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MASTER DRAINAGE REPORT
FOR
THE VILLAGE AT SKYLINE

MASTER DRAINAGE REPORT
FOR
THE VILLAGE AT SKYLINE

(Revised June 27, 1985)

Prepared For:
Jensen/Markley Group
1586 South 21st Street, Suite B
Colorado Springs, Colorado 80904

Prepared By:
Richards Engineering and
Land Surveying, Inc.
1685 W. Uintah, Suite 103
Colorado Springs, Colorado 80904



Job No. 1184-4

June 14, 1985

Mr. Gary Haynes
City Engineer
City of Colorado Springs
30 S. Nevada
Colorado Springs, CO 80903

Dear Mr. Haynes:

RE: Master Drainage Report for the
Village at Skyline

Transmitted herewith is a copy of the Master Drainage Report
for the Village at Skyline, for your review and approval.

If you have any questions, please contact our office.

Sincerely,

RICHARDS ENGINEERING AND
LAND SURVEYING, INC.

Michael J. Vinson P.E.
Project Engineer

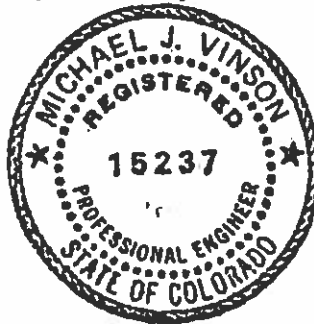
ald

DRAINAGE CERTIFICATION

ENGINEER'S CERTIFICATION

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City for drainage reports and said report is in general conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors, or omissions on my part in preparing this report.


Michael J. Vinson P.E. No. 15237



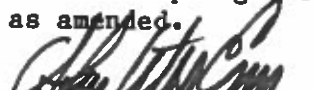
DEVELOPER'S STATEMENT

The owner/developer and/or his representative has read and will comply with all the requirements specified in this drainage report.


OWNER/DEVELOPER _____ DATE _____

CITY OF COLORADO SPRINGS FILING NOTE

Filed in accordance with section 15-3-906 of the Code of the City of Colorado Springs, 1980, as amended.


City Engineer _____ Date 7/31/85

JULY 26, 1985

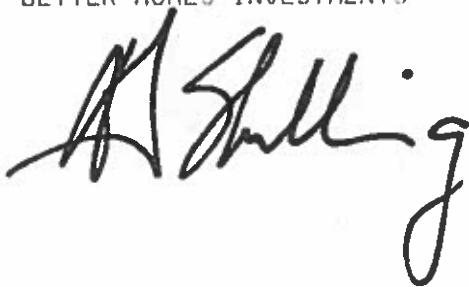
JENSEN/MARKLEY GROUP
C/O RICHARDS ENGINEERING AND
LAND SURVEYING
1687 W UINTAH SUITE 103
COLORADO SPRINGS,
COLORADO 80904

RE: DRAINAGE ONTO OUR PROPERTY FOR THE VILLAGE AT SKYLINE

SIRS;

We have reviewed the drainage report for the Village at Skyline with your Engineer. It is our understanding that the 5-year developed runoff onto our property will be about 6.5 c.f.s. less than the historic runoff, and that there will be a point discharge of about 4.8 c.f.s. at our South property line. We have no objections to this drainage plan. We request that we be kept informed of any changes that might be made.

Sincerely,
BETTER HOMES INVESTMENTS



→ CLAY HAMILTON ←

A.J. SCHELLING

JOHN FOURMAN

MASTER DRAINAGE REPORT
FOR
THE VILLAGE AT SKYLINE

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I. INTRODUCTION

This report is a master study for the Village at Skyline parcel. It is intended to show the proposed plan for routing storm runoff and its effects on proposed and existing structures. This report is not intended to replace the South 21st Street Master Drainage Basin Study. Certain generalities and assumptions were used to determine the downstream flows, and the adequacy of the downstream system. This report is to be used only if the Village at Skyline site is rezoned to PUD and developed as described herein.

II. LOCATION

The Village at Skyline is located in the South 21st Street Drainage Basin and consists of approximately 36 acres. The site is bounded on the North by Wheeler Street and Brewster Subdivision; on the East by the Village at Bear Creek; on the South by Lower Gold Camp Road; and on the West by unplatted land. According to the F.E.M.A. Flood Plain Maps, this site is not within a designated flood plain.

III. METHOD

Any assumed offsite flows stated in this report are from the South 21st Street Master Drainage Basin Study by R. Kieth Hook and Associates, Inc., dated June, 1977. Existing pipe sizes are from the "as built" South 21st Street Drainage Improvements Plans obtain from the Department of Public Works.

The method used for the onsite computations is the USDA/SCS synthetic hydrograph method as prescribed by the City of Colorado Springs. All interior flows are based on the "5-year" rainfall of 2.1 inches in 6 hours. The primary channel flows are based on a "100-year" rainfall of 3.5 inches in 6 hours. All computations are enclosed. Soils classifications were obtained from the SCS maps.

IV. SOILS

The soils on the project are from the Chaseville-Midway Complex. The Chaseville soil makes up about 70 percent of the complex and the Midway makes up about 20 percent. The Chaseville soil is on the steeper soils and the top of ridges. The Midway soil is usually at the lower part of the areas. The SCS soil survey of the El Paso County area, Colorado, puts the Chaseville soil in the Hydrologic Group "A" and the Midway is Hydrologic

Group "D". This site has been used for a gravel mine in the past. For runoff computations, a Hydrologic Group "B" was used.

