		6.0454	135.15		
NLC	OUT	POND	2	1.875		6.3794	5.01	6737.07	.708
NLC	OUT	POND	5	3.195		6.2959	11.97	6737.79	1.267
NLC	OUT	POND	10	4.166		6.2792	17.07	6738.38	1.691
NLC	OUT	POND	25	5.720		6.2792	23.49	6739.25	2.405
NLC	OUT	POND	50	6.805		6.2792	26.23	6739.71	2.950
NLC	OUT	POND	100	7.921		6.2959	28.62	6740.21	3.531
O1		AREA	2	.311		6.0454	4.73		
O1		AREA	5	.692		6.0287	12.73		
O1		AREA	10	1.003		6.0287	19.06		
O1		AREA	25	1.536		6.0287	29.60		
O1		AREA	50	1.928		6.0287	37.17		
O1		AREA	100	2.344		6.0120	45.16		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
O2B		AREA	2	.163		6.5965	.25		
O2B		AREA	5	.543		6.1289	4.42		
O2B		AREA	10	.894		6.1122	9.72		
O2B		AREA	25	1.543		6.0955	20.10		
O2B		AREA	50	2.044		6.0955	28.39		
O2B		AREA	100	2.593		6.0955	37.46		
PCC	IN	POND	2	1.990		6.0454	33.20		
PCC	IN	POND	5	3.708		6.0454	65.46		
PCC	IN	POND	10	5.020		6.0454	89.55		
PCC	IN	POND	25	7.174		6.0454	128.36		
PCC	IN	POND	50	8.708		6.0287	155.96		
PCC	IN	POND	100	10.305		6.0287	184.43		
PCC	OUT	POND	2	1.990		6.1790	14.72	6758.26	.420
PCC	OUT	POND	5	3.708		6.1623	31.59	6759.71	.903
PCC	OUT	POND	10	5.020		6.1623	39.19	6760.42	1.326
PCC	OUT	POND	25	7.174		6.1790	49.69	6761.47	2.146
PCC	OUT	POND	50	8.708		6.1790	56.38	6762.09	2.775
PCC	OUT	POND	100	10.305		6.1957	60.98	6762.40	3.488
VP-1		AREA	2	.064		6.0287	1.19		
VP-1		AREA	5	.104		6.0287	1.92		
VP-1		AREA	10	.133		6.0120	2.43		
VP-1		AREA	25	.178		6.0120	3.24		
VP-1		AREA	50	.210		6.0120	3.78		
VP-1		AREA	100	.242		6.0120	4.33		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = C Springs

Storm Tag Name = 5

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 5 yr
Total Rainfall Depth= 2.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	3.708		6.0454	65.46	B2	
		DL	3.708		6.0454	65.46		
		DN	3.708		6.0454	65.46	PCC	IN
ADDLINK 100	ADD	UN	.501		6.0120	9.25	B9	
		DL	.501		6.0120	9.25		
		DN	11.102		6.1623	76.61	DP 6	
ADDLINK 110	ADD	UN	.724		6.1957	5.12	A14	
		DL	.724		6.1957	5.12		
		DN	31.460		6.1623	279.60	DP 7	
ADDLINK 120	ADD	UN	1.313		6.0120	24.83	A6	
		DL	1.313		6.0120	24.83		
		DN	31.460		6.1623	279.60	DP 7	
ADDLINK 130	ADD	UN	2.037		6.2959	11.58	A2	
		DL	2.037		6.2959	11.58		
		DN	3.109		6.3293	15.82	DP 8	
ADDLINK 140	ADD	UN	4.940		6.0621	80.73	D1	
		DL	4.940		6.0621	80.73		
		DN	4.940		6.0621	80.73	EK1B	IN
ADDLINK 150	ADD	UN	1.997		6.0454	34.56	A3	
		DL	1.997		6.0454	34.56		
		DN	10.555		6.0955	98.35	DP 9	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 160	ADD	UN	.237		6.0955	3.38	A4	
		DL	.237		6.0955	3.38		
		DN	11.214		6.1289	106.44	DP 11	
ADDLINK 170	ADD	UN	.422		6.1289	5.67	A5	
		DL	.422		6.1289	5.67		
		DN	11.214		6.1289	106.44	DP 11	
ADDLINK 180	ADD	UN	2.884		6.1122	40.67	A9	
		DL	2.884		6.1122	40.67		
		DN	5.214		6.1289	71.50	DP12	
ADDLINK 190	ADD	UN	31.460		6.1623	279.60	DP 7	
		DL	31.460		6.1623	279.60		
		DN	31.460		6.1623	279.60	EK1A	IN
ADDLINK 20	ADD	UN	3.196		6.0621	54.19	B3	
		DL	3.196		6.0621	54.19		
		DN	3.196		6.0621	54.19	NLC	IN
ADDLINK 200	ADD	UN	.586		6.1790	4.38	A12	
		DL	.586		6.1790	4.38		
		DN	32.307		6.6132	19.92	DP 14	
ADDLINK 210	ADD	UN	32.307		6.6132	19.92	DP 14	
		DL	32.307		6.6132	19.92		
		DN	32.307		6.6132	19.92	I25-1	IN
ADDLINK 30	ADD	UN	.544		6.0621	8.42	B1	
		DL	.544		6.0621	8.42		
		DN	8.681		6.1289	56.08	DP 2	
ADDLINK 40	ADD	UN	.104		6.0287	1.92	VP-1	
		DL	.104		6.0287	1.92		
		DN	9.134		6.1456	58.66	DP 3	
ADDLINK 50	ADD	UN	.113		6.0287	2.07	IP-1	
		DL	.113		6.0287	2.07		
		DN	9.134		6.1456	58.66	DP 3	

NETWORK SUMMARY -- LINKS

(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	.236		6.0287	4.30	IP-2
		DL	.236		6.0287	4.30	
		DN	9.134		6.1456	58.66	DP 3
ADDLINK 70	ADD	UN	.479		6.1289	6.04	B5
		DL	.479		6.1289	6.04	
		DN	9.612		6.1456	64.51	DP 4
ADDLINK 80	ADD	UN	.347		6.1623	2.67	B6
		DL	.347		6.1623	2.67	
		DN	10.600		6.1623	74.53	DP 5
ADDLINK 90	ADD	UN	.641		6.1289	7.57	B7
		DL	.641		6.1289	7.57	
		DN	10.600		6.1623	74.53	DP 5
C1	REACH	UN	.692		6.0287	12.73	O1
		DL	.692		6.0621	10.55	
		DN	4.400		6.1122	39.67	DP 1
C10	REACH	UN	3.109		6.3293	15.82	DP 8
		DL	3.109		6.3961	15.34	
		DN	10.555		6.0955	98.35	DP 9
C11	REACH	UN	.519		6.1623	4.02	C1
		DL	.519		6.2458	3.43	
		DN	5.449		6.1456	68.33	DP 10
C12	ADD	UN	5.449		6.1456	68.33	DP 10
		DL	5.449		6.1456	68.33	
		DN	10.555		6.0955	98.35	DP 9
C13	REACH	UN	10.555		6.0955	98.35	DP 9
		DL	10.555		6.1289	97.45	
		DN	11.214		6.1289	106.44	DP 11
C14	REACH	UN	11.214		6.1289	106.44	DP 11
		DL	11.214		6.1289	106.23	
		DN	31.460		6.1623	279.60	DP 7

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C15	ADD	UN	2.330		6.1456	30.97	A8
		DL	2.330		6.1456	30.97	
		DN	5.214		6.1289	71.50	DP12
C16	REACH	UN	5.214		6.1289	71.50	DP12
		DL	5.214		6.1289	71.33	
		DN	31.460		6.1623	279.60	DP 7
C17	REACH	UN	1.893		6.1122	26.33	A7
		DL	1.893		6.1456	26.02	
		DN	31.460		6.1623	279.60	DP 7
C18	REACH	UN	31.460		8.5504	18.97	DP13
		DL	31.459		8.6005	18.97	
		DN	32.307		6.6132	19.92	DP 14
C19	REACH	UN	.262		6.1456	2.26	A13
		DL	.262		6.3460	.93	
		DN	32.307		6.6132	19.92	DP 14
C2	REACH	UN	4.400		6.1122	39.67	DP 1
		DL	4.400		6.1456	39.25	
		DN	8.681		6.1289	56.08	DP 2
C3	REACH	UN	.543		6.1289	4.42	O2B
		DL	.542		6.3961	1.57	
		DN	8.681		6.1289	56.08	DP 2
C4	REACH	UN	8.681		6.1289	56.08	DP 2
		DL	8.681		6.1623	55.36	
		DN	9.134		6.1456	58.66	DP 3
C5	REACH	UN	9.134		6.1456	58.66	DP 3
		DL	9.134		6.1623	58.56	
		DN	9.612		6.1456	64.51	DP 4
C6	REACH	UN	9.612		6.1456	64.51	DP 4
		DL	9.612		6.1623	64.45	
		DN	10.600		6.1623	74.53	DP 5

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
C7	REACH	UN	10.600		6.1623	74.53	DP 5	
		DL	10.600		6.1623	74.49		
		DN	11.102		6.1623	76.61	DP 6	
C8	REACH	UN	11.102		6.1623	76.61	DP 6	
		DL	11.101		6.2291	71.44		
		DN	31.460		6.1623	279.60	DP 7	
C9	REACH	UN	1.073		6.2625	6.55	A1	
		DL	1.073		6.4128	4.93		
		DN	3.109		6.3293	15.82	DP 8	
EK-1A STRUCTURE	PONDrt	UN	31.460		6.1623	279.60	EK1A	IN
EK-1A STRUCTURE			31.460		8.5504	18.97	EK1A	OUT
		DL	31.460		8.5504	18.97		
		DN	31.460		8.5504	18.97	DP13	
EK-1B STRUCTURE	PONDrt	UN	4.940		6.0621	80.73	EK1B	IN
EK-1B STRUCTURE			4.930		6.1456	65.96	EK1B	OUT
		DL	4.930		6.1456	65.96		
		DN	5.449		6.1456	68.33	DP 10	
I25-1 STRUCTURE	PONDrt	UN	32.307		6.6132	19.92	I25-1	IN
I25-1 STRUCTURE			32.308		8.1162	19.90	I25-1	OUT
		DL	32.308		8.1162	19.90		
		DN	32.308		8.1162	19.90	DP 18	
NLC STRUCTURE	PONDrt	UN	3.196		6.0621	54.19	NLC	IN
NLC STRUCTURE			3.195		6.2959	11.97	NLC	OUT
		DL	3.195		6.2959	11.97		
		DN	8.681		6.1289	56.08	DP 2	
PCC STRUCTURE	PONDrt	UN	3.708		6.0454	65.46	PCC	IN
PCC STRUCTURE			3.708		6.1623	31.59	PCC	OUT
		DL	3.708		6.1623	31.59		
		DN	4.400		6.1122	39.67	DP 1	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = C Springs

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 100 yr
Total Rainfall Depth= 4.4000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	10.305		6.0287	184.43	B2	
		DL	10.305		6.0287	184.43		
		DN	10.305		6.0287	184.43	PCC	IN
ADDLINK 100	ADD	UN	1.060		5.9953	18.59	B9	
		DL	1.060		5.9953	18.59		
		DN	32.388		6.1122	262.29	DP 6	
ADDLINK 110	ADD	UN	3.277		6.1456	39.80	A14	
		DL	3.277		6.1456	39.80		
		DN	89.072		6.1289	878.03	DP 7	
ADDLINK 120	ADD	UN	3.122		6.0120	57.02	A6	
		DL	3.122		6.0120	57.02		
		DN	89.072		6.1289	878.03	DP 7	
ADDLINK 130	ADD	UN	9.216		6.2291	92.62	A2	
		DL	9.216		6.2291	92.62		
		DN	14.070		6.2458	138.56	DP 8	
ADDLINK 140	ADD	UN	10.444		6.0454	168.68	D1	
		DL	10.444		6.0454	168.68		
		DN	10.444		6.0454	168.68	EK1B	IN
ADDLINK 150	ADD	UN	4.951		6.0287	85.49	A3	
		DL	4.951		6.0287	85.49		
		DN	31.802		6.1790	311.03	DP 9	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 160	ADD	UN	.657		6.0788	10.09	A4	
		DL	.657		6.0788	10.09		
		DN	33.604		6.1790	333.29	DP 11	
ADDLINK 170	ADD	UN	1.145		6.1122	16.61	A5	
		DL	1.145		6.1122	16.61		
		DN	33.604		6.1790	333.29	DP 11	
ADDLINK 180	ADD	UN	6.458		6.0955	93.39	A9	
		DL	6.458		6.0955	93.39		
		DN	11.886		6.1122	168.43	DP12	
ADDLINK 190	ADD	UN	89.072		6.1289	878.03	DP 7	
		DL	89.072		6.1289	878.03		
		DN	89.072		6.1289	878.03	EK1A	IN
ADDLINK 20	ADD	UN	7.922		6.0454	135.15	B3	
		DL	7.922		6.0454	135.15		
		DN	7.922		6.0454	135.15	NLC	IN
ADDLINK 200	ADD	UN	2.654		6.1289	33.67	A12	
		DL	2.654		6.1289	33.67		
		DN	92.277	R	7.2311	137.18	DP 14	
ADDLINK 210	ADD	UN	92.277	R	7.2311	137.18	DP 14	
		DL	92.277	R	7.2311	137.18		
		DN	92.277	R	7.2311	137.18	I25-1	IN
ADDLINK 30	ADD	UN	1.844		6.0454	32.02	B1	
		DL	1.844		6.0454	32.02		
		DN	25.007		6.0788	169.78	DP 2	
ADDLINK 40	ADD	UN	.242		6.0120	4.33	VP-1	
		DL	.242		6.0120	4.33		
		DN	26.062		6.0955	181.69	DP 3	
ADDLINK 50	ADD	UN	.264		6.0120	4.68	IP-1	
		DL	.264		6.0120	4.68		
		DN	26.062		6.0955	181.69	DP 3	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	.549		6.0120	9.74	IP-2
		DL	.549		6.0120	9.74	
		DN	26.062		6.0955	181.69	DP 3
ADDLINK 70	ADD	UN	1.523		6.0955	22.25	B5
		DL	1.523		6.0955	22.25	
		DN	27.585		6.0955	203.65	DP 4
ADDLINK 80	ADD	UN	1.570		6.1289	20.47	B6
		DL	1.570		6.1289	20.47	
		DN	31.328		6.1122	254.85	DP 5
ADDLINK 90	ADD	UN	2.173		6.1122	31.13	B7
		DL	2.173		6.1122	31.13	
		DN	31.328		6.1122	254.85	DP 5
C1	REACH	UN	2.344		6.0120	45.16	O1
		DL	2.344		6.0454	42.84	
		DN	12.650		6.0621	98.98	DP 1
C10	REACH	UN	14.070		6.2458	138.56	DP 8
		DL	14.070		6.2792	137.05	
		DN	31.802		6.1790	311.03	DP 9
C11	REACH	UN	2.347		6.1289	30.71	C1
		DL	2.346		6.1623	29.46	
		DN	12.781		6.1623	152.60	DP 10
C12	ADD	UN	12.781		6.1623	152.60	DP 10
		DL	12.781		6.1623	152.60	
		DN	31.802		6.1790	311.03	DP 9
C13	REACH	UN	31.802		6.1790	311.03	DP 9
		DL	31.802		6.1957	310.36	
		DN	33.604		6.1790	333.29	DP 11
C14	REACH	UN	33.604		6.1790	333.29	DP 11
		DL	33.604		6.1790	333.27	
		DN	89.072		6.1289	878.03	DP 7

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C15	ADD	UN	5.428		6.1289	75.24	A8
		DL	5.428		6.1289	75.24	
		DN	11.886		6.1122	168.43	DP12
C16	REACH	UN	11.886		6.1122	168.43	DP12
		DL	11.886		6.1122	168.10	
		DN	89.072		6.1289	878.03	DP 7
C17	REACH	UN	4.796		6.0955	70.36	A7
		DL	4.796		6.1122	69.92	
		DN	89.072		6.1289	878.03	DP 7
C18	REACH	UN	88.448	R	7.2812	132.49	DP13
		DL	88.438	R	7.3146	132.48	
		DN	92.277	R	7.2311	137.18	DP 14
C19	REACH	UN	1.186		6.0955	16.77	A13
		DL	1.186		6.2124	11.74	
		DN	92.277	R	7.2311	137.18	DP 14
C2	REACH	UN	12.650		6.0621	98.98	DP 1
		DL	12.650		6.0788	98.20	
		DN	25.007		6.0788	169.78	DP 2
C3	REACH	UN	2.593		6.0955	37.46	O2B
		DL	2.592		6.2124	24.70	
		DN	25.007		6.0788	169.78	DP 2
C4	REACH	UN	25.007		6.0788	169.78	DP 2
		DL	25.007		6.1122	168.19	
		DN	26.062		6.0955	181.69	DP 3
C5	REACH	UN	26.062		6.0955	181.69	DP 3
		DL	26.062		6.0955	181.40	
		DN	27.585		6.0955	203.65	DP 4
C6	REACH	UN	27.585		6.0955	203.65	DP 4
		DL	27.585		6.1122	203.36	
		DN	31.328		6.1122	254.85	DP 5

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
C7	REACH	UN	31.328		6.1122	254.85	DP 5	
		DL	31.328		6.1122	254.17		
		DN	32.388		6.1122	262.29	DP 6	
C8	REACH	UN	32.388		6.1122	262.29	DP 6	
		DL	32.388		6.1456	254.44		
		DN	89.072		6.1289	878.03	DP 7	
C9	REACH	UN	4.855		6.1957	52.06	A1	
		DL	4.855		6.2792	47.25		
		DN	14.070		6.2458	138.56	DP 8	
EK-1A STRUCTURE	PONDrt	UN	89.072		6.1289	878.03	EK1A	IN
EK-1A STRUCTURE			88.448	R	7.2812	132.49	EK1A	OUT
		DL	88.448	R	7.2812	132.49		
		DN	88.448	R	7.2812	132.49	DP13	
EK-1B STRUCTURE	PONDrt	UN	10.444		6.0454	168.68	EK1B	IN
EK-1B STRUCTURE			10.434		6.1623	123.14	EK1B	OUT
		DL	10.434		6.1623	123.14		
		DN	12.781		6.1623	152.60	DP 10	
I25-1 STRUCTURE	PONDrt	UN	92.277	R	7.2311	137.18	I25-1	IN
I25-1 STRUCTURE			92.276	R	7.4315	135.83	I25-1	OUT
		DL	92.276	R	7.4315	135.83		
		DN	92.276	R	7.4315	135.83	DP 18	
NLC STRUCTURE	PONDrt	UN	7.922		6.0454	135.15	NLC	IN
NLC STRUCTURE			7.921		6.2959	28.62	NLC	OUT
		DL	7.921		6.2959	28.62		
		DN	25.007		6.0788	169.78	DP 2	
PCC STRUCTURE	PONDrt	UN	10.305		6.0287	184.43	PCC	IN
PCC STRUCTURE			10.305		6.1957	60.98	PCC	OUT
		DL	10.305		6.1957	60.98		
		DN	12.650		6.0621	98.98	DP 1	

MASTER DESIGN STORM SUMMARY

Network Storm Collection: El Paso County

Return Event	Total Depth in	Rainfall Type	RNF ID
2	2.0000	Synthetic Curve	TYPEIIA 24HR
5	2.6000	Synthetic Curve	TYPEIIA 24HR
10	3.0000	Synthetic Curve	TYPEIIA 24HR
25	3.6000	Synthetic Curve	TYPEIIA 24HR
50	4.0000	Synthetic Curve	TYPEIIA 24HR
100	4.4000	Synthetic Curve	TYPEIIA 24HR

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 15	JCT	2	.295		6.0500	4.88		
DP 15	JCT	5	.481		6.0500	8.05		
DP 15	JCT	10	.615		6.0500	10.29		
DP 15	JCT	25	.826		6.0500	13.78		
DP 15	JCT	50	.972		6.0500	16.16		
DP 15	JCT	100	1.121		6.0500	18.56		
DP 16	JCT	2	.295		6.0500	4.71		
DP 16	JCT	5	.481		6.0500	7.87		
DP 16	JCT	10	.615		6.0500	10.12		
DP 16	JCT	25	.826		6.0500	13.64		
DP 16	JCT	50	.972		6.0500	16.04		
DP 16	JCT	100	1.121		6.0500	18.48		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 17	JCT	2	.987		6.1500	4.36		
DP 17	JCT	5	2.621		6.2000	18.46		
DP 17	JCT	10	4.068		6.2000	35.45		
DP 17	JCT	25	6.674		6.2000	68.25		
DP 17	JCT	50	8.656		6.1500	94.34		
DP 17	JCT	100	10.805		6.1500	123.89		
*DP 19	JCT	2	.987		6.2500	3.96		
*DP 19	JCT	5	2.621		6.5500	8.59		
*DP 19	JCT	10	4.068		6.5500	14.56		
*DP 19	JCT	25	6.673		6.5500	23.46		
*DP 19	JCT	50	8.656		6.6000	26.42		
*DP 19	JCT	100	10.805		6.6500	29.33		
E5	AREA	2	.209		6.6000	.38		
E5	AREA	5	.647		6.2000	4.53		
E5	AREA	10	1.044		6.1500	9.49		
E5	AREA	25	1.767		6.1500	19.29		
E5	AREA	50	2.322		6.1500	26.99		
E5	AREA	100	2.927		6.1500	35.43		
E7	AREA	2	.483		6.6000	.86		
E7	AREA	5	1.494		6.2000	9.69		
E7	AREA	10	2.410		6.2000	20.24		
E7	AREA	25	4.081		6.1500	40.79		
E7	AREA	50	5.362		6.1500	57.82		
E7	AREA	100	6.758		6.1500	76.64		
I25-2 IN	POND	2	.987		6.1500	4.36		
I25-2 IN	POND	5	2.621		6.2000	18.46		
I25-2 IN	POND	10	4.068		6.2000	35.45		
I25-2 IN	POND	25	6.674		6.2000	68.25		
I25-2 IN	POND	50	8.656		6.1500	94.34		
I25-2 IN	POND	100	10.805		6.1500	123.89		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
I25-2	OUT POND	2	.987		6.2500	3.96	6599.94	.037
I25-2	OUT POND	5	2.621		6.5500	8.59	6600.47	.343
I25-2	OUT POND	10	4.068		6.5500	14.56	6601.07	.726
I25-2	OUT POND	25	6.673		6.5500	23.46	6602.13	1.539
I25-2	OUT POND	50	8.656		6.6000	26.42	6602.60	2.342
I25-2	OUT POND	100	10.805		6.6500	29.33	6603.13	3.304
IP-3	AREA	2	.116		6.0500	1.91		
IP-3	AREA	5	.189		6.0500	3.16		
IP-3	AREA	10	.241		6.0500	4.04		
IP-3	AREA	25	.324		6.0500	5.40		
IP-3	AREA	50	.381		6.0500	6.34		
IP-3	AREA	100	.439		6.0500	7.28		
IP-4	AREA	2	.180		6.0500	2.97		
IP-4	AREA	5	.292		6.0500	4.89		
IP-4	AREA	10	.374		6.0500	6.26		
IP-4	AREA	25	.502		6.0500	8.37		
IP-4	AREA	50	.591		6.0500	9.82		
IP-4	AREA	100	.681		6.0500	11.28		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = El Paso County

Storm Tag Name = 5

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 5 yr
Total Rainfall Depth= 2.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	.189		6.0500	3.16	IP-3	
		DL	.189		6.0500	3.16		
		DN	.481		6.0500	8.05	DP 15	
ADDLINK 20	ADD	UN	.292		6.0500	4.89	IP-4	
		DL	.292		6.0500	4.89		
		DN	.481		6.0500	8.05	DP 15	
ADDLINK 30	ADD	UN	.647		6.2000	4.53	E5	
		DL	.647		6.2000	4.53		
		DN	2.621		6.2000	18.46	DP 17	
ADDLINK 40	ADD	UN	2.621		6.2000	18.46	DP 17	
		DL	2.621		6.2000	18.46		
		DN	2.621		6.2000	18.46	I25-2	IN
C20	REACH	UN	1.494		6.2000	9.69	E7	
		DL	1.493		6.3000	8.94		
		DN	2.621		6.2000	18.46	DP 17	
C21	REACH	UN	.481		6.0500	8.05	DP 15	
		DL	.481		6.0500	7.87		
		DN	.481		6.0500	7.87	DP 16	
C22	REACH	UN	.481		6.0500	7.87	DP 16	
		DL	.481		6.1000	6.96		
		DN	2.621		6.2000	18.46	DP 17	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
I25-2 STRUCTURE	PONDrt	UN	2.621		6.2000	18.46	I25-2	IN
I25-2 STRUCTURE			2.621		6.5500	8.59	I25-2	OUT
		DL	2.621		6.5500	8.59		
		DN	2.621		6.5500	8.59	DP 19	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = El Paso County

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 100 yr
Total Rainfall Depth= 4.4000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	.439		6.0500	7.28	IP-3	
		DL	.439		6.0500	7.28		
		DN	1.121		6.0500	18.56	DP 15	
ADDLINK 20	ADD	UN	.681		6.0500	11.28	IP-4	
		DL	.681		6.0500	11.28		
		DN	1.121		6.0500	18.56	DP 15	
ADDLINK 30	ADD	UN	2.927		6.1500	35.43	E5	
		DL	2.927		6.1500	35.43		
		DN	10.805		6.1500	123.89	DP 17	
ADDLINK 40	ADD	UN	10.805		6.1500	123.89	DP 17	
		DL	10.805		6.1500	123.89		
		DN	10.805		6.1500	123.89	I25-2	IN
C20	REACH	UN	6.758		6.1500	76.64	E7	
		DL	6.758		6.2000	74.97		
		DN	10.805		6.1500	123.89	DP 17	
C21	REACH	UN	1.121		6.0500	18.56	DP 15	
		DL	1.121		6.0500	18.48		
		DN	1.121		6.0500	18.48	DP 16	
C22	REACH	UN	1.121		6.0500	18.48	DP 16	
		DL	1.120		6.1000	17.56		
		DN	10.805		6.1500	123.89	DP 17	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
I25-2 STRUCTURE	PONDrt	UN	10.805		6.1500	123.89	I25-2	IN
I25-2 STRUCTURE			10.805		6.6500	29.33	I25-2	OUT
		DL	10.805		6.6500	29.33		
		DN	10.805		6.6500	29.33	DP 19	

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

HYDROLOGIC / HYDRAULIC CALCULATIONS
FULLY DEVELOPED CONDITIONS

COMPOSITE C_N VALUES - FULLY DEVELOPED CONDITIONS

BASIN (label)	BASIN AREA (ac)	LAND USE	SUB-AREA LAND USE 1		AREA (AC)	LAND USE	SUB-AREA LAND USE 2		AREA (AC)	TOTAL BASIN AREA (ac)	TOTAL BASIN AREA (sq mi)	WEIGHTED C_N
			ESTIMATED PERCENT IMPERIOUS	ESTIMATED			ESTIMATED PERCENT IMPERIOUS	ESTIMATED C_N				
02B	32.2	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	32.2	0.05031	60.0
O1	19.2	REPORT	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	19.2	0.03000	67.8
B1	15.1	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	15.1	0.02359	68.0
B2	62.7	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	62.7	0.09797	75.0
B3	40.0	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	40.0	0.06250	80.0
B5	11.4	UNDEV.	0	61	0.0	ROADS	100	88	11.4	11.4	0.01781	88.0
B6	18.4	UNDEV.	0	61	0.0	ROADS	100	88	18.4	18.4	0.02875	88.0
B7	17.8	UNDEV.	0	61	0.0	ROADS	100	88	17.8	17.8	0.02781	88.0
B9	4.1	UNDEV.	0	61	0.0	ROADS	100	88	4.1	4.1	0.00641	88.0
A1	56.9	UNDEV.	0	61	0.0	ROADS	100	88	56.9	56.9	0.08891	88.0
A2	108.0	UNDEV.	0	61	0.0	ROADS	100	88	108.0	108.0	0.16875	88.0
A3	25.0	UNDEV.	0	61	0.0	ROADS	100	88	25.0	25.0	0.03906	88.0
A4	4.0	UNDEV.	0	61	2.0	ROADS	100	88	2.0	4.0	0.00625	74.5
A5	6.7	UNDEV.	0	61	0.0	ROADS	100	88	6.7	6.7	0.01047	88.0
A6	14.7	UNDEV.	0	61	3.5	ROADS	100	88	11.2	14.7	0.02297	81.6
A7	25.1	UNDEV.	0	61	0.0	ROADS	100	88	25.1	25.1	0.03922	88.0
A8	24.7	UNDEV.	0	61	0.0	ROADS	100	88	24.7	24.7	0.03859	88.0
A9	27.5	UNDEV.	0	61	0.0	ROADS	100	88	27.5	27.5	0.04297	88.0
A12	31.1	UNDEV.	0	61	31.1	ROADS	100	88	0.0	31.1	0.04859	61.0
C1	62.5	UNDEV.	0	61	62.5	ROADS	100	88	0.0	62.5	0.09766	61.0
D1	40.4	UNDEV.	0	61	0.0	ROADS	100	88	40.4	40.4	0.06313	88.0
E5	34.3	UNDEV.	0	61	34.3	ROADS	100	88	0.0	34.3	0.05359	61.0
VP-1	1.1	UNDEV.	0	61	0.2	ROADS	100	88	0.9	1.1	0.00172	82.6
IP-1	1.2	UNDEV.	0	61	0.2	ROADS	100	88	1.0	1.2	0.00188	82.6
IP-2	2.5	UNDEV.	0	61	0.5	ROADS	100	88	2.0	2.5	0.00391	82.6
IP-3	2.0	UNDEV.	0	61	0.4	ROADS	100	88	1.6	2.0	0.00313	82.6
IP-4	3.1	UNDEV.	0	61	0.6	ROADS	100	88	2.5	3.1	0.00484	82.6
EK1A	3.6	UNDEV.	0	61	0.0	ROADS	100	88	3.6	3.6	0.00563	88.0
EK1B	7.7	UNDEV.	0	61	0.0	ROADS	100	88	7.7	7.7	0.01203	88.0
EK1C	4.8	UNDEV.	0	61	0.0	ROADS	100	88	4.8	4.8	0.00750	88.0
EK1D	8.3	UNDEV.	0	61	0.0	ROADS	100	88	8.3	8.3	0.01297	88.0
EK1E	12.4	UNDEV.	0	61	0.0	ROADS	100	88	12.4	12.4	0.01938	88.0
EK1F	5.5	UNDEV.	0	61	0.0	ROADS	100	88	5.5	5.5	0.00859	88.0
EK1G	4.6	UNDEV.	0	61	0.0	ROADS	100	88	4.6	4.6	0.00719	88.0
EK1H	10.9	UNDEV.	0	61	0.0	ROADS	100	88	10.9	10.9	0.01703	88.0
EK1I	5.9	UNDEV.	0	61	0.0	ROADS	100	88	5.9	5.9	0.00922	88.0
EKIJ	2.2	UNDEV.	0	61	0.0	ROADS	100	88	2.2	2.2	0.00344	88.0
EK1K	4.8	UNDEV.	0	61	0.0	ROADS	100	88	4.8	4.8	0.00750	88.0
EK1L	7.0	UNDEV.	0	61	0.0	ROADS	100	88	7.0	7.0	0.01094	88.0
EK1M	3.8	UNDEV.	0	61	0.0	ROADS	100	88	3.8	3.8	0.00594	88.0
EK1N	11.6	UNDEV.	0	61	0.0	ROADS	100	88	11.6	11.6	0.01813	88.0
EK1O	1.5	UNDEV.	0	61	1.5	ROADS	100	88	0.0	1.5	0.00234	61.0
EK2A	11.1	UNDEV.	0	61	0.0	ROADS	100	88	11.1	11.1	0.01734	88.0
EK2B	5.4	UNDEV.	0	61	0.0	ROADS	100	88	5.4	5.4	0.00844	88.0
EK2C	10.2	UNDEV.	0	61	0.0	ROADS	100	88	10.2	10.2	0.01594	88.0
EK2D	7.6	UNDEV.	0	61	1.5	ROADS	100	88	6.1	7.6	0.01188	82.7
EK2E	2.6	UNDEV.	0	61	2.6	ROADS	100	88	0.0	2.6	0.00406	61.0

