

DRAINAGE REPORT

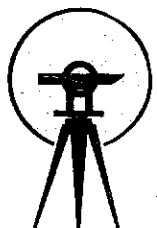
BOX CULVERT

at

STATE HIGHWAY 115 &

FISHER CANYON CHANNEL

MASTER



— DREXEL, BARRELL & CO. —

ENGINEERS — SURVEYORS

1700 38TH STREET

BOULDER, COLORADO 80301

(303) 442-4338

RETURN TO:
Land Development
201 West Castilla, Suite 122
Colorado Springs, CO 80903

DRAINAGE REPORT

BOX CULVERT

at

STATE HIGHWAY 115 &

FISHER CANYON CHANNEL

MASTER

Prepared for

Gates Land Development Co.

Prepared by

Drexel, Barrell & Co.

E-2589

May 25, 1982

RECEIVED

JUN 14 1982

1:00
PUBLIC WORKS
ENGINEERING

CITY OF COLORADO SPRINGS
The "America the Beautiful" City

DEPARTMENT OF PUBLIC WORKS CITY ENGINEERING DIVISION (303) 578-6606

30 S. NEVADA SUITE 403 P.O. BOX 1575

COLORADO SPRINGS, COLORADO 80901

July 6, 1982

Barbara N. Weiss
Drexel, Barrel & Co.
1700 38th Street
Boulder, CO 80301

Re: Box Culvert at State Highway 115 & Fisher Canyon Channel
Dear Barbara:

We are in receipt of your drainage report for the referenced project and will file said report subject to the following items:

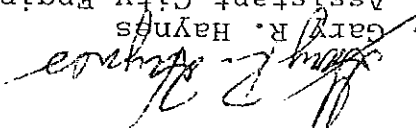
1. A transition channel is required from the east end of the proposed box culvert to the Cheyenne Meadows Channel. The transition channel must be designed for subcritical flow.

2. The SCS flow quantity given in your previous report must be used to design the capacity of the proposed facilities.

3. This report and the construction design drawings must be approved by the State Highway Department.

The City Engineering Division has no objections to your proceeding with the detailed design drawings subject to the above comments.

Sincerely,


Gary R. Haynes
Assistant City Engineer

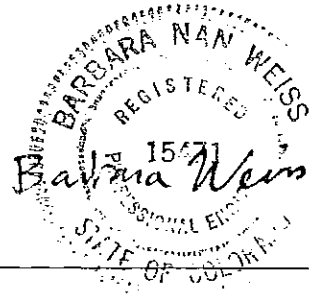
for Gerald J. Gromko
City Engineer

GRH/ro

cc: Bob Svejkovsky, Gates Land Company

CERTIFICATIONS

I, Barbara Weiss, a registered engineer in the State of Colorado, hereby certify that the attached drainage plan and report for a box culvert at Fisher Channel at Highway 115, were prepared under my direction and supervision and are correct to the best of my knowledge and belief. I further certify that said drainage report is in accordance with all City of Colorado Springs ordinances, specifications, and criteria to the best of my knowledge.



The developer has read and will comply with all of the requirements specified in this drainage report as approved by the City Engineer.

By 
Robert F. Svejksky
Director of Engineering

DRAINAGE REPORT
BOX CULVERT AT STATE HIGHWAY 115 AND
FISHER CANYON CHANNEL FOR GATES LAND DEVELOPMENT COMPANY

The purpose of this report is to request approval for a proposed box culvert at Fisher Canyon Channel under State Highway 115, Colorado Springs, Colorado. After approval is granted by the City of Colorado Springs, construction plans will be submitted. This report presents the drainage criteria used to design the box culvert.

The 100 year flow in Fisher Canyon Channel at Highway 115 is 1355 cfs. This flow was documented in a report prepared by Drexel, Barrell in August, 1978, (attached). The proposed size for the box culvert is a double 10' x 6' high box which has been approved by the State of Colorado. Please see the attached letter.

An hydraulic analysis was performed to determine the water surface profile thru the culvert. A hand analysis and check with HEC-2 computer program, both utilizing the Standard Step Method, was performed to determine energy losses from the channel section to the box culvert. Because supercritical flow exists the analysis proceeds downstream. Calculations show a double 10' x 6' high box culvert to be adequate. Maximum flow depth is 4'.

Calculations, vicinity map and plan of the culvert and transition follow in the appendix.