The offsite areas are on Razor-Midway complex soils. A Hydrologic Group "D" was used for this area.

V. ZONING AND LAND USE

A request for rezoning has been submitted to the City to zone the property P.U.D. It is the intent of the owners to develop the property as a retirement community, with small single family lots and central multi-family units.

VI. LOWER GOLD CAMP ROAD

At this time, Lower Gold Camp Road is designated as a major arterial roadway. About half of the length will be graded for the full width, while the West half will be tapered back to the existing width. Flows on Lower Gold Camp Road (10.S cfs) will be carried the historic direction to the East. All the flows up to the 100-year runoff are within the curb capacity.

VII. ONSITE FLOWS

BASIN "A": Basin "A" consists of about 5 acres and is located in the Southeast portion of the project. Flows from this basin (8.1 cfs) will discharge onto the Community at Bear Creek. These flows are shown to be accepted in the drainage report by Monument Valley Engineering for the Community at Bear Creek.

BASIN "B": Basin "B" is located in the Northeast portion of the project and consists of about 8.2 acres. Developed flows from this Basin (9.2 cfs, 5-year) will flow to the Northeast. Flows from Sub-basin B-1 (4.8 cfs, 5-year) will travel between two lots and dispersed at the lot line by means of a Rip-Rap channel. Flows from Sub-basin B-2 (4.4 cfs, 5-year) will sheet flow along historic patterns offsite. Most of the historic flow from this site (15.7 cfs, 5-year) discharged in this direction. Because most of the developed flows are being re-routed (See Basin "C"), the new flows represent a reduction of about 41% in the runoff to the Northeast. Most of the historic site discharged this direction. Because most of the developed flows are being re-routed, the new flows represent a reduction in runoff to the Northeast.

BASIN "C": Basin "C" is the central and Southwest portion of the project and consists of about 18.7 acres. Flows from this basin (25.9 cfs, 5-year) will be picked up in a private channel and diverted to a proposed inlet in Wheeler Avenue at the Northwest corner of this project. The runoff will then be carried in a proposed 27" RCP to the existing storm sewer system. The size and material for the diversion channel will be determined at the time the final plat is submitted for this area.

BASIN "D": Basin "D" is the portion along Wheeler Avenue and consists of about 4.2 acres. Flows from this area (0.81 cfs) will be picked up in a proposed DIOR inlet at the Northwest corner of this project.

VIII. DOWNSTREAM SYSTEM

Our calculations show that the downstream system is adequate to handle the flows projected in this report. For the most part, the downstream system has been constructed. The system begins with a 27" RCP at the corner of Wheeler and South Street. This pipe will be extended to the Northwest property corner and a 16'DIOR inlet will be constructed. The back of the inlet will be open to accept the flows from onsite Basin "C". The runoff calculated by this office was about two-thirds (2/3) the flows shown on the South 21st Street Master Drainage Basin Plan. One reason is that the South 21st Street report assumed that the vacant land would be developed with industrial usages. Instead, in this area the vacant ground at the "Village at Skyline" is being developed as residential. Also, the vacant ground in sub-basins OS-2, OS-3, and OS-4 is considered too steep for industrial usages, but could be developed as multi-family residential with open spaces.

Our calculations show that if the vacant areas do develop with industrial usages, the runoff would increase about 30% and the pipe system would still be adequate.

IX. PUBLIC AND PRIVATE IMPROVEMENTS

The only public improvements required for this project are the extension of the 27 inch RCP and the 6'DIOR inlet in Wheeler Avenue. The cost for the Public Improvements is presented as follows:

80 LF of 27" RCP @ \$30.00/LF	= \$ 2,400.00
1 ea. of 16'DIOR @ \$7,000.00/ea.	= \$ <u>7,000.00</u>
TOTAL	\$ 9,400.00

The private improvements will be determined in the final drainage report for each phase of this project.