TIME OF CONCENTRATION

BASIN	Cn	C(5)	Length (ft)	OVERLAND Height (ft)	Tc (min)	STREET / CHANNEL FLOW Length (ft)	Velocity (fps)	Tc (min)	Tc TOTAL (min)	Tc TOTAL (hr)	Lagtime TOTAL (hr)
02B	60.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	17.0000	0.2833	0.1700
O1	67.8	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	8.0000	0.1333	0.0800
B1	68.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	12.6000	0.2100	0.1260
B2	75.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	11.4000	0.1900	0.1140
B3	80.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	13.8000	0.2300	0.1380
B5	88.0	0.75	100	2	5.2	1375	3.0	7.6	12.8457	0.2141	0.1285
B6	88.0	0.75	100	2	5.2	1525	3.0	8.5	13.6790	0.2280	0.1368
B7	88.0	0.75	100	2	5.2	1425	3.0	7.9	13.1235	0.2187	0.1312
B9	88.0	0.75	100	2	5.2	500	3.0	2.8	7.9846	0.1331	0.0798
A1	88.0	0.75	100	2	5.2	2900	3.0	16.1	21.3179	0.3553	0.2132
A2	88.0	0.75	100	2	5.2	3400	3.0	18.9	24.0957	0.4016	0.2410
A3	88.0	0.75	100	2	5.2	1400	3.0	7.8	12.9846	0.2164	0.1298
A4	74.5	0.75	100	2	5.2	2500	3.0	13.9	19.0957	0.3183	0.1910
A5	88.0	0.75	100	2	5.2	1700	3.0	9.4	14.6512	0.2442	0.1465
A6	81.6	0.75	100	2	5.2	600	3.0	3.3	8.5401	0.1423	0.0854
A7	88.0	0.75	100	2	5.2	1700	3.0	9.4	14.6512	0.2442	0.1465
A8	88.0	0.75	100	2	5.2	2400	3.0	13.3	18.5401	0.3090	0.1854
A9	88.0	0.75	100	2	5.2	2000	3.0	11.1	16.3179	0.2720	0.1632
A12	61.0	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
C1	61.0	0.25	100	2	12.6	3350	3.0	18.6	31.2562	0.5209	0.3126
D1	88.0	0.75	100	2	5.2	2000	3.0	11.1	16.3179	0.2720	0.1632
E5	61.0	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
VP-1	82.6	0.75	50	2	2.9	1200	3.0	6.7	9.5956	0.1599	0.0960
IP-1	82.6	0.75	50	2	2.9	1350	3.0	7.5	10.4290	0.1738	0.1043
IP-2	82.6	0.75	50	2	2.9	1350	3.0	7.5	10.4290	0.1738	0.1043
IP-3	82.6	0.75	50	2	2.9	1900	3.0	10.6	13.4845	0.2247	0.1348
IP-4	82.6	0.75	50	2	2.9	1900	3.0	10.6	13.4845	0.2247	0.1348
EK1A	88.0	0.75	50	2	2.9	570	2.0	4.8	7.6790	0.1280	0.0768
EK1B	88.0	0.75	50	2	2.9	680	2.0	5.7	8.5956	0.1433	0.0860
EK1C	88.0	0.75	50	2	2.9	550	2.0	4.6	7.5123	0.1252	0.0751
EK1D	88.0	0.75	50	2	2.9	1000	2.0	8.3	11.2623	0.1877	0.1126
EK1E	88.0	0.75	50	2	2.9	980	2.0	8.2	11.0956	0.1849	0.1110
EK1F	88.0	0.75	50	2	2.9	940	2.0	7.8	10.7623	0.1794	0.1076
EK1G	88.0	0.75	50	2	2.9	980	2.0	8.2	11.0956	0.1849	0.1110
EK1H	88.0	0.75	50	2	2.9	1050	2.0	8.8	11.6790	0.1946	0.1168
EK1I	88.0	0.75	50	2	2.9	550	2.0	4.6	7.5123	0.1252	0.0751
EKIJ	88.0	0.75	50	2	2.9	400	2.0	3.3	6.2623	0.1044	0.0626
EK1K	88.0	0.75	50	2	2.9	680	2.0	5.7	8.5956	0.1433	0.0860
EK1L	88.0	0.75	50	2	2.9	870	2.0	7.3	10.1790	0.1696	0.1018
EK1M	88.0	0.75	50	2	2.9	500	2.0	4.2	7.0956	0.1183	0.0710
EK1N	88.0	0.75	50	2	2.9	950	2.0	7.9	10.8456	0.1808	0.1085
EK1O	61.0	0.25	50	2	7.1	250	2.0	2.1	9.1965	0.1533	0.0920
EK2A	88.0	0.75	50	2	2.9	1120	2.0	9.3	12.2623	0.2044	0.1226
EK2B	88.0	0.75	50	2	2.9	720	2.0	6.0	8.9290	0.1488	0.0893
EK2C	88.0	0.75	50	2	2.9	1040	2.0	8.7	11.5956	0.1933	0.1160
EK2D	82.7	0.75	50	2	2.9	800	2.0	6.7	9.5956	0.1599	0.0960
EK2E	61.0	0.75	50	2	2.9	400	2.0	3.3	6.2623	0.1044	0.0626

BASIN SUMMARY - FULLY DEVELOPED CONDITIONS

BASIN (label)	TOTAL BASIN AREA (acres)	TOTAL BASIN AREA (sq mi)	WEIGHTED CN	TOTAL LAG TIME (hours)	TOTAL Tc (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 10 Yr. (cfs)	Q 25 Yr. (cfs)	Q 50 Yr. (cfs)	Q 100 Yr. (cfs)
02B	32.2	0.0503	60.0	0.1700	0.2833	0.25	4.42	9.72	20.10	28.39	37.46
O1	19.2	0.0300	67.8	0.0800	0.1333	4.73	12.73	19.06	29.60	37.17	45.16
B1	15.1	0.0236	68.0	0.1260	0.2100	2.95	8.42	12.92	20.51	26.11	32.02
B2	62.7	0.0980	75.0	0.1140	0.1900	33.20	65.46	89.55	128.36	155.96	184.43
B3	40.0	0.0625	80.0	0.1380	0.2300	31.11	54.19	71.14	97.89	116.35	135.15
B5	11.4	0.0178	88.0	0.1285	0.2141	15.95	24.08	29.67	38.17	43.86	49.57
B6	18.4	0.0288	88.0	0.1368	0.2280	25.38	38.36	47.29	60.95	70.13	79.32
B7	17.8	0.0278	88.0	0.1312	0.2187	24.79	37.42	46.14	59.39	68.28	77.19
B9	4.1	0.0064	88.0	0.0798	0.1331	6.22	9.25	11.31	14.42	16.50	18.59
A1	56.9	0.0889	88.0	0.2132	0.3553	67.71	104.23	129.50	168.56	194.95	221.48
A2	108.0	0.1688	88.0	0.2410	0.4016	121.62	188.27	234.62	305.74	354.24	403.06
A3	25.0	0.0391	88.0	0.1298	0.2164	34.89	52.68	64.94	83.56	96.05	108.56
A4	4.0	0.0063	74.5	0.1910	0.3183	1.65	3.38	4.72	6.92	8.47	10.09
A5	6.7	0.0105	88.0	0.1465	0.2442	9.07	13.77	16.99	21.89	25.21	28.55
A6	14.7	0.0230	81.6	0.0854	0.1423	15.15	24.83	31.72	42.41	49.67	57.02
A7	25.1	0.0392	88.0	0.1465	0.2442	32.50	49.61	61.38	79.39	91.60	103.86
A8	24.7	0.0386	88.0	0.1854	0.3090	31.05	47.48	58.91	76.41	88.20	100.04
A9	27.5	0.0430	88.0	0.1632	0.2720	36.11	54.95	67.95	87.92	101.34	114.82
A12	31.1	0.0486	61.0	0.2209	0.3682	0.35	4.38	9.09	18.32	25.61	33.67
C1	62.5	0.0977	61.0	0.3126	0.5209	0.67	6.74	14.07	28.80	40.69	53.92
D1	40.4	0.0631	88.0	0.1632	0.2720	53.05	80.73	99.82	129.16	148.88	168.68
E5	34.3	0.0536	61.0	0.2209	0.3682	0.38	4.53	9.49	19.29	26.99	35.43
VP-1	1.1	0.0017	82.6	0.0960	0.1599	1.19	1.92	2.43	3.24	3.78	4.33
IP-1	1.2	0.0019	82.6	0.1043	0.1738	1.27	2.07	2.63	3.49	4.08	4.68
IP-2	2.5	0.0039	82.6	0.1043	0.1738	2.65	4.30	5.47	7.27	8.49	9.74
IP-3	2.0	0.0031	82.6	0.1348	0.2247	1.91	3.16	4.04	5.40	6.34	7.28
IP-4	3.1	0.0048	82.6	0.1348	0.2247	2.97	4.89	6.26	8.37	9.82	11.28
EK1A	3.6	0.0056	88.0	0.0768	0.1280	5.49	8.15	9.96	12.69	14.52	16.36
EK1B	7.7	0.0120	88.0	0.0860	0.1433	11.57	17.26	21.12	26.97	30.87	34.78
EK1C	4.8	0.0075	88.0	0.0751	0.1252	7.34	10.89	13.30	16.94	19.39	21.84
EK1D	8.3	0.0130	88.0	0.1126	0.1877	11.95	17.95	22.04	28.27	32.47	36.68
EK1E	12.4	0.0194	88.0	0.1110	0.1849	17.90	26.87	32.99	42.33	48.61	54.89
EK1F	5.5	0.0086	88.0	0.1076	0.1794	7.99	11.97	14.69	18.85	21.64	24.43
EK1G	4.6	0.0072	88.0	0.1110	0.1849	6.64	9.97	12.24	15.70	18.03	20.36
EK1H	10.9	0.0170	88.0	0.1168	0.1946	15.57	23.44	28.81	36.95	42.44	47.96
EK1I	5.9	0.0092	88.0	0.0751	0.1252	9.02	13.38	16.35	20.82	23.84	26.85
EKIJ	2.2	0.0034	88.0	0.0626	0.1044	3.41	5.05	6.17	7.85	8.98	10.10
EK1K	4.8	0.0075	88.0	0.0860	0.1433	7.21	10.76	13.17	16.81	19.24	21.68
EK1L	7.0	0.0109	88.0	0.1018	0.1696	10.26	15.35	18.85	24.15	27.70	31.25
EK1M	3.8	0.0059	88.0	0.0710	0.1183	5.84	8.65	10.56	13.46	15.40	17.35
EK1N	11.6	0.0181	88.0	0.1085	0.1808	16.82	25.22	30.94	39.72	45.59	51.48
EK1O	1.5	0.0023	61.0	0.0920	0.1533	0.03	0.37	0.73	1.36	1.84	2.37
EK2A	11.1	0.0173	88.0	0.1226	0.2044	15.18	22.88	28.32	36.61	42.18	47.77
EK2B	5.4	0.0084	88.0	0.0893	0.1488	7.89	11.87	14.58	18.68	21.44	24.19
EK2C	10.2	0.0159	88.0	0.1160	0.1933	14.04	21.32	26.35	34.01	39.15	44.30
EK2D	7.6	0.0119	82.7	0.0960	0.1599	7.81	12.85	16.41	21.93	25.70	29.50
EK2E	2.6	0.0041	61.0	0.0626	0.1044	0.05	0.66	1.32	2.50	3.38	4.32

CONVEYANCE CHART - FULLY DEVELOPED CONDITIONS

Conveyance Label	Type	Length ft.	Bottom Width ft.	Slope %	Side Slope H to 1	Manning's N value	Channel Depth ft.	Q 5 Yr. cfs	Q 100 Yr. cfs	Q Capacity cfs
C1	36" RCP	2650	3.0	0.0200	N/A	0.013	N/A	10.55	42.84	94.32
C2	48" RCP	1375	4.0	0.0225	N/A	0.013	N/A	39.25	98.20	215.45
C3	GRASSLINED SWALE	3475	6.0	0.0230	3.0	0.035	2.000	1.57	24.70	169.57
C4	54" RCP	1670	4.5	0.0186	N/A	0.013	N/A	55.36	168.19	268.18
C5	54" RCP	450	4.5	0.0200	N/A	0.013	N/A	58.56	181.40	278.09
C6	54" RCP	550	4.5	0.0200	N/A	0.013	N/A	75.51	224.99	278.09
C7	60" RCP	300	5.0	0.0217	N/A	0.013	N/A	145.19	374.92	383.63
C9	CONCRETE CHANNEL	1800	6.0	0.0167	1.5	0.150	5.000	102.76	219.81	1720.39
C10	CONCRETE CHANNEL	1100	6.0	0.0050	1.5	0.015	5.000	288.67	620.32	941.35
C11	42" RCP	2200	3.5	0.0177	N/A	0.013	N/A	6.37	52.96	133.85
C12	48" RCP	1050	4.0	0.0062	N/A	0.013	N/A	68.34	171.89	113.10
C13	CONCRETE CHANNEL	1750	6.0	0.0271	1.5	0.015	5.000	393.32	860.97	2191.56
C14	CONCRETE CHANNEL	600	6.0	0.0200	1.5	0.015	5.000	407.25	893.80	1882.71
C15	30" RCP	800	2.5	0.0250	N/A	0.013	N/A	47.48	100.04	64.85
C18	GRASS / RIPRAP SWALE	700	20.0	0.0257	2.0	0.035	3.000	70.53	166.30	934.14
C20	GRASSLINED SWALE	750	10.0	0.0227	10.0	0.035	2.000	0.68	8.81	432.24
C30	24" RCP	850	2.0	0.0050	N/A	0.013	N/A	8.06	16.29	16.00
C31	60" RCP	880	5.0	0.0306	N/A	0.013	N/A	175.09	438.38	455.56
C32	30" RCP	1300	2.5	0.0050	N/A	0.013	N/A	10.70	21.68	29.00
C33	42" RCP	750	3.5	0.0050	N/A	0.013	N/A	28.40	58.12	71.14
C34	78" RCP	650	6.0	0.0125	N/A	0.013	N/A	229.97	549.59	586.13
C35	30" RCP	150	2.5	0.0050	N/A	0.013	N/A	11.96	24.36	29.00
C36	36" RCP	200	3.0	0.0050	N/A	0.013	N/A	21.86	44.68	47.16
C37	84" RCP	430	7.0	0.0125	N/A	0.013	N/A	274.98	640.97	714.20
C38	84" RCP	350	7.0	0.0125	N/A	0.013	N/A	287.11	667.00	714.20
C39	84" RCP	100	7.0	0.0125	N/A	0.013	N/A	291.36	676.51	714.20
C40	30" RCP	800	2.5	0.0050	N/A	0.013	N/A	10.63	21.55	29.00
C41	42" RCP	100	3.5	0.0050	N/A	0.013	N/A	25.93	52.66	71.14
C42	54" RCP	150	4.5	0.0200	N/A	0.013	N/A	102.40	214.67	278.09
C43	48" RCP	500	4.0	0.0050	N/A	0.013	N/A	49.49	103.69	101.57
C44	54" RCP	250	4.5	0.0320	N/A	0.013	N/A	158.51	333.48	351.76
C45	54" RCP	100	4.5	0.0320	N/A	0.013	N/A	183.30	384.00	351.76
C46	30" RCP	1100	2.5	0.0050	N/A	0.013	N/A	8.05	18.56	29.00
C47	42" RCP	50	3.5	0.0050	N/A	0.013	N/A	30.88	66.17	71.14
C48	30" RCP	600	2.5	0.0050	N/A	0.013	N/A	11.87	24.19	29.00
C49	42" RCP	100	3.5	0.0050	N/A	0.013	N/A	33.19	68.49	71.14

DESIGN POINTS - FULLY DEVELOPED CONDITIONS

DESIGN POINT (label)	Q 2 Yr. Q (cfs) Ponding (ft)	Q 5 Yr. Q (cfs) Ponding (ft)	Q 10 Yr. Q (cfs) Ponding (ft)	Q 25 Yr. Q (cfs) Ponding (ft)	Q 50 Yr. Q (cfs) Ponding (ft)	Q 100 Yr. Q (cfs) Ponding (ft)
PCC IN	33.20	65.46	89.55	128.36	155.96	184.43
PCC OUT	14.72 / 6758.26	31.59 / 6759.71	39.19 / 6760.42	49.69 / 6761.47	56.38 / 6762.09	60.98 / 6762.40
1	17.26	39.67	53.49	72.12	85.95	98.98
NLC IN	31.11	54.19	71.14	97.89	116.35	135.15
NLC OUT	5.01 / 6737.07	11.97 / 6737.79	17.07 / 6738.38	23.49 / 6739.25	26.23 / 6739.71	28.62 / 6740.21
2	22.93	56.08	79.77	116.78	143.70	169.78
3	23.78	58.66	84.60	124.29	153.19	181.69
4	33.16	75.51	108.17	155.81	190.29	224.99
5	80.46	145.19	196.21	270.66	322.33	374.92
25	101.47	175.09	232.90	319.34	378.03	438.38
26	19.09	28.65	35.15	45.04	51.70	58.36
27	138.07	229.97	300.21	405.43	476.68	549.59
28	14.60	21.93	26.92	34.50	39.59	44.71
29	168.12	274.98	355.43	476.09	557.67	640.97
30	176.36	287.11	370.19	495.38	580.22	667.00
31	179.25	291.36	375.35	502.80	588.68	676.51
32	17.34	25.96	31.83	40.80	46.80	52.81
8	188.05	291.03	362.79	472.64	547.45	622.87
EK1B IN	53.05	80.73	99.82	129.16	148.88	168.68
EK1B OUT	42.25 / 6692.84	65.96 / 6693.83	79.25 / 6694.46	100.04 / 6695.50	112.94 / 6696.20	123.14 / 6696.88
10	42.29	68.34	87.64	122.15	148.01	171.89
9	252.33	394.97	493.82	649.62	756.73	862.68
11	259.21	407.63	510.14	672.37	784.76	895.08
12	66.98	102.40	126.80	164.01	189.27	214.67
33	103.76	158.51	196.67	254.92	294.13	333.48
34	119.82	183.30	227.03	293.70	338.55	384.00
7	554.86	891.81	1132.27	1493.71	1744.62	1999.54
EK1A IN	554.86	891.81	1132.27	1493.71	1744.62	1999.54
EK1A OUT	36.38 / 6616.11	70.54 / 6617.71	105.68 / 6618.78	129.83 / 6620.67	144.48 / 6621.85	166.31 / 6623.00
13	36.38	70.54	105.68	129.83	144.48	166.31
14	36.62	71.40	107.20	135.57	147.26	169.67
I25-1 IN	36.62	71.40	107.20	135.57	147.26	169.67
I25-1 OUT	36.61 / 6591.29	71.12 / 6592.34	106.58 / 6593.27	126.73 / 6593.78	145.76 / 6594.25	168.05 / 6594.79
20	36.61	71.12	106.58	126.73	145.76	168.05
15	4.88	8.05	10.29	13.78	16.16	18.56
35	20.07	30.88	38.34	50.12	58.12	66.17
36	21.90	33.19	40.93	52.69	60.58	68.49
37	49.23	76.62	95.62	124.74	144.39	164.15
EK2A-IN	49.23	76.62	95.62	124.74	144.39	164.15
EK-2A-OUT	0.57 / 6622.58	0.68 / 6623.58	0.74 / 6624.20	1.74 / 6624.79	4.43 / 6624.90	8.81 / 6625.04
38	0.57	0.68	0.74	1.74	4.43	8.81
17	0.93	5.27	10.47	20.60	28.53	37.35
I25-2 IN	0.93	5.27	10.47	20.60	28.53	37.35
I25-2 OUT	0.91 / 6599.41	4.47 / 6600.01	5.70 / 6600.15	8.91 / 6600.51	11.79 / 6600.80	16.11 / 6601.22
39	0.91	4.47	5.70	8.91	11.79	16.11

FULLY DEVELOPED CONDITIONS DETENTION POND SUMMARY

POND (label)	RISER DIMENSIONS (feet)	RISER CREST ELEV. (feet)	OUTLET PIPE DIAMETER (inches)	OUTLET PIPE INVERT (feet)	OUTLET PIPE SLOPE (%)	INVERT OF POND (feet)	INVERT OF EMERG. SPILLWAY (feet)	LENGTH OF EMERG. SPILLWAY (feet)	EMERGENCY SPILLWAY FLOW DEPTH (feet)	FREE-BOARD DEPTH (feet)	CREST OF DAM (feet)	100 YR. PONDING VOLUME (acre-feet)
EK-1A	4.0' x 3.0'	6614.00	54" RCP	6609.0	0.8	6609.00	6623.50	275	1.8	2.0	6627.30	86.165
EK-2A	4.0' x 4.0'	6624.75	18" RCP	6619.0	1.0	6619.00	6625.00	55	1.0	2.0	6628.00	6.241

FULLY DEVELOPED CONDITIONS - WATER QUALITY SUMMARY

POND (label)	WQ BASIN AREA (acres)	WQ BASIN % Impervious	WQ VOLUME REQUIRED (acre-feet)	WQ PONDING ELEV. (feet)	WQ PONDING HEIGHT (feet)	# WQ ORIFICE COLUMNS (feet)	# WQ ORIFICE ROWS (feet)	WQ ORIFICE DIAMETER (inches)
EK1A	305.2	72.00	8.679	6614.0	5.0	3	15	1.500
EK2A	30.2	72.00	0.859	6621.3	2.3	2	7	1.000

Design Procedure Form: Extended Detention Basin (EDB) - Sedimentation Facility

Designer: Matt Larson
Company: Classic Consulting
Date: July 20, 2007
Project: INTERQUEST SOUTH
Location: EK-1

1. Basin Storage Volume		
A) Tributary Area's Imperviousness Ratio (i = I_a / 100)	I_a = 72.00 % i = 0.72	
B) Contributing Watershed Area (Area)	Area = 305.20	acres
C) Water Quality Capture Volume (WQCV) (WQCV =1.0 * (0.91 * I^3 - 1.19 * I^2 + 0.78 * I))	WQCV = 0.28	watershed inches
D) Design Volume: Vol = (WQCV / 12) * Area * 1.2	Vol = 8.679	acre-feet
2. Outlet Works		
A) Outlet Type (Check One)	x Orifice Plate Perforated Riser Pipe Other:	
B) Depth at Outlet Above Lowest Perforation (H)	H = 5.00	feet
C) Required Maximum Outlet Area per Row, (A_o)	A_o = 5.46	square inches
D) Perforation Dimensions **(enter one only)**: i) Circular Perforation Diameter **OR** ii) 2" Height Rectangular Perforation Width	D = 1.5000 W =	inches, **OR** inches
E) Number of Columns (nc, See Table 6a-1 For Maximum)	nc = 3	number
F) Actual Design Outlet Area per Row (A_o)	A_o = 5.30	square inches
G) Number of Rows (nr)	nr = 15	number
H) Total Outlet Area (A_{ot})	A_{ot} = 79.52	square inches
3. Trash Rack		
A) Needed Open Area: A_t = 0.5 * (Figure 7 Value) * A_{ot}	A_t = 2,542	square inches
B) Type of Outlet Opening (Check One)	X ≤ 2" Diameter **Round** 2" High **Rectangular** Other:	
C) For 2", or Smaller, **Round Opening** (Ref.: Figure 6a):		
i) Width of Trash Rack and Concrete Opening (W_{conc}) from Table 6a-1	W_{conc} = 54	inches
ii) Height of Trash Rack Screen (H_{TR})	H_{TR} = 84	inches

JOB NAME:	Interquest South
JOB NUMBER:	2190.00
DATE:	07/23/07
CALCULATED BY:	JHB

EK-1A Detention Pond - BUILT-OUT Volumes - REDESIGNED

POND SIZING WITH PONDPACK EQUATION:

INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6609.00	500	0.01	acres
6610.00	11,118	0.26	acres
6612.00	89,478	2.05	acres
6614.00	234,372	5.38	acres
6616.00	332,561	7.63	acres
6618.00	375,315	8.62	acres
6620.00	403,528	9.26	acres
6622.00	430,604	9.89	acres
6624.00	471,957	10.83	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

							CUMMULATIVE VOLUME:
0.11	AC-FT	from	6,609	to	6,610		0.11
2.00	AC-FT	from	6,610	to	6,612		2.11
7.10	AC-FT	from	6,612	to	6,614		9.21
12.82	AC-FT	from	6,614	to	6,616		22.03
16.08	AC-FT	from	6,616	to	6,618		38.11
17.70	AC-FT	from	6,618	to	6,620		55.80
18.95	AC-FT	from	6,620	to	6,622		74.76
20.51	AC-FT	from	6,622	to	6,624		95.26
-	AC-FT	from	6,624	to	-		95.26
-	AC-FT	from	-	to	-		95.26
-	AC-FT	from	-	to	-		95.26

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

JOB NAME: Interquest South
JOB NUMBER: 2190.00
DATE: 07/23/07
CALCULATED BY: JHB

EK-1A Detention Pond - Required Storm Water Quality Volume

POND SIZING WITH PONDPACK EQUATION:
INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6606.50	-	-	acres
6606.50	4,705	0.11	acres
6607.00	5,575	0.13	acres
6608.00	7,494	0.17	acres
6609.00	9,636	0.22	acres
6610.00	11,118	0.26	acres
6612.00	89,478	2.05	acres
6613.00	159,744	3.67	acres
6613.80	210,037	4.82	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

						CUMMULATIVE VOLUME:
-	AC-FT	from	6,607	to	6,607	0.00
0.06	AC-FT	from	6,607	to	6,607	0.06
0.15	AC-FT	from	6,607	to	6,608	0.21
0.19	AC-FT	from	6,608	to	6,609	0.40
0.24	AC-FT	from	6,609	to	6,610	0.64
2.00	AC-FT	from	6,610	to	6,612	2.64
2.79	AC-FT	from	6,612	to	6,613	5.43
3.35	AC-FT	from	6,613	to	6,614	8.78
-	AC-FT	from	6,614	to	-	8.78
-	AC-FT	from	-	to	-	8.78
-	AC-FT	from	-	to	-	8.78

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

JOB NAME: Interquest South
JOB NUMBER: 2190.00
DATE: 07/25/07
CALCULATED BY: JHB

EK-1A Detention Pond - Provided Storm Water Quality Volume

POND SIZING WITH PONDPACK EQUATION:
INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6606.50	-	acres	
6606.50	4,705	0.11	acres
6607.00	5,575	0.13	acres
6608.00	7,494	0.17	acres
6609.00	9,636	0.22	acres
6610.00	11,118	0.26	acres
6612.00	89,478	2.05	acres
6614.00	234,372	5.38	acres
		-	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

							CUMMULATIVE VOLUME:
-	AC-FT	from	6,607	to	6,607		0.00
0.06	AC-FT	from	6,607	to	6,607		0.06
0.15	AC-FT	from	6,607	to	6,608		0.21
0.19	AC-FT	from	6,608	to	6,609		0.40
0.24	AC-FT	from	6,609	to	6,610		0.64
2.00	AC-FT	from	6,610	to	6,612		2.64
7.10	AC-FT	from	6,612	to	6,614		9.74
-	AC-FT	from	6,614	to	-		9.74
-	AC-FT	from	-	to	-		9.74
-	AC-FT	from	-	to	-		9.74
-	AC-FT	from	-	to	-		9.74

