



**MASTER DEVELOPMENT DRAINAGE REPORT (M.D.D.P.)
WOODMEN HEIGHTS ADDITONS 7, 8, AND 9 ANNEXATION
COLORADO SPRINGS, COLORADO**

*NOVEMBER 2005
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REVISED AUGUST 2006
REVISED MARCH 2007*

Prepared For:

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Job No. 0537.00

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COLORADO SPRINGS, COLORADO**

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KIOWA ENGINEERING'S HEC-1 ANALYSIS

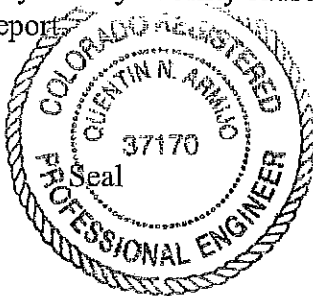
SAND CREEK DATA

DRAINAGE PLAN

ENGINEER'S STATEMENT:

The attached drainage plan and report were prepared under my direction and supervision and are correct to the best of my knowledge and belief. Said drainage report has been prepared according to the criteria established by the City for drainage reports and said report is in conformity with the master plan of the drainage basin. I accept responsibility for any liability caused by any negligent acts, errors or omissions on my part in preparing this report.

Quentin N. Primo 3/15/07
Name



Developer's Statement:

I, the developer have read and will comply with all of the requirements specified in this drainage report and plan.

SCHOOLER AND ASSOCIATES

Business Name

By: Myra Belevole

Title: Project Manager - First Church of the Nazarene

Address: 455 E. Pikes Peak Avenue, Suite 100, Colorado Springs, CO 80903

City of Colorado Springs Approval:

Filed in accordance with Section 7.7.906 of the Code of the City of Colorado Springs, 2001, as amended.

For City Engineer B. J. Kelly
Conditions

Date 3/19/07

**MASTER DEVELOPMENT DRAINAGE REPORT (M.D.D.P.)
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PURPOSE

The purpose of this Master Development Drainage Plan is to identify and analyze the proposed drainage patterns, determine proposed runoff quantities, size drainage structures for conveyance of developed runoff, and present solutions to drainage impacts on-site and off-site resulting from this development.

GENERAL DESCRIPTION

This Master Development Drainage Plan (M.D.D.P.) consists of three Additions, Woodmen Heights Addition 7, Addition 8 and Addition 9. Addition 9 consists of 3.87 acres. Addition 8 consists of 9.73 acres. Addition 7 consists of 62.2 acres. They are located in a portion of Section 8, Township 13 South, Range 65 West of the 6th Principal Meridian currently within unincorporated El Paso County, Colorado. The development of this site for Addition 8 and 9 consists of future proposed commercial sites. The site known as Addition 7 is to be developed as Quail Brush Creek and will be single family lots with open space. The MDDP also covers the improvements of Nebraska Lane from Woodmen Road south to the Addition 7 site. The entire site is bounded to the west by the Horseshoe Rancheros residential subdivision, to the north and west by unplatted county land and to the south by open space, Sand Creek and Ridgeview filing No. 26 residential subdivision.

The site is contained within the Sand Creek Basin. Flows from this site are tributary to Sand Creek.

Soils for this project are delineated by the map in the appendix as Blakeland loamy sand (8). Soils in the study area are shown as mapped by S.C.S. in the "Soils Survey of El Paso County Area" and are primarily of Hydrologic Group A. The study area consists of undeveloped land and large lot residential development with existing natural, grassy vegetation. The existing topography is sloping from the northwest to the southeast with an average slope of 7%.

EXISTING DRAINAGE CONDITIONS

This site was most recently studied in the “Master Development Drainage Plan Amendment No. II for the Easterly Portion of Ridgeview Subdivision and Preliminary Drainage Report for the Northeasterly Portion of Ridgeview Subdivision and Phase II Sand Creek Channel Improvements.” (MDDP Amend II) by JR Engineering, dated February 2004. The site was originally studied in the “Sand Creek Drainage Basin Planning Study Preliminary Design Report” (DBPS) prepared by Kiowa Engineering. This study was then updated in the “Sand Creek Drainage Basin Planning Study Preliminary Design Report Technical Addendum” by Kiowa, revised October 1995. Some of the runoff from the study area site and offsite areas upstream is tributary to existing natural channel that runs through the middle of Annexation Addition 7. This channel was studied as Reach 147 in the DBPS. In the MDDP Amend II prepared by JR Engineering a channel was designed at the north Boundary of Ridgeview Filing 26 that redirects Reach 147 southeast to a junction with Sand Creek. Other areas of the study site also are directed by sheet flow and small existing swales to Sand Creek, which is just south east of Annexation Addition 7. Annexation Additions 8 & 9 were the only Basins studied in the historic conditions so that preliminary detention could be calculated for these two future commercial areas. Annexation Addition 9 was studied as historic Basin H-D and Annexation Addition 8 was studied as Basin H-E.

Basin H-D’s 4.20 acres was analyzed as historic condition of 5 acre ranch area. Runoff currently sheet flows east off site and was calculated to be $Q_5=3$ cfs and $Q_{100}=8$ cfs. This runoff will be used as the allowable release rate for Addition 9.

Basin H-E’s 9.97 acres was analyzed as historic condition of 5 acre ranch area. Runoff currently sheet flows east off site and was calculated to be $Q_5=8$ cfs and $Q_{100}=19$ cfs. This runoff will be used as the allowable release rate for Addition 8.

PROPOSED DRAINAGE CONDITIONS

In the MDDP Amend II, the proposed site was part of Basins 142-2, 142-3, 147-47. Annexation Parcels 7-9 were recently studied at our request in the Sand Creek hydrological Analysis upper Sand Creek Basin Sand Creek Detention Basin No. 2” prepared by Kiowa Engineering and currently being

revised. Discussion was made with Kiowa Engineering who was in the process of reanalyzing the detention pond using HEC-1 to update our proposed sites as developed conditions and see how the change in ground cover would affect the downstream pond. The calculations performed by Kiowa showed that the channel and downstream detention basin will be able to handle this slight increase in developed runoff (See Appendix). With this analysis there is no need for on site detention for Annexation Parcel 7. It is proposed however for Annexation Parcels 7 & 8 to have on site detention when they are developed along with water quality. The downstream detention basin being designed by Kiowa Engineering will also provide additional water quality. To the flows calculated here in this report. Developed flows from the subject site will drain to the south east via proposed on site storm sewer and swales and then drain to the existing channel, the offsite swale designed in Ridgeview Filing 26, or to Sand Creek.

The following is a description of the onsite basins, offsite bypass flows and the overall proposed drainage characteristics for the development of Woodmen Heights Additions 7, 8, and 9. The following Design Points were determined using the Rational Method since each individual basin is less than 100 acres and the combined acreage at any Design Point is also less than 100 acres. This method offers a more conservative approach to calculating swale cross sections and storm drain sizes.

It is also assumed that runoff upstream of lots will be routed down side lot swales to avoid flooding of the homes.

The existing channel running through Woodmen Heights 7 was used not reanalyzed as MDDP Amend II updated the HEC-RAS model that was used in the original Sand Creek DBPS and Technical Addendum. In the MDDP Amend II Design Point 47 was moved to the south boundary of Addition 7 (north boundary of Ridgeview Filing 26). The runoff calculated in the channel was 644 cfs for the 100 year event. To be conservative this flow was assumed for the whole reach through Addition 7. A trapezoidal cross section of a 10' wide bottom with 4:1 slopes and an average slope of 1.54% was plugged into Haestad Methods Flowmaster. This cross section was proposed in the original DBPS. The results showed a 3.69' depth in the channel with a velocity of 5.01 ft/sec (See hydraulic calculation in appendix.) To account for freeboard the channel is shown to be designed

with a 6' depth. The one street crossing at Cat Tail Creek Drive will utilize 3 60" RCP Culverts to pass the storm.

Basin OS-1's 15.74 acres sits just west of the site and consist of 5 acre ranch property that sheet flows onto the proposed Woodmen Frontage Road the old Nebraska Lane. Basin OS-1A's 0.67 acres consist of the east half of Nebraska Lane, which will now be the above mentioned Woodmen Frontage Road. Since we first analyzed this area in our MDDP we have learned that the Woodmen Road improvements in this area will improve the north 700' of old Nebraska Lane (new Quail Brush Creek Drive and Woodmen Frontage Road name change and exact location not finalized), which coincides with a high point in old Nebraska Lane (new Quail Brush Creek Drive). Our limits of Quail Brush Creek to the north will end at the south end of the proposed Woodmen Frontage Road. With these improvements DMJM Harris and the El Paso County Development Services will study these areas and analyze the drainage for these Basins and Design Points. Per their design the storm drain will be installed with these road improvements and is no longer part of our study.

Offsite Basins OS-2, OS-3A, OS-4A and OS-4B, are currently 5 acre ranch lots. For the chance that the site does develop into some other type of zoning we have stubbed a 60" on the south west corner that will be able to handle the increase in flow if developed. Due to the upsizing that this offsite flows from the west become reimbursable and we have included this under our construction cost opinion for reimbursable drainage facilities.

Runoff ($Q_5=17$ cfs and $Q_{100}=38$ cfs) from Basin OS-2's 22.76 acres flows overland and onto the west half of Quail Brush Creek Drive (old Nebraska Lane). The runoff will be collected at Design Point 2A in 2-14' sump inlets located in Quail Brush Creek Drive. Basin OS-2A's 0.69 acres consists of the west half of Quail Brush Creek Drive. Runoff ($Q_5=3$ cfs and $Q_{100}=5$ cfs) is directed to a low point in the street section and captured in the above mentioned 2-14' sump inlets at Design Point 2A ($Q_5=19$ cfs and $Q_{100}=40$ cfs) along with the runoff from Basin OS-2. Basin OS-2B's 0.84 acres is the eastern street section of Quail Brush Creek Drive (old Nebraska Lane). Runoff ($Q_5=3$ cfs and $Q_{100}=6$ cfs) is captured in a 6' sump inlet at Design Point 2B and transported by Pipe Run 2A to a junction with Design Point 2A. The combined runoff ($Q_5=21$ cfs and $Q_{100}=42$ cfs) from Design Points 2A & 2B is routed south via a 36" storm drain (Pipe Run 2B).

Offsite Basin OS-3A's runoff ($Q_5=7$ cfs and $Q_{100}=15$ cfs) sheet flows east to a low point (Design point 3A) in Quail Brush Creek Drive where an 8' D-10R sump inlet will capture the flow at the north property line of Quail Brush Creek. Basin OS-3B's 0.17 acres consists of a portion of Quail Brush Creek Drive. Runoff from this basin is captured in 6' sump inlet and routed by Pipe run 3 an 18" storm drain to a junction with Design Point 3A and Pipe run 2B. A 36" storm drain (Pipe Run 3A) will carry the combined flow ($Q_5=27$ cfs and $Q_{100}=56$ cfs) south in a 30' tract at the west boundary of Quail Brush Creek Subdivision (Addition 7).

Offsite Basin OS-4A's runoff ($Q_5=8$ cfs and $Q_{100}=17$ cfs) sheet flows southeast into proposed Swale-1 (Section A-A) at the west boundary of Addition 7. The cross section for the trapezoidal swale consists of 2' bottom with 4:1 side slopes for a 100-year depth of 0.94'. The proposed design depth will be 2.5' to provide adequate freeboard. The SC250 fabric lined (North American Green Fabric see appendix for calculations) swale will then transport the runoff south to Design Point 4A. This SC250 fabric lined swale will be maintained by the HOA established for this subdivision. At DP-4A 2-10' long city standard D-9 inlets will capture the flow. From here the combined flow ($Q_5=33$ cfs and $Q_{100}=71$ cfs) of Basin OS-4A, A1 and Pipe run 3A are transported by Pipe run 4A, a 36" storm drain south to DP-4B.

Runoff from Basin OS-4B and Basin A2 is also captured by the above mentioned swale and directed to DP-4B. The combined runoff ($Q_5=39$ cfs and $Q_{100}=84$ cfs) will then be captured in 3-10' long city standard D-9 inlets. The captured flow from DP-4B and the flow in the Pipe run 4A are routed under the street section of Gold Drop Drive by Pipe Run 4B a 60" storm drain to a junction with Pipe runs 6A and 6B.

Basin B1's 4.34 acres consists of single family residential lots. Runoff ($Q_5=8$ cfs and $Q_{100}=16$ cfs) will be directed to a street section and transported south to Design Point 5A. At Design Point 5A an 8' sump inlet will capture the flow. Pipe Run 5A a 24" storm drain will transport the flow south down Tract Q and then east to a junction with Pipe run 5C.

Basin B2's 5.33 acres consists of single family lots in Addition 7. Runoff ($Q_5=10$ cfs and $Q_{100}=20$

cfs) will be directed to a street section and transported south to Design Point 5B. At Design Point 5B a 12' sump inlet will capture the flow. Pipe Run 5B a 24" storm drain will transport the flow south sown Tract R to Design Point 5C. At Design Point 5C a 6' sump inlet collects the runoff ($Q_5=7$ cfs and $Q_{100}=13$ cfs) from Basin B3's 3.35 acres consisting of single family development. The combined runoff ($Q_5=11$ cfs and $Q_{100}=22$ cfs) from Pipe runs 5A & 5B and DP-5C is transported in a 36" storm drain (Pipe Run 5C) south to Design point 5D a 6' sump inlet that will collect the runoff ($Q_5=2$ cfs and $Q_{100}=4$ cfs) from Basin B4's 1.19 acres. Pipe run 5D a 36" storm drain will route the flow south into the improved channel onto a proposed 20' x 15' d50 = 12" rip-rap pad.

Basin C1's 2.09 acres consists of single family residential lots and open space. Runoff ($Q_5=4$ cfs and $Q_{100}=9$ cfs) will be directed to a street section and transported south to Design Point 6A. At Design Point 6A a 20' sump inlet will capture 3 cfs in the 5 year event and 5 cfs in the 100 year event. Pipe Run 6A an 18" storm drain will transport the flow south to a junction with Pipe run 4B a 48" storm drain. Flow by ($Q_5=1$ cfs and $Q_{100}=3$ cfs) from DP-6A is routed south down Gold Drop Drive and into Ridgeview Filing 26 and then south down Flowering Almond Drive in the curb section to 2-5' curd cuts where the runoff is collected in a water quality pond. Basin C2's 0.93 acres consists of single family residential lots. Runoff ($Q_5=2$ cfs and $Q_{100}=4$ cfs) will be directed to a street section and transported south to Design Point 6B. At Design Point 6B a 12' sump inlet will capture 1 cfs in the 5 year event and 2 cfs in the 100 year event. Pipe Run 6B an 18" storm drain will transport the flow south to a junction with Pipe run 4B a 60" storm drain. The flow is then routed into the existing swale put in with Ridgeview Filing 27 at the south boundary of Addition 7 and onto a proposed 20' x 15' d50 = 12" rip-rap pad. Design Point 7 was calculated to show the combined runoff ($Q_5=79$ cfs and $Q_{100}=150$ cfs) that our system will discharge into the existing man made swale at the south boundary.

Basin D's 4.20 acres consists of area proposed to be a future commercial area. Runoff ($Q_5=12$ cfs and $Q_{100}=21$ cfs) will be directed overland to a low point (Design Point 8) in the southeast corner of the site. At the time of the Final Drainage Report for this site a detention pond with water quality will need to be designed. The allowable release will based upon the historic runoff numbers calculated for Basin H-D ($Q_5=3$ cfs and $Q_{100}=8$ cfs). A preliminary model was setup for the detention pond in Haestad's Pondpack. It was determined that a 0.49 ac-ft pond will be required. An 18" storm

drain pipe (Pipe Run 7) will transport the allowable runoff south east across Addition 8's site to the proposed detention basin area for Basin E.

Flow ($Q_5=30$ cfs and $Q_{100}=51$ cfs) from Basin E's 9.97 acres will be directed via a private storm drain system and curb and gutter to the south east corner of Addition 8 at the time of the Final Drainage Report. Addition 8 will consist of a future commercial site. A future detention basin with water quality will release the historic flow rates of $Q_5=11$ cfs and $Q_{100}=26$ cfs out of the future pond that was preliminary sized to be 1.46 ac-ft. A 30" storm drain pipe (Pipe run 8) will be routed south through Addition 7's site to Design point 11.