X. FEES

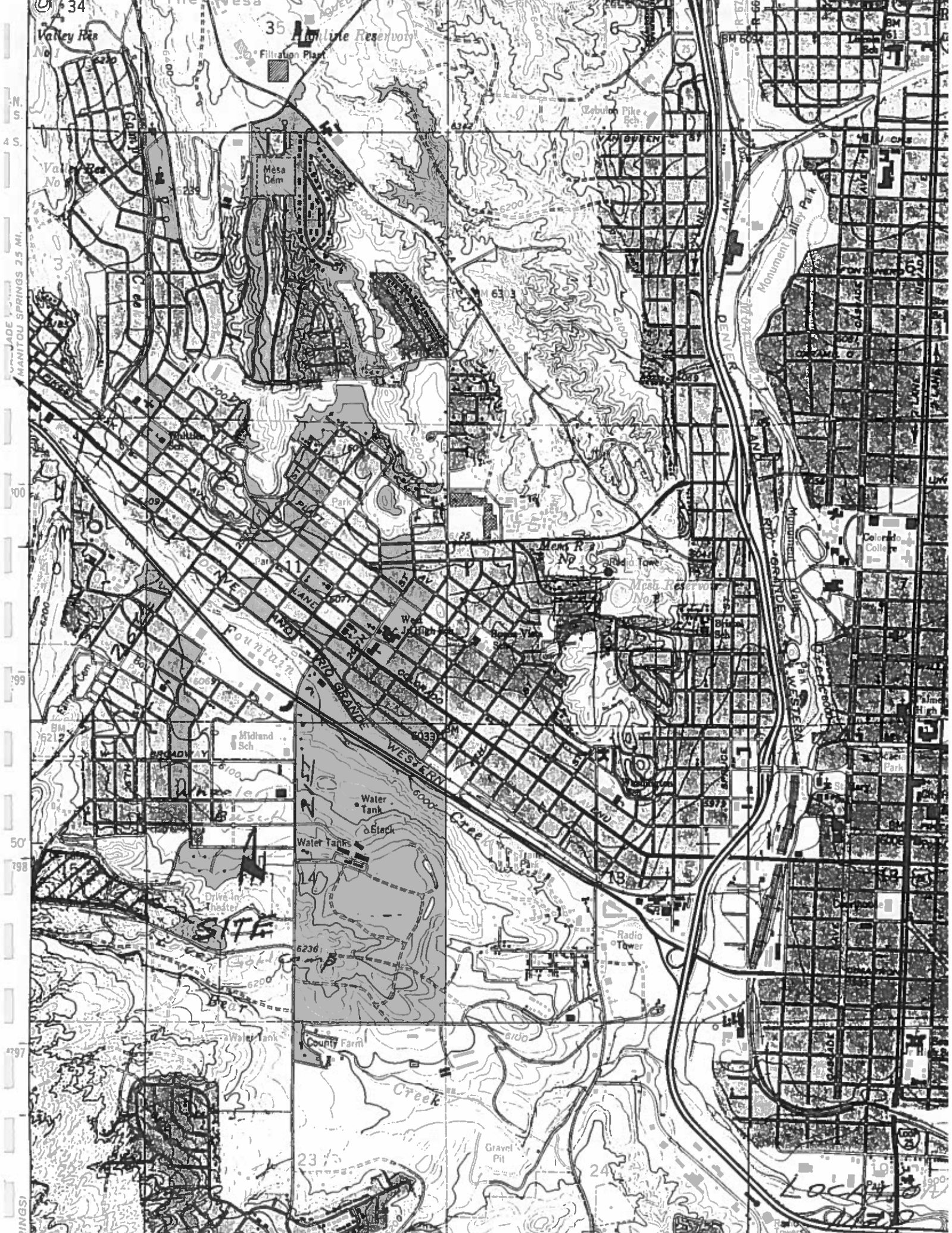
This project lays entirely within the South 21st Street Basin. The 1985 fees are presented as follows:

Drainage Fees: 36.6 acres @ 2,143.00/acre = \$78,433.80

Bridge Fees: None

The final amount of the drainage fees will be determined at the time of the final plat.

APPENDIX



T. 13 S.
T. 14 S.

370 000 FEET



CURVE NUMBERS
CALCULATIONS

Sub-Basins B-2

Total Acreage = 5.44 acres			
Res. 3 acres = CN of 85	-		46.88
Open 2.44 acres = CN of 61	-		<u>27.36</u>
	CN	=	74.23
<u>USE CN = 81</u>			

Sub-Basins C + D

Total Acreage = 22.848 acres			
Res. = 18.688 = CN 85	-		69.52
Open = 4.160 = CN 61	-		<u>11.11</u>
	CN	=	80.63
<u>USE CN = 81</u>			

Sub-Basins OS - 1 + C + D

Total Area = .0411 sq. mi.			
Res. = .0346 sq. mi. = CN 85	=		71.56
Open = .0065 sq. mi. = CN 61	=		<u>9.65</u>
	CN	=	81.20
<u>USE CN = 81</u>			

Sub-Basins C & D thru OS-6

Total area = .0678 sq. mi.			
CN 87 = .0046	=		5.9
CN 61 = .0065	=		5.8
CN 85 = .0567	=		<u>71.08</u>
	CN	=	82.83
<u>USE 85</u>			

Sub-Basin OS-1,2,3,6,7 & C&D

Total Area = .0986			
CN 87 = .0354	=		31.24
CN 61 = .0065	=		4.02
CN 85 = .0567	=		<u>48.88</u>
	TOTAL	=	84.14
<u>USE 85</u>			

Sub-Basins C&D + OS - 1 thru 7

Total Area = .1159		
CN 85 = .0567	=	41.58
CN 61 = .0065	=	3.42
CN 87 = .0527	=	<u>39.56</u>
	TOTAL	= 84.60
<u>USE 85</u>		

Sub-Basins C&D + OS - 1 thru 8

Total Area = .1379		
CN 85 = .0567	=	34.95
CN 61 = .0065	=	2.88
CN 87 = .0747	=	<u>47.12</u>
	TOTAL	= 84.95
<u>USE 85</u>		

Sub-Basins C & D + OS - 1 thru 10

Total Area = .1604		
CN 85 = .0567	=	30.05
CN 61 = .0065	=	2.48
CN 87 = .0972	=	<u>52.72</u>
	TOTAL	= 85.23
<u>USE 85</u>		