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

JOB NAME:	Interquest South
JOB NUMBER:	2190.00
DATE:	07/23/07
CALCULATED BY:	JHB

EK-1A Detention Pond - Micropool

POND SIZING WITH PONDPACK EQUATION:

INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	**AREA (BTM to TOP):**		
6606.50	-	-	acres
6606.50	4,705	0.11	acres
6607.00	5,575	0.13	acres
6608.00	7,494	0.17	acres
6609.00	9,636	0.22	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

							CUMMULATIVE VOLUME:
-	AC-FT	from	6,607	to	6,607		0.00
0.06	AC-FT	from	6,607	to	6,607		0.06
0.15	AC-FT	from	6,607	to	6,608		0.21
0.19	AC-FT	from	6,608	to	6,609		0.40
-	AC-FT	from	6,609	to	-		0.40
-	AC-FT	from	-	to	-		0.40
-	AC-FT	from	-	to	-		0.40
-	AC-FT	from	-	to	-		0.40
-	AC-FT	from	-	to	-		0.40
-	AC-FT	from	-	to	-		0.40
-	AC-FT	from	-	to	-		0.40

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

JOB NAME:	Interquest South
JOB NUMBER:	2190.00
DATE:	07/23/07
CALCULATED BY:	JHB

EK-1A Detention Pond - Additional Pond Volume

POND SIZING WITH PONDPACK EQUATION:

INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)
6623.00
6623.00
6624.00

AREA (BTM to TOP):		
-	-	acres
451,281	10.36	acres
471,957	10.83	acres
	-	acres
	-	acres
	-	acres
	-	acres
	-	acres
	-	acres
	-	acres
	-	acres
	-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

						CUMMULATIVE VOLUME:
-	AC-FT	from	6,623	to	6,623	0.00
10.49	AC-FT	from	6,623	to	6,624	10.49
-	AC-FT	from	6,624	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49
-	AC-FT	from	-	to	-	10.49

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

Design Procedure Form: Extended Detention Basin (EDB) - Sedimentation Facility

Designer: JHB
Company: Classic Consulting
Date: April 5, 2007
Project: Interquest South
Location: Pond EK-2A Fully Developed Conditions

1. Basin Storage Volume			
A) Tributary Area's Imperviousness Ratio (i = I_a / 100)	I_a =	72.00	%
	i =	0.72	
B) Contributing Watershed Area (Area)	Area =	30.20	acres
C) Water Quality Capture Volume (WQCV) (WQCV =1.0 * (0.91 * I^3 - 1.19 * I^2 + 0.78 * I))	WQCV =	0.28	watershed inches
D) Design Volume: Vol = (WQCV / 12) * Area * 1.2	Vol =	0.859	acre-feet
2. Outlet Works			
A) Outlet Type (Check One)		X	Orifice Plate
			Perforated Riser Pipe
			Other:
B) Depth at Outlet Above Lowest Perforation (H)	H =	2.30	feet
C) Required Maximum Outlet Area per Row, (A_o)	A_o =	1.51	square inches
D) Perforation Dimensions **(enter one only)**:			
i) Circular Perforation Diameter **OR**	D =	1.0000	inches, **OR**
ii) 2" Height Rectangular Perforation Width	W =		inches
E) Number of Columns (nc, See Table 6a-1 For Maximum)	nc =	2	number
F) Actual Design Outlet Area per Row (A_o)	A_o =	1.57	square inches
G) Number of Rows (nr)	nr =	7	number
H) Total Outlet Area (A_{ol})	A_{ol} =	10.84	square inches

JOB NAME: Interquest South
JOB NUMBER: 2190.00
DATE: 04/05/07
CALCULATED BY: JHB

EK-2A Detention Pond - BUILT-OUT Volumes

POND SIZING WITH PONDPACK EQUATION:
INSERT POND DESIGN SIZE INFO: (RED)

POND ELEVATION : (from lowest to highest)	AREA (BTM to TOP):		
6619.00	-	-	acres
6620.00	11,892	0.27	acres
6622.00	45,054	1.03	acres
6624.00	81,310	1.87	acres
6626.00	97,090	2.23	acres
6628.00	104,625	2.40	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres
		-	acres

PRELIMINARY SIZE:

VOLUME = 1/3{(EL2-EL1)*(A1+A2+((A1*A2)^.5))}

							CUMMULATIVE VOLUME:
0.09	AC-FT	from	6,619	to	6,620		0.09
1.21	AC-FT	from	6,620	to	6,622		1.30
2.83	AC-FT	from	6,622	to	6,624		4.14
4.05	AC-FT	from	6,624	to	6,626		8.18
4.58	AC-FT	from	6,626	to	6,628		12.77
-	AC-FT	from	6,628	to	-		12.77
-	AC-FT	from	-	to	-		12.77
-	AC-FT	from	-	to	-		12.77
-	AC-FT	from	-	to	-		12.77
-	AC-FT	from	-	to	-		12.77
-	AC-FT	from	-	to	-		12.77

*SIZING IS FOR PRELIMINARY PURPOSES ONLY.

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

POND PACK OUTPUT

FULLY DEVELOPED CONDITIONS

MASTER DESIGN STORM SUMMARY

Network Storm Collection: C Springs

Return Event	Total Depth in	Rainfall Type	RNF ID
2	2.0000	Synthetic Curve	TYPEIIA 24HR:
5	2.6000	Synthetic Curve	TYPEIIA 24HR:
10	3.0000	Synthetic Curve	TYPEIIA 24HR:
25	3.6000	Synthetic Curve	TYPEIIA 24HR:
50	4.0000	Synthetic Curve	TYPEIIA 24HR:
100	4.4000	Synthetic Curve	TYPEIIA 24HR:

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A1	AREA	2	4.577		6.0955	67.71		
A1	AREA	5	6.958		6.0955	104.23		
A1	AREA	10	8.621		6.0955	129.50		
A1	AREA	25	11.191		6.0788	168.56		
A1	AREA	50	12.940		6.0788	194.95		
A1	AREA	100	14.710		6.0788	221.48		
A12	AREA	2	.190		6.5965	.35		
A12	AREA	5	.586		6.1790	4.38		
A12	AREA	10	.946		6.1623	9.09		
A12	AREA	25	1.602		6.1456	18.32		
A12	AREA	50	2.106		6.1456	25.61		
A12	AREA	100	2.654		6.1289	33.67		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A2	AREA	2	8.687		6.1289	121.62		
A2	AREA	5	13.207		6.1122	188.27		
A2	AREA	10	16.364		6.1122	234.62		
A2	AREA	25	21.241		6.0955	305.74		
A2	AREA	50	24.560		6.0955	354.24		
A2	AREA	100	27.921		6.0955	403.06		
A3	AREA	2	2.011		6.0454	34.89		
A3	AREA	5	3.057		6.0287	52.68		
A3	AREA	10	3.788		6.0287	64.94		
A3	AREA	25	4.917		6.0287	83.56		
A3	AREA	50	5.685		6.0287	96.05		
A3	AREA	100	6.463		6.0287	108.56		
A4	AREA	2	.127		6.1122	1.65		
A4	AREA	5	.237		6.0955	3.38		
A4	AREA	10	.320		6.0955	4.72		
A4	AREA	25	.458		6.0955	6.92		
A4	AREA	50	.556		6.0955	8.47		
A4	AREA	100	.657		6.0788	10.09		
A5	AREA	2	.539		6.0454	9.07		
A5	AREA	5	.819		6.0454	13.77		
A5	AREA	10	1.015		6.0454	16.99		
A5	AREA	25	1.318		6.0454	21.89		
A5	AREA	50	1.524		6.0287	25.21		
A5	AREA	100	1.732		6.0287	28.55		
A6	AREA	2	.795		6.0287	15.15		
A6	AREA	5	1.313		6.0120	24.83		
A6	AREA	10	1.689		6.0120	31.72		
A6	AREA	25	2.285		6.0120	42.41		
A6	AREA	50	2.699		6.0120	49.67		
A6	AREA	100	3.122		6.0120	57.02		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A7	AREA	2	2.019		6.0621	32.50		
A7	AREA	5	3.069		6.0621	49.61		
A7	AREA	10	3.803		6.0621	61.38		
A7	AREA	25	4.936		6.0454	79.39		
A7	AREA	50	5.708		6.0454	91.60		
A7	AREA	100	6.489		6.0454	103.86		
A8	AREA	2	1.987		6.0788	31.05		
A8	AREA	5	3.021		6.0788	47.48		
A8	AREA	10	3.742		6.0621	58.91		
A8	AREA	25	4.858		6.0621	76.41		
A8	AREA	50	5.617		6.0621	88.20		
A8	AREA	100	6.386		6.0621	100.04		
A9	AREA	2	2.212		6.0621	36.11		
A9	AREA	5	3.363		6.0621	54.95		
A9	AREA	10	4.167		6.0454	67.95		
A9	AREA	25	5.408		6.0454	87.92		
A9	AREA	50	6.254		6.0454	101.34		
A9	AREA	100	7.109		6.0454	114.82		
B1	AREA	2	.245		6.0788	2.95		
B1	AREA	5	.544		6.0621	8.42		
B1	AREA	10	.788		6.0621	12.92		
B1	AREA	25	1.208		6.0621	20.51		
B1	AREA	50	1.516		6.0454	26.11		
B1	AREA	100	1.844		6.0454	32.02		
B2	AREA	2	1.990		6.0454	33.20		
B2	AREA	5	3.708		6.0454	65.46		
B2	AREA	10	5.020		6.0454	89.55		
B2	AREA	25	7.174		6.0454	128.36		
B2	AREA	50	8.708		6.0287	155.96		
B2	AREA	100	10.305		6.0287	184.43		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
B3	AREA	2	1.875		6.0621	31.11		
B3	AREA	5	3.196		6.0621	54.19		
B3	AREA	10	4.167		6.0454	71.14		
B3	AREA	25	5.720		6.0454	97.89		
B3	AREA	50	6.806		6.0454	116.35		
B3	AREA	100	7.922		6.0454	135.15		
B5	AREA	2	.917		6.0454	15.95		
B5	AREA	5	1.394		6.0287	24.08		
B5	AREA	10	1.727		6.0287	29.67		
B5	AREA	25	2.242		6.0287	38.17		
B5	AREA	50	2.592		6.0287	43.86		
B5	AREA	100	2.947		6.0287	49.57		
B6	AREA	2	1.480		6.0454	25.38		
B6	AREA	5	2.250		6.0454	38.36		
B6	AREA	10	2.788		6.0287	47.29		
B6	AREA	25	3.619		6.0287	60.95		
B6	AREA	50	4.184		6.0287	70.13		
B6	AREA	100	4.757		6.0287	79.32		
B7	AREA	2	1.432		6.0454	24.79		
B7	AREA	5	2.177		6.0287	37.42		
B7	AREA	10	2.697		6.0287	46.14		
B7	AREA	25	3.501		6.0287	59.39		
B7	AREA	50	4.048		6.0287	68.28		
B7	AREA	100	4.602		6.0287	77.19		
B9	AREA	2	.330		6.0120	6.22		
B9	AREA	5	.501		6.0120	9.25		
B9	AREA	10	.621		6.0120	11.31		
B9	AREA	25	.806		6.0120	14.42		
B9	AREA	50	.932		6.0120	16.50		
B9	AREA	100	1.060		5.9953	18.59		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
C1	AREA	2	.381		6.6800	.67		
C1	AREA	5	1.179		6.2792	6.74		
C1	AREA	10	1.902		6.2625	14.07		
C1	AREA	25	3.220		6.2458	28.80		
C1	AREA	50	4.232		6.2291	40.69		
C1	AREA	100	5.333		6.2291	53.92		
D1	AREA	2	3.250		6.0621	53.05		
D1	AREA	5	4.940		6.0621	80.73		
D1	AREA	10	6.121		6.0454	99.82		
D1	AREA	25	7.946		6.0454	129.16		
D1	AREA	50	9.187		6.0454	148.88		
D1	AREA	100	10.444		6.0454	168.68		
DP 1	JCT	2	2.301		6.1456	17.26		
DP 1	JCT	5	4.400		6.1122	39.67		
DP 1	JCT	10	6.023		6.0788	53.49		
DP 1	JCT	25	8.710		6.0788	72.12		
DP 1	JCT	50	10.636		6.0788	85.95		
DP 1	JCT	100	12.650		6.0621	98.98		
DP 10	JCT	2	3.620		6.1456	42.29		
DP 10	JCT	5	6.109		6.1623	68.34		
DP 10	JCT	10	8.013		6.1790	87.64		
DP 10	JCT	25	11.156		6.1957	122.15		
DP 10	JCT	50	13.409		6.2124	148.01		
DP 10	JCT	100	15.767		6.2124	171.89		
DP 11	JCT	2	19.561		6.1456	259.21		
DP 11	JCT	5	30.387		6.1289	407.63		
DP 11	JCT	10	38.121		6.1289	510.14		
DP 11	JCT	25	50.279		6.1122	672.37		
DP 11	JCT	50	58.673		6.1122	784.76		
DP 11	JCT	100	67.250		6.1122	895.08		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 14	JCT	2	27.541		7.7321	36.62		
DP 14	JCT	5	52.868		7.1977	71.40		
DP 14	JCT	10	71.162		7.0808	107.20		
DP 14	JCT	25	100.139		6.3961	135.57		
DP 14	JCT	50	120.259		7.1643	147.26		
DP 14	JCT	100	140.891		7.1476	169.67		
DP 2	JCT	2	4.583		6.1790	22.93		
DP 2	JCT	5	8.681		6.1289	56.08		
DP 2	JCT	10	11.871		6.1122	79.77		
DP 2	JCT	25	17.180		6.0955	116.78		
DP 2	JCT	50	21.001		6.0955	143.70		
DP 2	JCT	100	25.007		6.0788	169.78		
*DP 20	JCT	2	27.541		7.7989	36.61		
*DP 20	JCT	5	52.868		7.4315	71.12		
*DP 20	JCT	10	71.162		7.2645	106.58		
*DP 20	JCT	25	100.139		9.7194	126.73		
*DP 20	JCT	50	120.259		7.5317	145.76		
*DP 20	JCT	100	140.891		7.5818	168.05		
DP 25	JCT	2	9.928		6.0454	101.47		
DP 25	JCT	5	16.838		6.0454	175.09		
DP 25	JCT	10	21.995		6.0454	232.90		
DP 25	JCT	25	30.347		6.0454	319.34		
DP 25	JCT	50	36.242		6.0454	378.03		
DP 25	JCT	100	42.349		6.0454	438.38		
DP 26	JCT	2	1.054		6.0287	19.09		
DP 26	JCT	5	1.602		6.0287	28.65		
DP 26	JCT	10	1.985		6.0287	35.15		
DP 26	JCT	25	2.576		6.0120	45.04		
DP 26	JCT	50	2.979		6.0120	51.70		
DP 26	JCT	100	3.387		6.0120	58.36		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 27	JCT	2	11.979		6.0454	138.07		
DP 27	JCT	5	19.956		6.0454	229.97		
DP 27	JCT	10	25.858		6.0454	300.21		
DP 27	JCT	25	35.362		6.0454	405.43		
DP 27	JCT	50	42.041		6.0454	476.68		
DP 27	JCT	100	48.941		6.0454	549.59		
DP 28	JCT	2	.812		6.0287	14.60		
DP 28	JCT	5	1.235		6.0287	21.93		
DP 28	JCT	10	1.530		6.0287	26.92		
DP 28	JCT	25	1.986		6.0287	34.50		
DP 28	JCT	50	2.297		6.0120	39.59		
DP 28	JCT	100	2.611		6.0120	44.71		
DP 29	JCT	2	13.668		6.0454	168.12		
DP 29	JCT	5	22.524		6.0454	274.98		
DP 29	JCT	10	29.040		6.0454	355.43		
DP 29	JCT	25	39.492		6.0454	476.09		
DP 29	JCT	50	46.817		6.0454	557.67		
DP 29	JCT	100	54.370		6.0287	640.97		
DP 3	JCT	2	4.860		6.1957	23.78		
DP 3	JCT	5	9.134		6.1456	58.66		
DP 3	JCT	10	12.449		6.1122	84.60		
DP 3	JCT	25	17.957		6.1122	124.29		
DP 3	JCT	50	21.916		6.0955	153.19		
DP 3	JCT	100	26.062		6.0955	181.69		
DP 30	JCT	2	14.143		6.0454	176.36		
DP 30	JCT	5	23.245		6.0454	287.11		
DP 30	JCT	10	29.934		6.0454	370.19		
DP 30	JCT	25	40.653		6.0287	495.38		
DP 30	JCT	50	48.159		6.0287	580.22		
DP 30	JCT	100	55.895		6.0287	667.00		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 31	JCT	2	14.320		6.0454	179.25		
DP 31	JCT	5	23.514		6.0454	291.36		
DP 31	JCT	10	30.268		6.0454	375.35		
DP 31	JCT	25	41.085		6.0287	502.80		
DP 31	JCT	50	48.659		6.0287	588.68		
DP 31	JCT	100	56.464		6.0287	676.51		
DP 32	JCT	2	.949		6.0287	17.34		
DP 32	JCT	5	1.443		6.0287	25.96		
DP 32	JCT	10	1.788		6.0120	31.83		
DP 32	JCT	25	2.321		6.0120	40.80		
DP 32	JCT	50	2.683		6.0120	46.80		
DP 32	JCT	100	3.051		6.0120	52.81		
DP 33	JCT	2	6.523		6.0621	103.76		
DP 33	JCT	5	9.918		6.0454	158.51		
DP 33	JCT	10	12.288		6.0454	196.67		
DP 33	JCT	25	15.950		6.0454	254.92		
DP 33	JCT	50	18.443		6.0454	294.13		
DP 33	JCT	100	20.966		6.0454	333.48		
DP 34	JCT	2	7.456		6.0454	119.82		
DP 34	JCT	5	11.336		6.0454	183.30		
DP 34	JCT	10	14.046		6.0454	227.03		
DP 34	JCT	25	18.231		6.0454	293.70		
DP 34	JCT	50	21.081		6.0287	338.55		
DP 34	JCT	100	23.965		6.0287	384.00		
DP 4	JCT	2	5.777		6.1122	33.16		
DP 4	JCT	5	10.528		6.1122	75.51		
DP 4	JCT	10	14.177		6.0955	108.17		
DP 4	JCT	25	20.199		6.0955	155.81		
DP 4	JCT	50	24.508		6.0788	190.29		
DP 4	JCT	100	29.009		6.0788	224.99		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 5	JCT	2	8.689		6.0621	80.46		
DP 5	JCT	5	14.954		6.0788	145.19		
DP 5	JCT	10	19.662		6.0621	196.21		
DP 5	JCT	25	27.319		6.0621	270.66		
DP 5	JCT	50	32.740		6.0621	322.33		
DP 5	JCT	100	38.367		6.0621	374.92		
DP 7	JCT	2	43.081		6.0621	554.86		
DP 7	JCT	5	67.993		6.0621	891.81		
DP 7	JCT	10	85.911		6.0621	1132.27		
DP 7	JCT	25	114.201		6.0454	1493.71		
DP 7	JCT	50	133.794		6.0454	1744.62		
DP 7	JCT	100	153.852		6.0454	1999.54		
DP 8	JCT	2	13.264		6.1289	188.05		
DP 8	JCT	5	20.165		6.1122	291.03		
DP 8	JCT	10	24.985		6.1122	362.79		
DP 8	JCT	25	32.431		6.1122	472.64		
DP 8	JCT	50	37.500		6.0955	547.45		
DP 8	JCT	100	42.631		6.0955	622.87		
DP 9	JCT	2	18.895		6.1289	252.33		
DP 9	JCT	5	29.331		6.1122	394.97		
DP 9	JCT	10	36.785		6.1122	493.82		
DP 9	JCT	25	48.503		6.1122	649.62		
DP 9	JCT	50	56.594		6.1122	756.73		
DP 9	JCT	100	64.861		6.0955	862.68		
DP12	JCT	2	4.199		6.0621	66.98		
DP12	JCT	5	6.383		6.0621	102.40		
DP12	JCT	10	7.909		6.0621	126.80		
DP12	JCT	25	10.266		6.0621	164.01		
DP12	JCT	50	11.871		6.0454	189.27		
DP12	JCT	100	13.495		6.0454	214.67		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP13	JCT	2	27.343	R	7.6820	36.38		
DP13	JCT	5	52.254	R	7.2311	70.54		
DP13	JCT	10	70.170	R	7.0975	105.68		
DP13	JCT	25	98.460	R	9.4689	129.83		
DP13	JCT	50	118.052	R	7.2311	144.48		
DP13	JCT	100	138.109	R	7.2645	166.31		
EK1A	AREA	2	.290		6.0120	5.49		
EK1A	AREA	5	.440		6.0120	8.15		
EK1A	AREA	10	.545		6.0120	9.96		
EK1A	AREA	25	.708		6.0120	12.69		
EK1A	AREA	50	.819		5.9953	14.52		
EK1A	AREA	100	.931		5.9953	16.36		
EK1A IN	POND	2	43.081		6.0621	554.86		
EK1A IN	POND	5	67.993		6.0621	891.81		
EK1A IN	POND	10	85.911		6.0621	1132.27		
EK1A IN	POND	25	114.201		6.0454	1493.71		
EK1A IN	POND	50	133.794		6.0454	1744.62		
EK1A IN	POND	100	153.852		6.0454	1999.54		
EK1A OUT	POND	2	27.343	R	7.6820	36.38	6616.11	23.645
EK1A OUT	POND	5	52.254	R	7.2311	70.54	6617.71	36.538
EK1A OUT	POND	10	70.170	R	7.0975	105.68	6618.78	45.813
EK1A OUT	POND	25	98.460	R	9.4689	129.83	6620.67	63.138
EK1A OUT	POND	50	118.052	R	7.2311	144.48	6621.85	74.541
EK1A OUT	POND	100	138.109	R	7.2645	166.31	6623.00	86.165
EK1B	AREA	2	.619		6.0120	11.57		
EK1B	AREA	5	.942		6.0120	17.26		
EK1B	AREA	10	1.167		6.0120	21.12		
EK1B	AREA	25	1.514		6.0120	26.97		
EK1B	AREA	50	1.751		6.0120	30.87		
EK1B	AREA	100	1.991		6.0120	34.78		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK1B	IN POND	2	3.250		6.0621	53.05		
EK1B	IN POND	5	4.940		6.0621	80.73		
EK1B	IN POND	10	6.121		6.0454	99.82		
EK1B	IN POND	25	7.946		6.0454	129.16		
EK1B	IN POND	50	9.187		6.0454	148.88		
EK1B	IN POND	100	10.444		6.0454	168.68		
EK1B	OUT POND	2	3.240		6.1456	42.25	6692.84	.553
EK1B	OUT POND	5	4.930		6.1456	65.96	6693.83	.850
EK1B	OUT POND	10	6.111		6.1456	79.25	6694.46	1.080
EK1B	OUT POND	25	7.935		6.1456	100.04	6695.50	1.486
EK1B	OUT POND	50	9.177		6.1456	112.94	6696.20	1.778
EK1B	OUT POND	100	10.434		6.1623	123.14	6696.88	2.118
EK1C	AREA	2	.386		6.0120	7.34		
EK1C	AREA	5	.587		6.0120	10.89		
EK1C	AREA	10	.727		6.0120	13.30		
EK1C	AREA	25	.944		5.9953	16.94		
EK1C	AREA	50	1.092		5.9953	19.39		
EK1C	AREA	100	1.241		5.9953	21.84		
EK1D	AREA	2	.668		6.0287	11.95		
EK1D	AREA	5	1.015		6.0287	17.95		
EK1D	AREA	10	1.258		6.0287	22.04		
EK1D	AREA	25	1.632		6.0120	28.27		
EK1D	AREA	50	1.887		6.0120	32.47		
EK1D	AREA	100	2.146		6.0120	36.68		
EK1E	AREA	2	.997		6.0287	17.90		
EK1E	AREA	5	1.516		6.0287	26.87		
EK1E	AREA	10	1.879		6.0287	32.99		
EK1E	AREA	25	2.439		6.0120	42.33		
EK1E	AREA	50	2.820		6.0120	48.61		
EK1E	AREA	100	3.206		6.0120	54.89		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK1F	AREA	2	.442		6.0287	7.99		
EK1F	AREA	5	.673		6.0287	11.97		
EK1F	AREA	10	.833		6.0120	14.69		
EK1F	AREA	25	1.082		6.0120	18.85		
EK1F	AREA	50	1.251		6.0120	21.64		
EK1F	AREA	100	1.422		6.0120	24.43		
EK1G	AREA	2	.370		6.0287	6.64		
EK1G	AREA	5	.563		6.0287	9.97		
EK1G	AREA	10	.697		6.0287	12.24		
EK1G	AREA	25	.905		6.0120	15.70		
EK1G	AREA	50	1.046		6.0120	18.03		
EK1G	AREA	100	1.189		6.0120	20.36		
EK1H	AREA	2	.877		6.0287	15.57		
EK1H	AREA	5	1.333		6.0287	23.44		
EK1H	AREA	10	1.652		6.0287	28.81		
EK1H	AREA	25	2.144		6.0287	36.95		
EK1H	AREA	50	2.479		6.0120	42.44		
EK1H	AREA	100	2.818		6.0120	47.96		
EK1I	AREA	2	.475		6.0120	9.02		
EK1I	AREA	5	.721		6.0120	13.38		
EK1I	AREA	10	.894		6.0120	16.35		
EK1I	AREA	25	1.160		5.9953	20.82		
EK1I	AREA	50	1.342		5.9953	23.84		
EK1I	AREA	100	1.525		5.9953	26.85		
EK1J	AREA	2	.177		6.0120	3.41		
EK1J	AREA	5	.269		5.9953	5.05		
EK1J	AREA	10	.333		5.9953	6.17		
EK1J	AREA	25	.433		5.9953	7.85		
EK1J	AREA	50	.500		5.9953	8.98		
EK1J	AREA	100	.569		5.9953	10.10		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK1K	AREA	2	.386		6.0120	7.21		
EK1K	AREA	5	.587		6.0120	10.76		
EK1K	AREA	10	.727		6.0120	13.17		
EK1K	AREA	25	.944		6.0120	16.81		
EK1K	AREA	50	1.092		6.0120	19.24		
EK1K	AREA	100	1.241		6.0120	21.68		
EK1L	AREA	2	.563		6.0287	10.26		
EK1L	AREA	5	.856		6.0120	15.35		
EK1L	AREA	10	1.061		6.0120	18.85		
EK1L	AREA	25	1.377		6.0120	24.15		
EK1L	AREA	50	1.592		6.0120	27.70		
EK1L	AREA	100	1.810		6.0120	31.25		
EK1M	AREA	2	.306		6.0120	5.84		
EK1M	AREA	5	.465		6.0120	8.65		
EK1M	AREA	10	.576		6.0120	10.56		
EK1M	AREA	25	.747		5.9953	13.46		
EK1M	AREA	50	.864		5.9953	15.40		
EK1M	AREA	100	.982		5.9953	17.35		
EK1N	AREA	2	.933		6.0287	16.82		
EK1N	AREA	5	1.419		6.0287	25.22		
EK1N	AREA	10	1.758		6.0120	30.94		
EK1N	AREA	25	2.281		6.0120	39.72		
EK1N	AREA	50	2.638		6.0120	45.59		
EK1N	AREA	100	2.999		6.0120	51.48		
EK1O	AREA	2	.009		6.0621	.03		
EK1O	AREA	5	.028		6.0454	.37		
EK1O	AREA	10	.046		6.0454	.73		
EK1O	AREA	25	.077		6.0454	1.36		
EK1O	AREA	50	.102		6.0287	1.84		
EK1O	AREA	100	.128		6.0287	2.37		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
I25-1	IN POND	2	27.541		7.7321	36.62		
I25-1	IN POND	5	52.868		7.1977	71.40		
I25-1	IN POND	10	71.162		7.0808	107.20		
I25-1	IN POND	25	100.139		6.3961	135.57		
I25-1	IN POND	50	120.259		7.1643	147.26		
I25-1	IN POND	100	140.891		7.1476	169.67		
I25-1	OUT POND	2	27.541		7.7989	36.61	6591.29	.220
I25-1	OUT POND	5	52.868		7.4315	71.12	6592.34	.508
I25-1	OUT POND	10	71.162		7.2645	106.58	6593.27	.998
I25-1	OUT POND	25	100.139		9.7194	126.73	6593.78	1.264
I25-1	OUT POND	50	120.259		7.5317	145.76	6594.25	1.649
I25-1	OUT POND	100	140.891		7.5818	168.05	6594.79	2.214
IP-1	AREA	2	.070		6.0287	1.27		
IP-1	AREA	5	.113		6.0287	2.07		
IP-1	AREA	10	.145		6.0287	2.63		
IP-1	AREA	25	.194		6.0287	3.49		
IP-1	AREA	50	.229		6.0120	4.08		
IP-1	AREA	100	.264		6.0120	4.68		
IP-2	AREA	2	.145		6.0287	2.65		
IP-2	AREA	5	.236		6.0287	4.30		
IP-2	AREA	10	.301		6.0287	5.47		
IP-2	AREA	25	.405		6.0287	7.27		
IP-2	AREA	50	.476		6.0120	8.49		
IP-2	AREA	100	.549		6.0120	9.74		
NLC	IN POND	2	1.875		6.0621	31.11		
NLC	IN POND	5	3.196		6.0621	54.19		
NLC	IN POND	10	4.167		6.0454	71.14		
NLC	IN POND	25	5.720		6.0454	97.89		
NLC	IN POND	50	6.806		6.0454	116.35		
NLC	IN POND	100	7.922		6.0454	135.15		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
NLC	OUT POND	2	1.875		6.3794	5.01	6737.07	.708
NLC	OUT POND	5	3.195		6.2959	11.97	6737.79	1.267
NLC	OUT POND	10	4.166		6.2792	17.07	6738.38	1.691
NLC	OUT POND	25	5.720		6.2792	23.49	6739.25	2.405
NLC	OUT POND	50	6.805		6.2792	26.23	6739.71	2.950
NLC	OUT POND	100	7.921		6.2959	28.62	6740.21	3.531
O1	AREA	2	.311		6.0454	4.73		
O1	AREA	5	.692		6.0287	12.73		
O1	AREA	10	1.003		6.0287	19.06		
O1	AREA	25	1.536		6.0287	29.60		
O1	AREA	50	1.928		6.0287	37.17		
O1	AREA	100	2.344		6.0120	45.16		
O2B	AREA	2	.163		6.5965	.25		
O2B	AREA	5	.543		6.1289	4.42		
O2B	AREA	10	.894		6.1122	9.72		
O2B	AREA	25	1.543		6.0955	20.10		
O2B	AREA	50	2.044		6.0955	28.39		
O2B	AREA	100	2.593		6.0955	37.46		
PCC	IN POND	2	1.990		6.0454	33.20		
PCC	IN POND	5	3.708		6.0454	65.46		
PCC	IN POND	10	5.020		6.0454	89.55		
PCC	IN POND	25	7.174		6.0454	128.36		
PCC	IN POND	50	8.708		6.0287	155.96		
PCC	IN POND	100	10.305		6.0287	184.43		
PCC	OUT POND	2	1.990		6.1790	14.72	6758.26	.420
PCC	OUT POND	5	3.708		6.1623	31.59	6759.71	.903
PCC	OUT POND	10	5.020		6.1623	39.19	6760.42	1.326
PCC	OUT POND	25	7.174		6.1790	49.69	6761.47	2.146
PCC	OUT POND	50	8.708		6.1790	56.38	6762.09	2.775
PCC	OUT POND	100	10.305		6.1957	60.98	6762.40	3.488