Offsite Basin OS-5's 3.18 acres was analyzed as 5 acre ranch area per the land use figure from MDDP Amend II (Basin 147-47). Runoff ($Q_5=3$ cfs and $Q_{100}=6$ cfs) will sheet flow onto a part of the northern section of the proposed Addition 7. Side lot line swales will direct the runoff south onto the street section of River Bend Drive. Basin F1's 4.84 acres consists of single family residential lots. Runoff ($Q_5=8$ cfs and $Q_{100}=16$ cfs) is directed by curb and gutter south to Design Point 11. Basin F2's 1.33 acres consists of single family residential lots. Runoff ($Q_5=3$ cfs and $Q_{100}=6$ cfs) is directed by curb and gutter south to Design Point 11. Basin F3's 3.33 acres consists of single family residential lots. Runoff ($Q_5=6$ cfs and $Q_{100}=12$ cfs) is directed by curb and gutter south to Design Point 11. Basin F4's 1.26 acres consists of single family residential lots. Runoff ($Q_5=2$ cfs and $Q_{100}=5$ cfs) is directed by curb and gutter south to Design Point 11. The combined flow ($Q_5=18$ cfs and $Q_{100}=35$ cfs) of the above mentioned basins is captured by a 20' sump inlet. Pipe run 8 is routed through the 20' sump inlet and the combined flow ($Q_5=28$ cfs and $Q_{100}=58$ cfs) is routed into the proposed channel and onto a proposed 20' x 15' $d_{50} = 12$ " rip-rap pad by Pipe run 9 a 36" storm drain.

Basin G1's 3.36 acres consists of single family lots. Runoff ($Q_5=7$ cfs and $Q_{100}=13$ cfs) sheet flows to street sections and then is directed south to low point in the street (Design Point 10A). Basin G2's 1.70 acres also consists of single family lots. Runoff ($Q_5=3$ cfs and $Q_{100}=7$ cfs) sheet flows to street sections and then is directed east Design Point 10A. At DP-10A a 10' sump inlet will capture the combined flow ($Q_5=10$ cfs and $Q_{100}=19$ cfs) of Basin G1 & G2. A 24" storm drain (Pipe Run 10A) will transport the flow across the street to Design point 10B to the main channel running through Addition 7. At Design point 10B, Basin G3's 1.40 acres will be captured by 6 sump inlet. The

combined flow ($Q_5=13$ cfs and $Q_{100}=25$ cfs) in Pipe run 10B will be routed through Tract H and onto a proposed 20' x 15' d50 = 12" rip-rap pad and into the improved channel.

Basin H1's 2.72 acres consists of single family lots. Runoff ($Q_5=2$ cfs and $Q_{100}=5$ cfs) sheet flows to street sections and then is directed east to Design Point 12A low point in the street. A 6' sump inlet will capture the flow. Pipe run 11A will route the flow south to Design Point 12B. At Design point 12B a 6' sump inlet will capture the street runoff ($Q_5=1$ cfs and $Q_{100}=1$ cfs) from Basin H2's 0.22 acres. The combined flow ($Q_5=2$ cfs and $Q_{100}=6$ cfs) of DP-12A & 12B will be routed by Pipe run 11B into the improved channel.

Offsite Basin OS-6's 16.51 acres consists of large lot residential as based upon the land use map for MDDP Amend II. Runoff ($Q_5=9$ cfs and $Q_{100}=20$ cfs) from Basin OS-6's area is captured on the east boundary of Addition 7 (Quail Brush Creek) by a proposed 1' high berm that will direct the runoff south and in several locations open space Tracts I, L & M will route the flow west onto Narrow Rail Drive street section in Basin I2.

Basin I2's 3.21 acres will consist of a 1/8 acre residential lots. Runoff ($Q_5=6$ cfs and $Q_{100}=11$ cfs) will be directed by curb and gutter west to Design point 13, a low point where a 20' sump inlets will capture the flow.

Basin I1's 6.40 acres will consist of a 1/8 acre residential lots and park area. Runoff ($Q_5=13$ cfs and $Q_{100}=25$ cfs) will be directed by curb and gutter south to Design point 13, a low point where it will combine with Basin I2 and OS-6 runoff. The combined runoff ($Q_5=20$ cfs and $Q_{100}=40$ cfs) is captured by the 20 sump inlet at Design point 13. A 30" storm drain (Pipe Run 12) directs the flow west through Tract S onto a proposed 20' x 15' d50 = 12" rip-rap pad and into the improved channel.

Basin J's 1.19 acres consists of rear yards along the south boundary of Addition 7. Runoff ($Q_5=3$ cfs and $Q_{100}=6$ cfs) will be directed offsite down side lot line swales to the open space area just south of Quail Brush Creek that is city maintained. From here the runoff sheet flows into Sand Creek.

Basin K's 4.05 acres consists of rear lots and open space of Tract H. Runoff ($Q_5=4$ cfs and $Q_{100}=9$

cfs) sheet flows to the overland to the channel. The channel will remain natural for a majority of the length where the existing wetlands will be protected in place. A small portion of the channel will be improved by grading in a 10' trapezoidal section with 4:1 side slopes. The channel will route the flow south west along the same historic path. At the street crossing of Cat Tail drive 3-60' RCP culverts will be installed to pass the runoff under the street section.

Basin L's 8.31 acres consists of rear lots and open space Runoff ($Q_5=11$ cfs and $Q_{100}=24$ cfs) sheet flows to the overland to the improved channel. The improved channel will route the flow south west along the same historic path.

HYDROLOGIC CALCULATIONS

Hydrologic calculations were performed using the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual. The Rational Method was used to estimate storm water runoff anticipated from design storms with 5-year and 100-year recurrence intervals.

HYDRAULIC CALCULATIONS

Hydraulic calculations were estimated using the Manning's Formula and the methods described in the El Paso County and City of Colorado Springs Storm Drainage Design Criteria manual the pertinent data sheets are included in the appendix of this report.

FLOODPLAIN STATEMENT

No portion of this site is within a designated F.E.M.A. floodplain, as determined by Flood Insurance Rate Map No. 08041C0537 F dated March 17, 1997 (see appendix). Although Sand Creek sits just south of our site at the south east corner the floodplain does not impact our site. Due to the proximity of the creek to our property we looked at JR Engineering's "Sand Creek Channel Improvement Phase 1 & 2" plans, and HEC-RAS model for any concerns. We looked at sections 295, 300 and 305 (see appendix for exhibit). The 100-year and floodplain is approximately 80' from our south boundary at cross section 295 and has a calculated high water elevation of 6783.45. At cross section 300 the calculated high water elevation is 6786.70 and the 100-year floodplain is 60 feet away from the boundary. The elevation at the boundary is a 6788.00 and the finished floors of the building in this area will sit at about 6792.00 or above. After reviewing the above mentioned

plans and looking at our proposed grading it does not seem that we need to provide any kind of bank stabilization.

EROSION CONTROL

It is the policy of the City of Colorado Springs that we submit an erosion control plan with the drainage report. At this time we respectfully request that the erosion control plan be submitted in conjunction with the final grading plan. Proposed straw bale check dams, silt fence, vehicle traffic control, and reseeded are proposed as erosion control measures.

CONSTRUCTION COST OPINION FOR WOODMEN HEIGHTS 7 (QUAIL BRUSH CREEK) Public Drainage Facilities Reimbursable

(Storm Drain for future Adjacent Development to West)

1.	60" RCP	170 LF	\$ 120	<u>\$ 20,400</u>
Total				\$ 20,400

(Channel Improvements per DBPS)

Item	Description	Quantity	Unit Cost	Cost
1.	Channel Improvements	385 LF	\$ 200/LF*1.66	\$ 127,820
2.	60" RCP Culverts	360 LF	\$ 120	<u>\$ 43,200</u>
Total				\$ 171,020

Public Drainage Facilities Non-Reimbursable

1.	18" RCP	145 LF	\$ 45	\$ 6,525
2.	24" RCP	530 LF	\$ 50	\$ 26,500
3.	30" RCP	1175 LF	\$ 60	\$ 70,500
4.	36" RCP	1140 LF	\$ 70	\$ 79,800
5.	48" RCP	165 LF	\$ 100	\$ 16,500
6.	60" RCP	170 LF	\$ 120	\$ 20,400
7.	6' D-10R Inlet	7 EA	\$ 4,000	\$ 28,000
8.	8' D-10R Inlet	1 EA	\$ 4,500	\$ 4,500
9.	10' D-10R Inlet	1 EA	\$ 5,500	\$ 5,500
10.	12' D-10R Inlet	2 EA	\$ 6,000	\$ 12,000

11.	20' D-10R Inlet	2EA	\$ 7,200	\$ 14,400
12.	D-9 Inlets	4 EA	\$ 5,500	\$ 22,000
13.	Type 1 Manholes	8 EA	\$ 4,000	<u>\$ 32,000</u>
Total				\$ 338,625

CONSTRUCTION COST OPINION FOR WOODMEN HEIGHTS 8 Private Drainage

Facilities Non-Reimbursable

1.	18" RCP	960 LF	\$ 45	\$ 43,200
2.	Type 1 Manholes	2 EA	\$ 4,000	\$ 8,000
3.	Detention Pond	1 EA	\$ 10,000	<u>\$ 10,000</u>
Total				\$ 61,200

CONSTRUCTION COST OPINION FOR WOODMEN HEIGHTS 9 Private Drainage

Facilities Non-Reimbursable

1.	Detention Pond	1 EA	\$ 10,000	<u>\$ 10,000</u>
Total				\$ 10,000

SUMMARY

Development of this site will not adversely affect the surrounding development. Proposed flows, as detailed in this report, will follow the drainage patterns outlined in this report and will be conveyed overland to the channel or to Sand Creek where they will outfall as shown on the approved MDDP Amend II. Although there is an increase in the runoff it was anticipated in the MDDP and will be designed for in the outfall facilities downstream. Care will be taken to accommodate overland emergency flow routes on site and temporary drainage conditions.

PREPARED BY:
TERRA NOVA ENGINEERING, INC.

Quentin N. Armijo, P.E.
Project Engineer
Jobs/0537.00/word/md&p FINAL 3-15-07

BIBLIOGRAPHY

“El Paso County and City of Colorado Springs Drainage Criteria Manual”

SCS Soils Map for El Paso County

“Master Development Drainage Plan Amendmnet No. II For The Easterly Portion Of Ridgeview Subdivision And Preliminary Drainage Report For The Northeasterly Portion Of Ridgeview Subdivision And Phase II Sand Creek Channel Improvements.” Prepared by JR Engineering, February 2004.

“Final Drainage Report for The Black Forest Road Portion of Greenhaven Filing No. 1, a Portion of Dublin Boulevard and Final Design/ Analysis Report for Black Forest Storm Drain”, by JR Engineering, March 10, 2004.

“Final Drainage Report Greenhaven Filing No. 1 and 2 Colorado Springs, Colorado” by Terra Nova Engineering, Inc., dated March 2004.

“Sand Creek Drainage Basin Planning Study Preliminary Design Report” (DBPS) prepared by Kiowa Engineering, revised December 1998.

“Sand Creek Drainage Basin Planning Study Preliminary Design Report Technical Addendum” by Kiowa Engineering, revised October 1995.

GENERAL LOCATION MAP



- [Parcel Search](#)
- [Home](#)
- [Assessor's Office Home](#)
- [El Paso County Home](#)

El Paso County Geographic Information System

Parcel Highlighted (in red) is:

5308000098

Click Parcel Number for Property Information

Located on Assessor's MapSheet: 53080

Enter Parcel to Find:

Find

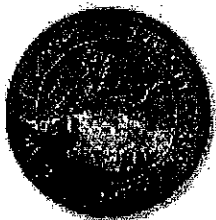
Click on Map to:

- Identify Parcel
- Zoom In
- Zoom Out
- Full County

Pan Bu

Center Butto to Full C

This site uses Unisys Patent No 4,558,302 and/or foreign counterparts and is provided free by Unisys as a public service.

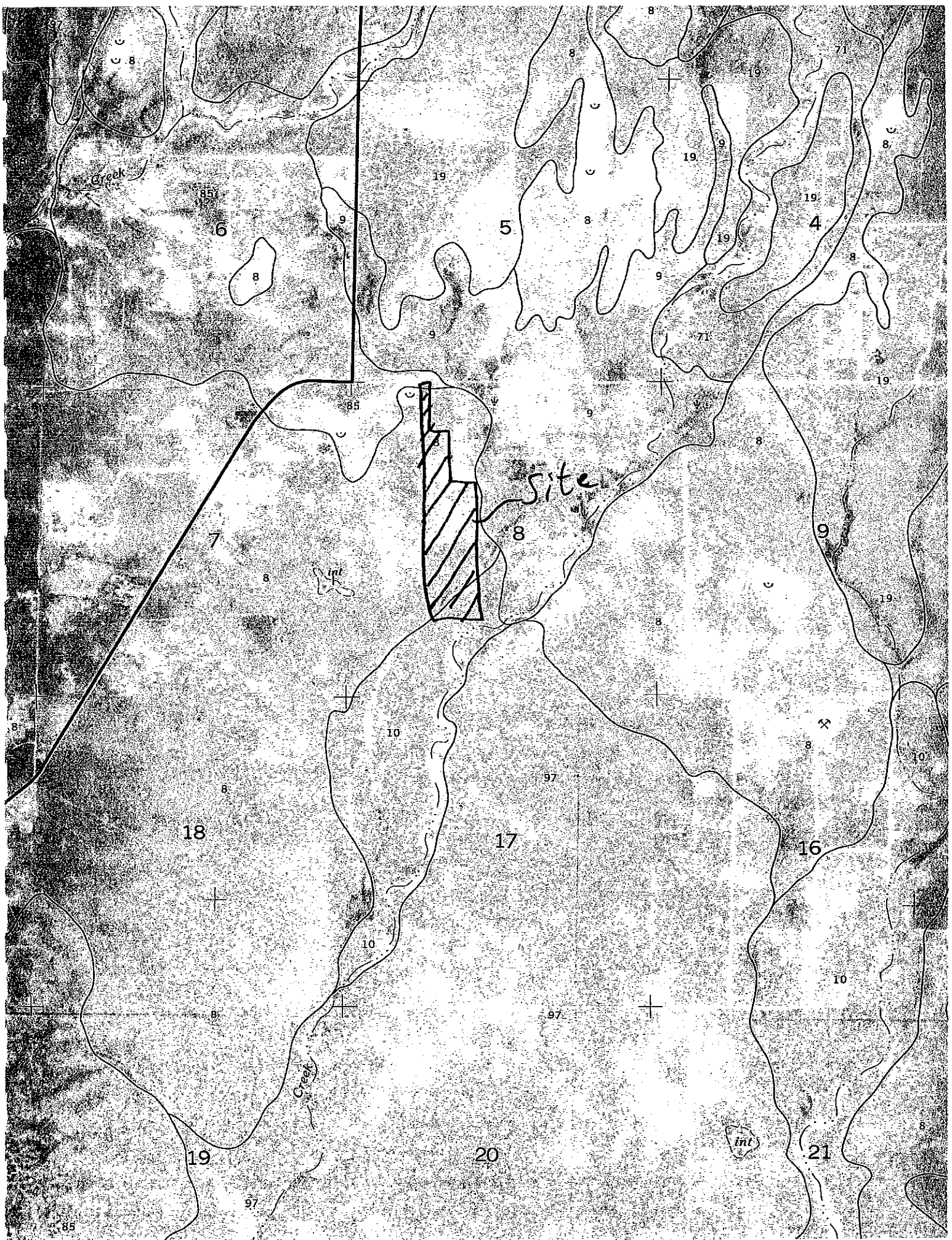


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Last updated 11/14/05

email: webmaster@elpasoco.com

S.C.S. SOILS MAP



FEMA FIRM MAP



APPROXIMATE SCALE IN FEET
 500 0 500

MAINE LANE

NEBRASKA LANE

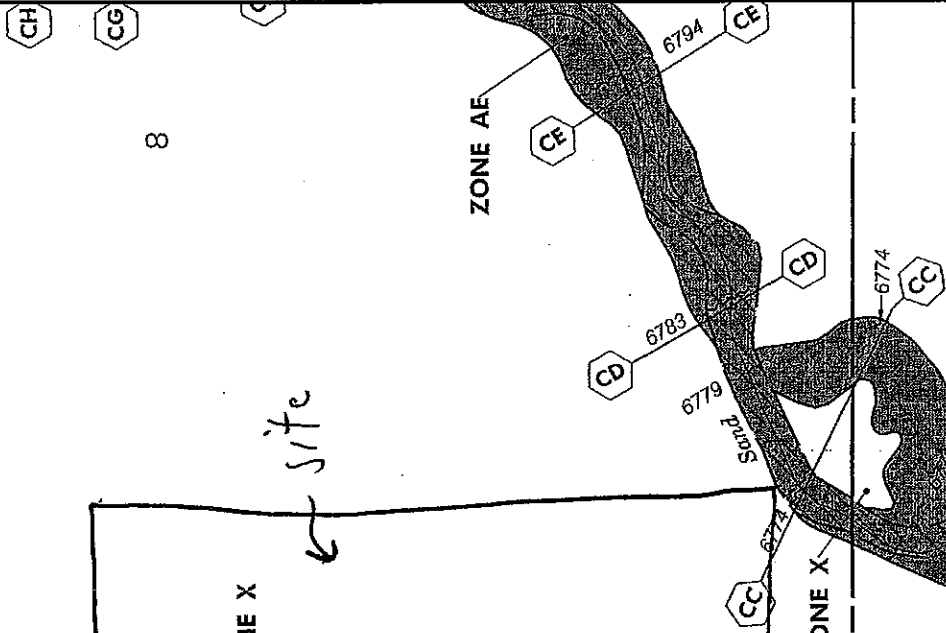
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NATIONAL FLOOD INSURANCE PROGRAM