Basins C & D & OS

Total Area = .19 sq. mi.		
CN 85 = .0567	=	25.37
CN 61 = .0065	=	2.08
CN 87 = .1268	=	<u>58.06</u>
	TOTAL	= 85.51
<u>USE 86</u>		

FIGURE 4

FLOW FOR CIRCULAR PIPE FLOWING FULL
BASED ON MANNING'S EQUATION $n=0.012$

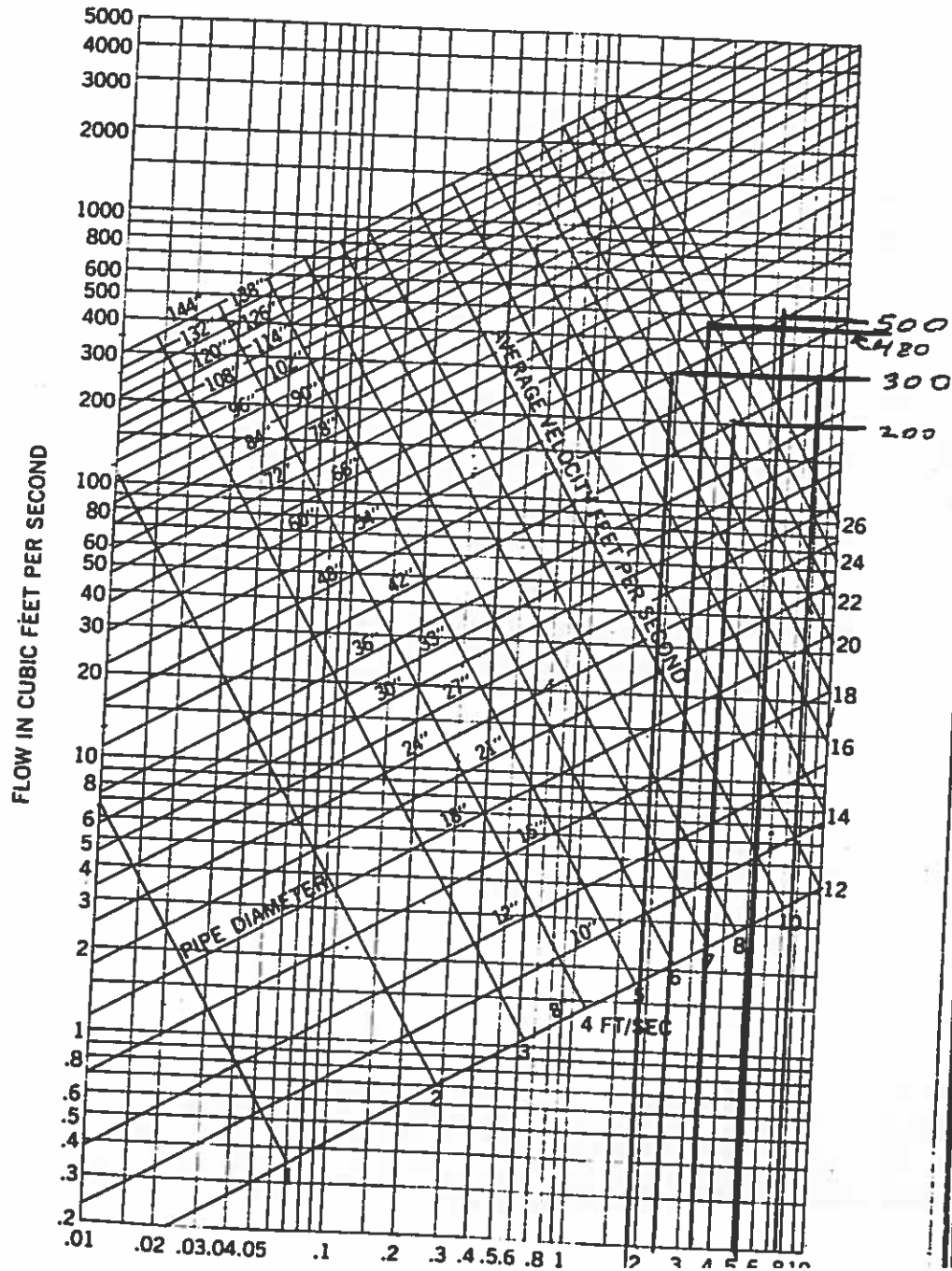


Figure III

MAJOR BASIN	SUB BASIN	AREA		BASIN		T _c	K	SOIL GROUP	DEV. TYPE	CURVE NO.	FLOW		
		Planim. Read.	MILE	LENGTH	HEIGHT						Q	qp	
GC	1	.	.0013	700	26	.0717	1230		Road	98	3.1	1.87	5yr.
											5.4	3.27	100yr.
+	2		.0055	3000	105	.2251	1020		Road	98	10.5	1.87	5yr.
											13.3	3.27	100yr.
A			.0078	1050	20	.127	1200	B	Res	85	2.14	0.87	5yr.
											18.90	2.02	100yr.
B	1		.0043	500	12	.05	1220	B	Res	85	4.8	0.87	5yr.
											11.1	2.02	100yr.
	2		.0085	200	40	.014	1280	B	Res & OPEN	74	4.4	0.40	5yr.
											13.5	1.24	100yr.
C			.0292	2500	60	.226	1020	B	Res	85	25.9	0.87	5-yr.
											60.2	2.02	100-yr.
D			.0065	900	20	.1061	1250	B	OPEN	61	0.81	0.10	5-yr.
											4.6	0.57	100-yr.
C+D			.0357			.226	1020	B	Res & OPEN	81	26.2	.72	5-yr.
											62.3	1.71	100yr.
HIST A B & C that discharge where Dev. B will			.0498	1600	80	.12	1210	B	OPEN	69	15.7	0.26	5-yr.
											57.2	0.95	100-yr.