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
VP-1	AREA	2	.064		6.0287	1.19		
VP-1	AREA	5	.104		6.0287	1.92		
VP-1	AREA	10	.133		6.0120	2.43		
VP-1	AREA	25	.178		6.0120	3.24		
VP-1	AREA	50	.210		6.0120	3.78		
VP-1	AREA	100	.242		6.0120	4.33		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = C Springs

Storm Tag Name = 5

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR:
Storm Frequency = 5 yr
Total Rainfall Depth= 2.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	3.708		6.0454	65.46	B2	
		DL	3.708		6.0454	65.46		
		DN	3.708		6.0454	65.46	PCC	IN
ADDLINK 100	ADD	UN	.501		6.0120	9.25	B9	
		DL	.501		6.0120	9.25		
		DN	16.838		6.0454	175.09	DP 25	
ADDLINK 110	ADD	UN	.465		6.0120	8.65	EK1M	
		DL	.465		6.0120	8.65		
		DN	9.918		6.0454	158.51	DP 33	
ADDLINK 120	ADD	UN	1.313		6.0120	24.83	A6	
		DL	1.313		6.0120	24.83		
		DN	67.993		6.0621	891.81	DP 7	
ADDLINK 130	ADD	UN	13.207		6.1122	188.27	A2	
		DL	13.207		6.1122	188.27		
		DN	20.165		6.1122	291.03	DP 8	
ADDLINK 140	ADD	UN	4.940		6.0621	80.73	D1	
		DL	4.940		6.0621	80.73		
		DN	4.940		6.0621	80.73	EK1B	IN
ADDLINK 150	ADD	UN	3.057		6.0287	52.68	A3	
		DL	3.057		6.0287	52.68		
		DN	29.331		6.1122	394.97	DP 9	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 160	ADD	UN	.237		6.0955	3.38	A4	
		DL	.237		6.0955	3.38		
		DN	30.387		6.1289	407.63	DP 11	
ADDLINK 170	ADD	UN	.819		6.0454	13.77	A5	
		DL	.819		6.0454	13.77		
		DN	30.387		6.1289	407.63	DP 11	
ADDLINK 180	ADD	UN	3.363		6.0621	54.95	A9	
		DL	3.363		6.0621	54.95		
		DN	6.383		6.0621	102.40	DP12	
ADDLINK 190	ADD	UN	67.993		6.0621	891.81	DP 7	
		DL	67.993		6.0621	891.81		
		DN	67.993		6.0621	891.81	EK1A	IN
ADDLINK 20	ADD	UN	3.196		6.0621	54.19	B3	
		DL	3.196		6.0621	54.19		
		DN	3.196		6.0621	54.19	NLC	IN
ADDLINK 200	ADD	UN	.586		6.1790	4.38	A12	
		DL	.586		6.1790	4.38		
		DN	52.868		7.1977	71.40	DP 14	
ADDLINK 210	ADD	UN	.942		6.0120	17.26	EK1B	
		DL	.942		6.0120	17.26		
		DN	16.838		6.0454	175.09	DP 25	
ADDLINK 220	ADD	UN	1.015		6.0287	17.95	EK1D	
		DL	1.015		6.0287	17.95		
		DN	1.602		6.0287	28.65	DP 26	
ADDLINK 230	ADD	UN	1.516		6.0287	26.87	EK1E	
		DL	1.516		6.0287	26.87		
		DN	19.956		6.0454	229.97	DP 27	
ADDLINK 240	ADD	UN	.563		6.0287	9.97	EK1G	
		DL	.563		6.0287	9.97		
		DN	1.235		6.0287	21.93	DP 28	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 250	ADD	UN	1.333		6.0287	23.44	EK1H	
		DL	1.333		6.0287	23.44		
		DN	22.524		6.0454	274.98	DP 29	
ADDLINK 260	ADD	UN	.721		6.0120	13.38	EK1I	
		DL	.721		6.0120	13.38		
		DN	23.245		6.0454	287.11	DP 30	
ADDLINK 270	ADD	UN	.269		5.9953	5.05	EK1J	
		DL	.269		5.9953	5.05		
		DN	23.514		6.0454	291.36	DP 31	
ADDLINK 280	ADD	UN	.856		6.0120	15.35	EK1L	
		DL	.856		6.0120	15.35		
		DN	1.443		6.0287	25.96	DP 32	
ADDLINK 290	ADD	UN	.028		6.0454	.37	EK1O	
		DL	.028		6.0454	.37		
		DN	52.868		7.1977	71.40	DP 14	
ADDLINK 30	ADD	UN	.544		6.0621	8.42	B1	
		DL	.544		6.0621	8.42		
		DN	8.681		6.1289	56.08	DP 2	
ADDLINK 300	ADD	UN	1.419		6.0287	25.22	EK1N	
		DL	1.419		6.0287	25.22		
		DN	11.336		6.0454	183.30	DP 34	
ADDLINK 310	ADD	UN	52.868		7.1977	71.40	DP 14	
		DL	52.868		7.1977	71.40		
		DN	52.868		7.1977	71.40	I25-1	IN
ADDLINK 40	ADD	UN	.104		6.0287	1.92	VP-1	
		DL	.104		6.0287	1.92		
		DN	9.134		6.1456	58.66	DP 3	
ADDLINK 50	ADD	UN	.113		6.0287	2.07	IP-1	
		DL	.113		6.0287	2.07		
		DN	9.134		6.1456	58.66	DP 3	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	.236		6.0287	4.30	IP-2
		DL	.236		6.0287	4.30	
		DN	9.134		6.1456	58.66	DP 3
ADDLINK 70	ADD	UN	1.394		6.0287	24.08	B5
		DL	1.394		6.0287	24.08	
		DN	10.528		6.1122	75.51	DP 4
ADDLINK 80	ADD	UN	2.250		6.0454	38.36	B6
		DL	2.250		6.0454	38.36	
		DN	14.954		6.0788	145.19	DP 5
ADDLINK 90	ADD	UN	2.177		6.0287	37.42	B7
		DL	2.177		6.0287	37.42	
		DN	14.954		6.0788	145.19	DP 5
C1	REACH	UN	.692		6.0287	12.73	O1
		DL	.692		6.0621	10.55	
		DN	4.400		6.1122	39.67	DP 1
C10	REACH	UN	20.165		6.1122	291.03	DP 8
		DL	20.165		6.1289	288.67	
		DN	29.331		6.1122	394.97	DP 9
C11	REACH	UN	1.179		6.2792	6.74	C1
		DL	1.179		6.3627	6.37	
		DN	6.109		6.1623	68.34	DP 10
C12	ADD	UN	6.109		6.1623	68.34	DP 10
		DL	6.109		6.1623	68.34	
		DN	29.331		6.1122	394.97	DP 9
C13	REACH	UN	29.331		6.1122	394.97	DP 9
		DL	29.331		6.1289	393.32	
		DN	30.387		6.1289	407.63	DP 11
C14	REACH	UN	30.387		6.1289	407.63	DP 11
		DL	30.387		6.1289	407.25	
		DN	67.993		6.0621	891.81	DP 7

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C15	ADD	UN	3.021		6.0788	47.48	A8
		DL	3.021		6.0788	47.48	
		DN	6.383		6.0621	102.40	DP12
C18	REACH	UN	52.254	R	7.2311	70.54	DP13
		DL	52.254		7.2645	70.53	
		DN	52.868		7.1977	71.40	DP 14
C2	REACH	UN	4.400		6.1122	39.67	DP 1
		DL	4.400		6.1456	39.25	
		DN	8.681		6.1289	56.08	DP 2
C3	REACH	UN	.543		6.1289	4.42	O2B
		DL	.542		6.3961	1.57	
		DN	8.681		6.1289	56.08	DP 2
C30	REACH	UN	.440		6.0120	8.15	EK1A
		DL	.440		6.0287	8.06	
		DN	16.838		6.0454	175.09	DP 25
C31	ADD	UN	16.838		6.0454	175.09	DP 25
		DL	16.838		6.0454	175.09	
		DN	19.956		6.0454	229.97	DP 27
C32	REACH	UN	.587		6.0120	10.89	EK1C
		DL	.587		6.0287	10.70	
		DN	1.602		6.0287	28.65	DP 26
C33	REACH	UN	1.602		6.0287	28.65	DP 26
		DL	1.602		6.0454	28.40	
		DN	19.956		6.0454	229.97	DP 27
C34	ADD	UN	19.956		6.0454	229.97	DP 27
		DL	19.956		6.0454	229.97	
		DN	22.524		6.0454	274.98	DP 29
C35	REACH	UN	.673		6.0287	11.97	EK1F
		DL	.673		6.0287	11.96	
		DN	1.235		6.0287	21.93	DP 28

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C36	REACH	UN	1.235		6.0287	21.93	DP 28
		DL	1.235		6.0287	21.86	
		DN	22.524		6.0454	274.98	DP 29
C37	ADD	UN	22.524		6.0454	274.98	DP 29
		DL	22.524		6.0454	274.98	
		DN	23.245		6.0454	287.11	DP 30
C38	ADD	UN	23.245		6.0454	287.11	DP 30
		DL	23.245		6.0454	287.11	
		DN	23.514		6.0454	291.36	DP 31
C39	ADD	UN	23.514		6.0454	291.36	DP 31
		DL	23.514		6.0454	291.36	
		DN	67.993		6.0621	891.81	DP 7
C4	REACH	UN	8.681		6.1289	56.08	DP 2
		DL	8.681		6.1623	55.36	
		DN	9.134		6.1456	58.66	DP 3
C40	REACH	UN	.587		6.0120	10.76	EK1K
		DL	.587		6.0287	10.63	
		DN	1.443		6.0287	25.96	DP 32
C41	REACH	UN	1.443		6.0287	25.96	DP 32
		DL	1.443		6.0287	25.93	
		DN	67.993		6.0621	891.81	DP 7
C42	ADD	UN	6.383		6.0621	102.40	DP12
		DL	6.383		6.0621	102.40	
		DN	9.918		6.0454	158.51	DP 33
C43	REACH	UN	3.069		6.0621	49.61	A7
		DL	3.069		6.0621	49.49	
		DN	9.918		6.0454	158.51	DP 33
C44	ADD	UN	9.918		6.0454	158.51	DP 33
		DL	9.918		6.0454	158.51	
		DN	11.336		6.0454	183.30	DP 34

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
C45	ADD	UN	11.336		6.0454	183.30	DP 34	
		DL	11.336		6.0454	183.30		
		DN	67.993		6.0621	891.81	DP 7	
C5	REACH	UN	9.134		6.1456	58.66	DP 3	
		DL	9.134		6.1623	58.56		
		DN	10.528		6.1122	75.51	DP 4	
C6	ADD	UN	10.528		6.1122	75.51	DP 4	
		DL	10.528		6.1122	75.51		
		DN	14.954		6.0788	145.19	DP 5	
C7	ADD	UN	14.954		6.0788	145.19	DP 5	
		DL	14.954		6.0788	145.19		
		DN	16.838		6.0454	175.09	DP 25	
C9	REACH	UN	6.958		6.0955	104.23	A1	
		DL	6.958		6.1122	102.76		
		DN	20.165		6.1122	291.03	DP 8	
EK-1A STRUCTURE	PONDrt	UN	67.993		6.0621	891.81	EK1A	IN
EK-1A STRUCTURE			52.254	R	7.2311	70.54	EK1A	OUT
		DL	52.254	R	7.2311	70.54		
		DN	52.254	R	7.2311	70.54	DP13	
EK-1B STRUCTURE	PONDrt	UN	4.940		6.0621	80.73	EK1B	IN
EK-1B STRUCTURE			4.930		6.1456	65.96	EK1B	OUT
		DL	4.930		6.1456	65.96		
		DN	6.109		6.1623	68.34	DP 10	
I25-1 STRUCTURE	PONDrt	UN	52.868		7.1977	71.40	I25-1	IN
I25-1 STRUCTURE			52.868		7.4315	71.12	I25-1	OUT
		DL	52.868		7.4315	71.12		
		DN	52.868		7.4315	71.12	DP 20	
NLC STRUCTURE	PONDrt	UN	3.196		6.0621	54.19	NLC	IN
NLC STRUCTURE			3.195		6.2959	11.97	NLC	OUT
		DL	3.195		6.2959	11.97		
		DN	8.681		6.1289	56.08	DP 2	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
PCC STRUCTURE	PONDrt	UN	3.708		6.0454	65.46	PCC	IN
PCC STRUCTURE			3.708		6.1623	31.59	PCC	OUT
		DL	3.708		6.1623	31.59		
		DN	4.400		6.1122	39.67	DP 1	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = C Springs

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR:
Storm Frequency = 100 yr
Total Rainfall Depth= 4.4000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	10.305		6.0287	184.43	B2	
		DL	10.305		6.0287	184.43		
		DN	10.305		6.0287	184.43	PCC	IN
ADDLINK 100	ADD	UN	1.060		5.9953	18.59	B9	
		DL	1.060		5.9953	18.59		
		DN	42.349		6.0454	438.38	DP 25	
ADDLINK 110	ADD	UN	.982		5.9953	17.35	EK1M	
		DL	.982		5.9953	17.35		
		DN	20.966		6.0454	333.48	DP 33	
ADDLINK 120	ADD	UN	3.122		6.0120	57.02	A6	
		DL	3.122		6.0120	57.02		
		DN	153.852		6.0454	1999.54	DP 7	
ADDLINK 130	ADD	UN	27.921		6.0955	403.06	A2	
		DL	27.921		6.0955	403.06		
		DN	42.631		6.0955	622.87	DP 8	
ADDLINK 140	ADD	UN	10.444		6.0454	168.68	D1	
		DL	10.444		6.0454	168.68		
		DN	10.444		6.0454	168.68	EK1B	IN
ADDLINK 150	ADD	UN	6.463		6.0287	108.56	A3	
		DL	6.463		6.0287	108.56		
		DN	64.861		6.0955	862.68	DP 9	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 160	ADD	UN	.657		6.0788	10.09	A4	
		DL	.657		6.0788	10.09		
		DN	67.250		6.1122	895.08	DP 11	
ADDLINK 170	ADD	UN	1.732		6.0287	28.55	A5	
		DL	1.732		6.0287	28.55		
		DN	67.250		6.1122	895.08	DP 11	
ADDLINK 180	ADD	UN	7.109		6.0454	114.82	A9	
		DL	7.109		6.0454	114.82		
		DN	13.495		6.0454	214.67	DP12	
ADDLINK 190	ADD	UN	153.852		6.0454	1999.54	DP 7	
		DL	153.852		6.0454	1999.54		
		DN	153.852		6.0454	1999.54	EK1A	IN
ADDLINK 20	ADD	UN	7.922		6.0454	135.15	B3	
		DL	7.922		6.0454	135.15		
		DN	7.922		6.0454	135.15	NLC	IN
ADDLINK 200	ADD	UN	2.654		6.1289	33.67	A12	
		DL	2.654		6.1289	33.67		
		DN	140.891		7.1476	169.67	DP 14	
ADDLINK 210	ADD	UN	1.991		6.0120	34.78	EK1B	
		DL	1.991		6.0120	34.78		
		DN	42.349		6.0454	438.38	DP 25	
ADDLINK 220	ADD	UN	2.146		6.0120	36.68	EK1D	
		DL	2.146		6.0120	36.68		
		DN	3.387		6.0120	58.36	DP 26	
ADDLINK 230	ADD	UN	3.206		6.0120	54.89	EK1E	
		DL	3.206		6.0120	54.89		
		DN	48.941		6.0454	549.59	DP 27	
ADDLINK 240	ADD	UN	1.189		6.0120	20.36	EK1G	
		DL	1.189		6.0120	20.36		
		DN	2.611		6.0120	44.71	DP 28	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 250	ADD	UN	2.818		6.0120	47.96	EK1H	
		DL	2.818		6.0120	47.96		
		DN	54.370		6.0287	640.97	DP 29	
ADDLINK 260	ADD	UN	1.525		5.9953	26.85	EK1I	
		DL	1.525		5.9953	26.85		
		DN	55.895		6.0287	667.00	DP 30	
ADDLINK 270	ADD	UN	.569		5.9953	10.10	EK1J	
		DL	.569		5.9953	10.10		
		DN	56.464		6.0287	676.51	DP 31	
ADDLINK 280	ADD	UN	1.810		6.0120	31.25	EK1L	
		DL	1.810		6.0120	31.25		
		DN	3.051		6.0120	52.81	DP 32	
ADDLINK 290	ADD	UN	.128		6.0287	2.37	EK1O	
		DL	.128		6.0287	2.37		
		DN	140.891		7.1476	169.67	DP 14	
ADDLINK 30	ADD	UN	1.844		6.0454	32.02	B1	
		DL	1.844		6.0454	32.02		
		DN	25.007		6.0788	169.78	DP 2	
ADDLINK 300	ADD	UN	2.999		6.0120	51.48	EK1N	
		DL	2.999		6.0120	51.48		
		DN	23.965		6.0287	384.00	DP 34	
ADDLINK 310	ADD	UN	140.891		7.1476	169.67	DP 14	
		DL	140.891		7.1476	169.67		
		DN	140.891		7.1476	169.67	I25-1	IN
ADDLINK 40	ADD	UN	.242		6.0120	4.33	VP-1	
		DL	.242		6.0120	4.33		
		DN	26.062		6.0955	181.69	DP 3	
ADDLINK 50	ADD	UN	.264		6.0120	4.68	IP-1	
		DL	.264		6.0120	4.68		
		DN	26.062		6.0955	181.69	DP 3	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 60	ADD	UN	.549		6.0120	9.74	IP-2
		DL	.549		6.0120	9.74	
		DN	26.062		6.0955	181.69	DP 3
ADDLINK 70	ADD	UN	2.947		6.0287	49.57	B5
		DL	2.947		6.0287	49.57	
		DN	29.009		6.0788	224.99	DP 4
ADDLINK 80	ADD	UN	4.757		6.0287	79.32	B6
		DL	4.757		6.0287	79.32	
		DN	38.367		6.0621	374.92	DP 5
ADDLINK 90	ADD	UN	4.602		6.0287	77.19	B7
		DL	4.602		6.0287	77.19	
		DN	38.367		6.0621	374.92	DP 5
C1	REACH	UN	2.344		6.0120	45.16	O1
		DL	2.344		6.0454	42.84	
		DN	12.650		6.0621	98.98	DP 1
C10	REACH	UN	42.631		6.0955	622.87	DP 8
		DL	42.631		6.1122	620.32	
		DN	64.861		6.0955	862.68	DP 9
C11	REACH	UN	5.333		6.2291	53.92	C1
		DL	5.333		6.2625	52.96	
		DN	15.767		6.2124	171.89	DP 10
C12	ADD	UN	15.767		6.2124	171.89	DP 10
		DL	15.767		6.2124	171.89	
		DN	64.861		6.0955	862.68	DP 9
C13	REACH	UN	64.861		6.0955	862.68	DP 9
		DL	64.861		6.1122	860.97	
		DN	67.250		6.1122	895.08	DP 11
C14	REACH	UN	67.250		6.1122	895.08	DP 11
		DL	67.250		6.1122	893.80	
		DN	153.852		6.0454	1999.54	DP 7

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C15	ADD	UN	6.386		6.0621	100.04	A8
		DL	6.386		6.0621	100.04	
		DN	13.495		6.0454	214.67	DP12
C18	REACH	UN	138.109	R	7.2645	166.31	DP13
		DL	138.109		7.2812	166.30	
		DN	140.891		7.1476	169.67	DP 14
C2	REACH	UN	12.650		6.0621	98.98	DP 1
		DL	12.650		6.0788	98.20	
		DN	25.007		6.0788	169.78	DP 2
C3	REACH	UN	2.593		6.0955	37.46	O2B
		DL	2.592		6.2124	24.70	
		DN	25.007		6.0788	169.78	DP 2
C30	REACH	UN	.931		5.9953	16.36	EK1A
		DL	.931		6.0120	16.29	
		DN	42.349		6.0454	438.38	DP 25
C31	ADD	UN	42.349		6.0454	438.38	DP 25
		DL	42.349		6.0454	438.38	
		DN	48.941		6.0454	549.59	DP 27
C32	REACH	UN	1.241		5.9953	21.84	EK1C
		DL	1.241		6.0120	21.68	
		DN	3.387		6.0120	58.36	DP 26
C33	REACH	UN	3.387		6.0120	58.36	DP 26
		DL	3.387		6.0287	58.12	
		DN	48.941		6.0454	549.59	DP 27
C34	ADD	UN	48.941		6.0454	549.59	DP 27
		DL	48.941		6.0454	549.59	
		DN	54.370		6.0287	640.97	DP 29
C35	REACH	UN	1.422		6.0120	24.43	EK1F
		DL	1.422		6.0287	24.36	
		DN	2.611		6.0120	44.71	DP 28

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C36	REACH	UN	2.611		6.0120	44.71	DP 28
		DL	2.611		6.0287	44.68	
		DN	54.370		6.0287	640.97	DP 29
C37	ADD	UN	54.370		6.0287	640.97	DP 29
		DL	54.370		6.0287	640.97	
		DN	55.895		6.0287	667.00	DP 30
C38	ADD	UN	55.895		6.0287	667.00	DP 30
		DL	55.895		6.0287	667.00	
		DN	56.464		6.0287	676.51	DP 31
C39	ADD	UN	56.464		6.0287	676.51	DP 31
		DL	56.464		6.0287	676.51	
		DN	153.852		6.0454	1999.54	DP 7
C4	REACH	UN	25.007		6.0788	169.78	DP 2
		DL	25.007		6.1122	168.19	
		DN	26.062		6.0955	181.69	DP 3
C40	REACH	UN	1.241		6.0120	21.68	EK1K
		DL	1.241		6.0120	21.55	
		DN	3.051		6.0120	52.81	DP 32
C41	REACH	UN	3.051		6.0120	52.81	DP 32
		DL	3.051		6.0287	52.66	
		DN	153.852		6.0454	1999.54	DP 7
C42	ADD	UN	13.495		6.0454	214.67	DP12
		DL	13.495		6.0454	214.67	
		DN	20.966		6.0454	333.48	DP 33
C43	REACH	UN	6.489		6.0454	103.86	A7
		DL	6.489		6.0621	103.68	
		DN	20.966		6.0454	333.48	DP 33
C44	ADD	UN	20.966		6.0454	333.48	DP 33
		DL	20.966		6.0454	333.48	
		DN	23.965		6.0287	384.00	DP 34

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
C45	ADD	UN	23.965		6.0287	384.00	DP 34	
		DL	23.965		6.0287	384.00		
		DN	153.852		6.0454	1999.54	DP 7	
C5	REACH	UN	26.062		6.0955	181.69	DP 3	
		DL	26.062		6.0955	181.40		
		DN	29.009		6.0788	224.99	DP 4	
C6	ADD	UN	29.009		6.0788	224.99	DP 4	
		DL	29.009		6.0788	224.99		
		DN	38.367		6.0621	374.92	DP 5	
C7	ADD	UN	38.367		6.0621	374.92	DP 5	
		DL	38.367		6.0621	374.92		
		DN	42.349		6.0454	438.38	DP 25	
C9	REACH	UN	14.710		6.0788	221.48	A1	
		DL	14.710		6.0955	219.81		
		DN	42.631		6.0955	622.87	DP 8	
EK-1A STRUCTURE	PONDrt	UN	153.852		6.0454	1999.54	EK1A	IN
EK-1A STRUCTURE			138.109	R	7.2645	166.31	EK1A	OUT
		DL	138.109	R	7.2645	166.31		
		DN	138.109	R	7.2645	166.31	DP13	
EK-1B STRUCTURE	PONDrt	UN	10.444		6.0454	168.68	EK1B	IN
EK-1B STRUCTURE			10.434		6.1623	123.14	EK1B	OUT
		DL	10.434		6.1623	123.14		
		DN	15.767		6.2124	171.89	DP 10	
I25-1 STRUCTURE	PONDrt	UN	140.891		7.1476	169.67	I25-1	IN
I25-1 STRUCTURE			140.891		7.5818	168.05	I25-1	OUT
		DL	140.891		7.5818	168.05		
		DN	140.891		7.5818	168.05	DP 20	
NLC STRUCTURE	PONDrt	UN	7.922		6.0454	135.15	NLC	IN
NLC STRUCTURE			7.921		6.2959	28.62	NLC	OUT
		DL	7.921		6.2959	28.62		
		DN	25.007		6.0788	169.78	DP 2	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
PCC STRUCTURE	PONDrt	UN	10.305		6.0287	184.43	PCC	IN
PCC STRUCTURE			10.305		6.1957	60.98	PCC	OUT
		DL	10.305		6.1957	60.98		
		DN	12.650		6.0621	98.98	DP 1	

MASTER DESIGN STORM SUMMARY

Network Storm Collection: El Paso County

Return Event	Total Depth in	Rainfall Type	RNF ID
2	2.0000	Synthetic Curve	TYPEIIA 24HR
5	2.6000	Synthetic Curve	TYPEIIA 24HR
10	3.0000	Synthetic Curve	TYPEIIA 24HR
25	3.6000	Synthetic Curve	TYPEIIA 24HR
50	4.0000	Synthetic Curve	TYPEIIA 24HR
100	4.4000	Synthetic Curve	TYPEIIA 24HR