Respectfully submitted,

Barbara Weiss

Barbara Weiss

APPENDIX

<u>Description</u>	<u>Exhibit</u>
Drainage Calculations at Star Ranch Road & Highway 115	A
Letter from State	B
Flow analysis	C
Computer analysis	D
Plan of Culvert on topo map	Back Cover

Project DRAINAGE - STARZ RANCH RD / ST. Hwy. 115		Job No E-1986
Client GATES LAND CO.	By DH	Date Aug 78

exhibit A

DRAINAGE CALCULATIONS

STARZ RANCH RD
@
STATE HIGHWAY 115

FOR

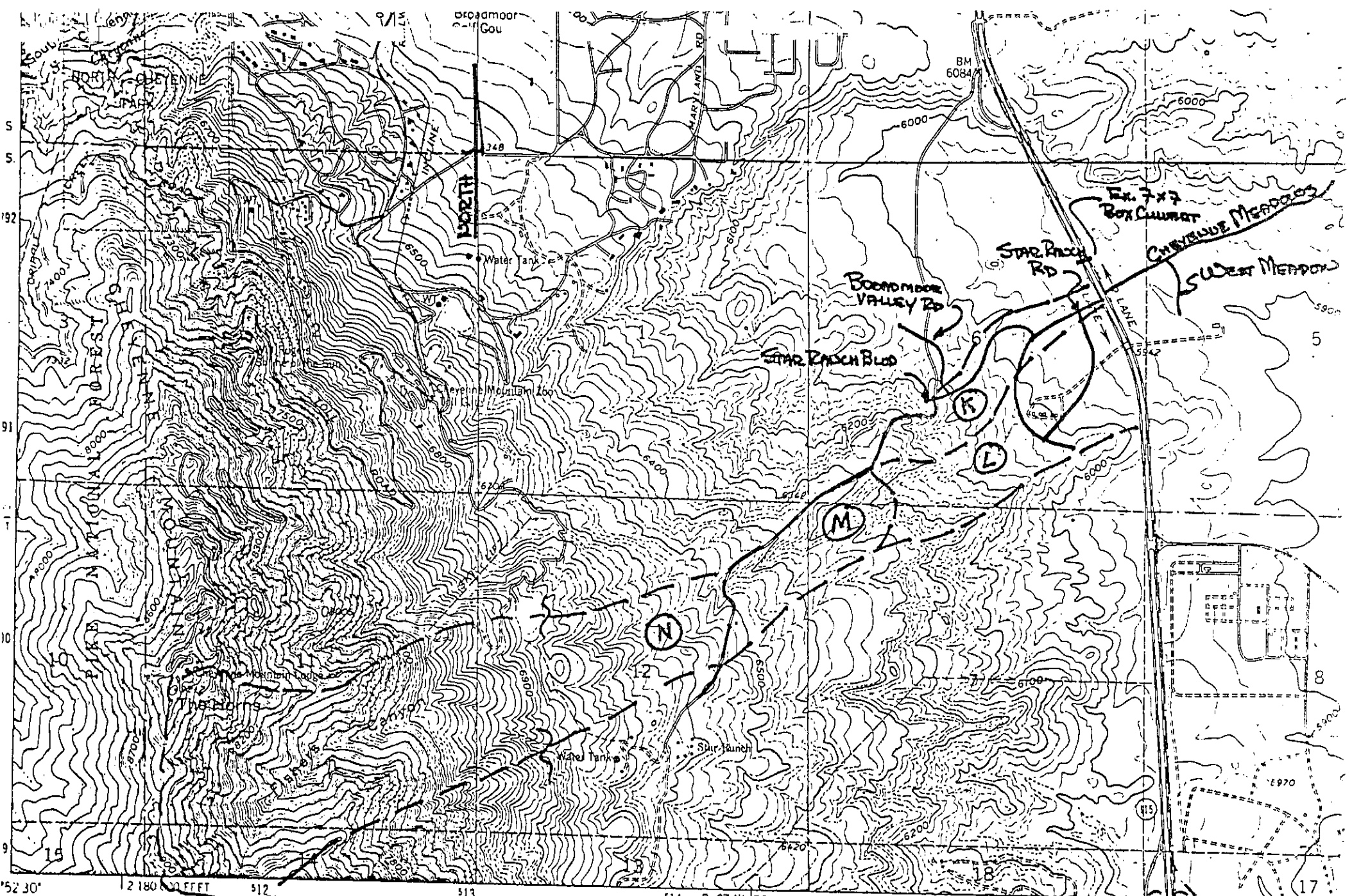
GATES LAND CO.
AUG 78

DESIGN METHOD:

SCS METHOD, MODIFIED TO
COLORADO SPRINGS, STANDARDS

CITY OF COLORADO SPRINGS

"DETERMINATION OF STORM RUNOFF CRITERIA"
MARCH 77



Mapped, edited, and published by the Geological Survey in cooperation with U. S. Corps of Engineers