HYDROLOGIC COMPUTATION - BASIC DATA

PROJ: Village at Skyline

By: MV
Date: 6/12/85

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of

4 Pages

MAJOR BASIN	SUB BASIN	AREA		BASIN		Tc	K	SOIL GROUP	DEV. TYPE	CURVE NO.	FLOW		
		Planim. Read.	MILE	LENGTH	HEIGHT						Q	qp	
OS	1		.0354	800	40	.07	1280	B	RES	85	6.01	0.87	5-yr
											14.0	2.02	100-yr
+CED			.0411				1020	B		81	30.2	0.72	5-yr
											71.7	1.71	100-yr
OS	2		.0221	1200	90	.13	1200	B	(Assumed) Res.	85	23.1	0.87	5-yr
											53.6	2.02	100-yr
1+2+C+D			.0632				1020	B		85	56.1	0.87	5-yr
											130.2	2.02	100-yr
OS	6		.0046	700	40	.06	1230	D		87	5.8	0.98	5-yr
											12.9	2.19	100-yr
CED +OS			.0678	3200	100	.247	1000			85	59.0	0.87	5-yr
											137.0	2.02	100-yr
OS	7		.0082	750	32	.09	1230	D		87	10.3	0.98	5-yr
											23.0	2.19	100-yr
OS	3		.0226	1700	102	.13	1200	D		87	26.6	0.98	5-yr
											59.4	2.19	100-yr
CED+OS 1,2,3,6+7			.0996	4050	132	.29	930			85	79.8	0.87	5-yr
											185.2	2.02	100-yr

HYDROLOGIC COMPUTATION - BASIC DATA

PROJ: Village at Skyline

By: MV
Date: 6/12/85

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of

4 Pages

MAJOR BASIN	SUB BASIN	AREA		BASIN		Tc	K	SOIL GROUP	DEV. TYPE	CURVE NO.	FLOW		
		Planim. Read.	MILE	LENGTH	HEIGHT						Q	qp	
OS	4		0.0140	1000	140	.06	1280	D	Res	87	17.6	.98	5yr
											39.2	2.19	100yr
OS	5		.0033	700	20	.08	1250	D	Res	87	4.1	0.98	5yr
											9.3	2.19	100yr
C&D +OS	1 thru 7		.1159	4700	150	.33	785			85	89.2	2.27	5yr
											207.2	2.22	100yr
OS	8		.022	1650	170	.09	1280	D	Res	87	27.6	.98	5-yr
											51.7	2.19	100-yr
Total			.1379	5050	164	.35	760			85	103.2	.87	5-yr
											239.6	2.02	100-yr
OS	10		.0125	1000	150	.06	1280	D		87	15.7	.98	5-yr
											35.0	2.19	100-yr
OS	9		.01	800	26	.08	1280	D		87	12.5	.98	5-yr
											28.0	2.19	100-yr
OS	10+9		.0225	1700	176	.10	1280				28.2		5-yr
											63.0		100-yr
C&D +OS	1 thru 10		.1604	5450	178	.37	840			85	117.2	.87	5-yr
											272.2	2.02	100-yr

HYDROLOGIC COMPUTATION - BASIC DATA

PROJ: Village at Skyline

By: M.V
Date: 8/12/85

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of

4 Pages

MAJOR BASIN	SUB BASIN	AREA		BASIN		Tc	K	SOIL GROUP	DEV. TYPE	CURVE NO.	FLOW		
		Ptopim. Read.	MILE	LENGTH	HEIGHT						Q	qp	
OS	11		.0129	700	36	.0246	1230	D	Res	87	16.2	.92	5-yr
											36.2	2.19	100-yr
											13.0	.98	5-yr
	12		.0104	250	26	.09	1230	D	Res	87	29.2	2.19	100-yr
											7.9	.98	5-yr
	13		.0063	750	26	.09	1280	D	Res	87	17.7	2.19	100-yr
CFD OS	ALL		.19	6050	123	.40	810			36	141.6	.92	5 yr
											323.2	2.10	100 yr

Table 1 -- Determination of Runoff Depth in inches for selected CN's and rainfall amounts

Curve ¹ Number	(P) Rainfall (Inches)	
	<u>2.10</u>	<u>3.50</u>
56	0.03	0.38
58	0.05	0.45
60	0.08	0.53
62	0.11	0.62
64	0.14	0.71
66	0.18	0.80
68	0.23	0.90
70	0.28	1.01
72	0.34	1.12
74	0.40	1.24
76	0.47	1.36
78	0.54	1.50
80	0.62	1.64
82	0.71	1.78
84	0.82	1.94
86	0.92	2.10
88	1.05	2.27
90	1.18	2.45
92	1.33	2.64
94	1.49	2.84
96	1.67	3.04
98	1.87	3.27