FIRM
FLOOD INSURANCE RATE MAP
 EL PASO COUNTY,
 COLORADO AND
 INCORPORATED AREAS

PANEL 537 OF 1300
 (SEE MAP INDEX FOR PANELS NOT PRINTED)

CONTAINS:	COMMUNITY	NUMBER	PANEL	SUFFIX
COLORADO SPRINGS, CITY OF	060260	0537	F	F
UNINCORPORATED AREAS	060260	0537	F	F

MAP NUMBER
 08041C0537 F

EFFECTIVE DATE:
 MARCH 17, 1997



Federal Emergency Management Agency

This is an official copy of a portion of the above referenced flood map. It was extracted using F-MIT On-Line. This map does not reflect changes or amendments which may have been made subsequent to the date on the title block. For the latest product information about National Flood Insurance Program flood maps check the FEMA Flood Map Store at www.msc.fema.gov

HYDROLOGIC CALCULATIONS

QUAIL BRUSH CREEK MDDP
(Area Runoff Coefficient Summary)
PROPOSED CONDITIONS

BASIN	TOTAL AREA (Acres)	STREETS / DEVELOPED			OVERLAND / UNDEVELOPED			WEIGHTED	
		AREA (Acres)	C ₅	C ₁₀₀	AREA (Acres)	C ₅	C ₁₀₀	C ₅	C ₁₀₀
<i>OS-1</i>	15.74	15.74	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>OS-1A</i>	0.67	0.67	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>OS-2</i>	22.76	22.76	0.30	0.40	0.00	0.30	0.40	<i>0.30</i>	<i>0.40</i>
<i>OS-2A</i>	0.69	0.69	0.90	0.95	0.00	0.25	0.35	<i>0.90</i>	<i>0.95</i>
<i>OS-2B</i>	0.84	0.84	0.90	0.95	0.00	0.25	0.35	<i>0.90</i>	<i>0.95</i>
<i>OS-3A</i>	7.46	7.46	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>OS-3B</i>	0.17	0.13	0.90	0.95	0.04	0.25	0.35	<i>0.76</i>	<i>0.82</i>
<i>OS-4A</i>	8.06	8.06	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>OS-4B</i>	8.21	8.21	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>OS-5</i>	3.18	3.18	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>OS-6</i>	16.51	16.51	0.30	0.40	0.00	0.25	0.35	<i>0.30</i>	<i>0.40</i>
<i>A1</i>	0.51	0.00	0.60	0.70	0.51	0.25	0.35	<i>0.25</i>	<i>0.35</i>
<i>A2</i>	0.83	0.00	0.60	0.70	0.83	0.25	0.35	<i>0.25</i>	<i>0.35</i>
<i>B1</i>	4.34	4.34	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>B2</i>	5.33	5.33	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>B3</i>	3.35	3.35	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>B4</i>	1.19	1.19	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>C1</i>	2.09	2.09	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>C2</i>	0.93	0.93	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>H-D*</i>	4.20	0.00	0.90	0.90	4.20	0.25	0.35	<i>0.25</i>	<i>0.35</i>
<i>D</i>	4.20	4.20	0.90	0.90	0.00	0.25	0.35	<i>0.90</i>	<i>0.90</i>
<i>H-E*</i>	9.97	0.00	0.90	0.90	9.97	0.25	0.35	<i>0.25</i>	<i>0.35</i>
<i>E</i>	9.97	9.97	0.90	0.90	0.00	0.25	0.35	<i>0.90</i>	<i>0.90</i>
<i>F1</i>	4.84	4.84	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>F2</i>	1.33	1.33	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>F3</i>	3.33	3.33	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>F4</i>	1.26	1.26	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>
<i>G1</i>	3.36	3.36	0.60	0.70	0.00	0.25	0.35	<i>0.60</i>	<i>0.70</i>

QUAIL BRUSH CREEK MDDP
(Area Runoff Coefficient Summary)
PROPOSED CONDITIONS

BASIN	TOTAL AREA (Acres)	STREETS / DEVELOPED			OVERLAND / UNDEVELOPED			WEIGHTED	
		AREA (Acres)	C ₅	C ₁₀₀	AREA (Acres)	C ₅	C ₁₀₀	C ₅	C ₁₀₀
G2	1.70	1.70	0.60	0.70	0.00	0.25	0.35	0.60	0.70
G3	1.40	1.40	0.60	0.70	0.00	0.25	0.35	0.60	0.70
H1	2.72	0.00	0.60	0.70	2.72	0.25	0.35	0.25	0.35
H2	0.22	0.00	0.60	0.70	0.22	0.25	0.35	0.25	0.35
II	6.40	6.40	0.60	0.70	0.00	0.25	0.35	0.60	0.70
I2	3.21	3.21	0.60	0.70	0.00	0.25	0.35	0.60	0.70
J	1.19	1.19	0.60	0.70	0.00	0.25	0.35	0.60	0.70
K	4.05	0.81	0.60	0.70	3.25	0.25	0.35	0.32	0.42
L	8.31	3.06	0.60	0.70	5.25	0.25	0.35	0.38	0.48

*** BASIN H-D & H-E ARE THE HISTORIC BASINS FOR D & E FOR THE PURPOSE OF CALCULATING ALLOWABLE RELEASE RATES FOR FUTURE COMMERCIAL AREAS.**

Calculated by: QNA
Date: 3/15/2007
Checked by: _____

QUAIL BRUSH CREEK MDDP AREA DRAINAGE SUMMARY

BASIN	AREA TOTAL (Acres)	WEIGHTED		OVERLAND				STREET / CHANNEL FLOW				T_t	INTENSITY		TOTAL FLOWS	
		C_5	C_{100}	C_5	Length (ft)	Height (ft)	T_C (min)	Length (ft)	Slope (%)	Velocity (fps)	T_t (min)	TOTAL (min)	I_5 (in/hr)	I_{100} (in/hr)	Q_5 (c.f.s.)	Q_{100} (c.f.s.)
<i>OS-1</i>	15.74	0.30	0.40	0.25	300	14.0	16.6	875	6.6%	1.9	7.7	24.2	2.8	4.6	13	29
<i>OS-1A</i>	0.67	0.30	0.40	0.25	20	0.5	5.3	250	4.0%	4.0	1.0	6.3	4.7	8.4	1	2
<i>OS-2</i>	22.76	0.30	0.40	0.25	300	10.0	18.5	1200	6.3%	1.9	10.5	29.0	2.5	4.1	17	38
<i>OS-2A</i>	0.69	0.90	0.95	0.25	20	0.5	5.3	800	4.0%	4.0	3.3	8.6	4.3	7.5	3	5
<i>OS-2B</i>	0.84	0.90	0.95	0.25	20	0.5	5.3	800	4.0%	4.0	3.3	8.6	4.3	7.5	3	6
<i>OS-3A</i>	7.46	0.30	0.40	0.25	300	32.0	12.6	850	7.1%	2.0	7.1	19.7	3.1	5.1	7	15
<i>OS-3B</i>	0.17	0.76	0.82	0.25	10	0.5	3.0	200	3.0%	3.0	1.1	5.0	5.0	9.1	1	1
<i>OS-4A</i>	8.06	0.30	0.40	0.25	300	22.0	14.3	550	6.9%	2.0	4.6	18.8	3.1	5.3	8	17
<i>OS-4B</i>	8.21	0.30	0.40	0.25	300	22.0	14.3	551	6.9%	3.0	3.1	17.3	3.3	5.5	8	18
<i>OS-5</i>	3.18	0.30	0.40	0.25	300	10.0	18.5	325	2.5%	1.1	4.9	23.4	2.8	4.7	3	6
<i>OS-6</i>	16.51	0.30	0.40	0.25	300	12.0	17.4	2250	2.7%	1.2	31.3	48.7	1.9	3.0	9	20
<i>A1</i>	0.51	0.25	0.35	0.25	10	1.0	2.4	670	1.8%	2.5	4.5	6.8	4.6	8.2	1	1
<i>A2</i>	0.83	0.25	0.35	0.25	10	1.0	2.4	1200	3.3%	4.0	5.0	7.4	4.5	8.0	1	2
<i>B1</i>	4.34	0.60	0.70	0.25	120	2.4	13.9	1100	2.7%	3.5	5.2	19.1	3.1	5.2	8	16
<i>B2</i>	5.33	0.60	0.70	0.25	140	4.0	13.3	880	2.7%	3.5	4.2	17.5	3.2	5.5	10	20
<i>B3</i>	3.35	0.60	0.70	0.25	100	2.0	12.6	340	0.8%	1.8	3.1	15.8	3.4	5.7	7	13
<i>B4</i>	1.19	0.60	0.70	0.25	120	2.4	13.9	570	0.8%	1.8	5.3	19.1	3.1	5.2	2	4
<i>C1</i>	2.09	0.60	0.70	0.25	120	4.0	11.7	640	3.0%	3.7	2.9	14.6	3.5	6.0	4	9
<i>C2</i>	0.93	0.60	0.70	0.25	55	1.1	9.4	640	3.0%	3.7	2.9	12.3	3.8	6.5	2	4
<i>H-D*</i>	4.20	0.25	0.35	0.25	100	2.0	12.6	800	2.5%	3.0	4.4	17.1	3.3	5.5	3	8
<i>D</i>	4.20	0.90	0.90	0.25	100	2.0	12.6	800	2.5%	3.0	4.4	17.1	3.3	5.5	12	21
<i>H-E*</i>	9.97	0.25	0.35	0.25	100	2.0	12.6	615	2.5%	3.0	3.4	16.1	3.4	5.7	8	20
<i>E</i>	9.97	0.90	0.90	0.25	100	2.0	12.6	615	2.5%	3.0	3.4	16.1	3.4	5.7	30	51
<i>F1</i>	4.84	0.60	0.70	0.25	185	5.0	15.6	1400	2.1%	2.8	8.3	23.9	2.8	4.6	8	16
<i>F2</i>	1.33	0.60	0.70	0.25	55	1.1	9.4	890	2.0%	2.7	5.5	14.9	3.5	5.9	3	6
<i>F3</i>	3.33	0.60	0.70	0.25	130	3.0	13.8	745	1.6%	2.4	5.2	18.9	3.1	5.2	6	12
<i>F4</i>	1.26	0.60	0.70	0.25	170	3.4	16.5	130	1.2%	2.3	0.9	17.4	3.2	5.5	2	5
<i>G1</i>	3.36	0.60	0.70	0.25	120	3.0	12.9	700	1.7%	2.5	4.7	17.5	3.2	5.5	7	13
<i>G2</i>	1.70	0.60	0.70	0.25	120	3.0	12.9	460	1.3%	2.3	3.3	16.2	3.4	5.7	3	7

QUAIL BRUSH CREEK MDDP AREA DRAINAGE SUMMARY

BASIN	AREA TOTAL (Acres)	WEIGHTED		OVERLAND				STREET / CHANNEL FLOW				T_t	INTENSITY		TOTAL FLOWS		
		C_5	C_{100}	C_5	Length (ft)	Height (ft)	T_C (min)	Length (ft)	Slope (%)	Velocity (fps)	T_t (min)	TOTAL (min)	I_5 (in/hr)	I_{100} (in/hr)	Q_5 (c.f.s.)	Q_{100} (c.f.s.)	
		<i>* For Calcs See Runoff Summary</i>															
G3	1.40	0.60	0.70	0.25	55	1.1	9.4	460	1.3%	2.3	3.3	12.7	3.7	6.4	3	6	
H1	2.72	0.25	0.35	0.25	120	3.0	12.9	670	1.2%	2.2	5.1	17.9	3.2	5.4	2	5	
H2	0.22	0.25	0.35	0.25	10	0.5	3.0	130	1.2%	2.2	1.0	5.0	5.0	9.1	1	1	
I1	6.40	0.60	0.70	0.25	55	1.1	9.4	1020	1.4%	2.3	7.4	16.8	3.3	5.6	13	25	
I2	3.21	0.60	0.70	0.25	80	1.6	11.3	1360	1.2%	2.2	10.3	21.6	2.9	4.9	6	11	
J	1.19	0.60	0.70	0.25	80	1.6	11.3	0	0.0%	0.0	0.0	11.3	3.9	6.7	3	6	
K	4.05	0.32	0.42	0.25	260	8.0	17.7	0	0.0%	0.0	0.0	17.7	3.2	5.4	4	9	
L	8.31	0.38	0.48	0.25	120	2.4	13.9	0	0.0%	0.0	0.0	13.9	3.6	6.1	11	24	

*** BASIN H-D & H-E ARE THE HISTORIC BASINS FOR D & E FOR THE PURPOSE OF CALCULATING ALLOWABLE RELEASE RATES FOR FUTURE COMMERCIAL AREAS.**

Calculated by: QNA

Date: 3/15/2007

Checked by: _____

QUAIL BRUSH CREEK SURFACE ROUTING SUMMARY

Design Point(s)	Contributing Basins	Equivalent CA ₅	Equivalent CA ₁₀₀	Maximum T _C	Intensity		Flow	
					I ₅	I ₁₀₀	Q ₅	Q ₁₀₀
1	OS-1	4.72	6.30	24.2	2.8	4.6	13	29
1A	OS-1A	0.60	0.64	6.3	4.7	8.4	3	5
2A	OS-2 & OS-2A	7.45	9.76	29.0	2.5	4.1	19	40
2B	OS-2B	0.75	0.80	8.6	4.3	7.5	3	6
3A	OS-3A	2.24	2.98	19.7	3.1	5.1	7	15
3B	OS-3B	0.13	0.14	5.0	5.0	9.1	1	1
4A	OS-4A, A1, PR-3A	13.11	17.08	29.0	2.5	4.1	33	71
4B	DP-4A, OS-4B & A2	15.57	20.36	29.0	2.5	4.1	39	84
5A	B1	2.60	3.04	19.1	3.1	5.2	8	16
5B	B2	3.20	3.73	17.5	3.2	5.5	10	20
5C	B3	2.01	2.34	15.8	3.4	5.7	7	13
5D	B4	0.71	0.83	19.1	3.1	5.2	2	4
5E	DP-5A - DP-5D	8.52	9.94	19.1	3.1	5.2	27	52
6A	C1	1.25	1.46	14.6	3.5	6.0	4	9
6B	C2	0.56	0.65	12.3	3.8	6.5	2	4
7	DP-2A thru DP-4B, DP-6A & DP-6B	17.38	22.47	29.0	2.5	4.1	44	93
H-8*	H-D	1.05	1.47	17.1	3.3	5.5	3	8
8	D	3.78	3.78	17.1	3.3	5.5	12	21
H-9*	H-D & H-E	3.54	4.96	17.1	3.3	5.5	12	27
9	D & E	12.76	12.76	16.1	3.4	5.7	43	73
10A	G1 & G2	3.03	3.54	17.5	3.2	5.5	10	19
10B	G3	0.84	0.98	12.7	3.7	6.4	3	6
11	F1, F2, F3, & F4	6.45	7.53	23.9	2.8	4.6	18	35

QUAIL BRUSH CREEK SURFACE ROUTING SUMMARY

<i>Design Point(s)</i>	<i>Contributing Basins</i>	<i>Equivalent CA₅</i>	<i>Equivalent CA₁₀₀</i>	<i>Maximum T_C</i>	<i>Intensity</i>		<i>Flow</i>	
					<i>I₅</i>	<i>I₁₀₀</i>	<i>Q₅</i>	<i>Q₁₀₀</i>
<i>12A</i>	<i>H1</i>	0.68	0.95	17.9	3.2	5.4	<i>2</i>	<i>5</i>
<i>12B</i>	<i>H2</i>	0.05	0.08	5.0	5.0	9.1	<i>1</i>	<i>1</i>
<i>13</i>	<i>I1 & I2</i>	10.72	13.33	48.7	1.9	3.0	<i>20</i>	<i>40</i>

* DESIGN POINTS H-8 & H-9 ARE THE HISTORIC FLOWS FOR DP-8 & DP-9 FOR THE PURPOSE OF CALCULATING DETENTION FOR FUTURE COMMERCIAL AREAS.

