(Amendment Letter) To the MASTER DEVELOPMENT DRAINAGE PLAN AND FINAL DRAINAGE REPORT for SHILOH MESA FILING NO. 1

January 2016

Prepared for:

COLA, LLC 1710 Jet Stream Drive Colorado Springs, CO 80921 Mike DeGrant

Prepared by:



20 Boulder Crescent, Suite 110 Colorado Springs, CO 80903 (719) 955-5485

(Amendment Letter)

To the

MASTER DEVELOPMENT DRAINAGE PLAN AND FINAL DRAINAGE REPORT for SHILOH MESA FILING NO. 1

DRAINAGE PLAN STATEMENTS

ENGINEER'S STATEMENT

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the established criteria for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or prissions on my part in preparing this report.

Virgil A.	Sanchez,	P.E.	#371	60

For and on Behalf of M & S Civil Consultants, Inc.

DEVELOPER'S STATEMENT

I, the developer, have read and will comply with all the requirements specified in this drainage report and plan.

COLA, LLC

TITLE:

Owner & Manager

DIRECTOR

ADDRESS:

COLA, LLC

1710 Jet Stream Drive

Colorado Springs, CO 80921

CITY OF COLORADO SPRINGS

Filed in accordance with Section 7-7-906 of the Code of the City of Colorado Springs, 2001, as amended.

For the City Engineer

DATE: 6/6/16

DATE: 5-31-16

CONDITIONS:

(Amendment Letter)

To the

MASTER DEVELOPMENT DRAINAGE PLAN AND FINAL DRAINAGE REPORT for SHILOH MESA FILING NO. 1

CERTIFICATION STATEMENT

"This report and plan for the final drainage design of Shiloh Mesa Filing No. 1 was prepared by me (or under my direct supervision) in accordance with the provisions of City of Colorado Springs Drainage Criteria Manual Volumes 1 and 2, Drainage Design and Technical Criteria for the owners thereof. I understand that the City of Colorado Springs does not and will not assume liability for drainage facilities designed by others."

SIGNATURE:

Virgil A. Sanonez, 10 RA#37160 For and on Behalf of Mr. 25 Civil Consultants, Inc.

"COLA, LLC hereby certifies that the drainage facilities for of Master Development Drainage Plan and Final Drainage Report for Shiloh Mesa & Shiloh Mesa Filing No. 1 shall be constructed according to the design presented in this report. COLA, LLC understand that the City of Colorado Springs does not and will not assume liability for the drainage facilities designed and/or certified by my engineer and that the City of Colorado Springs reviews drainage plans pursuant to Colorado Revised Statutes, Title 30, Article 28 (verify reference to CRS); but cannot, on behalf of Master Development Drainage Plan and Final Drainage Report for Shiloh Mesa & Shiloh Mesa Filing No. 1, guarantee that final drainage design review will absolve COLA, LLC and/or their successors and /or assigns of future liability for improper design. COLA, LLC further understand that approval of the final plat does not imply approval of my engineer's drainage design."

COLA, LLC

Mike DeGrant

AUTH REP

DATE: 5-31-16

May 31, 2016

City of Colorado Springs
Subdivision Engineering Review Team
30 South Nevada Avenue, Suite 401
Colorado Springs, Colorado 80903
Attn: Elizabeth Nijkamp

RE: Amendment Letter to the Master Development Drainage Plan and Final Drainage Report for Shiloh Mesa Filing No. 1.

Dear Elizabeth,

The following is the Final Drainage Letter to amend the Master Development Drainage Plan and Final Drainage Report for Shiloh Mesa & Shiloh Mesa Filing No. 1, prepared by M&S Civil Consultants dated December 2015.