1/ To obtain runoff depths for CN's and other rainfall amounts not shown in this table, use arithmetic interpolation or:

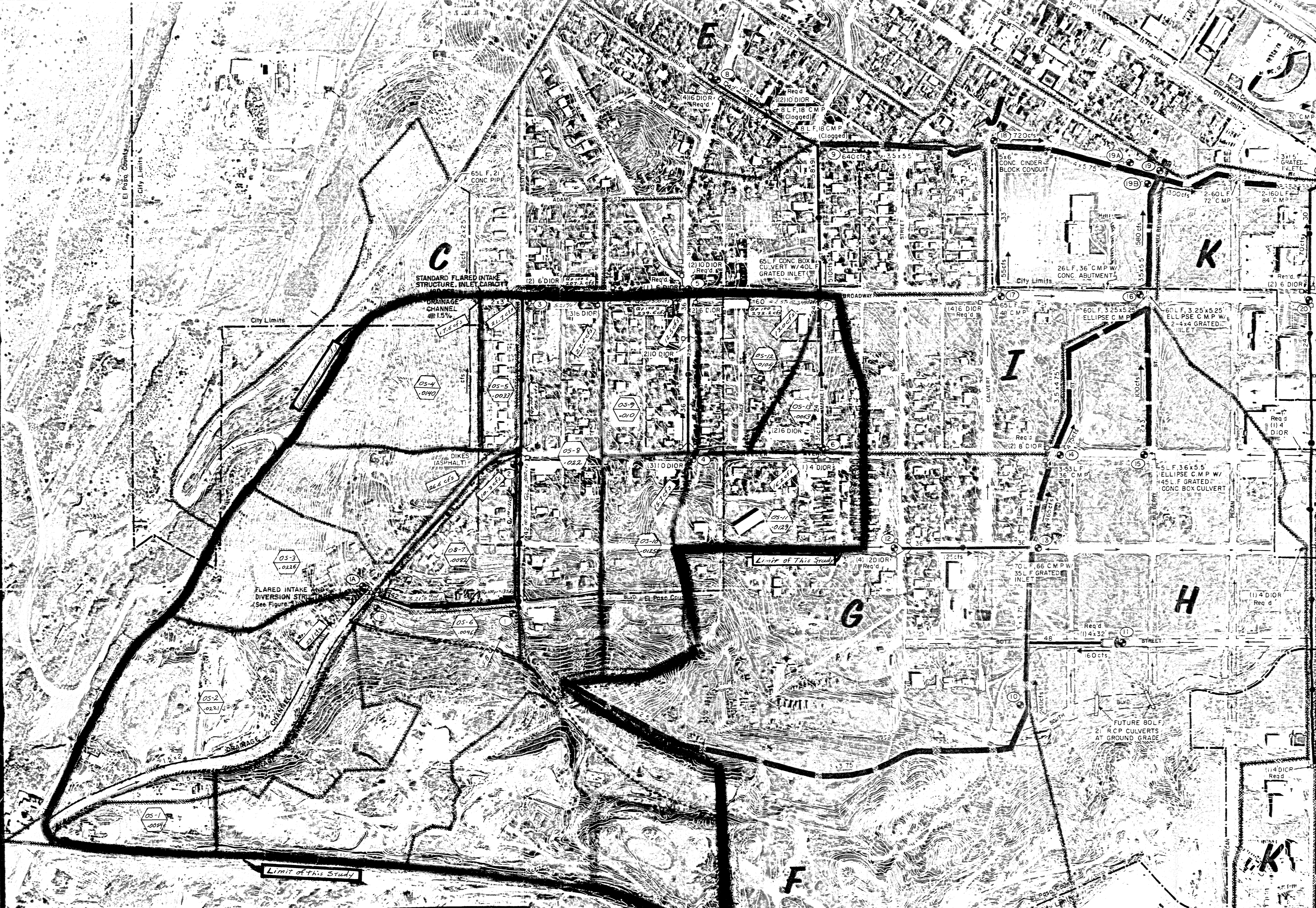
$$Q = \frac{CN (P + 2)^2 - 400 (P + 2 - \frac{100}{CN})}{CN (P - 8) + 800}$$

CURB OPENING INLET CAPACITIES (cfs)

Table 6

NOTE: This chart reflects approx. 60% pickup of street flows (not full pickup!)