Date: 3/15/2007

Checked by: _____

QUAIL BRUSH CREEK MDDP PIPE and SWALE ROUTING SUMMARY

<i>Pipe Run(s)</i>	<i>Contributing Design Points</i>	<i>Equivalent CA₅</i>	<i>Equivalent CA₁₀₀</i>	<i>Maximum T_C</i>	<i>Intensity</i>		<i>Flow</i>	
					<i>I₅</i>	<i>I₁₀₀</i>	<i>Q₅</i>	<i>Q₁₀₀</i>
<i>1</i>	<i>DP-1 & DP-1A</i>	5.33	6.93	24.2	2.8	4.6	<i>15</i>	<i>32</i>
<i>1A</i>	<i>DP-1A</i>	0.60	0.64	6.3	4.7	8.4	<i>3</i>	<i>5</i>
<i>2A</i>	<i>DP-2B</i>	0.75	0.80	8.6	4.3	7.5	<i>3</i>	<i>6</i>
<i>2B</i>	<i>DP-2A & DP-2B</i>	8.20	10.56	29.0	2.5	4.1	<i>21</i>	<i>44</i>
<i>3</i>	<i>DP-3B</i>	0.13	0.14	5.0	5.0	9.1	<i>1</i>	<i>1</i>
<i>3A</i>	<i>DP-3A, PR-3, & PR-2A</i>	10.57	13.67	29.0	2.5	4.1	<i>27</i>	<i>56</i>
<i>4A</i>	<i>DP-4A & PR-3A</i>	13.11	17.08	29.0	2.5	4.1	<i>33</i>	<i>71</i>
<i>4B</i>	<i>DP-4B</i>	15.57	20.36	29.0	2.5	4.1	<i>39</i>	<i>84</i>
<i>5A</i>	<i>DP-5A</i>	2.60	3.04	19.1	3.1	5.2	<i>8</i>	<i>16</i>
<i>5B</i>	<i>DP-5B</i>	3.20	3.73	17.5	3.2	5.5	<i>10</i>	<i>20</i>
<i>5C</i>	<i>PR-5A, PR-5B, DP-5C</i>	7.81	9.11	19.1	3.1	5.2	<i>24</i>	<i>48</i>
<i>5D</i>	<i>PR-5C & DP-5D</i>	8.52	9.94	19.1	3.1	5.2	<i>27</i>	<i>52</i>
<i>6A</i>	<i>PR-4B & DP-6A</i>	16.83	21.82	29.0	2.5	4.1	<i>42</i>	<i>90</i>
<i>6B</i>	<i>DP-6B & PR-6A</i>	17.38	22.47	29.0	2.5	4.1	<i>44</i>	<i>93</i>
<i>7**</i>	<i>DP-8</i>	1.05	1.47	17.1	3.3	5.5	<i>3</i>	<i>8</i>
<i>8**</i>	<i>DP-9</i>	3.54	4.96	17.1	3.3	5.5	<i>12</i>	<i>27</i>
<i>9</i>	<i>DP-11 & PR-8</i>	10.00	12.49	23.9	2.8	4.6	<i>28</i>	<i>58</i>
<i>10A</i>	<i>DP-10A</i>	3.03	3.54	17.5	3.2	5.5	<i>10</i>	<i>19</i>

QUAIL BRUSH CREEK MDDP PIPE and SWALE ROUTING SUMMARY

10B	DP-10A & DP-10B	3.88	4.52	17.5	3.2	5.5	13	25
11A	DP-12A	0.68	0.95	17.9	3.2	5.4	2	5
11B	DP-12A & DP-12B	0.73	1.03	17.9	3.2	5.4	2	6
12	DP-13	10.72	13.33	48.7	1.9	3.0	20	40

PIPERUNS 1 & 1A ARE SHOWN FOR INFORMATION ONL. THESE RUNS ARE TO BE CONSTRUCTED W/ DMJM IMPROVEMENTS

**** PIPERUNS 7 AND 8 ARE BASED UPON THE ALLOWABLE HISTORIC RELEASE FOR BASINS D & E**

Swale Run(s)	Contributing Design Points	Equivalent CA₅	Equivalent CA₁₀₀	Maximum T_c	Intensity		Flow	
					I₅	I₁₀₀	Q₅	Q₁₀₀
1	DP-3A	10.57	13.67	29.0	2.5	4.1	27	56
2	DP-4A	13.11	17.08	29.0	2.5	4.1	33	71

Calculated by: QNA

Date: 3/15/2007

Checked by: _____

HYDRAULIC CALCULATIONS

DESIGN POINT 1A

Total Flow: Q_5 = 3 cfs
 Q_{100} = 5 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.83 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i(1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

(Install a 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: #####
Checked by:

DESIGN POINT 2A

Total Flow: Q_5 = 19 cfs
 Q_{100} = 40 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = = 1.7(Li+1.8(W))(dmax + w/12)^{1.85}$$

Clogging Factor = 1.25

$Li (1.25)$ = Length of inlet opening

5-Year Event: 16 foot inlet required

100-Year Event: 24 foot inlet required

(Install two 14' D-10-R inlets to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA

Date: #####

Checked by:

DESIGN POINT 2B

Total Flow: Q_5 = 3 cfs
 Q_{100} = 6 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i(1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

(Install a 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: #####
Checked by:

DESIGN POINT 3B

Total Flow: Q_5 = 1 cfs
 Q_{100} = 1 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

(Install a 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: #####
Checked by:

Design Point 4A

Equation 7-2

$$Q = \frac{3 \times P \times d^{1.5}}{F}$$

Maxium Ponding Depth (d) =	1 ft
Discharge (Q) =	71 cfs
Clogging Factor (F) =	2
Grate perimeter (P) =	47.01 ft

Diameter (D) **Circular Grate Inlet**
D = 7.48 ft

Length of Each Side (L) **Rectangle Grate Inlet**
L = 11.75 ft

**Install 2 10' City Std. D-9 inlets
at this design point.**

Design Point 4B

Equation 7-2

$$Q = \frac{3 \times P \times d^{1.5}}{F}$$

Maxium Ponding Depth (d) =	1 ft
Discharge (Q) =	84 cfs
Clogging Factor (F) =	2
Grate perimeter (P) =	56.05 ft

Diameter (D) **Circular Grate Inlet**
D = 8.92 ft

Length of Each Side (L) **Rectangle Grate Inlet**
L = 14.01 ft

**Install 3 10' City Std. D-9 inlets
at this design point.**

DESIGN POINT 5A

Total Flow: Q_5 = 8 cfs
 Q_{100} = 16 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 6 foot inlet required

*(Install a 8' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows
at this design point.)*

Calculated by: QNA

Date: 5/05/2006

Checked by: _____

DESIGN POINT 5B

Total Flow: Q_5 = **10 cfs**
 Q_{100} = **20 cfs**

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

Q_i = $1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: **6** foot inlet required

100-Year Event: **10** foot inlet required

(Install a 12' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 5C

Total Flow: Q_5 = 7 cfs
 Q_{100} = 13 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

**(Install a 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows
at this design point.)**

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 5D

Total Flow: Q_5 = 2 cfs
 Q_{100} = 4 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i(1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

(Install a 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 6A

5-YR FLOW					
	Q(5)	4	I(5)	3.5	
	DEPTH	0.28	Fr	2.54	Inlet size ? L(i) = 20
	SPREAD	12.9	L(1)	25.2	If $L_i < L(2)$ then $Q_i = 3.5$
	CROSS SLOPE	2.0%	L(2)	15.1	If $L_i > L(2)$ then $Q_i = 3.0$
	STREET SLOPE	4.0%	L(3)	54.0	FB = 1
					CA(eqv.)= 0.41

100-YR FLOW					
	Q(100)	9	I(100)	6.0	
	DEPTH	0.35	Fr	2.65	Inlet size ? L(i) = 20
	SPREAD	16.1	L(1)	32.9	If $L_i < L(2)$ then $Q_i = 5.3$
	CROSS SLOPE	2.0%	L(2)	19.8	If $L_i > L(2)$ then $Q_i = 5.3$
	STREET SLOPE	4.0%	L(3)	70.6	FB = 3
					CA(eqv.)= 0.58

DESIGN POINT 6A

5-YR FLOW					
	Q(5)	2	I(5)	3.8	
	DEPTH	0.22	Fr	2.41	Inlet size ? L(i) = 12
	SPREAD	9.9	L(1)	18.3	If Li < L(2) then Qi = 1.4
	CROSS SLOPE	2.0%	L(2)	11.0	If Li > L(2) then Qi = 1.3
	STREET SLOPE	4.0%	L(3)	39.2	FB = 1
					CA(eqv.)= 0.21

100-YR FLOW					
	Q(100)	4	I(100)	6.5	
	DEPTH	0.28	Fr	2.53	Inlet size ? L(i) = 12
	SPREAD	12.6	L(1)	24.6	If Li < L(2) then Qi = 2.1
	CROSS SLOPE	2.0%	L(2)	14.8	If Li > L(2) then Qi = 2.3
	STREET SLOPE	4.0%	L(3)	52.7	FB = 2
					CA(eqv.)= 0.33

DESIGN POINT 10A

Total Flow: Q_5 = 10 cfs
 Q_{100} = 19 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 6 foot inlet required

100-Year Event: 8 foot inlet required

(Install a 10' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 10B

Total Flow: Q_5 = 3 cfs
 Q_{100} = 6 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

**(Install a 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows
at this design point.)**

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 11

Total Flow: Q_5 = 18 cfs
 Q_{100} = 35 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.67 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 16 foot inlet required

100-Year Event: 20 foot inlet required

(Install an 20' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 12A

Total Flow: Q_5 = 2 cfs
 Q_{100} = 5 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.83 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

(Install 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 12B

Total Flow: Q_5 = 1 cfs
 Q_{100} = 1 cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.83 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: 4 foot inlet required

100-Year Event: 4 foot inlet required

(Install 6' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

DESIGN POINT 13

Total Flow: Q_5 = **20** cfs
 Q_{100} = **40** cfs

Maximum allowable ponding depth at sump:

D_5 = 0.45
 D_{100} = 0.83 (dmax)

$$Q_i = 1.7(L_i + 1.8(W))(d_{max} + w/12)^{1.85}$$

Clogging Factor = 1.25
 $L_i (1.25)$ = Length of inlet opening

5-Year Event: **18** foot inlet required

100-Year Event: **16** foot inlet required

(Install 20' D-10-R inlet to accept both 5 yr. & 100 yr. developed flows at this design point.)

Calculated by: QNA
Date: 5/05/2006
Checked by: _____

Culvert Calculator Report

CHANNEL XING

Solve For: Section Size

Culvert Summary			
Allowable HW Elevation	6,800.00 ft	Headwater Depth/Height	1.54
Computed Headwater Elevation	6,798.71 ft	Discharge	618.00 cfs
Inlet Control HW Elev.	6,798.71 ft	Tailwater Elevation	6,793.63 ft
Outlet Control HW Elev.	6,798.44 ft	Control Type	Inlet Control
Grades			
Upstream Invert	6,791.00 ft	Downstream Invert	6,790.00 ft
Length	100.00 ft	Constructed Slope	1.0000 %
Hydraulic Profile			
Profile	S2	Depth, Downstream	3.53 ft
Slope Type	Steep	Normal Depth	3.36 ft
Flow Regime	Supercritical	Critical Depth	4.09 ft
Velocity Downstream	13.89 ft/s	Critical Slope	0.6276 %
Section			
Section Shape	Circular	Mannings Coefficient	0.013
Section Material	Concrete	Span	5.00 ft
Section Size	60 inch	Rise	5.00 ft
Number Sections	3		
Outlet Control Properties			
Outlet Control HW Elev.	6,798.44 ft	Upstream Velocity Head	2.23 ft
Ke	0.50	Entrance Loss	1.11 ft
Inlet Control Properties			
Inlet Control HW Elev.	6,798.71 ft	Flow Control	Submerged
Inlet Type	Square edge w/headwall	Area Full	58.9 ft ²
K	0.00980	HDS 5 Chart	1
M	2.00000	HDS 5 Scale	1
C	0.03980	Equation Form	1
Y	0.67000		

Pipe run 1A

Project Description

Worksheet	PR-1A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	18 in
Discharge	5.00 cfs

Results

Slope	0.23 %
Depth	1.50 ft
Flow Area	1.8 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	0.86 ft
Percent Full	100.0 %
Critical Slope	0.58 %
Velocity	2.83 ft/s
Velocity Head	0.12 ft
Specific Energy	1.62 ft
Froude Number	0.00
Maximum Discharge	5.38 cfs
Discharge Full	5.00 cfs
Slope Full	0.23 %
Flow Type	N/A

Pipe run 2A

Project Description

Worksheet	PR-2A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	18 in
Discharge	6.00 cfs

Results

Slope	0.33 %
Depth	1.50 ft
Flow Area	1.8 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	0.95 ft
Percent Full	100.0 %
Critical Slope	0.62 %
Velocity	3.40 ft/s
Velocity Head	0.18 ft
Specific Energy	1.68 ft
Froude Number	0.00
Maximum Discharge	6.45 cfs
Discharge Full	6.00 cfs
Slope Full	0.33 %
Flow Type	N/A

Pipe run 3

Project Description

Worksheet	PR-3
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	18 in
Discharge	1.00 cfs

Results

Slope	0.01 %
Depth	1.50 ft
Flow Area	1.8 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	0.37 ft
Percent Full	100.0 %
Critical Slope	0.49 %
Velocity	0.57 ft/s
Velocity Head	4.98e-3 ft
Specific Energy	1.50 ft
Froude Number	0.00
Maximum Discharge	1.08 cfs
Discharge Full	1.00 cfs
Slope Full	0.01 %
Flow Type	N/A

Piperun 3A

Project Description

Worksheet	PR-3A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	36 in
Discharge	56.00 cfs

Results

Slope	0.71 %
Depth	3.00 ft
Flow Area	7.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.43 ft
Percent Full	100.0 %
Critical Slope	0.72 %
Velocity	7.92 ft/s
Velocity Head	0.98 ft
Specific Energy	3.98 ft
Froude Number	0.00
Maximum Discharge	60.24 cfs
Discharge Full	56.00 cfs
Slope Full	0.71 %
Flow Type	N/A

Piperun 4A

Project Description

Worksheet	PR-4A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	48 in
Discharge	71.00 cfs

Results

Slope	0.24 %
Depth	4.00 ft
Flow Area	12.6 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.55 ft
Percent Full	100.0 %
Critical Slope	0.45 %
Velocity	5.65 ft/s
Velocity Head	0.50 ft
Specific Energy	4.50 ft
Froude Number	0.00
Maximum Discharge	76.38 cfs
Discharge Full	71.00 cfs
Slope Full	0.24 %
Flow Type	N/A

Piperun 4B

Project Description

Worksheet	PR-4B
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	48 in
Discharge	84.00 cfs