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 15	JCT	2	.295		6.0500	4.88		
DP 15	JCT	5	.481		6.0500	8.05		
DP 15	JCT	10	.615		6.0500	10.29		
DP 15	JCT	25	.826		6.0500	13.78		
DP 15	JCT	50	.972		6.0500	16.16		
DP 15	JCT	100	1.121		6.0500	18.56		
DP 17	JCT	2	3.095	R	6.5500	.93		
DP 17	JCT	5	4.889	R	6.2000	5.27		
DP 17	JCT	10	6.052	R	6.1500	10.47		
DP 17	JCT	25	7.946	R	6.1500	20.60		
DP 17	JCT	50	9.709	R	6.1500	28.53		
DP 17	JCT	100	11.549	R	6.1000	37.35		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 35	JCT	2	1.188		6.0500	20.07		
DP 35	JCT	5	1.838		6.0500	30.88		
DP 35	JCT	10	2.297		6.0500	38.34		
DP 35	JCT	25	3.009		6.0000	50.12		
DP 35	JCT	50	3.496		6.0000	58.12		
DP 35	JCT	100	3.990		6.0000	66.17		
DP 36	JCT	2	1.255		6.0000	21.90		
DP 36	JCT	5	1.908		6.0000	33.19		
DP 36	JCT	10	2.364		6.0000	40.93		
DP 36	JCT	25	3.068		6.0000	52.69		
DP 36	JCT	50	3.548		6.0000	60.58		
DP 36	JCT	100	4.033		6.0000	68.49		
DP 37	JCT	2	2.883		6.0000	49.23		
DP 37	JCT	5	4.463		6.0000	76.62		
DP 37	JCT	10	5.576		6.0000	95.62		
DP 37	JCT	25	7.307		6.0000	124.74		
DP 37	JCT	50	8.491		6.0000	144.39		
DP 37	JCT	100	9.693		6.0000	164.15		
DP 38	JCT	2	2.870	R	14.8500	.57		
DP 38	JCT	5	4.193	R	20.0500	.68		
DP 38	JCT	10	4.929	R	18.7500	.74		
DP 38	JCT	25	6.045	R	13.1000	1.74		
DP 38	JCT	50	7.210	R	8.1000	4.43		
DP 38	JCT	100	8.401	R	7.0500	8.81		
*DP 39	JCT	2	3.095	R	6.7000	.91		
*DP 39	JCT	5	4.883	R	6.3000	4.47		
*DP 39	JCT	10	6.045	R	6.4000	5.70		
*DP 39	JCT	25	7.938	R	6.4000	8.91		
*DP 39	JCT	50	9.701	R	6.4000	11.79		
*DP 39	JCT	100	11.542	R	6.6000	16.11		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
E5	AREA	2	.209		6.6000	.38		
E5	AREA	5	.647		6.2000	4.53		
E5	AREA	10	1.044		6.1500	9.49		
E5	AREA	25	1.767		6.1500	19.29		
E5	AREA	50	2.322		6.1500	26.99		
E5	AREA	100	2.927		6.1500	35.43		
EK2A	AREA	2	.893		6.0500	15.18		
EK2A	AREA	5	1.357		6.0000	22.88		
EK2A	AREA	10	1.682		6.0000	28.32		
EK2A	AREA	25	2.183		6.0000	36.61		
EK2A	AREA	50	2.524		6.0000	42.18		
EK2A	AREA	100	2.870		6.0000	47.77		
EK2A	IN POND	2	2.883		6.0000	49.23		
EK2A	IN POND	5	4.463		6.0000	76.62		
EK2A	IN POND	10	5.576		6.0000	95.62		
EK2A	IN POND	25	7.307		6.0000	124.74		
EK2A	IN POND	50	8.491		6.0000	144.39		
EK2A	IN POND	100	9.693		6.0000	164.15		
EK2A	OUT POND	2	2.870	R	14.8500	.57	6622.58	2.120
EK2A	OUT POND	5	4.193	R	20.0500	.68	6623.58	3.537
EK2A	OUT POND	10	4.929	R	18.7500	.74	6624.20	4.551
EK2A	OUT POND	25	6.045	R	13.1000	1.74	6624.79	5.732
EK2A	OUT POND	50	7.210	R	8.1000	4.43	6624.90	5.951
EK2A	OUT POND	100	8.401	R	7.0500	8.81	6625.04	6.241
EK2B	AREA	2	.434		6.0000	7.89		
EK2B	AREA	5	.660		6.0000	11.87		
EK2B	AREA	10	.818		6.0000	14.58		
EK2B	AREA	25	1.062		6.0000	18.68		
EK2B	AREA	50	1.228		6.0000	21.44		
EK2B	AREA	100	1.396		6.0000	24.19		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK2C		AREA	2	.820		6.0500	14.04		
EK2C		AREA	5	1.247		6.0000	21.32		
EK2C		AREA	10	1.545		6.0000	26.35		
EK2C		AREA	25	2.006		6.0000	34.01		
EK2C		AREA	50	2.320		6.0000	39.15		
EK2C		AREA	100	2.637		6.0000	44.30		
EK2D		AREA	2	.440		6.0000	7.81		
EK2D		AREA	5	.717		6.0000	12.85		
EK2D		AREA	10	.916		6.0000	16.41		
EK2D		AREA	25	1.231		6.0000	21.93		
EK2D		AREA	50	1.448		6.0000	25.70		
EK2D		AREA	100	1.670		6.0000	29.50		
EK3E		AREA	2	.016		6.0500	.05		
EK3E		AREA	5	.049		6.0000	.66		
EK3E		AREA	10	.079		6.0000	1.32		
EK3E		AREA	25	.134		6.0000	2.50		
EK3E		AREA	50	.176		6.0000	3.38		
EK3E		AREA	100	.222		6.0000	4.32		
I25-2	IN	POND	2	3.095	R	6.5500	.93		
I25-2	IN	POND	5	4.889	R	6.2000	5.27		
I25-2	IN	POND	10	6.052	R	6.1500	10.47		
I25-2	IN	POND	25	7.946	R	6.1500	20.60		
I25-2	IN	POND	50	9.709	R	6.1500	28.53		
I25-2	IN	POND	100	11.549	R	6.1000	37.35		
I25-2	OUT	POND	2	3.095	R	6.7000	.91	6599.41	.016
I25-2	OUT	POND	5	4.883	R	6.3000	4.47	6600.01	.045
I25-2	OUT	POND	10	6.045	R	6.4000	5.70	6600.15	.138
I25-2	OUT	POND	25	7.938	R	6.4000	8.91	6600.51	.364
I25-2	OUT	POND	50	9.701	R	6.4000	11.79	6600.80	.551
I25-2	OUT	POND	100	11.542	R	6.6000	16.11	6601.22	.822

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
IP-3	AREA	2	.116		6.0500	1.91		
IP-3	AREA	5	.189		6.0500	3.16		
IP-3	AREA	10	.241		6.0500	4.04		
IP-3	AREA	25	.324		6.0500	5.40		
IP-3	AREA	50	.381		6.0500	6.34		
IP-3	AREA	100	.439		6.0500	7.28		
IP-4	AREA	2	.180		6.0500	2.97		
IP-4	AREA	5	.292		6.0500	4.89		
IP-4	AREA	10	.374		6.0500	6.26		
IP-4	AREA	25	.502		6.0500	8.37		
IP-4	AREA	50	.591		6.0500	9.82		
IP-4	AREA	100	.681		6.0500	11.28		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = El Paso County

Storm Tag Name = 5

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 5 yr
Total Rainfall Depth= 2.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	.189		6.0500	3.16	IP-3	
		DL	.189		6.0500	3.16		
		DN	.481		6.0500	8.05	DP 15	
ADDLINK 20	ADD	UN	.292		6.0500	4.89	IP-4	
		DL	.292		6.0500	4.89		
		DN	.481		6.0500	8.05	DP 15	
ADDLINK 30	ADD	UN	1.357		6.0000	22.88	EK2A	
		DL	1.357		6.0000	22.88		
		DN	1.838		6.0500	30.88	DP 35	
ADDLINK 40	ADD	UN	.717		6.0000	12.85	EK2D	
		DL	.717		6.0000	12.85		
		DN	4.463		6.0000	76.62	DP 37	
ADDLINK 50	ADD	UN	1.247		6.0000	21.32	EK2C	
		DL	1.247		6.0000	21.32		
		DN	1.908		6.0000	33.19	DP 36	
ADDLINK 60	ADD	UN	4.463		6.0000	76.62	DP 37	
		DL	4.463		6.0000	76.62		
		DN	4.463		6.0000	76.62	EK2A	IN
ADDLINK 70	ADD	UN	.049		6.0000	.66	EK3E	
		DL	.049		6.0000	.66		
		DN	4.889	R	6.2000	5.27	DP 17	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 80	ADD	UN	.647		6.2000	4.53	E5	
		DL	.647		6.2000	4.53		
		DN	4.889	R	6.2000	5.27	DP 17	
ADDLINK 90	ADD	UN	4.889	R	6.2000	5.27	DP 17	
		DL	4.889	R	6.2000	5.27		
		DN	4.889	R	6.2000	5.27	I25-2	IN
C20	ADD	UN	4.193	R	20.0500	.68	DP 38	
		DL	4.193	R	20.0500	.68		
		DN	4.889	R	6.2000	5.27	DP 17	
C46	ADD	UN	.481		6.0500	8.05	DP 15	
		DL	.481		6.0500	8.05		
		DN	1.838		6.0500	30.88	DP 35	
C47	ADD	UN	1.838		6.0500	30.88	DP 35	
		DL	1.838		6.0500	30.88		
		DN	4.463		6.0000	76.62	DP 37	
C48	ADD	UN	.660		6.0000	11.87	EK2B	
		DL	.660		6.0000	11.87		
		DN	1.908		6.0000	33.19	DP 36	
C49	ADD	UN	1.908		6.0000	33.19	DP 36	
		DL	1.908		6.0000	33.19		
		DN	4.463		6.0000	76.62	DP 37	
EK2A STRUCTURE	PONDrt	UN	4.463		6.0000	76.62	EK2A	IN
EK2A STRUCTURE			4.193	R	20.0500	.68	EK2A	OUT
		DL	4.193	R	20.0500	.68		
		DN	4.193	R	20.0500	.68	DP 38	
I-25 STRUC	PONDrt	UN	4.889	R	6.2000	5.27	I25-2	IN
I-25 STRUC			4.883	R	6.3000	4.47	I25-2	OUT
		DL	4.883	R	6.3000	4.47		
		DN	4.883	R	6.3000	4.47	DP 39	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = El Paso County

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 100 yr
Total Rainfall Depth= 4.4000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	.439		6.0500	7.28	IP-3	
		DL	.439		6.0500	7.28		
		DN	1.121		6.0500	18.56	DP 15	
ADDLINK 20	ADD	UN	.681		6.0500	11.28	IP-4	
		DL	.681		6.0500	11.28		
		DN	1.121		6.0500	18.56	DP 15	
ADDLINK 30	ADD	UN	2.870		6.0000	47.77	EK2A	
		DL	2.870		6.0000	47.77		
		DN	3.990		6.0000	66.17	DP 35	
ADDLINK 40	ADD	UN	1.670		6.0000	29.50	EK2D	
		DL	1.670		6.0000	29.50		
		DN	9.693		6.0000	164.15	DP 37	
ADDLINK 50	ADD	UN	2.637		6.0000	44.30	EK2C	
		DL	2.637		6.0000	44.30		
		DN	4.033		6.0000	68.49	DP 36	
ADDLINK 60	ADD	UN	9.693		6.0000	164.15	DP 37	
		DL	9.693		6.0000	164.15		
		DN	9.693		6.0000	164.15	EK2A	IN
ADDLINK 70	ADD	UN	.222		6.0000	4.32	EK3E	
		DL	.222		6.0000	4.32		
		DN	11.549	R	6.1000	37.35	DP 17	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 80	ADD	UN	2.927		6.1500	35.43	E5	
		DL	2.927		6.1500	35.43		
		DN	11.549	R	6.1000	37.35	DP 17	
ADDLINK 90	ADD	UN	11.549	R	6.1000	37.35	DP 17	
		DL	11.549	R	6.1000	37.35		
		DN	11.549	R	6.1000	37.35	I25-2	IN
C20	ADD	UN	8.401	R	7.0500	8.81	DP 38	
		DL	8.401	R	7.0500	8.81		
		DN	11.549	R	6.1000	37.35	DP 17	
C46	ADD	UN	1.121		6.0500	18.56	DP 15	
		DL	1.121		6.0500	18.56		
		DN	3.990		6.0000	66.17	DP 35	
C47	ADD	UN	3.990		6.0000	66.17	DP 35	
		DL	3.990		6.0000	66.17		
		DN	9.693		6.0000	164.15	DP 37	
C48	ADD	UN	1.396		6.0000	24.19	EK2B	
		DL	1.396		6.0000	24.19		
		DN	4.033		6.0000	68.49	DP 36	
C49	ADD	UN	4.033		6.0000	68.49	DP 36	
		DL	4.033		6.0000	68.49		
		DN	9.693		6.0000	164.15	DP 37	
EK2A STRUCTURE	PONDrt	UN	9.693		6.0000	164.15	EK2A	IN
EK2A STRUCTURE			8.401	R	7.0500	8.81	EK2A	OUT
		DL	8.401	R	7.0500	8.81		
		DN	8.401	R	7.0500	8.81	DP 38	
I-25 STRUC	PONDrt	UN	11.549	R	6.1000	37.35	I25-2	IN
I-25 STRUC			11.542	R	6.6000	16.11	I25-2	OUT
		DL	11.542	R	6.6000	16.11		
		DN	11.542	R	6.6000	16.11	DP 39	

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

HYDROLOGIC / HYDRAULIC CALCULATIONS
FILING NO. 6 & 7 CONDITIONS

COMPOSITE C_N VALUES - FILING NO. 6 & 7 INTERIM CONDTIONS

BASIN (label)	BASIN AREA (ac)	LAND USE	SUB-AREA LAND USE 1		AREA (AC)	LAND USE	SUB-AREA LAND USE 2		AREA (AC)	TOTAL BASIN AREA (ac)	TOTAL BASIN AREA (sq mi)	WEIGHTED C_N
			ESTIMATED PERCENT IMPERIOUS	ESTIMATED C_N			ESTIMATED PERCENT IMPERIOUS	ESTIMATED C_N				
02B	32.2	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	32.2	0.05031	60.0
O1	19.2	REPORT	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	19.2	0.03000	67.8
B1	15.1	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	15.1	0.02359	68.0
B2	62.7	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	62.7	0.09797	75.0
B3	40.0	UNDEV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	40.0	0.06250	80.0
B5	11.4	UNDEV.	0	61	7.7	ROADS	100	88	3.7	11.4	0.01781	69.8
B6	18.4	UNDEV.	0	61	18.4	ROADS	100	88	0.0	18.4	0.02875	61.0
B7	17.8	UNDEV.	0	61	13.5	ROADS	100	88	4.3	17.8	0.02781	67.5
B9	4.1	UNDEV.	0	61	0.0	ROADS	100	88	4.1	4.1	0.00641	88.0
A1	56.9	UNDEV.	0	61	45.9	ROADS	100	88	11.0	56.9	0.08891	66.2
A2	108.0	UNDEV.	0	61	100.0	ROADS	100	88	8.0	108.0	0.16875	63.0
A3	25.0	UNDEV.	0	61	7.5	ROADS	100	88	17.5	25.0	0.03906	79.9
A4	4.0	UNDEV.	0	61	2.0	ROADS	100	88	2.0	4.0	0.00625	74.5
A5	6.7	UNDEV.	0	61	5.8	ROADS	100	88	0.9	6.7	0.01047	64.6
A6	14.7	UNDEV.	0	61	3.5	ROADS	100	88	11.2	14.7	0.02297	81.6
A7	25.1	UNDEV.	0	61	6.0	ROADS	100	88	19.1	25.1	0.03922	81.5
A8	24.7	UNDEV.	0	61	4.5	ROADS	100	88	20.2	24.7	0.03859	83.1
A9	27.5	UNDEV.	0	61	2.8	ROADS	100	88	24.7	27.5	0.04297	85.3
A12	31.1	UNDEV.	0	61	31.1	ROADS	100	88	0.0	31.1	0.04859	61.0
C1	62.5	UNDEV.	0	61	62.5	ROADS	100	88	0.0	62.5	0.09766	61.0
D1	40.4	UNDEV.	0	61	0.0	ROADS	100	88	40.4	40.4	0.06313	88.0
E5	34.3	UNDEV.	0	61	34.3	ROADS	100	88	0.0	34.3	0.05359	61.0
VP-1	1.1	UNDEV.	0	61	0.2	ROADS	100	88	0.9	1.1	0.00172	82.6
IP-1	1.2	UNDEV.	0	61	0.2	ROADS	100	88	1.0	1.2	0.00188	82.6
IP-2	2.5	UNDEV.	0	61	0.5	ROADS	100	88	2.0	2.5	0.00391	82.6
IP-3	2.0	UNDEV.	0	61	0.4	ROADS	100	88	1.6	2.0	0.00313	82.6
IP-4	3.1	UNDEV.	0	61	0.6	ROADS	100	88	2.5	3.1	0.00484	82.6
EK1C	4.8	UNDEV.	0	61	0.0	ROADS	100	88	3.8	3.8	0.00594	88.0
EK1D	8.3	UNDEV.	0	61	0.0	ROADS	100	88	11.6	11.6	0.01813	88.0
EK1M	3.8	UNDEV.	0	61	0.0	ROADS	100	88	3.8	3.8	0.00594	88.0
EK1N	11.6	UNDEV.	0	61	0.0	ROADS	100	88	11.6	11.6	0.01813	88.0
EK1Q	23.8	UNDEV.	0	69	18.8	ROADS	100	88	5.0	23.8	0.03719	73.0
EK1R	20.2	UNDEV.	0	69	4.0	ROADS	0	61	16.2	20.2	0.03156	62.6
EK1S	6.2	UNDEV.	0	69	6.2	ROADS	100	88	0.0	6.2	0.00969	69.0
EK1T	13.5	UNDEV.	0	69	1.5	ROADS	0	61	12.0	13.5	0.02109	61.9
EK2H	39.3	UNDEV.	0	69	3.2	ROADS	0	61	36.1	39.3	0.06141	61.7

TIME OF CONCENTRATION

BASIN	Cn	C(5)	OVERLAND Length (ft)	OVERLAND Height (ft)	OVERLAND Tc (min)	STREET / CHANNEL FLOW Length (ft)	STREET / CHANNEL FLOW Velocity (fps)	STREET / CHANNEL FLOW Tc (min)	Tc TOTAL (min)	Tc TOTAL (hr)	Lagtime TOTAL (hr)
02B	60.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	17.0000	0.2833	0.1700
O1	67.8	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	8.0000	0.1333	0.0800
B1	68.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	12.6000	0.2100	0.1260
B2	75.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	11.4000	0.1900	0.1140
B3	80.0	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	PREV.	13.8000	0.2300	0.1380
B5	69.8	0.25	100	2	12.6	1375	3.0	7.6	20.2839	0.3381	0.2028
B6	61.0	0.25	100	2	12.6	1525	3.0	8.5	21.1173	0.3520	0.2112
B7	67.5	0.25	100	2	12.6	1425	3.0	7.9	20.5617	0.3427	0.2056
B9	88.0	0.75	100	2	5.2	500	3.0	2.8	7.9846	0.1331	0.0798
A1	66.2	0.25	100	2	12.6	2900	3.0	16.1	28.7562	0.4793	0.2876
A2	63.0	0.25	100	2	12.6	3400	3.0	18.9	31.5339	0.5256	0.3153
A3	79.9	0.75	100	2	5.2	1400	3.0	7.8	12.9846	0.2164	0.1298
A4	74.5	0.75	100	2	5.2	2500	3.0	13.9	19.0957	0.3183	0.1910
A5	64.6	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
A6	81.6	0.75	100	2	5.2	600	3.0	3.3	8.5401	0.1423	0.0854
A7	81.5	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
A8	83.1	0.25	100	2	12.6	2400	3.0	13.3	25.9784	0.4330	0.2598
A9	85.3	0.25	100	2	12.6	2000	3.0	11.1	23.7562	0.3959	0.2376
A12	61.0	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
C1	61.0	0.25	100	2	12.6	3350	3.0	18.6	31.2562	0.5209	0.3126
D1	88.0	0.75	100	2	5.2	2000	3.0	11.1	16.3179	0.2720	0.1632
E5	61.0	0.25	100	2	12.6	1700	3.0	9.4	22.0895	0.3682	0.2209
VP-1	82.6	0.75	50	2	2.9	1200	3.0	6.7	9.5956	0.1599	0.0960
IP-1	82.6	0.75	50	2	2.9	1350	3.0	7.5	10.4290	0.1738	0.1043
IP-2	82.6	0.75	50	2	2.9	1350	3.0	7.5	10.4290	0.1738	0.1043
IP-3	82.6	0.75	50	2	2.9	1900	3.0	10.6	13.4845	0.2247	0.1348
IP-4	82.6	0.75	50	2	2.9	1900	3.0	10.6	13.4845	0.2247	0.1348
EK1C	88.0	0.75	50	2	2.9	550	2.0	4.6	7.5123	0.1252	0.0751
EK1D	88.0	0.75	50	2	2.9	1000	2.0	8.3	11.2623	0.1877	0.1126
EK1M	88.0	0.75	50	2	2.9	500	2.0	4.2	7.0956	0.1183	0.0710
EK1N	88.0	0.75	50	2	2.9	950	2.0	7.9	10.8456	0.1808	0.1085
EK1Q	73.0	0.25	100	2	12.6	1700	2.0	14.2	26.8117	0.4469	0.2681
EK1R	62.6	0.25	100	2	12.6	1100	2.0	9.2	21.8117	0.3635	0.2181
EK1S	69.0	0.25	100	2	12.6	800	2.0	6.7	19.3117	0.3219	0.1931
EK1T	61.9	0.25	100	2	12.6	1000	2.0	8.3	20.9784	0.3496	0.2098
EK2H	61.7	0.25	100	2	12.6	1400	2.0	11.7	24.3117	0.4052	0.2431

PREV. = DETERMINED FROM PREVIOUS REPORTS

BASIN SUMMARY - FILING NO. 6 & 7 INTERIM CONDITIONS

BASIN (label)	TOTAL BASIN AREA (acres)	TOTAL BASIN AREA (sq mi)	WEIGHTED CN	TOTAL LAG TIME (hours)	TOTAL Tc (hours)	Q 2 Yr. (cfs)	Q 5 Yr. (cfs)	Q 10 Yr. (cfs)	Q 25 Yr. (cfs)	Q 50 Yr. (cfs)	Q 100 Yr. (cfs)
02B	32.2	0.0503	60.0	0.1700	0.2833	0.25	4.42	9.72	20.10	28.39	37.46
O1	19.2	0.0300	67.8	0.0800	0.1333	4.73	12.73	19.06	29.60	37.17	45.16
B1	15.1	0.0236	68.0	0.1260	0.2100	2.95	8.42	12.92	20.51	26.11	32.02
B2	62.7	0.0980	75.0	0.1140	0.1900	33.20	65.46	89.55	128.36	155.96	184.43
B3	40.0	0.0625	80.0	0.1380	0.2300	31.11	54.19	71.14	97.89	116.35	135.15
B5	11.4	0.0178	69.8	0.2028	0.3381	2.31	6.04	9.10	14.37	18.20	22.25
B6	18.4	0.0288	61.0	0.2112	0.3520	0.21	2.67	5.54	11.14	15.58	20.47
B7	17.8	0.0278	67.5	0.2056	0.3427	2.49	7.57	11.92	19.51	25.14	31.13
B9	4.1	0.0064	88.0	0.0798	0.1331	6.22	9.25	11.31	14.42	16.50	18.59
A1	56.9	0.0889	66.2	0.2876	0.4793	3.94	14.97	25.19	43.78	57.91	73.26
A2	108.0	0.1688	63.0	0.3153	0.5256	2.72	16.85	31.66	60.16	82.61	107.29
A3	25.0	0.0391	79.9	0.1298	0.2164	19.83	34.56	45.24	62.06	73.64	85.49
A4	4.0	0.0063	74.5	0.1910	0.3183	1.65	3.38	4.72	6.92	8.47	10.09
A5	6.7	0.0105	64.6	0.2209	0.3682	2.83	5.67	7.84	11.43	13.98	16.61
A6	14.7	0.0230	81.6	0.0854	0.1423	15.15	24.83	31.72	42.41	49.67	57.02
A7	25.1	0.0392	81.5	0.2209	0.3682	14.32	26.33	35.34	49.80	59.91	70.36
A8	24.7	0.0386	83.1	0.2598	0.4330	18.22	30.97	40.16	54.82	64.93	75.24
A9	27.5	0.0430	85.3	0.2376	0.3682	25.01	40.67	51.88	69.34	81.26	93.39
A12	31.1	0.0486	61.0	0.2209	0.3682	0.35	4.38	9.09	18.32	25.61	33.67
C1	62.5	0.0977	61.0	0.3126	0.5209	0.67	6.74	14.07	28.80	40.69	53.92
A14	38.4	0.0600	61.0	0.2376	0.3959	53.05	80.73	99.82	129.16	148.88	168.68
E5	34.3	0.0536	61.0	0.2209	0.3682	0.38	4.53	9.49	19.29	26.99	35.43
VP-1	40.4	0.0631	88.0	0.1632	0.2720	1.19	1.92	2.43	3.24	3.78	4.33
E5	34.3	0.0536	61.0	0.2209	0.3682	1.27	2.07	2.63	3.49	4.08	4.68
E7	79.2	0.1238	61.0	0.2487	0.4145	2.65	4.30	5.47	7.27	8.49	9.74
IP-3	2.0	0.0031	82.6	0.1348	0.2247	1.91	3.16	4.04	5.40	6.34	7.28
IP-4	3.1	0.0048	82.6	0.1348	0.2247	2.97	4.89	6.26	8.37	9.82	11.28
EK1C	4.8	0.0075	88.0	0.0751	0.1252	7.34	10.89	13.30	16.94	19.39	21.84
EK1D	8.3	0.0130	88.0	0.1126	0.1877	11.95	17.95	22.04	28.27	32.47	36.68
EK1M	3.8	0.0059	88.0	0.0710	0.1183	5.84	8.65	10.56	13.46	15.40	17.35
EK1N	11.6	0.0181	88.0	0.1085	0.1808	16.82	25.22	30.94	39.72	45.59	51.47
EK1Q	23.8	0.0372	73.0	0.2681	0.4469	6.15	14.01	20.29	30.93	38.62	46.75
EK1R	20.2	0.0316	62.6	0.2181	0.3635	0.65	4.17	7.75	14.47	19.68	25.37
EK1S	6.2	0.0097	69.0	0.1931	0.3219	1.10	3.05	4.69	7.50	9.58	11.78
EK1T	13.5	0.0211	61.9	0.2098	0.3496	0.28	2.41	4.69	9.05	12.46	16.18
EK2H	39.3	0.0614	61.7	0.2431	0.4052	0.70	5.96	11.66	22.85	31.76	41.55

CONVEYANCE CHART - FILING NO. 6 & 7 INTERIM CONDITIONS

Conveyance Label	Type	Length ft.	Bottom Width ft.	Slope %	Side Slope H to 1	Manning's N value	Available Depth ft.	Q 5 Yr. cfs	Q 100 Yr. cfs	Q Capacity cfs
C1	36" RCP	2650	3.0	0.0200	N/A	0.013	N/A	10.55	42.84	94.32
C2	48" RCP	1375	4.0	0.0225	N/A	0.013	N/A	39.25	98.20	215.45
C3	GRASSLINED SWALE	3475	6.0	0.0230	3.0	0.035	2.000	1.57	24.70	169.57
C4	54" RCP	1670	4.5	0.0186	N/A	0.013	N/A	55.36	168.19	268.18
C5	54" RCP	450	4.5	0.0200	N/A	0.013	N/A	58.56	181.40	278.09
C6	54" RCP	550	4.5	0.0200	N/A	0.013	N/A	64.45	203.36	278.09
C7	60" RCP	600	5.0	0.0217	N/A	0.013	N/A	74.53	254.85	383.63
C9	GRASSLINED SWALE	1800	6.0	0.0167	3.0	0.035	3.000	12.35	67.91	365.58
C10	CONCRETE CHANNEL	1100	6.0	0.0050	1.5	0.015	5.000	28.06	172.86	1297.29
C11	42" RCP	2200	3.5	0.0177	N/A	0.013	N/A	6.37	52.96	133.85
C12	48" RCP	1050	4.0	0.0062	N/A	0.013	N/A	68.34	171.89	113.10
C13	CONCRETE CHANNEL	1750	6.0	0.0271	1.5	0.015	5.000	101.91	367.88	2191.56
C14	CONCRETE CHANNEL	600	6.0	0.0200	1.5	0.015	5.000	110.59	387.77	1882.71
C15	30" RCP	800	2.5	0.0188	N/A	0.013	N/A	30.97	75.24	56.24
C18	GRASS / RIPRAP SWALE	700	20.0	0.0257	2.0	0.035	3.000	28.62	128.01	934.14
C19	GRASSLINED SWALE	1850	10.0	0.0189	10.0	0.035	1.500	1.07	11.72	209.13
C20	GRASSLINED SWALE	750	10.0	0.0227	10.0	0.035	2.000	39.25	40.23	432.24
C21	30" RCP	650	2.5	0.0292	N/A	0.013	N/A	7.87	18.48	70.09
C22	GRASSLINED SWALE	1150	6.0	0.0182	2.0	0.035	1.500	6.96	17.56	80.50
C31	60" RCP	880	5.0	0.0306	N/A	0.013	N/A	77.01	263.79	455.56
C32	30" RCP	1300	2.5	0.0050	N/A	0.013	N/A	10.49	21.43	29.00
C33	42" RCP	750	3.5	0.0050	N/A	0.013	N/A	28.14	57.74	71.14
C34	78" RCP	650	6.0	0.0125	N/A	0.013	N/A	111.36	358.73	473.47
C37	84" RCP	880	7.0	0.0125	N/A	0.013	N/A	115.12	383.38	714.20
C42	54" RCP	150	4.5	0.0200	N/A	0.013	N/A	71.50	168.43	278.09
C43	48" RCP	500	4.0	0.0050	N/A	0.013	N/A	26.33	70.36	101.57
C44	54" RCP	250	4.5	0.0320	N/A	0.013	N/A	100.64	246.26	351.76
C45	54" RCP	100	4.5	0.0320	N/A	0.013	N/A	121.22	291.99	351.76

DESIGN POINTS - FILING NO. 6 & 7 INTERIM CONDITIONS

BASIN (label)	Q 2 Yr. Q (cfs) Ponding (ft)	Q 5 Yr. Q (cfs) Ponding (ft)	Q 10 Yr. Q (cfs) Ponding (ft)	Q 25 Yr. Q (cfs) Ponding (ft)	Q 50 Yr. Q (cfs) Ponding (ft)	Q 100 Yr. Q (cfs) Ponding (ft)
PCC IN	33.20	65.46	89.55	128.36	155.96	184.43
PCC OUT	14.72 / 6758.26	31.59 / 6759.71	39.19 / 6760.42	49.69 / 6761.47	56.38 / 6762.09	60.98 / 6762.40
1	17.26	39.67	53.49	72.12	85.95	98.98
NLC IN	31.11	54.19	71.14	97.89	116.35	135.15
NLC OUT	5.01 / 6737.07	11.97 / 6737.79	17.07 / 6738.38	23.49 / 6739.25	26.23 / 6739.71	28.62 / 6740.21
2	22.93	56.08	79.77	116.78	143.70	169.78
3	23.78	58.66	84.60	124.29	153.19	181.69
4	25.89	64.51	93.58	138.51	171.10	203.65
5	28.41	74.53	110.84	168.83	211.43	254.85
25	29.46	77.01	114.76	174.91	218.66	263.79
26	18.86	28.44	34.95	44.81	51.42	58.08
27	47.80	111.36	162.25	242.11	299.75	358.73
29	48.24	115.12	169.38	255.79	318.67	383.38
EK1B IN	53.05	80.73	99.82	129.16	148.88	168.68
EK1B OUT	42.25 / 6692.84	65.96 / 6693.83	79.25 / 6694.46	100.04 / 6695.50	112.94 / 6696.20	123.14 / 6696.88
10	42.29	68.34	87.64	122.15	148.01	171.89
8	5.41	28.74	52.84	98.75	134.88	174.40
9	58.70	102.56	146.44	233.23	300.33	368.62
11	62.20	110.74	156.47	246.73	316.26	387.77
12	43.06	71.50	91.85	123.85	145.96	168.43
33	59.04	100.64	130.85	178.95	212.27	246.26
34	72.05	121.22	156.95	213.35	252.28	291.99
7	189.59	357.91	492.95	717.34	885.01	1058.41
EK1A IN	189.59	357.91	492.95	717.34	885.01	1058.41
EK1A OUT	8.27 / 6614.37	28.63 / 6615.52	48.47 / 6616.29	87.91 / 6617.54	115.88 / 6618.39	128.02 / 6619.37
13	8.27	28.63	48.47	87.91	115.88	128.02
14	8.47	29.50	50.10	90.91	120.22	133.05
I25-1 IN	8.47	29.50	50.10	90.91	120.22	133.05
I25-1 OUT	8.47 / 6590.04	29.47 / 6591.04	50.09 / 6591.73	90.38 / 6592.85	119.55 / 6593.60	132.77 / 6593.93
20	8.47	29.47	50.09	90.38	119.55	132.77
15	4.88	8.05	10.29	13.78	16.16	18.56
16	4.71	7.87	10.12	13.64	16.04	18.48
17	4.39	15.28	27.49	51.38	70.19	90.65
I25-2 IN	4.39	15.28	27.49	51.38	70.19	90.65
I25-2 OUT	3.98 / 6599.94	7.38 / 6600.34	11.57 / 6600.78	19.81 / 6601.62	23.55 / 6602.14	25.87 / 6602.51
19	3.98	7.38	11.57	19.81	23.55	25.87