Opening Length (ft.)	4.0	6.0	8.0	10.0	12.0	14.0	16.0	18.0	20.0	22.0
Sump Capacity (cfs)	7.9	12.8	18.4	23.0	27.6	34.5	39.4	44.3	49.3	54.1
Street Slope %										
0.5	6.3	6.6	6.8	8.0	8.8	9.7	10.6	11.5	12.4	13.1
1.0	8.6	8.8	9.4	10.0	10.4	11.3	12.0	12.8	13.8	14.1
1.5	7.7	10.6	10.9	11.5	12.2	12.7	13.4	14.2	15.0	15.1
2.0	6.5	12.2	12.5	12.9	13.4	14.0	14.6	15.2	15.9	16.1
2.5	5.7	14.0	13.9	14.2	14.7	15.2	15.7	16.3	17.0	17.1
3.0	5.2	12.7	14.8	15.4	15.8	16.1	16.5	17.2	17.8	18.1
3.5	4.7	11.3	16.1	16.6	16.9	17.2	17.8	18.2	18.7	19.1
4.0	4.4	10.6	17.0	17.5	17.9	18.2	18.5	19.0	19.5	20.0
4.5	4.1	9.7	18.1	18.4	18.7	19.1	19.5	20.0	20.5	21.0
5.0	3.9	9.2	17.7	19.4	19.7	20.0	20.3	20.3	21.3	21.6
5.5	3.7	3.7	16.7	20.3	20.6	20.9	21.2	21.5	22.0	22.1
6.0	3.5	3.3	15.6	20.7	21.0	21.4	21.9	22.4	22.9	23.1
6.5	3.4	7.9	<u>14.9</u>	21.8	22.2	22.6	23.1	23.5	24.0	24.1
7.0	3.2	7.6	14.2	22.2	22.6	23.0	23.5	23.8	24.2	25.1
7.5	3.1	7.3	13.6	22.7	23.4	23.8	24.2	24.6	25.0	25.7
8.0	3.0	7.0	13.0	21.8	24.3	24.6	24.9	25.3	25.7	26.2
8.5	2.9	6.8	12.6	20.3	25.0	25.3	25.6	26.0	26.4	26.8
9.0	2.8	6.5	12.1	19.9	25.7	25.9	26.3	26.6	27.0	27.4
9.5	2.7	6.4	11.8	19.4	26.5	26.7	27.0	27.4	27.7	28.1
10.0	2.6	6.2	11.4	18.7	26.7	27.2	27.6	28.0	28.3	28.8



VILLAGE AT SKYLINE

**MASTER DRAINAGE PLAN
OFFSITE DRAINAGE PLAN**

RICHARDS ENGINEERING AND LAND SURVEYING
1665 S. DIXIE, SUITE 103
COLORADO SPRINGS,
COLORADO 80904

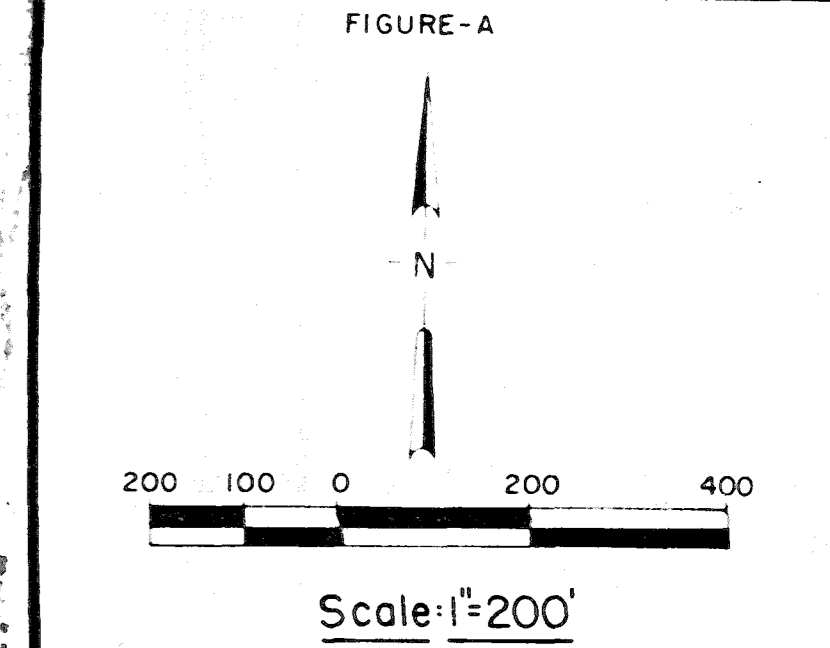
PHONE (303) 633-3768

PREPARED BY: _____ JOB NO. 1184-4

DATE: MAY 31, 1985 SHEET 1 OF 2

SCALE: HORIZONTAL 1"=50' REVISION BY DATE

VERTICAL 1"=5'