Results

Slope	0.34 %
Depth	4.00 ft
Flow Area	12.6 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.78 ft
Percent Full	100.0 %
Critical Slope	0.50 %
Velocity	6.68 ft/s
Velocity Head	0.69 ft
Specific Energy	4.69 ft
Froude Number	0.00
Maximum Discharge	90.36 cfs
Discharge Full	84.00 cfs
Slope Full	0.34 %
Flow Type	N/A

Pipe run 5A

Project Description

Worksheet	PR-5A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	24 in
Discharge	16.00 cfs

Results

Slope	0.50 %
Depth	2.00 ft
Flow Area	3.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	1.44 ft
Percent Full	100.0 %
Critical Slope	0.66 %
Velocity	5.09 ft/s
Velocity Head	0.40 ft
Specific Energy	2.40 ft
Froude Number	0.00
Maximum Discharge	17.21 cfs
Discharge Full	16.00 cfs
Slope Full	0.50 %
Flow Type	N/A

Pipe run 5B

Project Description

Worksheet	PR-5B
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	24 in
Discharge	20.00 cfs

Results

Slope	0.78 %
Depth	2.00 ft
Flow Area	3.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	1.61 ft
Percent Full	100.0 %
Critical Slope	0.81 %
Velocity	6.37 ft/s
Velocity Head	0.63 ft
Specific Energy	2.63 ft
Froude Number	0.00
Maximum Discharge	21.51 cfs
Discharge Full	20.00 cfs
Slope Full	0.78 %
Flow Type	N/A

Pipe run 5C

Project Description	
Worksheet	PR-5C
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data	
Mannings Coefficient	0.013
Diameter	36 in
Discharge	48.00 cfs

Results	
Slope	0.52 %
Depth	3.00 ft
Flow Area	7.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.26 ft
Percent Full	100.0 %
Critical Slope	0.62 %
Velocity	6.79 ft/s
Velocity Head	0.72 ft
Specific Energy	3.72 ft
Froude Number	0.00
Maximum Discharge	51.63 cfs
Discharge Full	48.00 cfs
Slope Full	0.52 %
Flow Type	N/A

Pipe run 5D

Project Description

Worksheet	PR-5D
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	36 in
Discharge	52.00 cfs

Results

Slope	0.61 %
Depth	3.00 ft
Flow Area	7.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.35 ft
Percent Full	100.0 %
Critical Slope	0.67 %
Velocity	7.36 ft/s
Velocity Head	0.84 ft
Specific Energy	3.84 ft
Froude Number	0.00
Maximum Discharge	55.94 cfs
Discharge Full	52.00 cfs
Slope Full	0.61 %
Flow Type	N/A

Piperun 6A

Project Description

Worksheet	PR-6A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	60 in
Discharge	90.00 cfs

Results

Slope	0.12 %
Depth	5.00 ft
Flow Area	19.6 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.69 ft
Percent Full	100.0 %
Critical Slope	0.37 %
Velocity	4.58 ft/s
Velocity Head	0.33 ft
Specific Energy	5.33 ft
Froude Number	0.00
Maximum Discharge	96.81 cfs
Discharge Full	90.00 cfs
Slope Full	0.12 %
Flow Type	N/A

Piperun 6B

Project Description

Worksheet	PR-6B
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	60 in
Discharge	93.00 cfs

Results

Slope	0.13 %
Depth	5.00 ft
Flow Area	19.6 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.74 ft
Percent Full	100.0 %
Critical Slope	0.38 %
Velocity	4.74 ft/s
Velocity Head	0.35 ft
Specific Energy	5.35 ft
Froude Number	0.00
Maximum Discharge	100.04 cfs
Discharge Full	93.00 cfs
Slope Full	0.13 %
Flow Type	N/A

Pipe run 7

Project Description

Worksheet	PR-7
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	18 in
Discharge	8.00 cfs

Results

Slope	0.58 %
Depth	1.50 ft
Flow Area	1.8 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	1.10 ft
Percent Full	100.0 %
Critical Slope	0.74 %
Velocity	4.53 ft/s
Velocity Head	0.32 ft
Specific Energy	1.82 ft
Froude Number	0.00
Maximum Discharge	8.61 cfs
Discharge Full	8.00 cfs
Slope Full	0.58 %
Flow Type	N/A

Pipe run 8

Project Description

Worksheet	PR-8
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	30 in
Discharge	27.00 cfs

Results

Slope	0.43 %
Depth	2.50 ft
Flow Area	4.9 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	1.77 ft
Percent Full	100.0 %
Critical Slope	0.60 %
Velocity	5.50 ft/s
Velocity Head	0.47 ft
Specific Energy	2.97 ft
Froude Number	0.00
Maximum Discharge	29.04 cfs
Discharge Full	27.00 cfs
Slope Full	0.43 %
Flow Type	N/A

Pipe run 9

Project Description

Worksheet	PR-9
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	36 in
Discharge	58.00 cfs

Results

Slope	0.76 %
Depth	3.00 ft
Flow Area	7.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.47 ft
Percent Full	100.0 %
Critical Slope	0.75 %
Velocity	8.21 ft/s
Velocity Head	1.05 ft
Specific Energy	4.05 ft
Froude Number	0.00
Maximum Discharge	62.39 cfs
Discharge Full	58.00 cfs
Slope Full	0.76 %
Flow Type	N/A

Pipe run 10A

Project Description

Worksheet	PR-10A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	24 in
Discharge	19.00 cfs

Results

Slope	0.71 %
Depth	2.00 ft
Flow Area	3.1 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	1.57 ft
Percent Full	100.0 %
Critical Slope	0.77 %
Velocity	6.05 ft/s
Velocity Head	0.57 ft
Specific Energy	2.57 ft
Froude Number	0.00
Maximum Discharge	20.44 cfs
Discharge Full	19.00 cfs
Slope Full	0.71 %
Flow Type	N/A

Pipe run 10B

Project Description

Worksheet	PR-10B
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	30 in
Discharge	25.00 cfs

Results

Slope	0.37 %
Depth	2.50 ft
Flow Area	4.9 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	1.70 ft
Percent Full	100.0 %
Critical Slope	0.57 %
Velocity	5.09 ft/s
Velocity Head	0.40 ft
Specific Energy	2.90 ft
Froude Number	0.00
Maximum Discharge	26.89 cfs
Discharge Full	25.00 cfs
Slope Full	0.37 %
Flow Type	N/A

Pipe run 11A

Project Description

Worksheet	PR-11A
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	18 in
Discharge	5.00 cfs

Results

Slope	0.23 %
Depth	1.50 ft
Flow Area	1.8 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	0.86 ft
Percent Full	100.0 %
Critical Slope	0.58 %
Velocity	2.83 ft/s
Velocity Head	0.12 ft
Specific Energy	1.62 ft
Froude Number	0.00
Maximum Discharge	5.38 cfs
Discharge Full	5.00 cfs
Slope Full	0.23 %
Flow Type	N/A

Pipe run 11B

Project Description

Worksheet	PR-11B
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	18 in
Discharge	6.00 cfs

Results

Slope	0.33 %
Depth	1.50 ft
Flow Area	1.8 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	0.95 ft
Percent Full	100.0 %
Critical Slope	0.62 %
Velocity	3.40 ft/s
Velocity Head	0.18 ft
Specific Energy	1.68 ft
Froude Number	0.00
Maximum Discharge	6.45 cfs
Discharge Full	6.00 cfs
Slope Full	0.33 %
Flow Type	N/A

Pipe run 12

Project Description

Worksheet	PR-12
Flow Element	Circular Channel
Method	Manning's Formula
Solve For	Full Flow Slope

Input Data

Mannings Coefficient	0.013
Diameter	30 in
Discharge	40.00 cfs

Results

Slope	0.95 %
Depth	2.50 ft
Flow Area	4.9 ft ²
Wetted Perimeter	0.00 ft
Top Width	0.00 ft
Critical Depth	2.13 ft
Percent Full	100.0 %
Critical Slope	0.89 %
Velocity	8.15 ft/s
Velocity Head	1.03 ft
Specific Energy	3.53 ft
Froude Number	0.00
Maximum Discharge	43.03 cfs
Discharge Full	40.00 cfs
Slope Full	0.95 %
Flow Type	N/A

Swale X-sect A-A

Project Description

Worksheet	SW-1 X-SECT A-A
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.030
Slope	2.80 %
Left Side Slope	4.00 H : V
Right Side Slope	4.00 H : V
Bottom Width	2.00 ft
Discharge	30.00 cfs

Results

Depth	0.94 ft
Flow Area	5.4 ft ²
Wetted Perimeter	9.72 ft
Top Width	9.48 ft
Critical Depth	1.06 ft
Critical Slope	1.58 %
Velocity	5.58 ft/s
Velocity Head	0.48 ft
Specific Energy	1.42 ft
Froude Number	1.31
Flow Type	Supercritical

Swale X-sect B-B

Project Description

Worksheet	SW-1 X-SECT B-B
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.030
Slope	1.04 %
Left Side Slope	4.00 H : V
Right Side Slope	4.00 H : V
Bottom Width	2.00 ft
Discharge	32.00 cfs

Results

Depth	1.20 ft
Flow Area	8.1 ft ²
Wetted Perimeter	11.89 ft
Top Width	11.59 ft
Critical Depth	1.10 ft
Critical Slope	1.56 %
Velocity	3.93 ft/s
Velocity Head	0.24 ft
Specific Energy	1.44 ft
Froude Number	0.83
Flow Type	Subcritical

Main Channel

Project Description

Worksheet	Main Channel
Flow Element	Trapezoidal Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.045
Slope	0.61 %
Left Side Slope	4.00 H : V
Right Side Slope	4.00 H : V
Bottom Width	10.00 ft
Discharge	644.00 cfs

Results

Depth	4.56 ft
Flow Area	128.7 ft ²
Wetted Perimeter	47.58 ft
Top Width	46.46 ft
Critical Depth	3.34 ft
Critical Slope	2.36 %
Velocity	5.01 ft/s
Velocity Head	0.39 ft
Specific Energy	4.95 ft
Froude Number	0.53
Flow Type	Subcritical

Berm at East boundary

Project Description

Worksheet	Triangular Channel - 1
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.035
Slope	2.00 %
Left Side Slope	4.00 H : V
Right Side Slope	50.00 H : V
Discharge	20.00 cfs

Results

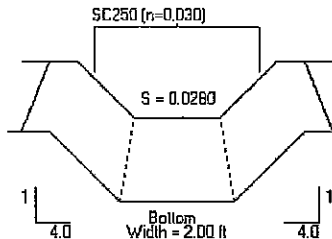
Depth	0.54 ft
Flow Area	8.0 ft ²
Wetted Perimeter	29.39 ft
Top Width	29.32 ft
Critical Depth	0.51 ft
Critical Slope	2.83 %
Velocity	2.51 ft/s
Velocity Head	0.10 ft
Specific Energy	0.64 ft
Froude Number	0.85
Flow Type	Subcriti cal

Swale A-A
SC250 Fabric Lined

North American Green - ECMD5 Version 4.3 8/2/2006 10:39 AM COMPUTED BY: Quentin Amijo
 PROJECT NAME: Quail Brush Creek PROJECT NO.: 0537.00
 FROM STATION/REACH: Swale B-B TO STATION/REACH: Swale B-B DRAINAGE AREA: DESIGN FREQUENCY: 100-year

HYDRAULIC RESULTS

Discharge (cfs)	Peak Flow Period (hrs)	Velocity (fps)	Area (sq.ft)	Hydraulic Radius(ft)	Normal Depth (ft)
80.0	2.0	7.17	11.16	0.80	1.44



LINER RESULTS

Not to Scale

Reach	Matting Type	Stability Analysis	Vegetation Characteristics				Permissible Shear Stress (psf)	Calculated Shear Stress (psf)	Safety Factor	Remarks
	Staple Pattern		Phase	Class	Type	Density				
Straight	SC250	Vegetation	2	C	Mix	50-75%	8.00	2.51	3.10	STABLE
	Staple E	Soil	Sandy Loam				2.500	0.533	4.69	STABLE

///

Back to Input Screen

KIOWA ENGINEERING HEC-1 ANALYSIS

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 11NOV05 TIME 08:16:07
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
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X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Sand Creek Detention Basin No. 2 Final design									
2	ID	FUTURE DEVELOPMENT CONDITION WITH SAND CREEK PER FINAL DESIGN									
3	ID	10-YEAR AND 100-YEAR 24-HOUR STORM FN:SC2-FD.DAT									
	*DIAGRAM										
4	IT	5	0	0	300						
5	IO	5	0								
6	JR	PREC	.68	1.0							
7	KK	SB82									
8	KM	RUNOFF FROM SUB-BASIN 82									
9	BA	.190									
10	IN	30									
11	PB	4.4									
12	PC	0.0000	0.0025	0.0050	0.0075	0.0100	0.0150	0.0200	0.0250	0.0300	0.0500

REVISED CHS
 STATION 23, 25+26


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90      KM  RUNOFF FROM SUB-BASIN 77
91      BA   .25
92      LS   0      67
93      UD   .336

94      KK  SB88
95      KM  RUNOFF FROM SUB-BASIN 88
96      BA   .1000
97      LS   0      67
98      UD   .204

99      KK  DP77
100     KM  DESIGN POINT 77 COMBINE RUNOFF FROM SUB-BASINS 77 AND 88 AND RT-6, AND RT-5
101     HC   4

102     KK  RT-8
103     KM  ROUTE FLOW FROM DESIGN POINT 77 TO DP 71
104     RK  3850   .025   .035           TRAP      20      10

105     KK  SB71
106     KM  RUNOFF FROM SUB-BASIN 71
107     BA   .36
108     LS   0      70
109     UD   .300

110     KK  DP71
111     KM  DESIGN POINT 71 COMBINE RUNOFF FROM SUB-BASIN 71 AND RT-8
112     HC   2

113     KK  RT-9
114     KM  ROUTE FLOW FROM DESIGN POINT 71 TO DP 70
115     RK  2500   .025   .035           TRAP      20      4

116     KK  SB70
117     KM  RUNOFF FROM SUB-BASIN 70
118     BA   .3100
119     LS   0      70
120     UD   .350

121     KK  DP70
122     KM  DESIGN POINT 70 COMBINE RUNOFF FROM SUB-BASIN 70 AND RT-9
123     HC   2

124     KK  RT-11
125     KM  ROUTE FLOW FROM DESIGN POINT 70 TO DP 87
126     RK  1200   .025   .035           TRAP      20      4
                                     HEC-1 INPUT

127     KK  SB72

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

128	KM	RUNOFF FROM SUB-BASIN 72				
129	BA	.2500				
130	LS	0	70			
131	UD	.138				
132	KK	RT-10				
133	KM	ROUTE FLOW FROM SUB-BASIN 72 TO DESIGN POINT 70				
134	RK	3000	.035	.035	TRAP 10	4
135	KK	SB69				
136	KM	RUNOFF FROM SUB-BASIN 69				
137	BA	.2500				
138	LS	0	75			
139	UD	.483				
140	KK	DP69				
141	KM	DESIGN POINT 69 COMBINE RUNOFF FROM SUB-BASIN 69 AND RT-10				
142	HC	2				
143	KK	RT-12				
144	KM	ROUTE FLOW FROM DESIGN POINT 69 TO DP 87				
145	RK	1400	.025	.035	TRAP 20	4
146	KK	SC3-15				
147	KM	RUNOFF FROM SUB-BASIN SC3-15				
148	BA	.0730				
149	LS	0	72.0			
150	UD	.230				
151	KK	DP87				
152	KM	DESIGN POINT 87 COMBINE RUNOFF FROM SUB-BASIN SC3-15, RT-12 AND RT-11				
153	HC	3				
154	KK	RT-13				
155	KM	ROUTE FLOW FROM DESIGN POINT 87 TO DP 63				
156	RK	4400	.025	.035	TRAP 40	4
157	KK	SC3-12				
158	KM	RUNOFF FROM SUB-BASIN SC3-12				
159	BA	.2380				
160	LS	0	70.6			
161	UD	.410				
162	KK	DP63				
163	KM	DESIGN POINT 63 COMBINE RUNOFF FROM SUB-BASIN SC3-12 AND RT-13				
164	HC	2				
165	KK	RT-17				
166	KM	ROUTE FLOW FROM DESIGN POINT 63 TO DP 60				
167	RK	4390	.015	.035	TRAP 10	4