FILING NO. 6 & 7 INTERIM CONDITIONS DETENTION POND SUMMARY

POND (label)	RISER DIMENSIONS (feet)	RISER CREST ELEV. (feet)	OUTLET PIPE DIAMETER (inches)	OUTLET PIPE INVERT (feet)	OUTLET PIPE SLOPE (%)	INVERT OF POND (feet)	INVERT OF EMERG. SPILLWAY (feet)	LENGTH OF EMERG. SPILLWAY (feet)	EMERGENCY SPILLWAY FLOW DEPTH (feet)	FREE-BOARD DEPTH (feet)	CREST OF DAM (feet)	100 YR. PONDING VOLUME (acre-feet)
EK-1A	4.0' x 3.0'	6614.00	54" RCP	6609.0	0.8	6609.00	6623.50	200	1.4	3.1	6628.00	51.160

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

POND PACK OUTPUT

FILING NO. 6 & 7 CONDITIONS

MASTER DESIGN STORM SUMMARY

Network Storm Collection: C Springs

Return Event	Total Depth in	Rainfall Type	RNF ID
2	2.0000	Synthetic Curve	TYPEIIA 24HR
5	2.6000	Synthetic Curve	TYPEIIA 24HR
10	3.0000	Synthetic Curve	TYPEIIA 24HR
25	3.6000	Synthetic Curve	TYPEIIA 24HR
50	4.0000	Synthetic Curve	TYPEIIA 24HR
100	4.4000	Synthetic Curve	TYPEIIA 24HR

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A1	AREA	2	.728		6.2625	3.94		
A1	AREA	5	1.738		6.2291	14.97		
A1	AREA	10	2.583		6.2124	25.19		
A1	AREA	25	4.055		6.1957	43.78		
A1	AREA	50	5.149		6.1957	57.91		
A1	AREA	100	6.318		6.1790	73.26		
A12	AREA	2	.190		6.5965	.35		
A12	AREA	5	.586		6.1790	4.38		
A12	AREA	10	.946		6.1623	9.09		
A12	AREA	25	1.602		6.1456	18.32		
A12	AREA	50	2.106		6.1456	25.61		
A12	AREA	100	2.654		6.1289	33.67		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A2	AREA	2	.915		6.3460	2.72		
A2	AREA	5	2.505		6.2792	16.85		
A2	AREA	10	3.895		6.2458	31.66		
A2	AREA	25	6.380		6.2291	60.16		
A2	AREA	50	8.260		6.2291	82.61		
A2	AREA	100	10.291		6.2124	107.29		
A3	AREA	2	1.172		6.0621	19.83		
A3	AREA	5	1.997		6.0454	34.56		
A3	AREA	10	2.604		6.0454	45.24		
A3	AREA	25	3.575		6.0454	62.06		
A3	AREA	50	4.254		6.0454	73.64		
A3	AREA	100	4.951		6.0287	85.49		
A4	AREA	2	.127		6.1122	1.65		
A4	AREA	5	.237		6.0955	3.38		
A4	AREA	10	.320		6.0955	4.72		
A4	AREA	25	.458		6.0955	6.92		
A4	AREA	50	.556		6.0955	8.47		
A4	AREA	100	.657		6.0788	10.09		
A5	AREA	2	.231		6.1456	2.83		
A5	AREA	5	.422		6.1289	5.67		
A5	AREA	10	.567		6.1122	7.84		
A5	AREA	25	.803		6.1122	11.43		
A5	AREA	50	.971		6.1122	13.98		
A5	AREA	100	1.145		6.1122	16.61		
A6	AREA	2	.795		6.0287	15.15		
A6	AREA	5	1.313		6.0120	24.83		
A6	AREA	10	1.689		6.0120	31.72		
A6	AREA	25	2.285		6.0120	42.41		
A6	AREA	50	2.699		6.0120	49.67		
A6	AREA	100	3.122		6.0120	57.02		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
A7	AREA	2	1.093		6.1289	14.32		
A7	AREA	5	1.893		6.1122	26.33		
A7	AREA	10	2.486		6.1122	35.34		
A7	AREA	25	3.439		6.1122	49.80		
A7	AREA	50	4.107		6.1122	59.91		
A7	AREA	100	4.796		6.0955	70.36		
A8	AREA	2	1.431		6.1456	18.22		
A8	AREA	5	2.330		6.1456	30.97		
A8	AREA	10	2.978		6.1289	40.16		
A8	AREA	25	3.999		6.1289	54.82		
A8	AREA	50	4.706		6.1289	64.93		
A8	AREA	100	5.428		6.1289	75.24		
A9	AREA	2	1.822		6.1289	25.01		
A9	AREA	5	2.884		6.1122	40.67		
A9	AREA	10	3.640		6.1122	51.88		
A9	AREA	25	4.821		6.1122	69.34		
A9	AREA	50	5.632		6.1122	81.26		
A9	AREA	100	6.458		6.0955	93.39		
B1	AREA	2	.245		6.0788	2.95		
B1	AREA	5	.544		6.0621	8.42		
B1	AREA	10	.788		6.0621	12.92		
B1	AREA	25	1.208		6.0621	20.51		
B1	AREA	50	1.516		6.0454	26.11		
B1	AREA	100	1.844		6.0454	32.02		
B2	AREA	2	1.990		6.0454	33.20		
B2	AREA	5	3.708		6.0454	65.46		
B2	AREA	10	5.020		6.0454	89.55		
B2	AREA	25	7.174		6.0454	128.36		
B2	AREA	50	8.708		6.0287	155.96		
B2	AREA	100	10.305		6.0287	184.43		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
B3	AREA	2	1.875		6.0621	31.11		
B3	AREA	5	3.196		6.0621	54.19		
B3	AREA	10	4.167		6.0454	71.14		
B3	AREA	25	5.720		6.0454	97.89		
B3	AREA	50	6.806		6.0454	116.35		
B3	AREA	100	7.922		6.0454	135.15		
B5	AREA	2	.229		6.1456	2.31		
B5	AREA	5	.479		6.1289	6.04		
B5	AREA	10	.679		6.1122	9.10		
B5	AREA	25	1.017		6.1122	14.37		
B5	AREA	50	1.263		6.1122	18.20		
B5	AREA	100	1.523		6.0955	22.25		
B6	AREA	2	.112		6.2625	.21		
B6	AREA	5	.347		6.1623	2.67		
B6	AREA	10	.560		6.1456	5.54		
B6	AREA	25	.948		6.1456	11.14		
B6	AREA	50	1.246		6.1289	15.58		
B6	AREA	100	1.570		6.1289	20.47		
B7	AREA	2	.288		6.1623	2.49		
B7	AREA	5	.641		6.1289	7.57		
B7	AREA	10	.929		6.1289	11.92		
B7	AREA	25	1.424		6.1122	19.51		
B7	AREA	50	1.787		6.1122	25.14		
B7	AREA	100	2.173		6.1122	31.13		
B9	AREA	2	.330		6.0120	6.22		
B9	AREA	5	.501		6.0120	9.25		
B9	AREA	10	.621		6.0120	11.31		
B9	AREA	25	.806		6.0120	14.42		
B9	AREA	50	.932		6.0120	16.50		
B9	AREA	100	1.060		5.9953	18.59		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
C1	AREA	2	.381		6.6800	.67		
C1	AREA	5	1.179		6.2792	6.74		
C1	AREA	10	1.902		6.2625	14.07		
C1	AREA	25	3.220		6.2458	28.80		
C1	AREA	50	4.232		6.2291	40.69		
C1	AREA	100	5.333		6.2291	53.92		
D1	AREA	2	3.250		6.0621	53.05		
D1	AREA	5	4.940		6.0621	80.73		
D1	AREA	10	6.121		6.0454	99.82		
D1	AREA	25	7.946		6.0454	129.16		
D1	AREA	50	9.187		6.0454	148.88		
D1	AREA	100	10.444		6.0454	168.68		
DP 1	JCT	2	2.301		6.1456	17.26		
DP 1	JCT	5	4.400		6.1122	39.67		
DP 1	JCT	10	6.023		6.0788	53.49		
DP 1	JCT	25	8.710		6.0788	72.12		
DP 1	JCT	50	10.636		6.0788	85.95		
DP 1	JCT	100	12.650		6.0621	98.98		
DP 10	JCT	2	3.620		6.1456	42.29		
DP 10	JCT	5	6.109		6.1623	68.34		
DP 10	JCT	10	8.013		6.1790	87.64		
DP 10	JCT	25	11.156		6.1957	122.15		
DP 10	JCT	50	13.409		6.2124	148.01		
DP 10	JCT	100	15.767		6.2124	171.89		
DP 11	JCT	2	6.793		6.1289	62.20		
DP 11	JCT	5	13.008		6.1456	110.74		
DP 11	JCT	10	17.982		6.1957	156.47		
DP 11	JCT	25	26.426		6.2124	246.73		
DP 11	JCT	50	32.597		6.2124	316.26		
DP 11	JCT	100	39.129		6.2124	387.77		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 14	JCT	2	16.642	R	12.8757	8.47		
DP 14	JCT	5	33.110	R	8.1830	29.50		
DP 14	JCT	10	46.336	R	7.7989	50.10		
DP 14	JCT	25	68.585	R	7.2812	90.91		
DP 14	JCT	50	84.706	R	7.0975	120.22		
DP 14	JCT	100	101.668	R	7.0975	133.05		
DP 2	JCT	2	4.583		6.1790	22.93		
DP 2	JCT	5	8.681		6.1289	56.08		
DP 2	JCT	10	11.871		6.1122	79.77		
DP 2	JCT	25	17.180		6.0955	116.78		
DP 2	JCT	50	21.001		6.0955	143.70		
DP 2	JCT	100	25.007		6.0788	169.78		
*DP 20	JCT	2	16.639	R	12.9425	8.47		
*DP 20	JCT	5	33.106	R	8.2498	29.47		
*DP 20	JCT	10	46.332	R	7.8657	50.09		
*DP 20	JCT	25	68.584	R	7.4983	90.38		
*DP 20	JCT	50	84.705	R	7.3313	119.55		
*DP 20	JCT	100	101.667	R	7.2311	132.77		
DP 25	JCT	2	5.819		6.1957	29.46		
DP 25	JCT	5	11.102		6.1456	77.01		
DP 25	JCT	10	15.238		6.1289	114.76		
DP 25	JCT	25	22.152		6.1122	174.91		
DP 25	JCT	50	27.144		6.1122	218.66		
DP 25	JCT	100	32.388		6.0955	263.79		
DP 26	JCT	2	1.054		6.0287	18.86		
DP 26	JCT	5	1.602		6.0287	28.44		
DP 26	JCT	10	1.985		6.0287	34.95		
DP 26	JCT	25	2.576		6.0287	44.81		
DP 26	JCT	50	2.979		6.0120	51.42		
DP 26	JCT	100	3.387		6.0120	58.08		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 27	JCT	2	7.508		6.1122	47.80		
DP 27	JCT	5	13.938		6.1122	111.36		
DP 27	JCT	10	18.924		6.0955	162.25		
DP 27	JCT	25	27.202		6.0955	242.11		
DP 27	JCT	50	33.153		6.0955	299.75		
DP 27	JCT	100	39.386		6.0955	358.73		
DP 29	JCT	2	7.679		6.1122	48.24		
DP 29	JCT	5	14.407		6.1122	115.12		
DP 29	JCT	10	19.652		6.1122	169.38		
DP 29	JCT	25	28.396		6.0955	255.79		
DP 29	JCT	50	34.698		6.0955	318.67		
DP 29	JCT	100	41.310		6.0955	383.38		
DP 3	JCT	2	4.860		6.1957	23.78		
DP 3	JCT	5	9.134		6.1456	58.66		
DP 3	JCT	10	12.449		6.1122	84.60		
DP 3	JCT	25	17.957		6.1122	124.29		
DP 3	JCT	50	21.916		6.0955	153.19		
DP 3	JCT	100	26.062		6.0955	181.69		
DP 33	JCT	2	4.651		6.1289	59.04		
DP 33	JCT	5	7.572		6.1122	100.64		
DP 33	JCT	10	9.679		6.1122	130.85		
DP 33	JCT	25	13.007		6.0955	178.95		
DP 33	JCT	50	15.309		6.0955	212.27		
DP 33	JCT	100	17.664		6.0955	246.26		
DP 34	JCT	2	5.585		6.0788	72.05		
DP 34	JCT	5	8.991		6.0621	121.22		
DP 34	JCT	10	11.436		6.0621	156.95		
DP 34	JCT	25	15.288		6.0621	213.35		
DP 34	JCT	50	17.947		6.0621	252.28		
DP 34	JCT	100	20.663		6.0621	291.99		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 4	JCT	2	5.089		6.1957	25.89		
DP 4	JCT	5	9.612		6.1456	64.51		
DP 4	JCT	10	13.128		6.1289	93.58		
DP 4	JCT	25	18.974		6.1122	138.51		
DP 4	JCT	50	23.179		6.1122	171.10		
DP 4	JCT	100	27.585		6.0955	203.65		
DP 5	JCT	2	5.490		6.1957	28.41		
DP 5	JCT	5	10.600		6.1623	74.53		
DP 5	JCT	10	14.617		6.1289	110.84		
DP 5	JCT	25	21.346		6.1289	168.83		
DP 5	JCT	50	26.212		6.1122	211.43		
DP 5	JCT	100	31.328		6.1122	254.85		
DP 7	JCT	2	20.964		6.0955	189.59		
DP 7	JCT	5	37.960		6.0955	357.91		
DP 7	JCT	10	51.106		6.0955	492.95		
DP 7	JCT	25	72.919		6.0955	717.34		
DP 7	JCT	50	88.594		6.0955	885.01		
DP 7	JCT	100	105.017		6.0955	1058.41		
DP 8	JCT	2	1.643		6.4128	5.41		
DP 8	JCT	5	4.243		6.3126	28.74		
DP 8	JCT	10	6.478		6.2792	52.84		
DP 8	JCT	25	10.435		6.2625	98.75		
DP 8	JCT	50	13.408		6.2458	134.88		
DP 8	JCT	100	16.609		6.2291	174.40		
DP 9	JCT	2	6.435		6.0955	58.70		
DP 9	JCT	5	12.349		6.1289	102.56		
DP 9	JCT	10	17.095		6.1957	146.44		
DP 9	JCT	25	25.165		6.2124	233.23		
DP 9	JCT	50	31.071		6.2124	300.33		
DP 9	JCT	100	37.327		6.2124	368.62		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
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Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP12		JCT	2	3.253		6.1456	43.06		
DP12		JCT	5	5.214		6.1289	71.50		
DP12		JCT	10	6.617		6.1289	91.85		
DP12		JCT	25	8.820		6.1122	123.85		
DP12		JCT	50	10.338		6.1122	145.96		
DP12		JCT	100	11.886		6.1122	168.43		
DP13		JCT	2	16.359	R	12.8590	8.27		
DP13		JCT	5	32.245	R	8.2832	28.63		
DP13		JCT	10	44.946	R	7.8323	48.47		
DP13		JCT	25	66.241	R	7.3480	87.91		
DP13		JCT	50	81.631	R	7.2812	115.88		
DP13		JCT	100	97.799	R	7.3814	128.02		
EK1A	IN	POND	2	20.964		6.0955	189.59		
EK1A	IN	POND	5	37.960		6.0955	357.91		
EK1A	IN	POND	10	51.106		6.0955	492.95		
EK1A	IN	POND	25	72.919		6.0955	717.34		
EK1A	IN	POND	50	88.594		6.0955	885.01		
EK1A	IN	POND	100	105.017		6.0955	1058.41		
EK1A	OUT	POND	2	16.359	R	12.8590	8.27	6614.37	11.868
EK1A	OUT	POND	5	32.245	R	8.2832	28.63	6615.52	19.244
EK1A	OUT	POND	10	44.946	R	7.8323	48.47	6616.29	25.033
EK1A	OUT	POND	25	66.241	R	7.3480	87.91	6617.54	35.114
EK1A	OUT	POND	50	81.631	R	7.2812	115.88	6618.39	42.425
EK1A	OUT	POND	100	97.799	R	7.3814	128.02	6619.37	51.156
EK1B	IN	POND	2	3.250		6.0621	53.05		
EK1B	IN	POND	5	4.940		6.0621	80.73		
EK1B	IN	POND	10	6.121		6.0454	99.82		
EK1B	IN	POND	25	7.946		6.0454	129.16		
EK1B	IN	POND	50	9.187		6.0454	148.88		
EK1B	IN	POND	100	10.444		6.0454	168.68		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK1B	OUT POND	2	3.240		6.1456	42.25	6692.84	.553
EK1B	OUT POND	5	4.930		6.1456	65.96	6693.83	.850
EK1B	OUT POND	10	6.111		6.1456	79.25	6694.46	1.080
EK1B	OUT POND	25	7.935		6.1456	100.04	6695.50	1.486
EK1B	OUT POND	50	9.177		6.1456	112.94	6696.20	1.778
EK1B	OUT POND	100	10.434		6.1623	123.14	6696.88	2.118
EK1C	AREA	2	.386		6.0120	7.34		
EK1C	AREA	5	.587		6.0120	10.89		
EK1C	AREA	10	.727		6.0120	13.30		
EK1C	AREA	25	.944		5.9953	16.94		
EK1C	AREA	50	1.092		5.9953	19.39		
EK1C	AREA	100	1.241		5.9953	21.84		
EK1D	AREA	2	.668		6.0287	11.95		
EK1D	AREA	5	1.015		6.0287	17.95		
EK1D	AREA	10	1.258		6.0287	22.04		
EK1D	AREA	25	1.632		6.0120	28.27		
EK1D	AREA	50	1.887		6.0120	32.47		
EK1D	AREA	100	2.146		6.0120	36.68		
EK1M	AREA	2	.306		6.0120	5.84		
EK1M	AREA	5	.465		6.0120	8.65		
EK1M	AREA	10	.576		6.0120	10.56		
EK1M	AREA	25	.747		5.9953	13.46		
EK1M	AREA	50	.864		5.9953	15.40		
EK1M	AREA	100	.982		5.9953	17.35		
EK1N	AREA	2	.933		6.0287	16.82		
EK1N	AREA	5	1.419		6.0287	25.22		
EK1N	AREA	10	1.758		6.0120	30.94		
EK1N	AREA	25	2.281		6.0120	39.72		
EK1N	AREA	50	2.638		6.0120	45.59		
EK1N	AREA	100	2.999		6.0120	51.48		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
EK1Q		AREA	2	.635		6.1957	6.15		
EK1Q		AREA	5	1.235		6.1790	14.01		
EK1Q		AREA	10	1.700		6.1623	20.29		
EK1Q		AREA	25	2.474		6.1623	30.93		
EK1Q		AREA	50	3.029		6.1623	38.62		
EK1Q		AREA	100	3.611		6.1456	46.75		
EK1R		AREA	2	.171		6.2124	.65		
EK1R		AREA	5	.469		6.1623	4.17		
EK1R		AREA	10	.729		6.1456	7.75		
EK1R		AREA	25	1.193		6.1456	14.47		
EK1R		AREA	50	1.545		6.1289	19.68		
EK1R		AREA	100	1.925		6.1289	25.37		
EK1S		AREA	2	.112		6.1456	1.10		
EK1S		AREA	5	.241		6.1122	3.05		
EK1S		AREA	10	.346		6.1122	4.69		
EK1S		AREA	25	.524		6.1122	7.50		
EK1S		AREA	50	.654		6.0955	9.58		
EK1S		AREA	100	.792		6.0955	11.78		
EK1T		AREA	2	.098		6.2124	.28		
EK1T		AREA	5	.283		6.1623	2.41		
EK1T		AREA	10	.448		6.1456	4.69		
EK1T		AREA	25	.746		6.1289	9.05		
EK1T		AREA	50	.972		6.1289	12.46		
EK1T		AREA	100	1.218		6.1289	16.18		
I25-1	IN	POND	2	16.642	R	12.8757	8.47		
I25-1	IN	POND	5	33.110	R	8.1830	29.50		
I25-1	IN	POND	10	46.336	R	7.7989	50.10		
I25-1	IN	POND	25	68.585	R	7.2812	90.91		
I25-1	IN	POND	50	84.706	R	7.0975	120.22		
I25-1	IN	POND	100	101.668	R	7.0975	133.05		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
I25-1	OUT POND	2	16.639	R	12.9425	8.47	6590.04	.027
I25-1	OUT POND	5	33.106	R	8.2498	29.47	6591.04	.180
I25-1	OUT POND	10	46.332	R	7.8657	50.09	6591.73	.288
I25-1	OUT POND	25	68.584	R	7.4983	90.38	6592.85	.779
I25-1	OUT POND	50	84.705	R	7.3313	119.55	6593.60	1.170
I25-1	OUT POND	100	101.667	R	7.2311	132.77	6593.93	1.343
IP-1	AREA	2	.070		6.0287	1.27		
IP-1	AREA	5	.113		6.0287	2.07		
IP-1	AREA	10	.145		6.0287	2.63		
IP-1	AREA	25	.194		6.0287	3.49		
IP-1	AREA	50	.229		6.0120	4.08		
IP-1	AREA	100	.264		6.0120	4.68		
IP-2	AREA	2	.145		6.0287	2.65		
IP-2	AREA	5	.236		6.0287	4.30		
IP-2	AREA	10	.301		6.0287	5.47		
IP-2	AREA	25	.405		6.0287	7.27		
IP-2	AREA	50	.476		6.0120	8.49		
IP-2	AREA	100	.549		6.0120	9.74		
NLC	IN POND	2	1.875		6.0621	31.11		
NLC	IN POND	5	3.196		6.0621	54.19		
NLC	IN POND	10	4.167		6.0454	71.14		
NLC	IN POND	25	5.720		6.0454	97.89		
NLC	IN POND	50	6.806		6.0454	116.35		
NLC	IN POND	100	7.922		6.0454	135.15		
NLC	OUT POND	2	1.875		6.3794	5.01	6737.07	.708
NLC	OUT POND	5	3.195		6.2959	11.97	6737.79	1.267
NLC	OUT POND	10	4.166		6.2792	17.07	6738.38	1.691
NLC	OUT POND	25	5.720		6.2792	23.49	6739.25	2.405
NLC	OUT POND	50	6.805		6.2792	26.23	6739.71	2.950
NLC	OUT POND	100	7.921		6.2959	28.62	6740.21	3.531

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID		Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
O1		AREA	2	.311		6.0454	4.73		
O1		AREA	5	.692		6.0287	12.73		
O1		AREA	10	1.003		6.0287	19.06		
O1		AREA	25	1.536		6.0287	29.60		
O1		AREA	50	1.928		6.0287	37.17		
O1		AREA	100	2.344		6.0120	45.16		
O2B		AREA	2	.163		6.5965	.25		
O2B		AREA	5	.543		6.1289	4.42		
O2B		AREA	10	.894		6.1122	9.72		
O2B		AREA	25	1.543		6.0955	20.10		
O2B		AREA	50	2.044		6.0955	28.39		
O2B		AREA	100	2.593		6.0955	37.46		
PCC	IN	POND	2	1.990		6.0454	33.20		
PCC	IN	POND	5	3.708		6.0454	65.46		
PCC	IN	POND	10	5.020		6.0454	89.55		
PCC	IN	POND	25	7.174		6.0454	128.36		
PCC	IN	POND	50	8.708		6.0287	155.96		
PCC	IN	POND	100	10.305		6.0287	184.43		
PCC	OUT	POND	2	1.990		6.1790	14.72	6758.26	.420
PCC	OUT	POND	5	3.708		6.1623	31.59	6759.71	.903
PCC	OUT	POND	10	5.020		6.1623	39.19	6760.42	1.326
PCC	OUT	POND	25	7.174		6.1790	49.69	6761.47	2.146
PCC	OUT	POND	50	8.708		6.1790	56.38	6762.09	2.775
PCC	OUT	POND	100	10.305		6.1957	60.98	6762.40	3.488
VP-1		AREA	2	.064		6.0287	1.19		
VP-1		AREA	5	.104		6.0287	1.92		
VP-1		AREA	10	.133		6.0120	2.43		
VP-1		AREA	25	.178		6.0120	3.24		
VP-1		AREA	50	.210		6.0120	3.78		
VP-1		AREA	100	.242		6.0120	4.33		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = C Springs

Storm Tag Name = 5

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 5 yr
Total Rainfall Depth= 2.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	3.708		6.0454	65.46	B2	
		DL	3.708		6.0454	65.46		
		DN	3.708		6.0454	65.46	PCC	IN
ADDLINK 100	ADD	UN	.501		6.0120	9.25	B9	
		DL	.501		6.0120	9.25		
		DN	11.102		6.1456	77.01	DP 25	
ADDLINK 120	ADD	UN	1.313		6.0120	24.83	A6	
		DL	1.313		6.0120	24.83		
		DN	37.960		6.0955	357.91	DP 7	
ADDLINK 130	ADD	UN	2.505		6.2792	16.85	A2	
		DL	2.505		6.2792	16.85		
		DN	4.243		6.3126	28.74	DP 8	
ADDLINK 140	ADD	UN	4.940		6.0621	80.73	D1	
		DL	4.940		6.0621	80.73		
		DN	4.940		6.0621	80.73	EK1B	IN
ADDLINK 150	ADD	UN	1.997		6.0454	34.56	A3	
		DL	1.997		6.0454	34.56		
		DN	12.349		6.1289	102.56	DP 9	
ADDLINK 160	ADD	UN	.237		6.0955	3.38	A4	
		DL	.237		6.0955	3.38		
		DN	13.008		6.1456	110.74	DP 11	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 170	ADD	UN	.422		6.1289	5.67	A5	
		DL	.422		6.1289	5.67		
		DN	13.008		6.1456	110.74	DP 11	
ADDLINK 180	ADD	UN	2.884		6.1122	40.67	A9	
		DL	2.884		6.1122	40.67		
		DN	5.214		6.1289	71.50	DP12	
ADDLINK 190	ADD	UN	37.960		6.0955	357.91	DP 7	
		DL	37.960		6.0955	357.91		
		DN	37.960		6.0955	357.91	EK1A	IN
ADDLINK 20	ADD	UN	3.196		6.0621	54.19	B3	
		DL	3.196		6.0621	54.19		
		DN	3.196		6.0621	54.19	NLC	IN
ADDLINK 200	ADD	UN	.586		6.1790	4.38	A12	
		DL	.586		6.1790	4.38		
		DN	33.110	R	8.1830	29.50	DP 14	
ADDLINK 210	ADD	UN	33.110	R	8.1830	29.50	DP 14	
		DL	33.110	R	8.1830	29.50		
		DN	33.110	R	8.1830	29.50	I25-1	IN
ADDLINK 220	ADD	UN	1.235		6.1790	14.01	EK1Q	
		DL	1.235		6.1790	14.01		
		DN	13.938		6.1122	111.36	DP 27	
ADDLINK 230	ADD	UN	1.015		6.0287	17.95	EK1D	
		DL	1.015		6.0287	17.95		
		DN	1.602		6.0287	28.44	DP 26	
ADDLINK 240	ADD	UN	.469		6.1623	4.17	EK1R	
		DL	.469		6.1623	4.17		
		DN	14.407		6.1122	115.12	DP 29	
ADDLINK 250	ADD	UN	.465		6.0120	8.65	EK1M	
		DL	.465		6.0120	8.65		
		DN	7.572		6.1122	100.64	DP 33	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 260	ADD	UN	1.419		6.0287	25.22	EK1N
		DL	1.419		6.0287	25.22	
		DN	8.991		6.0621	121.22	DP 34
ADDLINK 270	ADD	UN	.241		6.1122	3.05	EK1S
		DL	.241		6.1122	3.05	
		DN	37.960		6.0955	357.91	DP 7
ADDLINK 30	ADD	UN	.544		6.0621	8.42	B1
		DL	.544		6.0621	8.42	
		DN	8.681		6.1289	56.08	DP 2
ADDLINK 40	ADD	UN	.104		6.0287	1.92	VP-1
		DL	.104		6.0287	1.92	
		DN	9.134		6.1456	58.66	DP 3
ADDLINK 50	ADD	UN	.113		6.0287	2.07	IP-1
		DL	.113		6.0287	2.07	
		DN	9.134		6.1456	58.66	DP 3
ADDLINK 60	ADD	UN	.236		6.0287	4.30	IP-2
		DL	.236		6.0287	4.30	
		DN	9.134		6.1456	58.66	DP 3
ADDLINK 70	ADD	UN	.479		6.1289	6.04	B5
		DL	.479		6.1289	6.04	
		DN	9.612		6.1456	64.51	DP 4
ADDLINK 80	ADD	UN	.347		6.1623	2.67	B6
		DL	.347		6.1623	2.67	
		DN	10.600		6.1623	74.53	DP 5
ADDLINK 90	ADD	UN	.641		6.1289	7.57	B7
		DL	.641		6.1289	7.57	
		DN	10.600		6.1623	74.53	DP 5
C1	REACH	UN	.692		6.0287	12.73	O1
		DL	.692		6.0621	10.55	
		DN	4.400		6.1122	39.67	DP 1