This letter is prepared to accompany the approved Development Plan amendment for Shiloh Mesa at Woodmen Heights (68.88 acres, 232 lots) and the recorded Final Plat for Shiloh Mesa Filing No. 1 (19.956 acres, 46 lots). The amendment to the Development Plan and Final Plat consisted of a four items; (1) reduction of lots from 236 to 232, (2) Minor lot line adjustments, (3) Incorporation of an eyebrow along Kenosha Drive between Barham Place and Sandsmere Drive, (4) Revision to the foot print of the south Water Quality Pond to better facilitate the construction and the aesthetics. Specifically, the walls within the pond were eliminated and the pond slopes were laid back at 3:1. The emergency spillway was moved from the west side along Marksheffel Road, to the south side of the pond along Kenosha Drive. Emergency overflow will exit onto Kenosha Drive and be routed via curb and gutter to Marksheffel Road. The sand filter area and pond volume requirements are still met as indicated by the attached Design Procedure Form: Sand Filter (SF) and the revised SDI Design Data Sheet (The SDI data sheet was revised - Exhibit 3). All storm sewer pipe and storm sewer structures out falling, and discharging from the pond were not be altered (see Exhibit 1 revised South WQ Pond). Due to changes of the pond side slopes, the placement of boulders adjacent to the 42" RCP and rip rap pad were needed. In general, no impacts to drainage patterns and/or quantities to storm facilities were required.

It is important to note, no changes to the fees were necessary, as a result of the approved minor amendments. Refer to the approved Master Development Drainage Plan and Final Drainage Report for Shiloh Mesa & Shiloh Mesa Filing No. 1, for Construction Cost Opinion, Drainage/Bridge/Pond Fees and Drainage Cost Comparison and Credit Summary fees.

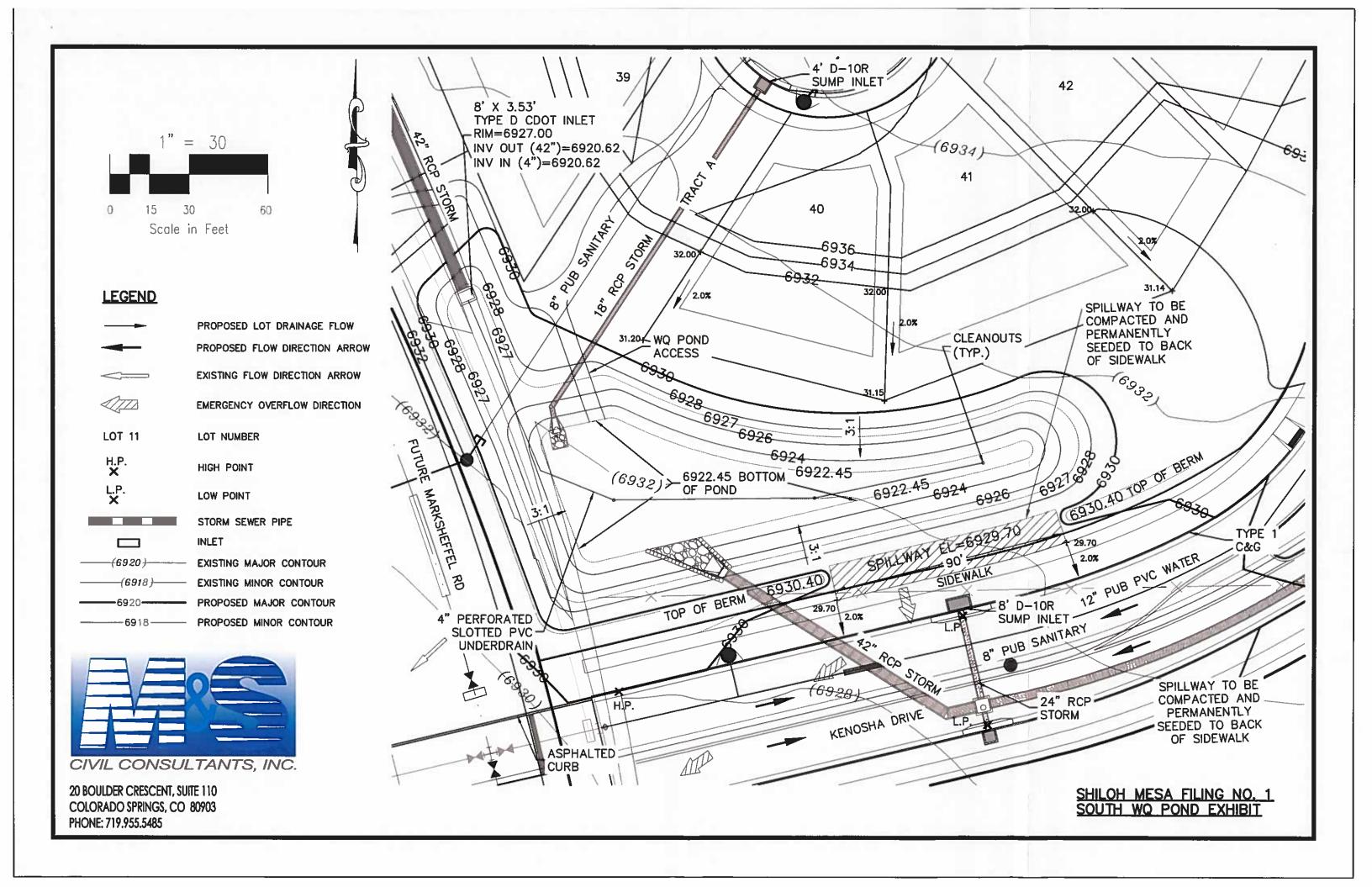
This amendment drainage letter and development of the Shiloh Mesa residential subdivisions will not adversely affect adjacent or downstream property and is in accordance with the Master Development Drainage Plan and Final Drainage Report for Shiloh Mesa & Shiloh Mesa Willing No. 1.