207 KM ROUTE FLOW FROM DP 60 TO DESIGN POINT 53
 208 RK 2690 .015 .035 TRAP 10 4
 HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

209 KK SC3-2
 210 KM RUNOFF FROM SUB-BASIN SC3-2
 211 BA .1920
 212 LS 0 76.1
 213 UD .310

214 KK SC3-11
 215 KM RUNOFF FROM SUB-BASIN SC3-11
 216 BA .2750
 217 LS 0 80.8
 218 UD .430

219 KK RT-19
 220 KM ROUTE FLOW FROM SUB-BASIN SC3-11 TO DESIGN POINT 55
 221 RK 2600 .022 .035 TRAP 10 6

222 KK SC3-10
 223 KM RUNOFF FROM SUB-BASIN SC3-10
 224 BA .1130
 225 LS 0 75.0
 226 UD .290

227 KK DP55
 228 KM DESIGN POINT 55 COMBINE RUNOFF FROM SUB-BASIN SC3-10 AND RT-19
 229 HC 2

230 KK RT-20
 231 KM ROUTE FLOW FROM DESIGN POINT 55 TO DESIGN POINT 56
 232 RK 3670 .012 .035 TRAP 10 10

233 KK SC3-1
 234 KM RUNOFF FROM SUB-BASIN SC3-1
 235 BA .1290
 236 LS 0 80.0
 237 UD .320

238 KK DP56
 239 KM DESIGN POINT 56 COMBINE RUNOFF FROM SUB-BASIN SC3-1 AND RT-20
 240 HC 2

241 KK DB95
 242 KM ROUTE FLOW FROM DP 56 THROUGH DBPS STRUCTURE 95
 243 RS 1 ELEV 100
 244 SV 0 20 34 39 42 50
 245 SQ 0 10 20 30 40 43

246 SE 100 104 107 108 109 110
 247 KK RT-21
 248 KM ROUTE FLOW FROM DETENTION BASIN DB95 TO DESIGN POINT 53
 249 RK 870 .023 .035 TRAP 10 10
 HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

250 KK DP53
 251 KM DESIGN POINT 53 COMBINE RUNOFF FROM SUB-BASIN SC3-2, RT-21 AND RT-18
 252 HC 3

253 KK DB94
 254 KM ROUTE FLOW FROM DP 53 THROUGH DBFS STRUCTURE 94
 255 RS 1 ELEV 100
 256 SV 0 30 40 60 80 120 130
 257 SQ 0 80 90 300 800 1750 2000
 258 SE 100 104 105 107 108 109 110

259 KK RT-27
 260 KM ROUTE FLOW FROM DETETNION BASIN DB94 TO DESIGN POINT 50
 261 RK 1380 .022 .035 TRAP 10 10

262 KK SC2-24
 263 KM RUNOFF FROM SUB-BASIN SC2-24
 264 BA .0660
 265 LS 0 90.7
 266 UD .190

267 KK RT-29
 268 KM ROUTE FLOW FROM SUB-BASIN SC2-24 TO DESIGN POINT 50
 269 RK 330 .020 .013 CIRC 4

270 KK SC2-27
 271 KM RUNOFF FROM SUB-BASIN SC2-27
 272 BA .0780
 273 LS 0 92.0
 274 UD .210

275 KK DP50
 276 KM DESIGN POINT 50 COMBINE RUNOFF FROM SC2-27, RT-27 AND RT-29
 277 HC 3

278 KK RT-31
 279 KM ROUTE FLOW FROM DESIGN POINT 50 TO DESIGN POINT 49
 280 RK 940 .020 .035 TRAP 40 6

281 KK SC3-8
 282 KM RUNOFF FROM SUB-BASIN SC3-8
 283 BA .2420

284 LS 0 67.0
 285 UD .290

 286 KK RT-25
 287 KM ROUTE FLOW FROM SUB-BASIN SC3-8 TO DESIGN POINT 51
 288 RK 2690 .015 .035 TRAP 20 6
 HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

289 KK SC3-3
 290 KM RUNOFF FROM SUB-BASIN SC3-3
 291 BA .1270
 292 LS 0 88.5
 293 UD .280

 294 KK DP51
 295 KM DESIGN POINT 51 COMBINE RUNOFF FROM SC3-3 AND RT-25
 296 HC 2

 297 KK DB93
 298 KM ROUTE FLOW FROM DP 51 THROUGH DBPS STRUCTURE 93
 299 RS 1 ELEV 100
 300 SV 0 10 20 30
 301 SQ 0 20 100 290
 302 SE 100 104 106 108

 303 KK RT-30
 304 KM ROUTE FLOW FROM DETENTION BASIN DB93 TO DESIGN POINT 49
 305 RK 1590 .019 .035 TRAP 20 10

 306 KK DP49
 307 KM DESIGN POINT 49 COMBINE RUNOFF FROM RT-31 AND RT-30
 308 HC 2

 309 KK RT-32
 310 KM ROUTE FLOW FROM DESIGN POINT 49 TO DESIGN POINT 48
 311 RK 4930 .017 .035 TRAP 40 6

 312 KK SC3-6
 313 KM RUNOFF FROM SUB-BASIN SC3-6
 314 BA .2420
 315 LS 0 70.0
 316 UD .310

 317 KK RT-23
 318 KM ROUTE FLOW FROM SUB-BASIN SC3-6 TO DESIGN POINT 54
 319 RK 1830 .023 .035 TRAP 10 10

 320 KK SC3-5
 321 KM RUNOFF FROM SUB-BASIN SC3-5

322 BA .1150
 323 LS 0 88.7
 324 UD .280

 325 KK RT-24
 326 KM ROUTE FLOW FROM SUB-BASIN SC3-5 TO DESIGN POINT 54
 327 RK 1760 .041 .035 TRAP 10 4

1

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

328 KK SC3-7
 329 KM RUNOFF FROM SUB-BASIN SC3-7
 330 BA .5380
 331 LS 0 81.5
 332 UD .530

 333 KK RT-22
 334 KM ROUTE FLOW FROM SUB-BASIN SC3-7 TO DESIGN POINT 54
 335 RK 2860 .021 .035 TRAP 30 6

 336 KK SC3-4
 337 KM RUNOFF FROM SUB-BASIN SC3-4
 338 BA .3210
 339 LS 0 81.9
 340 UD .330

 341 KK DP54
 342 KM DESIGN POINT 54 COMBINE RUNOFF FROM SUB-BASIN SC3-4, RT-22, RT-23 AND RT-24
 343 HC 4

 344 KK DB92
 345 KM ROUTE FLOW FROM DP 54 THROUGH DBPS STRUCTURE 92
 346 RS 1 ELEV 100
 347 SV 0 40 60 90 110 120 130
 348 SQ 0 40 60 82 90 91 95
 349 SE 100 104 105 106 108 109 110

 350 KK RT-26
 351 KM ROUTE FLOW FROM DETENTION BASIN 92 TO DESIGN POINT 47
 352 RK 4680 .028 .035 TRAP 40 10

 353 KK SC2-26
 354 KM RUNOFF FROM SUB-BASIN SC2-26
 355 BA .1860
 356 LS 0 79
 357 UD .320

 358 KK RT-34
 359 KM ROUTE FLOW FROM SUB-BASIN SC2-26 TO DESIGN POINT 47
 360 RK 1620 .014 .035 TRAP 4 4

399 LS 0 79.0
 400 UD .220

 401 KK DP46
 402 KM DESIGN POINT 46 COMBINE RUNOFF FROM SUB-BASIN SC2-22 AND RT-35
 403 HC 2

 404 KK RT-36
 405 KM ROUTE FLOW FROM DESIGN POINT 46 TO DESIGN POINT 42
 406 RK 1790 .017 .013 TRAP 30 4

 407 KK DP42
 408 KM DESIGN POINT 42 COMBINE RUNOFF FROM RT- 37 AND RT-36
 409 HC 2

1

HEC-1 INPUT

PAGE 11

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10
410	KK	RT-38									
411	KM	ROUTE FLOW FROM DESIGN POINT 42 TO DESIGN POINT 85									
412	RK	1790	.013	.035	TRAP	60	6				
413	KK	SC2-20									
414	KM	RUNOFF FROM SUB-BASIN SC2-20									
415	BA	.3240									
416	LS	0	79.0								
417	UD	.380									
418	KK	RT-41									
419	KM	ROUTE FLOW FROM SUB-BASIN SC2-20 TO DESIGN POINT 85									
420	RK	1560	.020	.013	CIRC	6.5					
421	KK	SC2-15									
422	KM	RUNOFF FROM SUB-BASIN SC2-15									
423	BA	.1810									
424	LS	0	79.0								
425	UD	.240									
426	KK	RT-39									
427	KM	ROUTE FLOW FROM SUB-BASIN SC2-15 TO DESIGN POINT 85									
428	RK	990	.022	.013	CIRC	6.5					
429	KK	SC2-13									
430	KM	RUNOFF FROM SUB-BASIN SC2-13									
431	BA	.1630									
432	LS	0	79.0								
433	UD	.280									
434	KK	RT-40									
435	KM	ROUTE FLOW FROM SUB-BASIN SC2-13 TO DESIGN POINT 85									
436	RK	300	.033	.013	CIRC	5.5					

437 KK DP85
 438 KM DESIGN POINT 85 COMBINE RUNOFF FROM RT-41, RT-38, RT-39 AND RT-40
 439 HC 4

 440 KK RT-42
 441 KM ROUTE FLOW FROM DESIGN POINT 85 TO DESIGN POINT 37
 442 RK 2900 .016 .035 TRAP 60 6

 443 KK SC2-14
 444 KM RUNOFF FROM SUB-BASIN SC2-14
 445 BA .2650
 446 LS 0 76.2
 447 UD .450

 448 KK SC2-16
 449 KM RUNOFF FROM SUB-BASIN SC2-16
 450 BA .2120
 451 LS 0 76.2
 452 UD .310

HEC-1 INPUT

1

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
453	KK DP37
454	KM DESIGN POINT 37 COMBINE RUNOFF FROM RT-42, SUB-BASINS SC2-14 AND SC2-16
455	HC 3
456	KK RT-43
457	KM ROUTE FLOW FROM DESIGN POINT 37 TO DESIGN POINT 36
458	RK 3380 .016 .035 TRAP 60 4
459	KK SC2-12
460	KM RUNOFF FROM SUB-BASIN SC2-12
461	BA .1790
462	LS 0 70.6
463	UD .280
464	KK RT-46
465	KM ROUTE FLOW FROM SUB-BASIN SC2-12 TO DESIGN POINT 33
466	RK 2280 .024 .035 TRAP 20 4
467	KK SC2-8
468	KM RUNOFF FROM SUB-BASIN SC2-8
469	BA .3480
470	LS 0 79.0
471	UD .440
472	KK DP33
473	KM DESIGN POINT 33 COMBINE RUNOFF FROM RT-46 AND SUB-BASIN SC2-8
474	HC 2

475 KK RT-45
 476 KM ROUTE FLOW FROM DESIGN POINT 33 TO DESIGN POINT 36
 477 RK 850 .021 .035 TRAP 20 4

478 KK DP36
 479 KM DESIGN POINT 36 COMBINE RUNOFF FROM RT-45 AND RT-43
 480 HC 2

481 KK RT-44
 482 KM ROUTE FLOW FROM DESIGN POINT 36 TO DESIGN POINT 35
 483 RK 1740 .016 .035 TRAP 60 4

484 KK SC2-11
 485 KM RUNOFF FROM SUB-BASIN SC2-11
 486 BA .2100
 487 LS 0 79.0
 488 UD .240

489 KK RT-52
 490 KM ROUTE FLOW FROM SUB-BASIN SC2-11 TO DESIGN POINT 39
 491 RK 2680 .008 .035 TRAP 30 4

1

HEC-1 INPUT

PAGE 13

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

492 KK SC2-19
 493 KM RUNOFF FROM SUB-BASIN SC2-19
 494 BA .1820
 495 LS 0 86.0
 496 UD .300

497 KK RT-51
 498 KM ROUTE FLOW FROM SUB-BASIN SC2-19 TO DESIGN POINT 39
 499 RK 3100 .022 .013 CIRC 6

500 KK SC2-18
 501 KM RUNOFF FROM SUB-BASIN SC2-18
 502 BA .1980
 503 LS 0 88.0
 504 UD .290

505 KK DP39
 506 KM DESIGN POINT 39 COMBINE RUNOFF FROM RT-52, RT-51 AND SUB-BASIN SC2-18
 507 HC 3

508 KK RT-49
 509 KM ROUTE FLOW FROM DESIGN POINT 39 TO DESIGN POINT 34
 510 RK 1160 .024 .035 TRAP 20 4

511 KK SC2-17

512 KM RUNOFF FROM SUB-BASIN SC2-17
 513 BA .1600
 514 LS 0 79.0
 515 UD .320

 516 KK RT-50
 517 KM ROUTE FLOW FROM SUB-BASIN SC2-17 TO DESIGN POINT 34
 518 RK 1840 .025 .035 TRAP 20 4

 519 KK SC2-29
 520 KM RUNOFF FROM SUB-BASIN SC2-29
 521 BA .1510
 522 LS 0 88.0
 523 UD .210

 524 KK DP34
 525 KM DESIGN POINT 34 COMBINE RUNOFF FROM RT-49, RT-50 AND SUB-BASIN SC2-29
 526 HC 3

 527 KK RT-48
 528 KM ROUTE FLOW FROM DESIGN POINT 34 TO DESIGN POINT 32
 529 RK 1470 .020 .04 TRAP 20 4

 530 KK SC2-10
 531 KM RUNOFF FROM SUB-BASIN SC2-10
 532 BA .3290
 533 LS 0 78.4
 534 UD .390

1

HEC-1 INPUT

PAGE 14

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

 535 KK DP32
 536 KM DESIGN POINT 32 COMBINE RUNOFF FROM RT-48 AND SUB-BASIN SC2-10
 537 HC 2

 538 KK RT-47
 539 KM ROUTE FLOW FROM SUB-BASIN DESIGN POINT 32 TO DESIGN POINT 35
 540 RK 991 .028 .031 TRAP 20 4

 541 KK SC2-9
 542 KM RUNOFF FROM SUB-BASIN SC2-9
 543 BA .2060
 544 LS 0 73.0
 545 UD .520

 546 KK DP35
 547 KM DESIGN POINT 35 COMBINE RUNOFF FROM RT-47, RT-44 AND SUB-BASIN SC2-9
 548 HC 3

 549 KK RT-53

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550      KM  ROUTE FLOW FROM DESIGN POINT 35 TO DESIGN POINT 29
551      RK  1120   .020   .035           TRAP    60    6

552      KK  SC2-7
553      KM  RUNOFF FROM SUB-BASIN SC2-7
554      BA  .1440
555      LS  0     79.0
556      UD  .320

557      KK  DP29
558      KM  DESIGN POINT 29 COMBINE RUNOFF FROM RT-53 AND SUB-BASIN SC2-7
559      HC  2

560      KK  RT-54
561      KM  ROUTE FLOW FROM DESIGN POINT 29 TO DESIGN POINT 28
562      RK  1560   .011   .035           TRAP    60    4

563      KK  SC2-6
564      KM  RUNOFF FROM SUB-BASIN SC2-6
565      BA  .1500
566      LS  0     81.6
567      UD  .240

568      KK  RT-55
569      KM  ROUTE FLOW FROM SUB-BASIN SC2-6 TO DESIGN POINT 28
570      RD  950   .020   .035           TRAP    10    4

571      KK  SC2-2
572      KM  RUNOFF FROM SUB-BASIN SC2-2
573      BA  .1670
574      LS  0     80.8
575      UD  .280

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HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

576      KK  DP28
577      KM  DESIGN POINT 28 COMBINE RUNOFF FROM RT-54, RT-55 AND SUB-BASIN SC2-2
578      HC  3

579      KK  RT-56
580      KM  ROUTE FLOW FROM DESIGN POINT 28 TO DESIGN POINT 21
581      RK  1280   .017   .035           TRAP    60    4