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C10	REACH	UN	4.243		6.3126	28.74	DP 8
		DL	4.243		6.3460	28.06	
		DN	12.349		6.1289	102.56	DP 9
C11	REACH	UN	1.179		6.2792	6.74	C1
		DL	1.179		6.3627	6.37	
		DN	6.109		6.1623	68.34	DP 10
C12	ADD	UN	6.109		6.1623	68.34	DP 10
		DL	6.109		6.1623	68.34	
		DN	12.349		6.1289	102.56	DP 9
C13	REACH	UN	12.349		6.1289	102.56	DP 9
		DL	12.349		6.1456	101.91	
		DN	13.008		6.1456	110.74	DP 11
C14	REACH	UN	13.008		6.1456	110.74	DP 11
		DL	13.008		6.1623	110.59	
		DN	37.960		6.0955	357.91	DP 7
C15	ADD	UN	2.330		6.1456	30.97	A8
		DL	2.330		6.1456	30.97	
		DN	5.214		6.1289	71.50	DP12
C18	REACH	UN	32.245	R	8.2832	28.63	DP13
		DL	32.241	R	8.3166	28.62	
		DN	33.110	R	8.1830	29.50	DP 14
C19	REACH	UN	.283		6.1623	2.41	EK1T
		DL	.283		6.3794	1.07	
		DN	33.110	R	8.1830	29.50	DP 14
C2	REACH	UN	4.400		6.1122	39.67	DP 1
		DL	4.400		6.1456	39.25	
		DN	8.681		6.1289	56.08	DP 2
C3	REACH	UN	.543		6.1289	4.42	O2B
		DL	.542		6.3961	1.57	
		DN	8.681		6.1289	56.08	DP 2

NETWORK SUMMARY -- LINKS

(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C31	ADD	UN	11.102		6.1456	77.01	DP 25
		DL	11.102		6.1456	77.01	
		DN	13.938		6.1122	111.36	DP 27
C32	REACH	UN	.587		6.0120	10.89	EK1C
		DL	.587		6.0287	10.49	
		DN	1.602		6.0287	28.44	DP 26
C33	REACH	UN	1.602		6.0287	28.44	DP 26
		DL	1.602		6.0454	28.14	
		DN	13.938		6.1122	111.36	DP 27
C34	ADD	UN	13.938		6.1122	111.36	DP 27
		DL	13.938		6.1122	111.36	
		DN	14.407		6.1122	115.12	DP 29
C37	ADD	UN	14.407		6.1122	115.12	DP 29
		DL	14.407		6.1122	115.12	
		DN	37.960		6.0955	357.91	DP 7
C4	REACH	UN	8.681		6.1289	56.08	DP 2
		DL	8.681		6.1623	55.36	
		DN	9.134		6.1456	58.66	DP 3
C42	ADD	UN	5.214		6.1289	71.50	DP12
		DL	5.214		6.1289	71.50	
		DN	7.572		6.1122	100.64	DP 33
C43	ADD	UN	1.893		6.1122	26.33	A7
		DL	1.893		6.1122	26.33	
		DN	7.572		6.1122	100.64	DP 33
C44	ADD	UN	7.572		6.1122	100.64	DP 33
		DL	7.572		6.1122	100.64	
		DN	8.991		6.0621	121.22	DP 34
C45	ADD	UN	8.991		6.0621	121.22	DP 34
		DL	8.991		6.0621	121.22	
		DN	37.960		6.0955	357.91	DP 7

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
C5	REACH	UN	9.134		6.1456	58.66	DP 3	
		DL	9.134		6.1623	58.56		
		DN	9.612		6.1456	64.51	DP 4	
C6	REACH	UN	9.612		6.1456	64.51	DP 4	
		DL	9.612		6.1623	64.45		
		DN	10.600		6.1623	74.53	DP 5	
C7	ADD	UN	10.600		6.1623	74.53	DP 5	
		DL	10.600		6.1623	74.53		
		DN	11.102		6.1456	77.01	DP 25	
C9	REACH	UN	1.738		6.2291	14.97	A1	
		DL	1.738		6.3460	12.35		
		DN	4.243		6.3126	28.74	DP 8	
EK-1A STRUCTURE	PONDrt	UN	37.960		6.0955	357.91	EK1A	IN
EK-1A STRUCTURE			32.245	R	8.2832	28.63	EK1A	OUT
		DL	32.245	R	8.2832	28.63		
		DN	32.245	R	8.2832	28.63	DP13	
EK-1B STRUCTURE	PONDrt	UN	4.940		6.0621	80.73	EK1B	IN
EK-1B STRUCTURE			4.930		6.1456	65.96	EK1B	OUT
		DL	4.930		6.1456	65.96		
		DN	6.109		6.1623	68.34	DP 10	
I25-1 STRUCTURE	PONDrt	UN	33.110	R	8.1830	29.50	I25-1	IN
I25-1 STRUCTURE			33.106	R	8.2498	29.47	I25-1	OUT
		DL	33.106	R	8.2498	29.47		
		DN	33.106	R	8.2498	29.47	DP 20	
NLC STRUCTURE	PONDrt	UN	3.196		6.0621	54.19	NLC	IN
NLC STRUCTURE			3.195		6.2959	11.97	NLC	OUT
		DL	3.195		6.2959	11.97		
		DN	8.681		6.1289	56.08	DP 2	
PCC STRUCTURE	PONDrt	UN	3.708		6.0454	65.46	PCC	IN
PCC STRUCTURE			3.708		6.1623	31.59	PCC	OUT
		DL	3.708		6.1623	31.59		
		DN	4.400		6.1122	39.67	DP 1	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = C Springs

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 100 yr
Total Rainfall Depth= 4.4000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	10.305		6.0287	184.43	B2	
		DL	10.305		6.0287	184.43		
		DN	10.305		6.0287	184.43	PCC	IN
ADDLINK 100	ADD	UN	1.060		5.9953	18.59	B9	
		DL	1.060		5.9953	18.59		
		DN	32.388		6.0955	263.79	DP 25	
ADDLINK 120	ADD	UN	3.122		6.0120	57.02	A6	
		DL	3.122		6.0120	57.02		
		DN	105.017		6.0955	1058.41	DP 7	
ADDLINK 130	ADD	UN	10.291		6.2124	107.29	A2	
		DL	10.291		6.2124	107.29		
		DN	16.609		6.2291	174.40	DP 8	
ADDLINK 140	ADD	UN	10.444		6.0454	168.68	D1	
		DL	10.444		6.0454	168.68		
		DN	10.444		6.0454	168.68	EK1B	IN
ADDLINK 150	ADD	UN	4.951		6.0287	85.49	A3	
		DL	4.951		6.0287	85.49		
		DN	37.327		6.2124	368.62	DP 9	
ADDLINK 160	ADD	UN	.657		6.0788	10.09	A4	
		DL	.657		6.0788	10.09		
		DN	39.129		6.2124	387.77	DP 11	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 170	ADD	UN	1.145		6.1122	16.61	A5	
		DL	1.145		6.1122	16.61		
		DN	39.129		6.2124	387.77	DP 11	
ADDLINK 180	ADD	UN	6.458		6.0955	93.39	A9	
		DL	6.458		6.0955	93.39		
		DN	11.886		6.1122	168.43	DP12	
ADDLINK 190	ADD	UN	105.017		6.0955	1058.41	DP 7	
		DL	105.017		6.0955	1058.41		
		DN	105.017		6.0955	1058.41	EK1A	IN
ADDLINK 20	ADD	UN	7.922		6.0454	135.15	B3	
		DL	7.922		6.0454	135.15		
		DN	7.922		6.0454	135.15	NLC	IN
ADDLINK 200	ADD	UN	2.654		6.1289	33.67	A12	
		DL	2.654		6.1289	33.67		
		DN	101.668	R	7.0975	133.05	DP 14	
ADDLINK 210	ADD	UN	101.668	R	7.0975	133.05	DP 14	
		DL	101.668	R	7.0975	133.05		
		DN	101.668	R	7.0975	133.05	I25-1	IN
ADDLINK 220	ADD	UN	3.611		6.1456	46.75	EK1Q	
		DL	3.611		6.1456	46.75		
		DN	39.386		6.0955	358.73	DP 27	
ADDLINK 230	ADD	UN	2.146		6.0120	36.68	EK1D	
		DL	2.146		6.0120	36.68		
		DN	3.387		6.0120	58.08	DP 26	
ADDLINK 240	ADD	UN	1.925		6.1289	25.37	EK1R	
		DL	1.925		6.1289	25.37		
		DN	41.310		6.0955	383.38	DP 29	
ADDLINK 250	ADD	UN	.982		5.9953	17.35	EK1M	
		DL	.982		5.9953	17.35		
		DN	17.664		6.0955	246.26	DP 33	

NETWORK SUMMARY -- LINKS

(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
ADDLINK 260	ADD	UN	2.999		6.0120	51.48	EK1N
		DL	2.999		6.0120	51.48	
		DN	20.663		6.0621	291.99	DP 34
ADDLINK 270	ADD	UN	.792		6.0955	11.78	EK1S
		DL	.792		6.0955	11.78	
		DN	105.017		6.0955	1058.41	DP 7
ADDLINK 30	ADD	UN	1.844		6.0454	32.02	B1
		DL	1.844		6.0454	32.02	
		DN	25.007		6.0788	169.78	DP 2
ADDLINK 40	ADD	UN	.242		6.0120	4.33	VP-1
		DL	.242		6.0120	4.33	
		DN	26.062		6.0955	181.69	DP 3
ADDLINK 50	ADD	UN	.264		6.0120	4.68	IP-1
		DL	.264		6.0120	4.68	
		DN	26.062		6.0955	181.69	DP 3
ADDLINK 60	ADD	UN	.549		6.0120	9.74	IP-2
		DL	.549		6.0120	9.74	
		DN	26.062		6.0955	181.69	DP 3
ADDLINK 70	ADD	UN	1.523		6.0955	22.25	B5
		DL	1.523		6.0955	22.25	
		DN	27.585		6.0955	203.65	DP 4
ADDLINK 80	ADD	UN	1.570		6.1289	20.47	B6
		DL	1.570		6.1289	20.47	
		DN	31.328		6.1122	254.85	DP 5
ADDLINK 90	ADD	UN	2.173		6.1122	31.13	B7
		DL	2.173		6.1122	31.13	
		DN	31.328		6.1122	254.85	DP 5
C1	REACH	UN	2.344		6.0120	45.16	O1
		DL	2.344		6.0454	42.84	
		DN	12.650		6.0621	98.98	DP 1

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C10	REACH	UN	16.609		6.2291	174.40	DP 8
		DL	16.608		6.2625	172.86	
		DN	37.327		6.2124	368.62	DP 9
C11	REACH	UN	5.333		6.2291	53.92	C1
		DL	5.333		6.2625	52.96	
		DN	15.767		6.2124	171.89	DP 10
C12	ADD	UN	15.767		6.2124	171.89	DP 10
		DL	15.767		6.2124	171.89	
		DN	37.327		6.2124	368.62	DP 9
C13	REACH	UN	37.327		6.2124	368.62	DP 9
		DL	37.327		6.2291	367.88	
		DN	39.129		6.2124	387.77	DP 11
C14	REACH	UN	39.129		6.2124	387.77	DP 11
		DL	39.129		6.2124	387.67	
		DN	105.017		6.0955	1058.41	DP 7
C15	ADD	UN	5.428		6.1289	75.24	A8
		DL	5.428		6.1289	75.24	
		DN	11.886		6.1122	168.43	DP12
C18	REACH	UN	97.799	R	7.3814	128.02	DP13
		DL	97.796	R	7.3981	128.01	
		DN	101.668	R	7.0975	133.05	DP 14
C19	REACH	UN	1.218		6.1289	16.18	EK1T
		DL	1.218		6.2458	11.72	
		DN	101.668	R	7.0975	133.05	DP 14
C2	REACH	UN	12.650		6.0621	98.98	DP 1
		DL	12.650		6.0788	98.20	
		DN	25.007		6.0788	169.78	DP 2
C3	REACH	UN	2.593		6.0955	37.46	O2B
		DL	2.592		6.2124	24.70	
		DN	25.007		6.0788	169.78	DP 2

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points
C31	ADD	UN	32.388		6.0955	263.79	DP 25
		DL	32.388		6.0955	263.79	
		DN	39.386		6.0955	358.73	DP 27
C32	REACH	UN	1.241		5.9953	21.84	EK1C
		DL	1.241		6.0287	21.43	
		DN	3.387		6.0120	58.08	DP 26
C33	REACH	UN	3.387		6.0120	58.08	DP 26
		DL	3.387		6.0287	57.74	
		DN	39.386		6.0955	358.73	DP 27
C34	ADD	UN	39.386		6.0955	358.73	DP 27
		DL	39.386		6.0955	358.73	
		DN	41.310		6.0955	383.38	DP 29
C37	ADD	UN	41.310		6.0955	383.38	DP 29
		DL	41.310		6.0955	383.38	
		DN	105.017		6.0955	1058.41	DP 7
C4	REACH	UN	25.007		6.0788	169.78	DP 2
		DL	25.007		6.1122	168.19	
		DN	26.062		6.0955	181.69	DP 3
C42	ADD	UN	11.886		6.1122	168.43	DP12
		DL	11.886		6.1122	168.43	
		DN	17.664		6.0955	246.26	DP 33
C43	ADD	UN	4.796		6.0955	70.36	A7
		DL	4.796		6.0955	70.36	
		DN	17.664		6.0955	246.26	DP 33
C44	ADD	UN	17.664		6.0955	246.26	DP 33
		DL	17.664		6.0955	246.26	
		DN	20.663		6.0621	291.99	DP 34
C45	ADD	UN	20.663		6.0621	291.99	DP 34
		DL	20.663		6.0621	291.99	
		DN	105.017		6.0955	1058.41	DP 7

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
C5	REACH	UN	26.062		6.0955	181.69	DP 3	
		DL	26.062		6.0955	181.40		
		DN	27.585		6.0955	203.65	DP 4	
C6	REACH	UN	27.585		6.0955	203.65	DP 4	
		DL	27.585		6.1122	203.36		
		DN	31.328		6.1122	254.85	DP 5	
C7	ADD	UN	31.328		6.1122	254.85	DP 5	
		DL	31.328		6.1122	254.85		
		DN	32.388		6.0955	263.79	DP 25	
C9	REACH	UN	6.318		6.1790	73.26	A1	
		DL	6.318		6.2625	67.91		
		DN	16.609		6.2291	174.40	DP 8	
EK-1A STRUCTURE	PONDrt	UN	105.017		6.0955	1058.41	EK1A	IN
EK-1A STRUCTURE			97.799	R	7.3814	128.02	EK1A	OUT
		DL	97.799	R	7.3814	128.02		
		DN	97.799	R	7.3814	128.02	DP13	
EK-1B STRUCTURE	PONDrt	UN	10.444		6.0454	168.68	EK1B	IN
EK-1B STRUCTURE			10.434		6.1623	123.14	EK1B	OUT
		DL	10.434		6.1623	123.14		
		DN	15.767		6.2124	171.89	DP 10	
I25-1 STRUCTURE	PONDrt	UN	101.668	R	7.0975	133.05	I25-1	IN
I25-1 STRUCTURE			101.667	R	7.2311	132.77	I25-1	OUT
		DL	101.667	R	7.2311	132.77		
		DN	101.667	R	7.2311	132.77	DP 20	
NLC STRUCTURE	PONDrt	UN	7.922		6.0454	135.15	NLC	IN
NLC STRUCTURE			7.921		6.2959	28.62	NLC	OUT
		DL	7.921		6.2959	28.62		
		DN	25.007		6.0788	169.78	DP 2	
PCC STRUCTURE	PONDrt	UN	10.305		6.0287	184.43	PCC	IN
PCC STRUCTURE			10.305		6.1957	60.98	PCC	OUT
		DL	10.305		6.1957	60.98		
		DN	12.650		6.0621	98.98	DP 1	

MASTER DESIGN STORM SUMMARY

Network Storm Collection: El Paso County

Return Event	Total Depth in	Rainfall Type	RNF ID
2	2.0000	Synthetic Curve	TYPEIIA 24HR
5	2.6000	Synthetic Curve	TYPEIIA 24HR
10	3.0000	Synthetic Curve	TYPEIIA 24HR
25	3.6000	Synthetic Curve	TYPEIIA 24HR
50	4.0000	Synthetic Curve	TYPEIIA 24HR
100	4.4000	Synthetic Curve	TYPEIIA 24HR

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 15	JCT	2	.295		6.0500	4.88		
DP 15	JCT	5	.481		6.0500	8.05		
DP 15	JCT	10	.615		6.0500	10.29		
DP 15	JCT	25	.826		6.0500	13.78		
DP 15	JCT	50	.972		6.0500	16.16		
DP 15	JCT	100	1.121		6.0500	18.56		
DP 16	JCT	2	.295		6.0500	4.71		
DP 16	JCT	5	.481		6.0500	7.87		
DP 16	JCT	10	.615		6.0500	10.12		
DP 16	JCT	25	.826		6.0500	13.64		
DP 16	JCT	50	.972		6.0500	16.04		
DP 16	JCT	100	1.121		6.0500	18.48		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
DP 17	JCT	2	.788		6.1500	4.39		
DP 17	JCT	5	1.952		6.2000	15.28		
DP 17	JCT	10	2.962		6.2000	27.49		
DP 17	JCT	25	4.764		6.1500	51.38		
DP 17	JCT	50	6.124		6.1500	70.19		
DP 17	JCT	100	7.594		6.1500	90.65		
*DP 19	JCT	2	.788		6.2500	3.98		
*DP 19	JCT	5	1.952		6.5000	7.38		
*DP 19	JCT	10	2.962		6.5000	11.57		
*DP 19	JCT	25	4.764		6.5000	19.81		
*DP 19	JCT	50	6.124		6.5000	23.55		
*DP 19	JCT	100	7.594		6.5500	25.87		
E5	AREA	2	.209		6.6000	.38		
E5	AREA	5	.647		6.2000	4.53		
E5	AREA	10	1.044		6.1500	9.49		
E5	AREA	25	1.767		6.1500	19.29		
E5	AREA	50	2.322		6.1500	26.99		
E5	AREA	100	2.927		6.1500	35.43		
EK2H	AREA	2	.284		6.2500	.70		
EK2H	AREA	5	.824		6.2000	5.96		
EK2H	AREA	10	1.304		6.2000	11.66		
EK2H	AREA	25	2.171		6.1500	22.85		
EK2H	AREA	50	2.831		6.1500	31.76		
EK2H	AREA	100	3.547		6.1500	41.55		
I25-2 IN	POND	2	.788		6.1500	4.39		
I25-2 IN	POND	5	1.952		6.2000	15.28		
I25-2 IN	POND	10	2.962		6.2000	27.49		
I25-2 IN	POND	25	4.764		6.1500	51.38		
I25-2 IN	POND	50	6.124		6.1500	70.19		
I25-2 IN	POND	100	7.594		6.1500	90.65		

MASTER NETWORK SUMMARY
SCS Unit Hydrograph Method

(*Node=Outfall; +Node=Diversion;)
(Trun= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left&Rt)

Node ID	Type	Return Event	HYG Vol ac-ft	Trun	Qpeak hrs	Qpeak cfs	Max WSEL ft	Max Pond Storage ac-ft
I25-2	OUT POND	2	.788		6.2500	3.98	6599.94	.038
I25-2	OUT POND	5	1.952		6.5000	7.38	6600.34	.259
I25-2	OUT POND	10	2.962		6.5000	11.57	6600.78	.537
I25-2	OUT POND	25	4.764		6.5000	19.81	6601.62	1.079
I25-2	OUT POND	50	6.124		6.5000	23.55	6602.14	1.563
I25-2	OUT POND	100	7.594		6.5500	25.87	6602.51	2.182
IP-3	AREA	2	.116		6.0500	1.91		
IP-3	AREA	5	.189		6.0500	3.16		
IP-3	AREA	10	.241		6.0500	4.04		
IP-3	AREA	25	.324		6.0500	5.40		
IP-3	AREA	50	.381		6.0500	6.34		
IP-3	AREA	100	.439		6.0500	7.28		
IP-4	AREA	2	.180		6.0500	2.97		
IP-4	AREA	5	.292		6.0500	4.89		
IP-4	AREA	10	.374		6.0500	6.26		
IP-4	AREA	25	.502		6.0500	8.37		
IP-4	AREA	50	.591		6.0500	9.82		
IP-4	AREA	100	.681		6.0500	11.28		

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = El Paso County

Storm Tag Name = 5

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 5 yr
Total Rainfall Depth= 2.6000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	.189		6.0500	3.16	IP-3	
		DL	.189		6.0500	3.16		
		DN	.481		6.0500	8.05	DP 15	
ADDLINK 20	ADD	UN	.292		6.0500	4.89	IP-4	
		DL	.292		6.0500	4.89		
		DN	.481		6.0500	8.05	DP 15	
ADDLINK 30	ADD	UN	.647		6.2000	4.53	E5	
		DL	.647		6.2000	4.53		
		DN	1.952		6.2000	15.28	DP 17	
ADDLINK 40	ADD	UN	1.952		6.2000	15.28	DP 17	
		DL	1.952		6.2000	15.28		
		DN	1.952		6.2000	15.28	I25-2	IN
C20	REACH	UN	.824		6.2000	5.96	EK2H	
		DL	.824		6.3000	5.32		
		DN	1.952		6.2000	15.28	DP 17	
C21	REACH	UN	.481		6.0500	8.05	DP 15	
		DL	.481		6.0500	7.87		
		DN	.481		6.0500	7.87	DP 16	
C22	REACH	UN	.481		6.0500	7.87	DP 16	
		DL	.481		6.1000	6.96		
		DN	1.952		6.2000	15.28	DP 17	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
I25-2 STRUCTURE	PONDrt	UN	1.952		6.2000	15.28	I25-2	IN
I25-2 STRUCTURE			1.952		6.5000	7.38	I25-2	OUT
		DL	1.952		6.5000	7.38		
		DN	1.952		6.5000	7.38	DP 19	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

DEFAULT Design Storm File,ID = El Paso County

Storm Tag Name = 100

Data Type, File, ID = Synthetic Storm TYPEIIA 24HR
Storm Frequency = 100 yr
Total Rainfall Depth= 4.4000 in
Duration Multiplier = 1
Resulting Duration = 24.0000 hrs
Resulting Start Time= .0000 hrs Step= .2500 hrs End= 24.0000 hrs

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
ADDLINK 10	ADD	UN	.439		6.0500	7.28	IP-3	
		DL	.439		6.0500	7.28		
		DN	1.121		6.0500	18.56	DP 15	
ADDLINK 20	ADD	UN	.681		6.0500	11.28	IP-4	
		DL	.681		6.0500	11.28		
		DN	1.121		6.0500	18.56	DP 15	
ADDLINK 30	ADD	UN	2.927		6.1500	35.43	E5	
		DL	2.927		6.1500	35.43		
		DN	7.594		6.1500	90.65	DP 17	
ADDLINK 40	ADD	UN	7.594		6.1500	90.65	DP 17	
		DL	7.594		6.1500	90.65		
		DN	7.594		6.1500	90.65	I25-2	IN
C20	REACH	UN	3.547		6.1500	41.55	EK2H	
		DL	3.547		6.2000	40.23		
		DN	7.594		6.1500	90.65	DP 17	
C21	REACH	UN	1.121		6.0500	18.56	DP 15	
		DL	1.121		6.0500	18.48		
		DN	1.121		6.0500	18.48	DP 16	
C22	REACH	UN	1.121		6.0500	18.48	DP 16	
		DL	1.120		6.1000	17.56		
		DN	7.594		6.1500	90.65	DP 17	

NETWORK SUMMARY -- LINKS
(UN=Upstream Node; DL=DNstream End of Link; DN=DNstream Node)
(Trun.= HYG Truncation: Blank=None; L=Left; R=Rt; LR=Left & Rt)

Link ID	Type		HYG Vol ac-ft	Trun.	Peak Time hrs	Peak Q cfs	End Points	
I25-2 STRUCTURE	PONDrt	UN	7.594		6.1500	90.65	I25-2	IN
I25-2 STRUCTURE			7.594		6.5500	25.87	I25-2	OUT
		DL	7.594		6.5500	25.87		
		DN	7.594		6.5500	25.87	DP 19	

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

HYDROLOGIC / HYDRAULIC CALCULATIONS
LOTS 1 & 2, INTERQUEST FILING NO. 6
FINAL DRAINAGE REPORT

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

FINAL DRAINAGE REPORT ~ BASIN RUNOFF COEFFICIENT SUMMARY

BASIN	TOTAL AREA (AC)	IMPERVIOUS AREA / STREETS			LANDSCAPE/UNDEVELOPED AREAS			WEIGHTED		WEIGHTED CA	
		AREA (AC)	C(5)	C(100)	AREA (AC)	C(5)	C(100)	C(5)	C(100)	CA(5)	CA(100)
A8.2.1	1.35	0.05	0.90	0.95	1.30	0.25	0.35	0.27	0.37	**0.37**	**0.50**
A8.2.2	5.60	5.60	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**5.04**	**5.32**
A8.2.3	3.30	1.43	0.90	0.95	1.87	0.25	0.35	0.53	0.61	**1.75**	**2.01**
A8.2.4	11.60	7.59	0.90	0.95	4.01	0.25	0.35	0.68	0.74	**7.83**	**8.61**
A8.2.5	2.00	0.00	0.90	0.95	2.00	0.25	0.35	0.25	0.35	**0.50**	**0.70**
A8.2.6	0.31	0.31	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.28**	**0.29**
A8.2.7	0.54	0.54	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.49**	**0.51**
A8.2.8	0.63	0.60	0.90	0.95	0.03	0.25	0.35	0.87	0.92	**0.55**	**0.58**
A9.0.1	1.98	1.98	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**1.78**	**1.88**
A9.0.2	7.60	5.37	0.90	0.95	2.23	0.25	0.35	0.71	0.77	**5.39**	**5.88**
A9.0.3	0.60	0.00	0.90	0.95	0.60	0.25	0.35	0.25	0.35	**0.15**	**0.21**
A9.0.4	5.20	2.00	0.90	0.95	3.20	0.25	0.35	0.50	0.58	**2.60**	**3.02**
A9.0.5	4.00	2.90	0.90	0.95	1.10	0.25	0.35	0.72	0.79	**2.89**	**3.14**
A9.0.6	0.54	0.30	0.90	0.95	0.24	0.25	0.35	0.61	0.68	**0.33**	**0.37**
EX-D1	1.75	1.75	0.80	0.85	0.00	0.25	0.35	0.80	0.85	**1.40**	**1.49**
EX-D2	4.04	4.04	0.80	0.85	0.00	0.25	0.35	0.80	0.85	**3.23**	**3.43**
EX-D3	0.94	0.85	0.90	0.95	0.09	0.30	0.40	0.84	0.90	**0.79**	**0.84**
EX-D4	6.59	6.59	0.80	0.85	0.00	0.25	0.35	0.80	0.85	**5.27**	**5.60**
EX-D5	6.36	6.36	0.80	0.85	0.00	0.25	0.35	0.80	0.85	**5.09**	**5.41**
EX-D6	3.61	3.61	0.80	0.85	0.00	0.25	0.35	0.80	0.85	**2.89**	**3.07**
EX-D7	0.36	0.36	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.32**	**0.34**
EX-D8	1.64	1.47	0.90	0.95	0.17	0.25	0.35	0.83	0.89	**1.37**	**1.46**
EX-D9	1.04	1.04	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.94**	**0.99**
EX-D10	0.23	0.23	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.21**	**0.22**
EX-D11	0.19	0.19	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.17**	**0.18**
EX-OS2	2.65	0.00	0.90	0.95	2.65	0.70	0.8	0.70	0.80	**1.86**	**2.12**
EX-OS3	0.95	0.00	0.90	0.95	0.95	0.70	0.8	0.70	0.80	**0.67**	**0.76**
EX-AA	3.44	0.20	0.90	0.95	3.24	0.25	0.35	0.29	0.38	**0.99**	**1.32**

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

FINAL DRAINAGE REPORT ~ BASIN RUNOFF COEFFICIENT SUMMARY

BASIN	TOTAL AREA (AC)	IMPERVIOUS AREA / STREETS			LANDSCAPE/UNDEVELOPED AREAS			WEIGHTED		WEIGHTED CA	
		AREA (AC)	C(5)	C(100)	AREA (AC)	C(5)	C(100)	C(5)	C(100)	CA(5)	CA(100)
A	0.74	0.56	0.90	0.95	0.18	0.25	0.35	0.74	0.80	**0.55**	**0.60**
B	0.07	0.07	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.06**	**0.07**
C	0.85	0.75	0.90	0.95	0.10	0.25	0.35	0.82	0.88	**0.70**	**0.75**
D	0.77	0.69	0.90	0.95	0.08	0.25	0.35	0.83	0.89	**0.64**	**0.68**
E	0.60	0.60	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.54**	**0.57**
F	0.73	0.73	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.66**	**0.69**
G	0.82	0.72	0.90	0.95	0.10	0.25	0.35	0.82	0.88	**0.67**	**0.72**
H	0.67	0.62	0.90	0.95	0.05	0.25	0.35	0.85	0.91	**0.57**	**0.61**
I	0.85	0.55	0.90	0.95	0.30	0.25	0.35	0.67	0.74	**0.57**	**0.63**
J	0.46	0.42	0.90	0.95	0.04	0.25	0.35	0.84	0.90	**0.39**	**0.41**
K	0.91	0.91	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.82**	**0.86**
L	0.89	0.89	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.80**	**0.85**
M	0.31	0.28	0.90	0.95	0.03	0.25	0.35	0.84	0.89	**0.26**	**0.28**
N	0.72	0.72	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.65**	**0.68**
O	0.69	0.49	0.90	0.95	0.20	0.25	0.35	0.71	0.78	**0.49**	**0.54**
P	0.80	0.75	0.90	0.95	0.05	0.25	0.35	0.86	0.91	**0.69**	**0.73**
Q	0.61	0.43	0.90	0.95	0.18	0.25	0.35	0.71	0.77	**0.43**	**0.47**
R	0.62	0.62	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**0.56**	**0.59**
S	1.11	1.11	0.90	0.95	0.00	0.25	0.35	0.90	0.95	**1.00**	**1.05**
T	0.79	0.00	0.90	0.95	0.79	0.30	0.4	0.30	0.40	**0.24**	**0.32**
U	0.65	0.00	0.90	0.95	0.65	0.30	0.4	0.30	0.40	**0.20**	**0.26**