Control by USGS and USC&GS

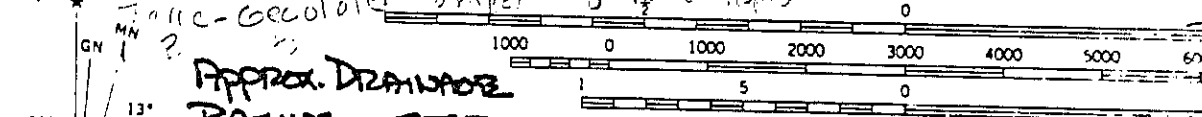
Topography by photogrammetric methods from aerial photographs taken 1947 Field checked 1948 Revised from aerial photographs taken 1960 Field checked 1961

Polyconic projection 1927 North American datum

I-1-EF rock sample - Tolson 2m 9-210%
 X10-F Jar-Terets
 Talle-Geolater sample 3-16% 10%

514 R. 67 W. 150° 515 R. 66 W. ICHEYENNE MOUNTAINS PENROSE 28 MI CANON CITY 46 MI

SCALE 1:24 000



APPROX. DRAINAGE BASINS - STAR RANCH RD ST. HWY. 115

CONTOUR INTERVAL 20 FEET

Project DRAINAGE - STARRANCH RD / ST. HWY 115		Job No E-1986
Client GATES LAND CO.	By DH	Date Aug 78

AREA K - (TRIBUTARY TO STARRANCH ROAD & BOULEVARD)

AREA = 40 A

$$T_c = \left(\frac{11.9 \left(\frac{5200}{5280} \right)^3}{350} \right)^{.385} \times 60 = 16.4 \text{ min.}$$

SOIL GROUP = C+ (SCS REGIONAL OFFICE)

CN = 84 (MEDIUM DENSITY HOUSING - 1/4 ACRE)

5 year - 6 hour storm

$P = 2.1" \Rightarrow Q = 0.82"$

$q_p = 950 \text{ csm/m.}$

$$q = q_p A Q = (950) \left(\frac{40}{640} \right) (.82) = 48.7 \text{ cfs}$$

100 year - 6 hour storm

$P = 3.5" \Rightarrow Q = 1.94"$

$$q = (950) \left(\frac{40}{640} \right) (1.94) = 115.2 \text{ cfs}$$

$t_p = 3.16 \text{ hrs} \Rightarrow t_p = \frac{D}{2} + 0.6 t_c = 3.16 \text{ hrs}$
(by 10 Grants Manual)

$t_b = 8.45 \text{ hrs.}$

100 yr - 6 hr / HISTORIC

CN = 76, $P = 3.5" \Rightarrow Q = 1.36$

$$q = (950) \left(\frac{40}{640} \right) (1.36) = 80.8 \text{ cfs}$$

$t_p = 3.16 \text{ hr}$

$t_b = 8.45 \text{ hr}$

Project DRAINAGE - STAR RANCH RD / ST. HWY 115		Job No. E-1986
Client GATES LAND CO.	By DH	Date Aug 78

AREA 1 -

AREA = 178 ACRES

$$P_c = \left(\frac{11.9 \left(\frac{5200}{5280} \right)^3}{240} \right)^{.385} \times 60 = 19 \text{ min.}$$

SOIL GROUP = C (SCS REGIONAL OFFICE)

CN = 83 (1/4 acre residential)

5 year - 6 hour storm

$$P = 2.1'' \Rightarrow Q = .76''$$

$$g_p = 900 \text{ csm/in}$$

$$g = g_p A Q = 900 \left(\frac{178}{640} \right) (.76) = 190 \text{ cfs}$$

100 year - 6 hour storm

$$P = 3.5'' \Rightarrow Q = 1.86''$$

$$g = 465.6 \text{ cfs}$$

$$t_p = 3.19 \text{ hrs}$$

$$t_b = 8.52 \text{ hrs.}$$

100 yr - 6 hr / HISTORIC

$$CN = 74, P = 3.5'' \Rightarrow Q = 1.24$$

$$g = 400 \left(\frac{178}{640} \right) (1.24) = 310.4 \text{ cfs}$$

$$t_p = 3.19, t_b = 8.52 \text{ hr}$$

Project DRAINAGE - STAR RANCH RD / ST. Hwy 115		Job No E-1986
Client GATES LAND CO.	By DH	Date Aug 78

AREA M -

$$\text{AREA} = 75.2 \text{ A}$$

$$T_c = \left(\frac{11.9 \left(\frac{3000}{5280} \right)^3}{270} \right)^{.385} \times 60 = 9.4 \text{ min.}$$