582      KK  SC2-5
583      KM  RUNOFF FROM SUB-BASIN SC2-5
584      BA  .1610
585      LS  0     92.0
586      UD  .280

587      KK  RT-58

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588 KM ROUTE FLOW FROM SUB-BASIN SC2-5 TO DESIGN POINT 21
589 RK 1670 .027 .024 CIRC 5

590 KK SC2-0
591 KM RUNOFF FROM SUB-BASIN SC2-0
592 BA .1500
593 LS 0 73.4
594 UD .320

595 KK RT-57
596 KM ROUTE FLOW FROM SUB-BASIN SC2-0 TO DESIGN POINT 21
597 RK 800 .032 .035 TRAP 10 4

598 KK DP21
599 KM DESIGN POINT 21 COMBINE RUNOFF FROM RT-58, RT-56 AND RT- 57
600 HC 3

601 KK RT-59
602 KM ROUTE FLOW FROM DESIGN POINT 21 TO DESIGN POINT 20
603 RK 830 .01 .035 TRAP 80 4

604 KK SC2-4
605 KM RUNOFF FROM SUB-BASIN SC2-4
606 BA .2850
607 LS 0 92.0
608 UD .320

609 KK RT-61
610 KM ROUTE FLOW FROM SUB-BASIN SC2-4 TO DESIGN POINT 22
611 RK 1670 .019 .035 TRAP 20 3

612 KK SC2-3
613 KM RUNOFF FROM SUB-BASIN SC2-3
614 BA .0710
615 LS 0 92.0
616 UD .210

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

617 KK DP22
618 KM DESIGN POINT 22 COMBINE RUNOFF FROM RT-61 AND SC2-3
619 HC 2

620 KK RT-60
621 KM ROUTE FLOW FROM DESIGN POINT 22 TO DESIGN POINT 20
622 RK 1010 .018 .035 TRAP 20 3

623 KK SC2-1
624 KM RUNOFF FROM SUB-BASIN SC2-1
625 BA .1850

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626      LS      0      80.0
627      UD      .290

628      KK      DP20
629      KM      DESIGN POINT 20 COMBINE RUNOFF FROM RT-60, RT-59 AND SB SC2-1
630      HC      3

631      KK      DBSC2
632      KM      ROUTE FLOW FROM DP 20 THROUGH SAND CREEK DETENTION BASIN DBSC2
633      RS      1      ELEV 6496.5
634      SV      0      6.75      29.8      58.8      92.2      129.4      168.6      209.8      255.0      302.2
635      SQ      0      10      30      100      400      800      2000      4300      5000      7000
636      SE      6496.5      6498      6500      6502      6504      6506      6508      6510      6512      6514

637      KK      RT-63
638      KM      ROUTE FLOW FROM DETENTION BASIN SC2 TO DESIGN POINT 18
639      RK      1290      .007      .035      TRAP      100      3

640      KK      SC1-10
641      KM      RUNOFF FROM SUB-BASIN SC1-10
642      BA      .2880
643      LS      0      78.2
644      UD      .390

645      KK      DP18
646      KM      DESIGN POINT 18 COMBINE RUNOFF FROM RT-63 AND SUB-BASIN SC1-10
647      HC      2

648      KK      RT-66
649      KM      ROUTE FLOW FROM DESIGN POINT 18 TO DESIGN POINT 17
650      RK      320      .007      .035      TRAP      100      3

651      KK      SC1-13
652      KM      RUNOFF FROM SUB-BASIN SC1-13
653      BA      .1300
654      LS      0      87.3
655      UD      .270

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HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

656      KK      RT-64
657      KM      ROUTE FLOW FROM SUB-BASIN SC1-13 TO DESIGN POINT 19
658      RK      1970      .026      .013      CIRC      4.5

659      KK      SC1-14
660      KM      RUNOFF FROM SUB-BASIN SC1-14
661      BA      .1100
662      LS      0      92.0
663      UD      .280

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664      KK      DP19
665      KM      DESIGN POINT 19 COMBINE RUNOFF FROM RT-64 AND SUB-BASIN SC1-14
666      HC          2

667      KK      RT-65
668      KM      ROUTE FLOW FROM DESIGN POINT 19 TO DESIGN POINT 17
669      RK      700      .022      .013      TRAP      10      .1

670      KK      DP17
671      KM      DESIGN POINT 17 COMBINE RUNOFF FROM RT-65 AND RT-66
672      HC          2

673      KK      RT-67
674      KM      ROUTE FLOW FROM DESIGN POINT 17 TO DESIGN POINT 13
675      RK      910      .007      .035      TRAP      100      3

676      KK      SC1-9
677      KM      RUNOFF FROM SUB-BASIN SC1-9
678      BA      .2400
679      LS      0      76.9
680      UD      .240

681      KK      DBSR29
682      KM      ROUTE FLOW FROM SB SC1-9 THROUGH SPRINGS RANCH DETENTION BASIN DBSR29
683      RS      1      ELEV      100
684      SV      0      .03      0.47      1.20      2.60      6.70      8.8      10.7
685      SQ      0      .2      7.8      22      40      73      97      113
686      SE      100      100.5      101.5      102.5      103.5      104.5      105.5      106.5

687      KK      RT-70
688      KM      ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR29 TO DESIGN POINT 14
689      RD      2750      .013      .013      CIRC      5.5

690      KK      SC1-8
691      KM      RUNOFF FROM SUB-BASIN SC1-8
692      BA      .2100
693      LS      0      79.5
694      UD      .320

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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695      KK      RT-71
696      KM      ROUTE FLOW FROM SUB-BASIN SC1-8 TO DESIGN POINT 14
697      RK      600      .010      .013      CIRC      5

698      KK      DP14
699      KM      DESIGN POINT 14 COMBINE RUNOFF FROM RT-71 AND RT-70
700      HC          2

701      KK      RT-69

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702      KM  ROUTE FLOW FROM DESIGN POINT 14 TO DESIGN POINT 12
703      RK  1550  .015  .013          TRAP      7      .1

704      KK  SC1-7
705      KM  RUNOFF FROM SUB-BASIN SC1-7
706      BA  .0960
707      LS  0      80.1
708      UD  .230

709      KK  DP12
710      KM  DESIGN POINT 12 COMBINE RUNOFF FROM SUB-BASIN SC1-7 AND RT-69
711      HC  2

712      KK  RT-68
713      KM  ROUTE FLOW FROM DESIGN POINT 12 TO DESIGN POINT 13
714      RK  390   .025  .013          TRAP      7      .1

715      KK  DP13
716      KM  DESIGN POINT 13 COMBINE RUNOFF FROM RT-67 AND RT-68
717      HC  2

718      KK  RT-72
719      KM  ROUTE FLOW FROM DESIGN POINT 13 TO DESIGN POINT 10
720      RK  1150  .007  .04          TRAP     100     3

721      KK  SC1-6
722      KM  RUNOFF FROM SUB-BASIN SC1-6
723      BA  .1590
724      LS  0      80.6
725      UD  .220

726      KK  DBSR15
727      KM  ROUTE FLOW FROM SB SC1-6 THROUGH SPRINGS RANCH DETENTION BASIN DBSR15
728      RS  1      ELEV    100
729      SV  0      1.50    3.80    6.50    9.70    13.80
730      SQ  0      80     125    150     180     210
731      SE  100    102    104    106    108     110

732      KK  RT-74
733      KM  ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR15 TO DESIGN POINT 11
734      RK  2080  .021  .013          CIRC     3.5

HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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735      KK  SC1-5
736      KM  RUNOFF FROM SUB-BASIN SC1-5
737      BA  .1730
738      LS  0      79.0
739      UD  .310

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740	KK	DP11							
741	KM	DESIGN POINT 11 COMBINE RUNOFF FROM SUB-BASIN SC1-5 AND RT-74							
742	HC	2							
743	KK	RT-73							
744	KM	ROUTE FLOW FROM DESIGN POINT 11 TO DESIGN POINT 10							
745	RK	580 .024 .013 CIRC 3.5							
746	KK	DP10							
747	KM	DESIGN POINT 10 COMBINE RUNOFF FROM RT-72 AND RT-73							
748	HC	2							
749	KK	RT-75							
750	KM	ROUTE FLOW FROM DESIGN POINT 10 TO DESIGN POINT 9							
751	RK	2270 .007 .035 TRAP 100 3							
752	KK	SC1-2							
753	KM	RUNOFF FROM SUB-BASIN SC1-2							
754	BA	.1360							
755	LS	0 79.0							
756	UD	.320							
757	KK	RT-76							
758	KM	ROUTE FLOW FROM SUB-BASIN SC1-2 TO DESIGN POINT 9							
759	RD	500 .039 .013 CIRC 3							
760	KK	DP9							
761	KM	DESIGN POINT 9 COMBINE RUNOFF FROM RT-75 AND RT-76							
762	HC	2							
763	KK	RT-78							
764	KM	ROUTE FLOW FROM DESIGN POINT 9 TO DESIGN POINT 4							
765	RK	1530 .007 .035 TRAP 100 3							
766	KK	SC1-12							
767	KM	RUNOFF FROM SUB-BASIN SC1-12							
768	BA	.2480							
769	LS	0 84.0							
770	UD	.370							
771	KK	RT-81							
772	KM	ROUTE FLOW FROM SUB-BASIN SC1-12 TO DESIGN POINT 8							
773	RK	1400 .008 .035 TRAP 30 4							

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

774	KK	SC1-11							
775	KM	RUNOFF FROM SUB-BASIN SC1-11							
776	BA	.1600							
777	LS	0 92.0							

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 819 ZZ

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 * *
 809 KK * DBSC1 *
 * *

811 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	RATIO 2
				.68	1.00
HYDROGRAPH AT					
+ SB82	.19	1	FLOW	84.	232.
			TIME	6.50	6.50
ROUTED TO					
+ RT-4	.19	1	FLOW	83.	220.
			TIME	6.58	6.58
HYDROGRAPH AT					
+ SB74	.18	1	FLOW	49.	143.
			TIME	6.67	6.67
2 COMBINED AT					
+ DP74	.37	1	FLOW	122.	345.
			TIME	6.58	6.58
ROUTED TO					
+ RT-3	.37	1	FLOW	119.	331.
			TIME	6.67	6.67
HYDROGRAPH AT					

+	SB80	.22	1	FLOW TIME	68. 6.67	190. 6.67
ROUTED TO						
+	RT-2	.22	1	FLOW TIME	67. 6.75	185. 6.75
HYDROGRAPH AT						
+	SB81	.39	1	FLOW TIME	100. 6.67	298. 6.67
ROUTED TO						
+	RT-1	.39	1	FLOW TIME	99. 6.83	296. 6.75
HYDROGRAPH AT						
+	SB73	.12	1	FLOW TIME	31. 6.75	90. 6.67
HYDROGRAPH AT						
+	SB75	.12	1	FLOW TIME	25. 6.83	72. 6.75
HYDROGRAPH AT						
+	SB76	.17	1	FLOW TIME	35. 6.83	105. 6.75
6 COMBINED AT						
+	DP75	1.39	1	FLOW TIME	364. 6.75	1022. 6.75
ROUTED TO						
+	RT-5	1.39	1	FLOW TIME	345. 6.75	1021. 6.75
HYDROGRAPH AT						
+	SB79	.29	1	FLOW TIME	111. 6.58	306. 6.58
ROUTED TO						
+	RT-7	.29	1	FLOW TIME	107. 6.67	281. 6.58
HYDROGRAPH AT						
+	SB78	.88	1	FLOW TIME	259. 6.67	738. 6.67
2 COMBINED AT						
+	DP78	1.17	1	FLOW TIME	367. 6.67	1020. 6.67
ROUTED TO						

+	RT-6	1.17	1	FLOW TIME	351. 6.75	993. 6.67
HYDROGRAPH AT						
+	SB77	.25	1	FLOW TIME	61. 6.75	177. 6.75
HYDROGRAPH AT						
+	SB88	.10	1	FLOW TIME	34. 6.58	97. 6.58
4 COMBINED AT						
+	DP77	2.91	1	FLOW TIME	780. 6.75	2207. 6.67
ROUTED TO						
+	RT-8	2.91	1	FLOW TIME	761. 6.83	2201. 6.75
HYDROGRAPH AT						
+	SB71	.36	1	FLOW TIME	127. 6.67	334. 6.67
2 COMBINED AT						
+	DP71	3.27	1	FLOW TIME	861. 6.83	2508. 6.75
ROUTED TO						
+	RT-9	3.27	1	FLOW TIME	842. 6.83	2445. 6.75
HYDROGRAPH AT						
+	SB70	.31	1	FLOW TIME	99. 6.75	256. 6.75
2 COMBINED AT						
+	DP70	3.58	1	FLOW TIME	931. 6.83	2701. 6.75
ROUTED TO						
+	RT-11	3.58	1	FLOW TIME	905. 6.83	2653. 6.75
HYDROGRAPH AT						
+	SB72	.25	1	FLOW TIME	135. 6.58	321. 6.50
ROUTED TO						
+	RT-10	.25	1	FLOW TIME	128. 6.58	321. 6.58
HYDROGRAPH AT						

+	SB69	.25	1	FLOW TIME	93. 6.83	216. 6.83
2 COMBINED AT						
+	DP69	.50	1	FLOW TIME	191. 6.67	453. 6.58
ROUTED TO						
+	RT-12	.50	1	FLOW TIME	187. 6.67	446. 6.67
HYDROGRAPH AT						
+	SC3-15	.07	1	FLOW TIME	35. 6.58	87. 6.58
3 COMBINED AT						
+	DP87	4.15	1	FLOW TIME	1068. 6.83	3104. 6.75
ROUTED TO						
+	RT-13	4.15	1	FLOW TIME	1048. 6.92	3020. 6.83
HYDROGRAPH AT						
+	SC3-12	.24	1	FLOW TIME	70. 6.83	183. 6.75
2 COMBINED AT						
+	DP63	4.39	1	FLOW TIME	1111. 6.92	3198. 6.83
ROUTED TO						
+	RT-17	4.39	1	FLOW TIME	1083. 7.00	3152. 6.83
HYDROGRAPH AT						
+	SB68	.22	1	FLOW TIME	97. 6.75	222. 6.75
ROUTED TO						
+	RT-14	.22	1	FLOW TIME	95. 6.83	220. 6.83
HYDROGRAPH AT						
+	SC3-14	.31	1	FLOW TIME	88. 6.83	234. 6.75
2 COMBINED AT						
+	DP67	.53	1	FLOW TIME	183. 6.83	448. 6.83
ROUTED TO						

+	RT-15	.53	1	FLOW TIME	178. 6.92	446. 6.83
HYDROGRAPH AT						
+	SC3-13	.25	1	FLOW TIME	174. 6.75	346. 6.75
2 COMBINED AT						
+	DP64	.79	1	FLOW TIME	338. 6.83	763. 6.75
ROUTED TO						
+	RT-16	.79	1	FLOW TIME	332. 6.92	758. 6.83
HYDROGRAPH AT						
+	SC3-9	.24	1	FLOW TIME	123. 6.67	279. 6.67
3 COMBINED AT						
+	DP60	5.42	1	FLOW TIME	1462. 6.92	4126. 6.83
ROUTED TO						
+	RT-18	5.42	1	FLOW TIME	1456. 7.00	4013. 6.92
HYDROGRAPH AT						
+	SC3-2	.19	1	FLOW TIME	108. 6.67	239. 6.67
HYDROGRAPH AT						
+	SC3-11	.28	1	FLOW TIME	167. 6.75	341. 6.75
ROUTED TO						
+	RT-19	.28	1	FLOW TIME	166. 6.83	336. 6.83
HYDROGRAPH AT						
+	SC3-10	.11	1	FLOW TIME	61. 6.67	138. 6.67
2 COMBINED AT						
+	DP55	.39	1	FLOW TIME	211. 6.75	448. 6.75
ROUTED TO						
+	RT-20	.39	1	FLOW TIME	209. 6.92	445. 6.83
HYDROGRAPH AT						