From the
**SOUTH 21st STREET
MASTER DRAINAGE BASIN PLAN**

R. KEITH HOOK & ASSOCIATES, INC.
CONSULTING ENGINEERS
COLORADO SPRINGS, COLORADO

DRAWN BY: _____ JOB NO. 770083 FILE NO. 7128

CHECKED BY: _____

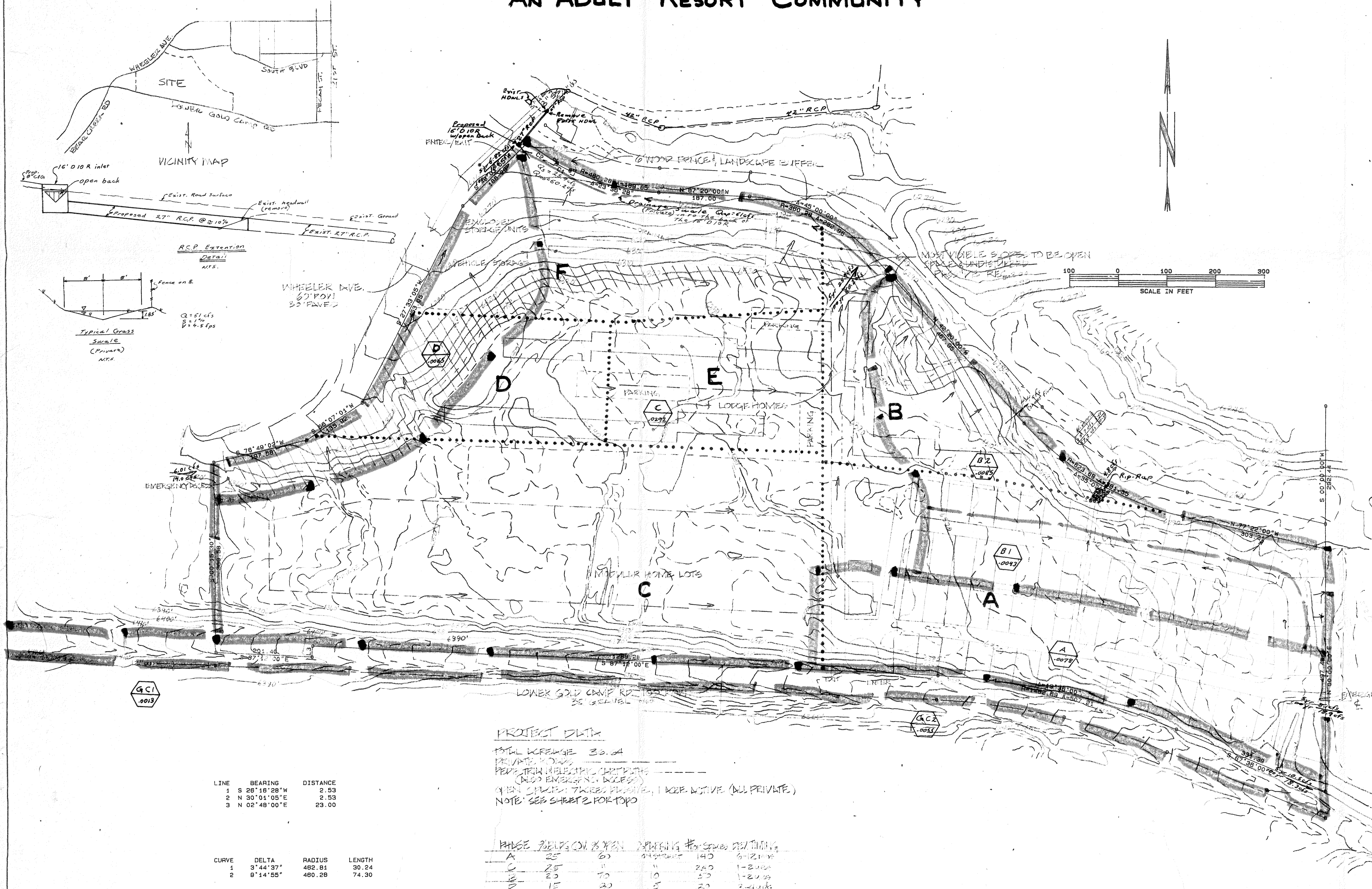
REFERENCE

LEGEND		BASIN DATA			
		Sub-Basin	Acreege	Basin	Acreege
≡≡≡	EXISTING DRAINAGE STRUCTURES	OS-1	3.45	OS-10	8.00
—	PROPOSED DRAINAGE STRUCTURES	OS-2	14.17	OS-11	8.25
24"	R.C.P. STORM DRAIN PIPE W/SIZE DIA.	OS-3	14.46	OS-12	6.65
□	CATCH BASIN W/CONNECTOR PIPE.	OS-4	8.96	OS-13	4.03
●	MANHOLE	OS-5	2.11		
—	GRATED INLET	OS-6	2.94		
—	DRAINAGE CHANNEL	OS-7	5.24		
—	MAJOR DRAINAGE CHANNEL	OS-8	14.08		
		OS-9	6.40		

NOTES:

- All pipe is 45° bend or less.
- For required improvement to existing drainage structures at hydrograph point location. See chart on page 36-37.

DRAINAGE PLAN THE VILLAGE AT SKYLINE AN ADULT RESORT COMMUNITY



LINE	BEARING	DISTANCE
1	S 28°16'28"W	2.53
2	N 30°01'05"E	2.53
3	N 02°48'00"E	23.00

CURVE	DELTA	RADIUS	LENGTH
1	3°44'37"	482.81	30.24
2	9°14'55"	480.28	74.30

PROJECT DATA
 TOTAL DRAINAGE 30.4
 PERCENT OPEN DRAINAGE
 PERCENT UNDEVELOPED DRAINAGE
 (ALSO EMERGENCY ACCESS)
 OPEN CHANNEL: 7.00 FEET, 1 DEEP DITCH (ALL PRIVATE)
 NOTE: SEE SHEET 2 FOR TOP

PHASE	DELTA	RADIUS	LENGTH	AREA
A	25	70	140	4-12.1000
B	25	70	140	1-2.1000
C	15	20	20	3-2.1000
D	25	70	140	2-2.1000
E	15	20	20	1-2.1000

PREPARED APRIL 17, 1985 BY: LMM
 RICHARDS ENGINEERING
 AND LAND SURVEYING INC.
 1687 W. UINAH STREET
 COLORADO SPRINGS COLORADO
 633-3768 80904
 JOB NO. 1184-4 SHEET 1 OF 2
 SCALE: 1" = 100'