SOIL GROUP = B (SCS REGIONAL OFFICE)

CN = 75 (1/4 ACRE - RESIDENTIAL)

5 year - 6 hour storm -

$$P = 2.1" \Rightarrow Q = 0.43"$$

$$q_p = 1150 \text{ csm/in}$$

$$q = q_p A Q = 1150 \left(\frac{75.2}{640} \right) (.43) = 58.1 \text{ cfs}$$

100 year - 6 hour storm -

$$P = 3.5" \Rightarrow Q = 1.3"$$

$$q = 175.7 \text{ cfs}$$

$$t_p = 3.09 \text{ hrs}$$

$$t_b = 8.26 \text{ hrs}$$

100 yr - 6 hr / HISTORIC

$$CN = 61, P = 3.5" \Rightarrow Q = 0.59$$

$$q = 1150 \left(\frac{75.2}{640} \right) (0.59) = 79.7 \text{ cfs}$$

$$T_p = 3.09 \text{ hr} \quad T_b = 8.26 \text{ hr}$$

Project DRAINAGE - STAR RANCH RD / ST. HWY 115		Job No E-1936
Client GATES LAND CO.	By DH	Date Aug 78
<p><u>AREA N</u></p> <p>AREA = 469 A</p> <p>$T_c = \left(\frac{11.9 \left(\frac{11500}{5280} \right)^3}{2550} \right)^{.385} \times 60 = 18.7 \text{ min.}$</p> <p>SOIL GROUP = B (SCS REGIONAL OFFICE)</p> <p>CN = 70 (1/4 acre residential ≠ open space)</p> <p><u>5 year - 6 hour storm</u></p> <p>$P = 2.1'' \Rightarrow Q = .28''$</p> <p>$q_p = 905 \text{ csm/in.}$</p> <p>$q = q_p A Q = (905) \left(\frac{469}{640} \right) (.28) = 185.7 \text{ cfs}$</p> <p><u>100 year - 6 hour storm</u></p> <p>$P = 3.5'' \Rightarrow Q = 1.01''$</p> <p>$q = 669.8 \text{ cfs}$</p> <p>$t_p = 3.19 \text{ hrs.}$</p> <p>$t_b = 8.51$</p> <hr/> <p><u>100 yr - 6 hr / HISTORIC</u></p> <p>CN = 61, P = 3.5' Q = 0.59</p> <p>$q = 905 \left(\frac{469}{640} \right) (0.59) = 391.3 \text{ cfs}$</p> <p>$t_p = 3.19 \quad t_b = 8.51$</p>		