Respectfully,

Virgil A. Sanchez, P.E. M&S Civil Consultants, Inc.



Attachements: Exhibit 1-South WQ Pond Exhibit 2-Design Procedure Form: Sand Filter (SF) Exhibit 3-SDI Design Data Sheet



	Design Procedure Fo	rm: Sand Filter (SF)				
Designer:	Eugene Tellez	Sheet 1 of 2				
Company:	MS Civil Consultants					
Date:	March 17, 2015					
Project:	Shiloh Mesa South Water Quality Pond					
Location:	Northeast of Marksheffel Road and Kenosha Road Intersection					
1. Basin Sto	prage Volume	T				
A) Effecti (100%	ive Imperviousness of Tributary Area, 1, 5 if all paved and roofed areas upstream of sand filter)	I ₄ = <u>65.0</u> %				
	ary Area's imperviousness Ratio (i = I _s /100)	i = 0.650				
C) Water WQ0	r Quality Capture Volume (WQCV) Based on 12-hour Drain Time CV= 0.9 ° (0.91° I³ - 1.19 ° I² + 0.78 ° i)	WQCV = 0.23 watershed inches				
D) Contr	tbuting Watershed Area (including sand filter area)	Area = <u>945,256</u> sq ft				
E) Water Vwqc	r Quality Capture Volume (WQCV) Design Volume _{SV} = WQCV / 12 * Area	V _{wacv} = <u>18,017</u> cu ft				
F) For W Avera	/atersheds Outside of the Denver Region, Depth of age Runoff Producing Storm	d _e = in				
	Vatersheds Outside of the Denver Region, r Quality Capture Volume (WQCV) Design Volume	Vwqcyoner =cu ft				
H) User (Only i	Input of Water Quality Capture Volume (WQCV) Design Volume if a different WQCV Design Volume is desired)	V _{WQCVUSER} =cu ft				
2. Basin Ge	ometry					
A) WQCV	/ Depth	D _{WQCV} = 6.0 ft				
B) Sand F 4:1 or	Filter Side Slopes (Hortzontal distance per unit vertical, flatter preferred). Use "0" if sand filter has vertical walls.	Z = 3.00 ft / ft DIFFICULT TO MAINTAIN, INCREASE WHERE POSSIBLE				
C) Mimim	eum Filter Area (Flat Surface Area)	A _{the} = 4004 sq ft				
D) Actual	Filter Area	A _{retur} =4145 sq ft				
E) Volume	e Provided	V _T =33,811_cuft				
3. Filter Mate	erial	Choose One				
		18" CDOT Class C Filter Material Other (Explain):				
4. Underdrai	in System	- Chassa One -				
A) Are un	derdrains provided?	Choose One O YES O NO				
B) Under	drain system orifice diameter for 12 hour drain time	L				
	Distance From Lowest Elevation of the Storage Volume to the Center of the Orifice	y= <u>1.8</u> ft				
	ii) Volume to Drain in 12 Hours	Vol ₁₂ = 18.017 cu ft				
	iii) Orifice Diameter, 3/8" Minimum	D _o = in				