JOB NAME: *INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER: *2190.00*
DATE: *07/25/07*
CALC'D BY: *MAL*

FINAL DRAINAGE REPORT ~ BASIN RUNOFF SUMMARY

BASIN	WEIGHTED CA(5)	WEIGHTED CA(100)	OVERLAND C(5)	OVERLAND Length (ft)	OVERLAND Height (ft)	OVERLAND Tc (min)	STREET / CHANNEL FLOW Length (ft)	STREET / CHANNEL FLOW Slope (%)	STREET / CHANNEL FLOW Velocity (fps)	STREET / CHANNEL FLOW Tc (min)	Tc TOTAL (min)	INTENSITY I(2) (in/hr)	INTENSITY I(5) (in/hr)	INTENSITY I(10) (in/hr)	INTENSITY I(25) (in/hr)	INTENSITY I(50) (in/hr)	INTENSITY I(100) (in/hr)	TOTAL FLOWS Q(2) (cfs)	TOTAL FLOWS Q(5) (cfs)	TOTAL FLOWS Q(10) (cfs)	TOTAL FLOWS Q(25) (cfs)	TOTAL FLOWS Q(50) (cfs)	TOTAL FLOWS Q(100) (cfs)
A8.2.1	0.37	0.50	0.25	15	10	1.5	0	0.0%	0.0	14.0	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	2	2	4	4	5
A8.2.2	5.04	5.32	0.25	10	3	1.6	0	0.0%	0.0	#DIV/0!	10.0	2.98	4.10	4.79	6.15	6.97	7.29	15	21	24	33	37	39
A8.2.3	1.75	2.01	0.25	100	10	7.4	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	7	9	10	15	17	18
A8.2.4	7.83	8.61	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	17.0	2.37	3.27	3.81	4.90	5.55	5.81	19	26	30	42	48	50
A8.2.5	0.50	0.70	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	9.0	3.10	4.27	4.98	6.40	7.25	7.58	2	2	2	4	5	5
A8.2.6	0.28	0.29	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	2	2	3	3
A8.2.7	0.49	0.51	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	2	3	4	4	5
A8.2.8	0.55	0.58	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	4	5	5
A9.0.1	1.78	1.88	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	7	9	11	14	16	17
A9.0.2	5.39	5.88	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	9.0	3.10	4.27	4.98	6.40	7.25	7.58	17	23	27	38	43	45
A9.0.3	0.15	0.21	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	1	2	2	2
A9.0.4	2.60	3.02	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	7.0	3.37	4.64	5.41	6.96	7.89	8.25	9	12	14	21	24	25
A9.0.5	2.89	3.14	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.6	3.60	4.95	5.78	7.43	8.42	8.81	10	14	17	23	26	28
A9.0.6	0.33	0.37	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	2	2	3	3	3
EX-D1	1.40	1.49	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	14.0	2.60	3.57	4.17	5.36	6.07	6.35	4	5	6	8	9	9
EX-D2	3.23	3.43	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	8.0	3.23	4.44	5.18	6.67	7.55	7.90	10	14	17	23	26	27
EX-D3	0.79	0.84	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	13.4	2.65	3.64	4.25	5.46	6.19	6.47	2	3	3	5	5	5
EX-D4	5.27	5.60	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	16.6	2.40	3.30	3.85	4.95	5.61	5.87	13	17	20	26	31	33
EX-D5	5.09	5.41	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	15.7	2.46	3.39	3.96	5.09	5.76	6.03	13	17	20	27	31	33
EX-D6	2.89	3.07	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	17.1	2.37	3.26	3.80	4.88	5.54	5.79	7	9	11	15	17	18
EX-D7	0.32	0.34	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	2	2	3	3	3
EX-D8	1.37	1.46	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	5	7	8	11	13	13
EX-D9	0.94	0.99	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	3	5	6	8	9	9
EX-D10	0.21	0.22	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	1	2	2	2
EX-D11	0.17	0.18	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	1	1	2	2
EX-OS2	1.86	2.12	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	12.5	2.73	3.75	4.38	5.63	6.38	6.67	5	7	8	12	14	14
EX-OS3	0.67	0.76	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	10.1	2.97	4.08	4.77	6.13	6.95	7.27	2	3	3	5	5	6
EX-AA	0.99	1.32	0.25	350	10	21.0	0	0.0%	0.0	#DIV/0!	21.0	2.13	2.94	3.43	4.41	4.99	5.22	2	3	3	6	7	7

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FINAL DRAINAGE REPORT ~ BASIN RUNOFF SUMMARY

BASIN	WEIGHTED CA(5)	WEIGHTED CA(100)	OVERLAND C(5)	OVERLAND Length (ft)	OVERLAND Height (ft)	OVERLAND Tc (min)	STREET / CHANNEL FLOW Length (ft)	STREET / CHANNEL FLOW Slope (%)	STREET / CHANNEL FLOW Velocity (fps)	STREET / CHANNEL FLOW Tc (min)	Tc TOTAL (min)	INTENSITY I(2) (in/hr)	INTENSITY I(5) (in/hr)	INTENSITY I(10) (in/hr)	INTENSITY I(25) (in/hr)	INTENSITY I(50) (in/hr)	INTENSITY I(100) (in/hr)	TOTAL FLOWS Q(2) (cfs)	TOTAL FLOWS Q(5) (cfs)	TOTAL FLOWS Q(10) (cfs)	TOTAL FLOWS Q(25) (cfs)	TOTAL FLOWS Q(50) (cfs)	TOTAL FLOWS Q(100) (cfs)
A	0.55	0.60	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	5	5	5
B	0.06	0.07	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	0	0	0	1	1	1
C	0.70	0.75	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	3	4	4	6	6	7
D	0.64	0.68	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	4	5	6	6
E	0.54	0.57	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	4	5	5
F	0.66	0.69	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	4	5	6	6
G	0.67	0.72	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	4	6	6	7
H	0.57	0.61	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	5	5	6
I	0.57	0.63	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	5	5	6
J	0.39	0.41	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	2	2	3	4	4
K	0.82	0.86	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	3	4	5	7	8	8
L	0.80	0.85	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	3	4	5	6	7	8
M	0.26	0.28	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	2	2	2	3
N	0.65	0.68	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	4	5	6	6
O	0.49	0.54	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	4	5	5
P	0.69	0.73	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	3	4	4	6	6	7
Q	0.43	0.47	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	2	3	4	4	4
R	0.56	0.59	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	2	3	3	5	5	5
S	1.00	1.05	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	4	5	6	8	9	10
T	0.24	0.32	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	1	2	3	3
U	0.20	0.26	0.25	0	0	#DIV/0!	0	0.0%	0.0	#DIV/0!	5.0	3.71	5.10	5.96	7.66	8.68	9.07	1	1	1	2	2	2

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FINAL DRAINAGE REPORT ~ SURFACE ROUTING SUMMARY

Design Point(s)	Contributing Basins	Equivalent CA(5)	Equivalent CA(100)	Maximum Tc	Intensity		Flow		Inlet Size
					I(5)	I(100)	Q(5)	Q(100)	
FA-1	Basin A8.2.4	7.83	8.61	17.0	3.27	5.81	26	50	Multiple existing inlets/pipe system
FA-2	FA-1 + Basins A8.2.3 & A8.2.5 + Basin A8.2.8	10.64	11.91	17.0	3.27	5.81	35	69	Swale flow, existing depressed area
FA-3	FA-2 + Basins A8.2.1 & A8.2.2 + Basin A8.2.7	16.53	18.24	17.0	3.27	5.81	54	106	Existing 'stilling basin' + Overflow to Proposed Site
FA-5	Portion of Basin A9.0.5	1.75	1.75	5.6	4.95	8.81	9	15	Surface flow into existing drive
FA-6	FA-5 + Basin A9.0.4	4.35	4.77	7.0	4.64	8.25	20	39	Inlet Pickup to FA-7 + Overflow to FA-8
FA-7	FA-6 Pickup + Basin A9.0.3	3.17	1.91	7.0	4.64	8.25	15	16	30" RCP under private driveway adequate
FA-8	FA-6 Overflow + Basin A9.0.6	1.66	3.44	7.0	4.64	8.25	8	28	Existing 4.0' Sump Inlet + Flowby to FA-9
FA-9	FA-8 Flowby + Basin A9.0.1 & Basin EX+AA	3.57	5.92	9.0	4.27	7.58	15	45	Existing 8.0' At-Grade Inlet + Flowby to DP-10
10a	FA-9 Flowby + Basin EX-D-3	3.34	5.47	13.4	3.64	6.47	12	35	Federal Street Flows (15 cfs overtops crown to DP-11, 2.5 cfs overtops crown to DP-12)
10b	Basins EX-D-5, EX-D-7, EX-D-8, & EX-OS2	8.63	9.32	15.7	3.39	6.03	29	56	Federal Street Flows (24 cfs overtops crown to DP-13, 6 cfs overtops crown to DP-12)
10a	Actual Flow to Inlet	5.77	3.25	13.4	3.64	6.47	21	21	Ex. 14.0' sump inlet (includes 9cfs (5yr) & 3.5cfs (100yr) from DP-10b.
10b	Actual Flow to Inlet	5.01	2.82	15.7	3.39	6.03	17	17	Ex. 10.0' sump inlet (inlet overtops to DP-10a + 5.5cfs (100yr) to DP-12)
11	Basin A8.2.6 + Overtopping from 10a (15 cfs - 100 yr)	0.28	2.61	13.4	3.64	6.47	1	17	Surface Flow Into Proposed Site (15cfs (100yr) from DP-10a)
12	Overflow DP 10a & 10b + Basins EX-D-10 & EX-D-11	1.26	2.72	15.7	3.39	6.03	4	16	Surface Flow Into Proposed Site (3cfs (5yr) from DP-10b, 14 cfs (100yr) from DP-10a & DP-10b
13	Basin EX-D9 + Overtopping from 10b (24 cfs - 100 yr)	0.94	4.97	15.7	3.39	6.03	3	30	Surface Flow Into Proposed Site (24 cfs (100yr) from DP-10b)

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FINAL DRAINAGE REPORT ~ SURFACE ROUTING SUMMARY

Design Point(s)	Contributing Basins	Equivalent CA(5)	Equivalent CA(100)	Maximum Tc	Intensity		Flow		Inlet Size
					I(5)	I(100)	Q(5)	Q(100)	
IF-1	Basins EX-OS3 + EX-D-6	3.55	3.83	17.1	3.26	5.79	12	22	Existing inlets
IF-2	Basin EX-D-2	3.23	3.43	8.0	4.44	7.90	14	27	Existing inlets
IF-3	Basin EX-D-4	5.27	5.60	16.6	3.30	5.87	17	33	Existing inlets
14	Basin C	0.70	0.75	5.0	5.10	9.07	4	7	Proposed 6.0' Sump Inlet
15	DP-12 + Basin J	1.65	3.13	15.7	3.39	6.03	6	19	Proposed 14.0' Sump Inlet
16	DP-11 + Basin I	0.85	3.24	13.4	3.64	6.47	3	21	Proposed 16.0' Sump Inlet
17	Basins E & F	1.20	1.26	5.0	5.10	9.07	6	11	StormCeptor (Roof Drains)
18	Basins D & M	0.90	0.96	5.0	5.10	9.07	5	9	Proposed 6.0' Sump Inlet
19	Basin G	0.67	0.72	5.0	5.10	9.07	3	7	Proposed 4.0' Sump Inlet
20	Basin H	0.57	0.61	5.0	5.10	9.07	3	6	Proposed 4.0' Sump Inlet
21	Basin N	0.65	0.68	5.0	5.10	9.07	3	6	Proposed 4.0' Sump Inlet
22	Basin O	0.49	0.54	5.0	5.10	9.07	3	5	Proposed 4.0' Sump Inlet
23	Basin P	0.69	0.73	5.0	5.10	9.07	4	7	Proposed 4.0' Sump Inlet
24	Basin Q	0.43	0.47	5.0	5.10	9.07	2	4	Proposed 4.0' Sump Inlet
25	Basins K & L	1.62	1.71	5.0	5.10	9.07	8	16	StormCeptor (Roof Drains)
26	Basin R	0.56	0.59	5.0	5.10	9.07	3	5	Proposed 4.0' Sump Inlet
27	Basin S	1.00	1.05	5.0	5.10	9.07	5	10	Proposed 6.0' Sump Inlet
28	DP-13 + Basin A	1.49	5.56	15.7	3.39	6.03	5	34	Proposed 18.0' Sump Inlet

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* PIPES ARE LISTED AT MAXIMUM SIZE REQUIRED TO ACCOMMODATE Q100 FLOWS AT MINIMUM GRADE.
REFER TO INDIVIDUAL PIPE SHEETS FOR HYDRAULIC INFORMATION.

FINAL DRAINAGE REPORT ~ PIPE ROUTING SUMMARY

Pipe Run	Contributing Basins	Equivalent CA(5)	Equivalent CA(100)	Maximum Tc	Intensity		Flow		Pipe Size*
					I(5)	I(100)	Q(5)	Q(100)	
1	Intercepted flow from FA-6	3.02	1.70	7.0	4.64	8.25	14	14	Existing pipe to FA-7
2	FA-7	3.17	1.91	7.0	4.64	8.25	15	16	Existing 30" RCP
3	Pipe 2 + Pickup FA-8 + Basin A9.0.2	9.42	8.52	9.0	4.27	7.58	40	65	Existing 30" RCP
4	Pipe 3 + Pickup FA-9	10.44	9.81	9.0	4.27	7.58	45	74	Existing 30" RCP
5	DPs IF-1 & iF-2	6.79	7.26	17.1	3.26	5.79	22	42	Existing 30" storm - Interquest No. 1
6	Pipe 5 + DP IF-3 + Basin EX-D-1	13.46	14.35	17.1	3.26	5.79	44	83	Existing storm - Interquest No. 1
7	Pipe 6 + Int. DP10a + Int. DP10b	24.24	20.42	17.1	3.26	5.79	79	118	Re-routed inlet Lateral - Federal
8	Pipe 4 + Pipe 7	34.68	30.23	17.1	3.26	5.79	113	175	Proposed 54" RCP - (off-site flows)
9	DP-16	0.85	3.24	13.4	3.64	6.47	3	21	Proposed 24" RCP
10	Pipe 9 + DP-15	2.50	6.37	15.7	3.39	6.03	8	38	Proposed 24" RCP
11	DP-14	0.70	0.75	5.0	5.10	9.07	4	7	Proposed 18" RCP
12	Pipes 8 + 10 + 11	37.88	37.35	17.1	3.26	5.79	123	216	Proposed 54" RCP
13	DP-17	1.20	1.26	5.0	5.10	9.07	6	11	Proposed 18" RCP
14	Pipe 12 + Pipe 13	39.08	38.61	17.1	3.26	5.79	127	224	Proposed 54" RCP

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

* PIPES ARE LISTED AT MAXIMUM SIZE REQUIRED TO ACCOMMODATE Q100 FLOWS AT MINIMUM GRADE.
REFER TO INDIVIDUAL PIPE SHEETS FOR HYDRAULIC INFORMATION.

FINAL DRAINAGE REPORT ~ PIPE ROUTING SUMMARY

Pipe Run	Contributing Basins	Equivalent CA(5)	Equivalent CA(100)	Maximum Tc	Intensity		Flow		Pipe Size*
					I(5)	I(100)	Q(5)	Q(100)	
15	DP-18	0.90	0.96	5.0	5.10	9.07	5	9	Proposed 18" RCP
16	Pipe 14 + Pipe 15	39.98	39.57	17.1	3.26	5.79	130	229	Proposed 60" RCP
17	DP-19	0.67	0.72	5.0	5.10	9.07	3	7	Proposed 18" RCP
18	Pipe 17 + DP-20	1.24	1.33	5.0	5.10	9.07	6	12	Proposed 18" RCP
19	DP-21	0.65	0.68	5.0	5.10	9.07	3	6	Proposed 18" RCP
20	Pipes 16 + 18 + 19	41.87	41.58	17.1	3.26	5.79	136	241	Proposed 60" RCP Outfall to Pond
21	DP FA-3	16.53	18.24	17.0	3.27	5.81	54	106	Proposed 42" RCP
22	Pipe 21 + DP-22	17.02	18.78	17.0	3.27	5.81	56	109	Proposed 42" RCP
23	Pipe 22 + DP-23	17.71	19.51	17.0	3.27	5.81	58	113	Proposed 42" RCP
24	Pipe 23 + DP-24	18.14	19.98	18.4	3.14	5.58	57	112	Proposed 48" RCP
25	DP-25	1.62	1.71	5.0	5.10	9.07	8	16	Proposed 24" RCP
26	Pipe 24 + Pipe 25	19.76	21.69	18.4	3.14	5.58	62	121	Proposed 48" RCP
27	DP-26	0.56	0.59	5.0	5.10	9.07	3	5	Proposed 18" RCP
28	Pipe 27 + DP-27	1.56	1.64	5.0	5.10	9.07	8	15	Proposed 18" RCP
29	Pipe 26 + Pipe 28	21.32	23.33	18.4	3.14	5.58	67	130	Proposed 48" RCP

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

FINAL DRAINAGE REPORT ~ PIPE TRAVEL TIMES

PIPE RUN	STREET / CHANNEL FLOW			
	Length (ft)	Slope (%)	Velocity (fps)	Tc (min)
24	300	1.0%	3.5	1.4
25	0	0.0%	0.0	#DIV/0!
26	0	0.0%	0.0	#DIV/0!
27	0	0.0%	0.0	#DIV/0!
28	0	0.0%	0.0	#DIV/0!
29	0	0.0%	0.0	#DIV/0!
30	0	0.0%	0.0	#DIV/0!
31	0	0.0%	0.0	#DIV/0!
32	0	0.0%	0.0	#DIV/0!
33	0	0.0%	0.0	#DIV/0!
34	0	0.0%	0.0	#DIV/0!
35	0	0.0%	0.0	#DIV/0!
36	0	0.0%	0.0	#DIV/0!
37	0	0.0%	0.0	#DIV/0!
38	0	0.0%	0.0	#DIV/0!
39	0	0.0%	0.0	#DIV/0!
40	0	0.0%	0.0	#DIV/0!

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT **FA-6**

Total Flow: Q_5 = 14 cfs

Q_{100} = 14 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 8 foot inlet required

100-Year Event: 8 foot inlet required

INSTALL A PUBLIC 8 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT **FA-8**

Total Flow: Q_5 = 6 cfs

Q_{100} = 6

Maximum allowable ponding depth at sump:

D_5 = 0.30

D_{100} = 0.30 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: [4] foot inlet required

100-Year Event: [4] foot inlet required

INSTALL A PUBLIC [4] FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT FA-9 **100 YEAR FLOW**

Q(100)	45	I(100)	7.6		
DEPTH	0.50	Fr	1.93	Inlet size ? L(i) =	6
SPREAD	18.5	L(1)	27.4	If Li < L(2) then Qi =	10
CROSS SLOPE	2.0%	L(2)	16.5	If Li > L(2) then Qi =	18
STREET SLOPE	2.0%	L(3)	58.8	FB =	35
				CA(eqv.)=	4.62

5 YEAR FLOW

Q(5)	15	I(5)	4.3		
DEPTH	0.42	Fr	1.84	Inlet size ? L(i) =	6
SPREAD	14.8	L(1)	20.9	If Li < L(2) then Qi =	4
CROSS SLOPE	2.0%	L(2)	12.6	If Li > L(2) then Qi =	7
STREET SLOPE	2.0%	L(3)	44.9	FB =	11
				CA(eqv.)=	2.55

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 10-A

Total Flow: Q_5 = 21 cfs

Q_{100} = 21

Maximum allowable ponding depth at sump:

D_5 = 0.53

D_{100} = 0.53 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event:	14	foot inlet required
100-Year Event:	14	foot inlet required
INSTALL A PUBLIC	14	FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 10-B

Total Flow: Q_5 = 17 cfs

Q_{100} = 17

Maximum allowable ponding depth at sump:

D_5 = 0.53

D_{100} = 0.53 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 10 foot inlet required

100-Year Event: 10 foot inlet required

INSTALL A PUBLIC 10 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 10-B

Total Flow: Q_5 = 31 cfs

Q_{100} = 31

Maximum allowable ponding depth at sump:

D_5 = 0.53

D_{100} = 0.53 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 24 foot inlet required

100-Year Event: 24 foot inlet required

INSTALL A PUBLIC 24 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 13

Total Flow: Q_5 = 3 cfs

Q_{100} = 30 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 26 foot inlet required

INSTALL A PUBLIC 8 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT **14**

Total Flow: Q_5 = 4 cfs

Q_{100} = 7 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: [4] foot inlet required

100-Year Event: [4] foot inlet required

INSTALL A PUBLIC [6] FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 15

Total Flow: Q_5 = 6 cfs

Q_{100} = 19 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 14 foot inlet required

INSTALL A PUBLIC 14 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 16

Total Flow: Q_5 = 3 cfs

Q_{100} = 21 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 16 foot inlet required

INSTALL A PUBLIC 16 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 18

Total Flow: Q_5 = 5 cfs

Q_{100} = 9 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: [4] foot inlet required

100-Year Event: [4] foot inlet required

INSTALL A PUBLIC [6] FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 19

Total Flow: Q_5 = 3 cfs

Q_{100} = 7 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

INSTALL A PUBLIC 4 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 20

Total Flow: Q_5 = 3 cfs

Q_{100} = 6 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

INSTALL A PUBLIC 4 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 21

Total Flow: Q_5 = 3 cfs

Q_{100} = 6 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: [4] foot inlet required

100-Year Event: [4] foot inlet required

INSTALL A PUBLIC [4] FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 22

Total Flow: Q_5 = 3 cfs

Q_{100} = 5 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

INSTALL A PUBLIC 4 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 23

Total Flow: Q_5 = 4 cfs

Q_{100} = 7 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: [4] foot inlet required

100-Year Event: [4] foot inlet required

INSTALL A PUBLIC [4] FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 24

Total Flow: Q_5 = 2 cfs

Q_{100} = 4 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

INSTALL A PUBLIC 4 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 26

Total Flow: Q_5 = 3 cfs

Q_{100} = 5 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

INSTALL A PUBLIC 4 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 27

Total Flow: Q_5 = 5 cfs

Q_{100} = 10 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.50 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

INSTALL A PUBLIC 4 FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME:	*INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER:	*2190.00*
DATE:	*07/25/07*
CALCULATED BY:	*MAL*

DESIGN POINT 28

Total Flow: Q_5 = 5 cfs

Q_{100} = 34 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.50

D_{100} = 0.67 (dmax)

Qi = 1.7(Li+1.8(W))(dmax + w/12)^1.85

Clogging Factor = 1.25

Li (1.25) = Length of inlet opening

5-Year Event: [4] foot inlet required

100-Year Event: [18] foot inlet required

INSTALL A PUBLIC [18] FT D-10-R INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

JOB NAME: *INTERQUEST FILING NO. 6 - LOTS 1 & 2*
JOB NUMBER: *2190.00*
DATE: *7/25/2007*
CALCULATED BY: *MAL*

DESIGN POINT 3

Total Flow: Q(5) = 0 cfs
Q(100) = 0 cfs

Maximum allowable ponding depth at sump:

D(5) = 0.50 (d)
D(100) = 0.67 (dmax)

Qi = [(3.0)(P)(d1.5)]/F (Weir Conditions)

Clogging Factor (F) = 2.0

5-Year Event: 0.0 foot perimeter required

100-Year Event: 0.0 foot perimeter required

INSTALL A PUBLIC 0 FT D-9 INLET TO ACCEPT BOTH 5YR & 100 YR DEVELOPED FLOWS AT THIS DESIGN POINT.

Worksheet for Gutter - 10A-to dp-11

Project Description

Solve For	Discharge

Input Data

Channel Slope	0.01600	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Spread	20.00	ft
Roughness Coefficient	0.016	

Results

Discharge	20.16	ft^3/s
Flow Area	4.09	ft^2
Depth	0.49	ft
Gutter Depression	0.09	ft
Velocity	4.93	ft/s

Worksheet for Gutter - 10A-to dp-12

Project Description

Solve For	Discharge

Input Data

Channel Slope	0.01200	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Spread	20.00	ft
Roughness Coefficient	0.016	

Results

Discharge	17.46	ft^3/s
Flow Area	4.09	ft^2
Depth	0.49	ft
Gutter Depression	0.09	ft
Velocity	4.27	ft/s

Worksheet for Gutter - 10B-to dp-13

Project Description

Solve For	Discharge

Input Data

Channel Slope	0.04000	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Spread	20.00	ft
Roughness Coefficient	0.016	

Results

Discharge	31.87	ft^3/s
Flow Area	4.09	ft^2
Depth	0.49	ft
Gutter Depression	0.09	ft
Velocity	7.80	ft/s

Worksheet for Gutter - 10B-to dp-12

Project Description

Solve For	Discharge

Input Data

Channel Slope	0.02700	ft/ft
Gutter Width	2.00	ft
Gutter Cross Slope	0.06	ft/ft
Road Cross Slope	0.02	ft/ft
Spread	20.00	ft
Roughness Coefficient	0.016	

Results

Discharge	26.19	ft^3/s
Flow Area	4.09	ft^2
Depth	0.49	ft
Gutter Depression	0.09	ft
Velocity	6.41	ft/s

CLASSIC
CONSULTING
ENGINEERS & SURVEYORS

DETENTION POND EK-1A IMPROVEMENTS

Detention Pond EK-1A
Pond Improvement Cost Estimate
10/31/2007

Item	Quantity	Units	Unit Price	Total Costs
SURVEYING				
Construction Staking / As-builts	1	ls	$5,000.00	$5,000.00
SUBTOTAL				**$5,000.00**
SOILS ENGINEERING				
Soils Engineering	1	ls	$5,000.00	$5,000.00
SUBTOTAL				**$5,000.00**
EXCAVATION				
Mobilization	1	ls	$1,500.00	$1,500.00
Clear and Grub	3.5	acre	$1,500.00	$5,250.00
Import Fill / Structural Fill	11566	cy	$6.00	$69,396.00
SUBTOTAL				**$76,146.00**
EROSION CONTROL				
Silt Fence	1000	lf	$1.30	$1,300.00
Vehicle Tracking Control	2	ea	$1,000.00	$2,000.00
Erosion Control Blankets	11500	sy	$1.23	$14,145.00
Seeding	3	acre	$450.00	$1,350.00
SUBTOTAL				**$18,795.00**
STORM SEWER				
Outlet Strucuture with SWQ	1	ea	$20,000.00	$20,000.00
Relocate Low Flow Channel	510	lf	$10.00	$5,100.00
Emergency Spillway Rip Rap	1388	cy	$45.00	$62,460.00
Emergency Spillway Rip Rap - Sand Base	400	cy	$20.00	$8,000.00
Emergency Spillway Anchor Wall	55	cy	$400.00	$22,000.00
54" Outlet Pipe Filter Diaphragm	1	ea	$5,000.00	$5,000.00
Forebay Rip Rap Protection	1000	cy	$45.00	$45,000.00
SUBTOTAL				**$167,560.00**
MISCELLANEOUS				
Replace Existing Gravel Trail	222	cy	$30.00	$6,660.00
Replace Existing Concrete Road Section	83	cy	$100.00	$8,300.00
SUBTOTAL				**$14,960.00**

CONST. SUBTOTAL	$287,461.00
CONTINGENCY 10%	$28,746.10
SUBTOTAL	$316,207.10
CONST. MANAGE. 5%	$15,810.36
TOTAL COSTS	$332,017.46

Classic Consulting Engineers & Surveyors cannot and does not guarantee that the construction cost will not vary from these opinions of probable construction costs. These opinions represent our best judgment as design professionals familiar with the construction industry and this development in particular.

PROPERTY OWNER	%	Share of Costs
SRKO FAMILY LTD PARTNERSHIP (Jannie)	52.49	$174,275.96
INTERQUEST SOUTH (COPT)	31.77	$105,481.95
UNITED CHRISTIAN CAMPS (Jannie)	7.77	$25,797.76
TNC INTERQUEST B6-7 LLC	4.87	$16,169.25
INTERQUEST LAND DEVELOPMENT	1.85	$6,142.32
TNC INTERQUEST B5 LLC	1.25	$4,150.22
TOTAL	100.00	$332,017.46

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DRAINAGE MAP
EXISTING CONDITIONS

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DRAINAGE MAP
FULLY DEVELOPED CONDITIONS

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DRAINAGE MAP
FILING NO. 6 & 7 INTERIM CONDITIONS

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DRAINAGE MAP

LOTS 1 & 2

FILING NO. 6 FINAL DRAINAGE REPORT

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DETENTION POND EK-1A IMPROVEMENTS
PARTICIPATION MAP AND PROPOSED IMPROVEMENTS PLAN

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UNDEVELOPED BASIN EXHIBIT