Project

Hydrographs - 100yr - Historic

Job No

E-1986

Client

By

DH

Date

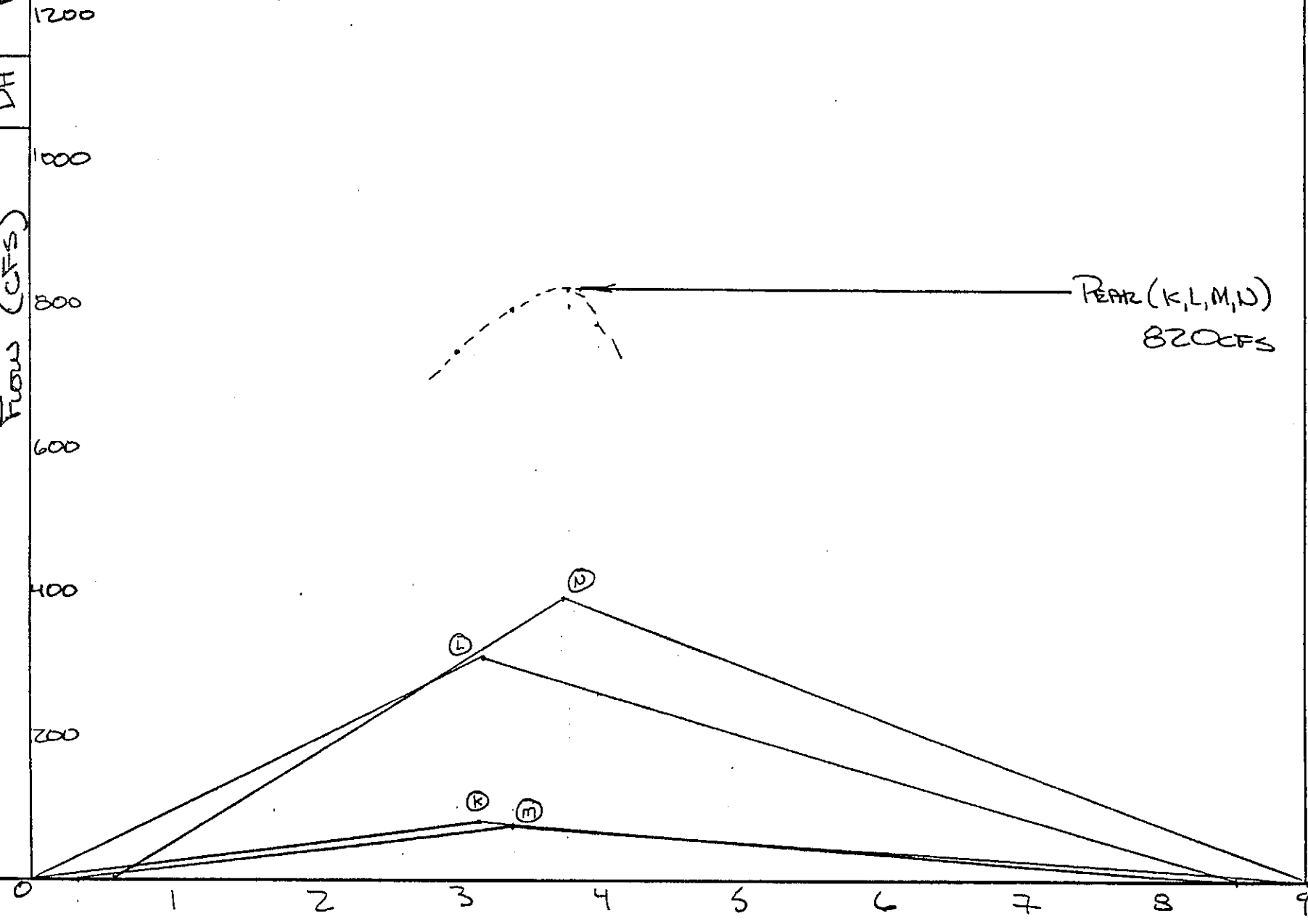
Aug 79

Flow (cfs)

1200
800
600
400
200
0

0 1 2 3 4 5 6 7 8 9

Time (hrs)



Project

HYDROGRAPHS - 100 YEAR - DEVELOPED

Job No

E-1986

Client

By

DH

Date

Aug 78

FLOW (CFS)



TIME (HRS)

Project

HYDROGRAPHS - 5 YEAR - DEVELOPED

Job No

E-1986

Client

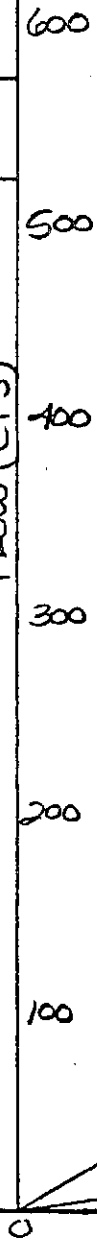
By

DA

Date

Aug 78

FLOW (CFS)



TIME (HRS)



EXHIBIT B
COLORADO STATE DEPARTMENT OF HIGHWAYS

DIVISION OF HIGHWAYS

May 12, 1982

RECEIVED

MAY 14 '82

Gates Land Company
155 West Lake Avenue
Colorado Springs, CO 80906

GATES LAND COMPANY

ATTENTION: ROBERT F. SVEJKOVSKY, DIRECTOR OF ENGINEERING

Dear Mr. Svejksky:

We wish to clarify the intent of your letter of October 13, 1981. If we understand it correctly, Gates Land Company will construct, at its expense, a double 10'x6' concrete box culvert under S.H. 115, south of Star Ranch Road. They require that the Colorado Department of Highways construct and maintain a detour for S.H. 115 traffic during the course of the construction.

If this is your intention please notify us and we will have an agreement written by our Denver Headquarters.