EXHIBIT 2

	Design Procedure	Form: Sand Filter (SF)
Designer: Company: Date: Project: Location:	Eugene Tellez MS Civil Consultants March 17, 2015 Shiloh Mesa South Water Quality Pond Northeast of Marksheffel Road and Kenosha Road Intersection	Sheet 2 of
A) is an	hable Geomembrane Liner and Geotextile Separator Fabric Impermeable liner provided due to proximity ructures or groundwater contamination?	Choose One O YES ® NO
6-7. Inlet / Ou A) Descriconve	ribet Works ribe the type of energy dissipation at inlet points and means of eying flows in excess of the WQCV through the outlet	Riprap pad provided where needed. Emergency overflow provided to carry excess flow.
Notes:		

Stormwater Detention and Infiltration Design Data Sheet

User Defined

Stage [ft]

0.00

0.55

User Defined

Area [ft^2]

4,145

4,792

Workbook Protected

Worksheet Protected

User Defined

Stage [ft]

0.00

0.55

User Defined

Discharge [cfs]

0.00

0.29

Stormwater Facility Name: Shiloh Mesa - Southern WQ Sand Filtration Basin

Facility Location & Jurisdiction: SW corner of the Shiloh Mesa Site, Colorado Spring

User (Input) Watershed Characteristics

Watershed Slope =	0.011	ft/ft
Watershed Length-to-Width Ratio =	1.40	L:W
Watershed Area =	21.70	acres
Watershed Imperviousness =	65.0%	percent
Percentage Hydrologic Soil Group A =	57.4%	percent
Percentage Hydrologic Soil Group B =	42.6%	percent
Percentage Hydrologic Soil Groups C/D =	0.0%	percent
		•

Location for 1-hr Rainfall Depths (use dropdown):

User Input	▼

User Input: Detention Basin Characteristics
WQCV Design Drain Time = 12.00 hours

1.55	6,098	1.55	0.50
2.55	7,841	2.55	0.62
3.55	9,148	3.55	0.75
4.55	11,761	4.55	0.90
5.55	13,504	5.55	59.00
6.55	15,682	6.55	96.00
7.25	16,988	7.25	115.00
1			
6			

After completing and printing this worksheet to a pdf, go to: https://maperture.digitaldataservices.com/gvh/?viewer=cswdif create a new stormwater facility, and attach the pdf of this worksheet to that record.

Routed Hydrograph Results

	Kouted Hydra	graph Results					_
Design Storm Return Period =	WQCV	2 Year	5 Year	10 Year	50 Year	100 Year	
One-Hour Rainfall Depth =	0.50	1.19	1.50	1.75	2.25	2.52	in
Calculated Runoff Volume =	0.364	1.227	1.668	2.080	3.126	3.660	acre-ft
OPTIONAL Override Runoff Volume =							acre-ft
Inflow Hydrograph Volume =	0.363	1.227	1.668	2.079	3.125	3.659	acre-ft
Time to Drain 97% of Inflow Volume =	12.0	17.3	16.7	16.2	15.1	14.6	hours
Time to Drain 99% of Inflow Volume =	14.4	19.9	19.2	18.7	17.9	17.5	hours
Maximum Ponding Depth =	2.31	4.73	4.92	5.10	5.51	5.74	ft
Maximum Ponded Area =	0.170	0.277	0.285	0.292	0.308	0.319	acres
Maximum Volume Stored =	0.299	0.826	0.880	0.930	1.052	1.126	acre-ft

EXHIBIT 3

Stormwater Detention and Infiltration Design Data Sheet