Sincerely,

E. N. HAASE
CHIEF ENGINEER

By H. W. Harris
H. W. HARRIS
District Engineer

HWH:ss

RECEIVED

MAY 17 1982

OREXEL BARRELL & CO

Project

FISHER CANYON AT Hwy 115

Job No

Client

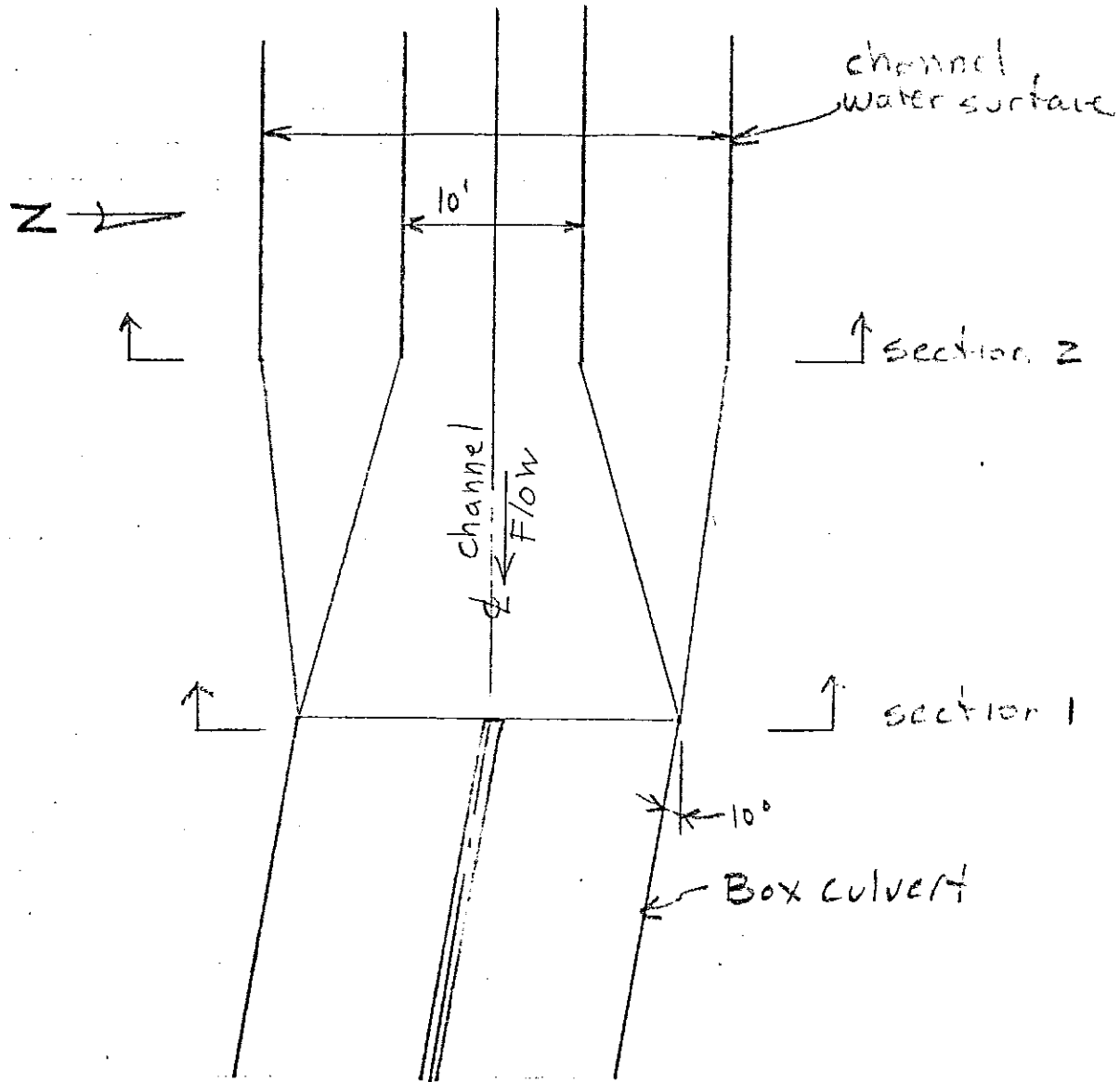
GATES EXHIBIT C

By

BNW

Date

21 May 82



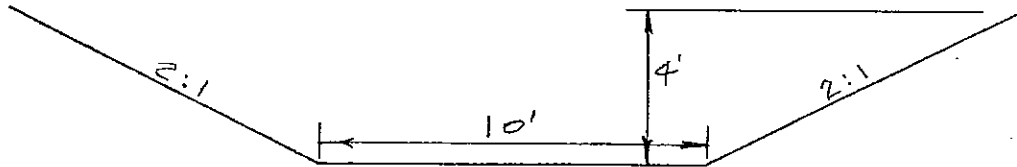
Plan view Transition to Box culvert

Scale: 1" = 10'

Project: FISHER CANYON AT Hwy 115
Job No: E 2589

Client: GATES
By: BNW
Date: 21 MAY 82

CROSS SECTION OF TRANSITION TO BOX CULVERT



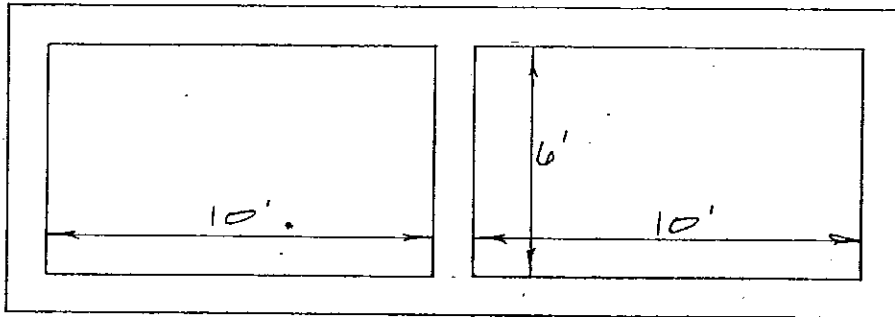
SECTION 2

DESIGN DATA: $S = 1.04\%$ $n = 0.015$ $Q = 1355 \text{ cfs}$

$D = 4'$ $D_c = 5.7'$ $Z = 2$ $V = 19 \text{ fps}$

$$F = \frac{V}{\sqrt{gL}} \quad L = \frac{7Z}{26} = 2.77$$

$F = 2.01$



SECTION 1
DBL. BOX CULVERT

$$F = \frac{V}{\sqrt{gL}} \quad L = \frac{6D}{20} = 3.45 \text{ @ entrance } F = 1.85$$

$D_c = 5.2'$ $S = 1.04\%$ $D_{\text{normal}} = 4'$

Project: FISHER CANYON AT Hwy 115
Job No: E 2589

Client: GATES
By: BNW
Date: 21 May 82

WATER SURFACE ELEVATION

Std. Step Method

$$a. WS_2 + \frac{\alpha_2 V_2^2}{2g} = WS_1 + \frac{\alpha_1 V_1^2}{2g} + h_e$$

$$b. h_e = L \bar{S}f + C \left[\frac{\alpha_2 V_2^2}{2g} - \frac{\alpha_1 V_1^2}{2g} \right]$$

$L = 20'$ $\alpha = 1.0$ $V_2 = 19 \text{ fps}$ $C = 0.3$
 ASSUME $d_1 = 3.45$ $A = 60 \text{ sq ft}$ $\therefore V_1 = 19.6 \text{ fps}$
 $Sf @ 2 = 0.0104$

$$Sf @ 1 = \frac{(0.015) 1355}{1.49(60)(1.6)} = 0.015 = Sf_1$$

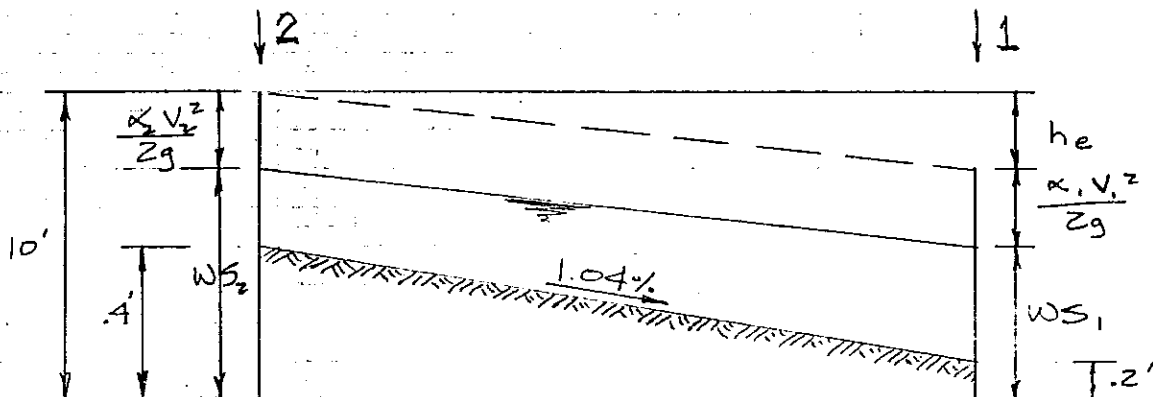
$$\bar{S}f = \frac{Sf_1 + Sf_2}{2} = 0.013 \quad \bar{S}f(L) = 0.26$$

$$b) h_e = 0.26 + .3 \left| (5.6 - 5.97) \right| = 0.37$$

$$\frac{\alpha_1 V_1^2}{2g} = 5.6 \quad \frac{\alpha_2 V_2^2}{2g} = 5.97$$

$$a) 10 = 3.65 + 5.97 + .37$$

$$10 = 9.99 \quad \checkmark$$



```

X   X   XXXXXXX   XXXXX           XXXXX
X   X   X         X   X           X   X
X   X   X         X                 X
XXXXXXXX XXXX     X           XXXXX   XXXXX
X   X   X         X                 X
X   X   X         X   X           X
X   X   XXXXXXX   XXXXX           XXXXXXX

```

1
06/04/82. 09.39.10.

```

*****
HEC2 RELEASE DATED NOV 76 UPDATED APRIL 1980
ERROR CORR - 01,02,03,04
MODIFICATION - 50,51,52,53,54
*****

```

T1 E2589
T2 GATES
T3

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	0.	0.	0.	1.	0.000000	0.00	0.0	1355.	34.200	0.000
NC	0.000	0.000	.015	.100	.300	0.000	0.000	0.000	0.000	0.000
X1	1.000	4.000	10.000	40.000	0.000	0.000	20.000	0.000	0.000	0.000
GR	35.200	10.000	30.200	20.000	30.200	30.000	35.200	40.000	0.000	0.000
X1	1.500	4.000	12.200	37.500	0.000	0.000	0.000	0.000	0.000	0.000
GR	35.100	12.200	30.100	17.000	30.100	33.000	35.100	37.500	0.000	0.000
X1	2.000	8.000	14.500	35.510	0.000	0.000	10.000	0.000	0.000	0.000
GR	36.000	14.500	30.000	14.510	30.000	24.500	36.000	24.510	36.000	36.000
GR	30.000	25.510	30.000	35.500	36.000	35.510	0.000	0.000	0.000	0.000
X1	3.000	8.000	14.500	35.510	0.000	0.000	0.000	0.000	0.000	0.000
GR	35.900	14.500	29.900	14.510	29.900	24.500	35.900	24.510	35.900	35.900
GR	29.900	25.510	29.900	35.500	35.900	35.510	0.000	0.000	0.000	0.000
EJ	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

1
06/04/82. 09.39.10.

SECNO	DEPTH	CWSEL	CRISW	WSELK	EG	HV	HL	GLOSS	BANK ELEV
Q	QLOS	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLCB	VCH	VROB	XLN	XLNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOEL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

GR	25.200	10.000	30.200	20.000	30.200	30.000	35.200	40.000	0.000
X1	1.500	4.000	12.200	37.500	0.000	0.000	0.000	0.000	0.000
GR	35.100	12.200	30.100	17.000	30.100	33.000	35.100	37.500	0.000
X1	2.000	8.000	14.500	35.510	0.000	0.000	10.000	0.000	0.000
GR	36.000	14.500	30.000	14.510	30.000	24.500	36.000	24.510	36.000
GR	30.000	25.510	30.000	35.500	36.000	35.510	0.000	0.000	0.000
X1	3.000	8.000	14.500	35.510	0.000	0.000	0.000	0.000	0.000
GR	35.900	14.500	29.900	14.510	29.900	24.500	35.900	24.510	35.900
GR	29.900	25.510	29.900	35.500	35.900	35.510	0.000	0.000	0.000
EJ	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000

1
06/04/82. 09.39.10.

Water surface elevation

SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	QLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLDB	VCH	VRDB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

*PROF 1

CCHV= .100 CEHV= .300

*SECNO 1.000

1.00	4.00	34.20	35.85	34.20	39.70	5.50	0.00	0.00	35.20
1355.	0.	1355.	0.	0.	72.	0.	0.	0.	35.20
0.00	0.00	18.82	0.00	.000	.015	.000	0.000	30.20	12.00
.010189	0.	0.	0.	0	10	0	0.00	26.00	38.00

0

*SECNO 1.500

1.50	3.64	33.74	35.47	0.00	39.48	5.73	.20	.02	35.10
1355.	0.	1355.	0.	0.	71.	0.	0.	0.	35.10
.00	0.00	19.21	0.00	.000	.015	.000	.014	30.10	13.51
.009913	0.	20.	0.	9	11	0	0.00	22.77	36.27

0

*SECNO 2.000

3265 DIVIDED FLOW

2.00	3.45	33.45	35.22	0.00	39.45	6.00	0.00	.03	36.00
1355.	0.	1355.	0.	0.	69.	0.	0.	0.	36.00
.00	0.00	19.66	0.00	.000	.015	.000	.014	30.00	14.50
.015205	0.	0.	0.	7	11	0	0.00	20.00	35.51

0

*SECNO 3.000

3265 DIVIDED FLOW

3.00	3.49	33.39	35.13	0.00	39.26	5.87	.15	.04	35.90
1355.	0.	1355.	0.	0.	70.	0.	0.	0.	35.90
.00	0.00	19.45	0.00	.000	.015	.000	.014	29.90	14.50
.014756	0.	10.	0.	3	11	0	0.00	20.00	35.51

0

1

06/04/82. 09.39.10.