+	SC3-1	.13	1	FLOW TIME	92. 6.67	187. 6.67
2 COMBINED AT						
+	DP56	.52	1	FLOW TIME	274. 6.83	586. 6.83
ROUTED TO						
+	DB95	.52	1	FLOW TIME	13. 11.42	41. 11.00
** PEAK STAGES IN FEET **						
			1	STAGE TIME	105.02 11.42	109.39 11.00
ROUTED TO						
+	RT-21	.52	1	FLOW TIME	13. 11.42	41. 11.00
3 COMBINED AT						
+	DP53	6.13	1	FLOW TIME	1500. 7.00	4163. 6.83
ROUTED TO						
+	DB94	6.13	1	FLOW TIME	440. 10.75	1908. 7.25
** PEAK STAGES IN FEET **						
			1	STAGE TIME	107.28 10.75	109.63 7.25
ROUTED TO						
+	RT-27	6.13	1	FLOW TIME	438. 10.75	1895. 7.25
HYDROGRAPH AT						
+	SC2-24	.07	1	FLOW TIME	105. 6.58	172. 6.50
ROUTED TO						
+	RT-29	.07	1	FLOW TIME	105. 6.58	172. 6.58
HYDROGRAPH AT						
+	SC2-27	.08	1	FLOW TIME	128. 6.58	207. 6.58
3 COMBINED AT						
+	DP50	6.27	1	FLOW TIME	442. 10.75	1922. 7.25
ROUTED TO						

+	RT-31	6.27	1	FLOW TIME	440. 10.83	1920. 7.33
HYDROGRAPH AT						
+	SC3-8	.24	1	FLOW TIME	66. 6.67	192. 6.67
ROUTED TO						
+	RT-25	.24	1	FLOW TIME	63. 6.75	190. 6.75
HYDROGRAPH AT						
+	SC3-3	.13	1	FLOW TIME	155. 6.67	270. 6.58
2 COMBINED AT						
+	DP51	.37	1	FLOW TIME	198. 6.67	437. 6.67
ROUTED TO						
+	DB93	.37	1	FLOW TIME	25. 10.50	77. 7.17
** PEAK STAGES IN FEET **						
1	STAGE				104.11	105.42
	TIME				10.50	7.17
ROUTED TO						
+	RT-30	.37	1	FLOW TIME	25. 10.58	77. 7.25
2 COMBINED AT						
+	DP49	6.64	1	FLOW TIME	464. 10.83	1996. 7.33
ROUTED TO						
+	RT-32	6.64	1	FLOW TIME	463. 10.92	1988. 7.33
HYDROGRAPH AT						
+	SC3-6	.24	1	FLOW TIME	83. 6.67	219. 6.67
ROUTED TO						
+	RT-23	.24	1	FLOW TIME	82. 6.75	213. 6.75
HYDROGRAPH AT						
+	SC3-5	.12	1	FLOW TIME	142. 6.67	247. 6.58
ROUTED TO						

+	RT-24	.12	1	FLOW TIME	142. 6.67	243. 6.67
HYDROGRAPH AT						
+	SC3-7	.54	1	FLOW TIME	290. 6.83	589. 6.83
ROUTED TO						
+	RT-22	.54	1	FLOW TIME	290. 6.92	585. 6.92
HYDROGRAPH AT						
+	SC3-4	.32	1	FLOW TIME	252. 6.67	496. 6.67
4 COMBINED AT						
+	DP54	1.22	1	FLOW TIME	681. 6.75	1393. 6.75
ROUTED TO						
+	DB92	1.22	1	FLOW TIME	54. 10.92	89. 10.92
** PEAK STAGES IN FEET **						
	1	STAGE			104.72	107.66
		TIME			10.92	10.92
ROUTED TO						
+	RT-26	1.22	1	FLOW TIME	54. 11.08	89. 11.08
HYDROGRAPH AT						
+	SC2-26	.19	1	FLOW TIME	124. 6.67	259. 6.67
ROUTED TO						
+	RT-34	.19	1	FLOW TIME	121. 6.75	249. 6.67
HYDROGRAPH AT						
+	SC2-25	.44	1	FLOW TIME	235. 6.83	497. 6.75
3 COMBINED AT						
+	DP47	1.85	1	FLOW TIME	355. 6.75	749. 6.75
ROUTED TO						
+	RT-33	1.85	1	FLOW TIME	352. 6.75	745. 6.75
HYDROGRAPH AT						

+	SC2-28	.27	1	FLOW TIME	48. 6.92	142. 6.92
HYDROGRAPH AT						
+	SC2-21	.19	1	FLOW TIME	113. 6.75	236. 6.75
4 COMBINED AT						
+	DP48	8.95	1	FLOW TIME	728. 6.75	2353. 7.33
ROUTED TO						
+	RT-37	8.95	1	FLOW TIME	715. 6.75	2350. 7.33
HYDROGRAPH AT						
+	SC2-23	.20	1	FLOW TIME	128. 6.67	267. 6.67
ROUTED TO						
+	RT-35	.20	1	FLOW TIME	125. 6.67	264. 6.67
HYDROGRAPH AT						
+	SC2-22	.08	1	FLOW TIME	67. 6.58	137. 6.58
2 COMBINED AT						
+	DP46	.28	1	FLOW TIME	187. 6.67	386. 6.67
ROUTED TO						
+	RT-36	.28	1	FLOW TIME	183. 6.67	383. 6.67
2 COMBINED AT						
+	DP42	9.23	1	FLOW TIME	889. 6.75	2407. 7.33
ROUTED TO						
+	RT-38	9.23	1	FLOW TIME	863. 6.83	2399. 7.33
HYDROGRAPH AT						
+	SC2-20	.32	1	FLOW TIME	193. 6.75	401. 6.75
ROUTED TO						
+	RT-41	.32	1	FLOW TIME	192. 6.75	400. 6.75
HYDROGRAPH AT						

+	SC2-15	.18	1	FLOW TIME	141. 6.58	291. 6.58
ROUTED TO						
+	RT-39	.18	1	FLOW TIME	138. 6.58	288. 6.58
HYDROGRAPH AT						
+	SC2-13	.16	1	FLOW TIME	118. 6.67	241. 6.67
ROUTED TO						
+	RT-40	.16	1	FLOW TIME	118. 6.67	241. 6.67
4 COMBINED AT						
+	DP85	9.90	1	FLOW TIME	1265. 6.75	2615. 6.75
ROUTED TO						
+	RT-42	9.90	1	FLOW TIME	1235. 6.83	2586. 6.75
HYDROGRAPH AT						
+	SC2-14	.26	1	FLOW TIME	114. 6.83	254. 6.83
HYDROGRAPH AT						
+	SC2-16	.21	1	FLOW TIME	120. 6.67	265. 6.67
3 COMBINED AT						
+	DP37	10.37	1	FLOW TIME	1442. 6.83	3082. 6.75
ROUTED TO						
+	RT-43	10.37	1	FLOW TIME	1438. 6.83	3007. 6.83
HYDROGRAPH AT						
+	SC2-12	.18	1	FLOW TIME	71. 6.67	179. 6.67
ROUTED TO						
+	RT-46	.18	1	FLOW TIME	69. 6.75	171. 6.67
HYDROGRAPH AT						
+	SC2-8	.35	1	FLOW TIME	184. 6.83	390. 6.75

2 COMBINED AT

+	DP33	.53	1	FLOW TIME	252. 6.75	559. 6.75
ROUTED TO						
+	RT-45	.53	1	FLOW TIME	247. 6.83	553. 6.75
2 COMBINED AT						
+	DP36	10.90	1	FLOW TIME	1685. 6.83	3537. 6.83
ROUTED TO						
+	RT-44	10.90	1	FLOW TIME	1647. 6.83	3536. 6.83
HYDROGRAPH AT						
+	SC2-11	.21	1	FLOW TIME	163. 6.58	338. 6.58
ROUTED TO						
+	RT-52	.21	1	FLOW TIME	160. 6.67	335. 6.67
HYDROGRAPH AT						
+	SC2-19	.18	1	FLOW TIME	191. 6.67	345. 6.67
ROUTED TO						
+	RT-51	.18	1	FLOW TIME	189. 6.67	343. 6.67
HYDROGRAPH AT						
+	SC2-18	.20	1	FLOW TIME	234. 6.67	405. 6.67
3 COMBINED AT						
+	DP39	.59	1	FLOW TIME	582. 6.67	1083. 6.67
ROUTED TO						
+	RT-49	.59	1	FLOW TIME	570. 6.67	1073. 6.67
HYDROGRAPH AT						
+	SC2-17	.16	1	FLOW TIME	107. 6.67	222. 6.67
ROUTED TO						
+	RT-50	.16	1	FLOW TIME	105. 6.75	214. 6.67
HYDROGRAPH AT						

+	SC2-29	.15	1	FLOW TIME	208. 6.58	358. 6.58
3 COMBINED AT						
+	DP34	.90	1	FLOW TIME	846. 6.67	1587. 6.67
ROUTED TO						
+	RT-48	.90	1	FLOW TIME	827. 6.67	1574. 6.67
HYDROGRAPH AT						
+	SC2-10	.33	1	FLOW TIME	185. 6.75	391. 6.75
2 COMBINED AT						
+	DP32	1.23	1	FLOW TIME	999. 6.67	1949. 6.67
ROUTED TO						
+	RT-47	1.23	1	FLOW TIME	980. 6.67	1930. 6.67
HYDROGRAPH AT						
+	SC2-9	.21	1	FLOW TIME	61. 6.92	150. 6.83
3 COMBINED AT						
+	DP35	12.34	1	FLOW TIME	2561. 6.83	5419. 6.75
ROUTED TO						
+	RT-53	12.34	1	FLOW TIME	2545. 6.83	5359. 6.75
HYDROGRAPH AT						
+	SC2-7	.14	1	FLOW TIME	96. 6.67	200. 6.67
2 COMBINED AT						
+	DP29	12.48	1	FLOW TIME	2621. 6.83	5544. 6.75
ROUTED TO						
+	RT-54	12.48	1	FLOW TIME	2582. 6.83	5445. 6.83
HYDROGRAPH AT						
+	SC2-6	.15	1	FLOW TIME	137. 6.58	269. 6.58

ROUTED TO

+	RT-55	.15	1	FLOW TIME	134. 6.67	258. 6.58
HYDROGRAPH AT						
+	SC2-2	.17	1	FLOW TIME	135. 6.67	266. 6.67
3 COMBINED AT						
+	DP28	12.80	1	FLOW TIME	2754. 6.83	5874. 6.75
ROUTED TO						
+	RT-56	12.80	1	FLOW TIME	2717. 6.83	5789. 6.75
HYDROGRAPH AT						
+	SC2-5	.16	1	FLOW TIME	235. 6.58	386. 6.58
ROUTED TO						
+	RT-58	.16	1	FLOW TIME	231. 6.67	379. 6.58
HYDROGRAPH AT						
+	SC2-0	.15	1	FLOW TIME	67. 6.67	160. 6.67
ROUTED TO						
+	RT-57	.15	1	FLOW TIME	66. 6.75	156. 6.67
3 COMBINED AT						
+	DP21	13.11	1	FLOW TIME	2928. 6.83	6263. 6.75
ROUTED TO						
+	RT-59	13.11	1	FLOW TIME	2896. 6.83	6190. 6.75
HYDROGRAPH AT						
+	SC2-4	.28	1	FLOW TIME	393. 6.67	641. 6.67
ROUTED TO						
+	RT-61	.28	1	FLOW TIME	388. 6.67	637. 6.67
HYDROGRAPH AT						
+	SC2-3	.07	1	FLOW TIME	117. 6.58	188. 6.58
2 COMBINED AT						

+ DP22 .36 1 FLOW 485. 793.
TIME 6.67 6.67

ROUTED TO

+ RT-60 .36 1 FLOW 483. 793.
TIME 6.67 6.67

HYDROGRAPH AT

+ SC2-1 .19 1 FLOW 140. 282.
TIME 6.67 6.67

3 COMBINED AT

+ DP20 13.65 1 FLOW 3359. 7151.
TIME 6.83 6.75

IN SC#2

ROUTED TO

+ DBSC2 13.65 1 FLOW 1153. 4357.
TIME 7.42 7.08

OUT SC#2

** PEAK STAGES IN FEET **

1 STAGE 6506.59 6510.16
TIME 7.42 7.08

ROUTED TO

+ RT-63 13.65 1 FLOW 1150. 4348.
TIME 7.42 7.08

HYDROGRAPH AT

+ SC1-10 .29 1 FLOW 160. 339.
TIME 6.75 6.75

2 COMBINED AT

+ DP18 13.94 1 FLOW 1185. 4499.
TIME 7.42 7.08

ROUTED TO

+ RT-66 13.94 1 FLOW 1183. 4493.
TIME 7.42 7.08

HYDROGRAPH AT

+ SC1-13 .13 1 FLOW 152. 271.
TIME 6.58 6.58

ROUTED TO

+ RT-64 .13 1 FLOW 151. 267.
TIME 6.67 6.58

HYDROGRAPH AT

+ SC1-14 .11 1 FLOW 160. 263.
TIME 6.58 6.58

2 COMBINED AT

+	DP19	.24	1	FLOW TIME	308. 6.58	530. 6.58
ROUTED TO						
+	RT-65	.24	1	FLOW TIME	308. 6.67	527. 6.58
2 COMBINED AT						
+	DP17	14.18	1	FLOW TIME	1219. 7.42	4589. 7.08
ROUTED TO						
+	RT-67	14.18	1	FLOW TIME	1213. 7.50	4575. 7.08
HYDROGRAPH AT						
+	SC1-9	.24	1	FLOW TIME	162. 6.58	352. 6.58
ROUTED TO						
+	DBSR29	.24	1	FLOW TIME	51. 6.92	103. 6.92
** PEAK STAGES IN FEET **						
1	STAGE	103.85	105.90			
	TIME	6.92	6.92			
ROUTED TO						
+	RT-70	.24	1	FLOW TIME	51. 7.00	103. 7.00
HYDROGRAPH AT						
+	SC1-8	.21	1	FLOW TIME	145. 6.67	298. 6.67
ROUTED TO						
+	RT-71	.21	1	FLOW TIME	144. 6.67	297. 6.67
2 COMBINED AT						
+	DP14	.45	1	FLOW TIME	183. 6.75	365. 6.67
ROUTED TO						
+	RT-69	.45	1	FLOW TIME	182. 6.75	363. 6.75
HYDROGRAPH AT						
+	SC1-7	.10	1	FLOW TIME	83. 6.58	166. 6.58
2 COMBINED AT						

Preliminary Design
Hydrologic

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1*****
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*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
CORPS OF ENGINEERS *
* JUN 1998 *
ENGINEERING CENTER *
* VERSION 4.1 *
SECOND STREET *
*
CALIFORNIA 95616 *
* RUN DATE 06AUG04 TIME 13:04:30 *
756-1104 *
*
*****
*****

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SC# 2 w/
DBPS + SR# 15
+ SR# 24

U.S. ARMY
HYDROLOGIC
609
DAVIS,
(916)

PER SC# 2
PRELIMINARY DESIGN

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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PAGE 1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Sand Creek Detention Basin No. 2 Preliminary design									
2	ID	Future development condition with Sand Creek No. 1									
3	ID	DETENTION BASIN MODELED PER DBPS WITH SPRINGS RANCH DETENTION									
4	ID	BASINS 15 AND 29									
5	ID	10-year and 100 Year, 24 hr Type IIA Storm fn: SC1-FD.dat									
	*DIAGRAM										
6	IT	5	0	0	300						
7	IO	5	0								
8	JR	PREC	.68	1.0							
9	KK	SB82									
10	KM	RUNOFF FROM SUB-BASIN 82									
11	BA	.190									
12	IN	30									
13	PB	4.4									
14	PC	0.0000	0.0025	0.0050	0.0075	0.0100	0.0150	0.0200	0.0250	0.0300	0.0500
15	PC	0.0600	0.1000	0.1000	0.7000	0.7000	0.7500	0.7800	0.7980	0.8200	0.8300
16	PC	0.9150	0.9210	0.9270	0.9330	0.9400	0.9450	0.9500	0.9525	0.9550	0.9600
17	PC	0.9650	0.9700	0.9750	0.9800	0.9830	0.9850	0.9880	0.9900	0.9930	0.9950

123 KK DP70
 124 KM DESIGN POINT 70 COMBINE RUNOFF FROM SUB-BASIN 70 AND RT-9
 125 HC 2

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

126 KK RT-11
 127 KM ROUTE FLOW FROM DESIGN POINT 70 TO DP 87
 128 RK 1200 .025 .035 TRAP 20 4
 129 KK SB72
 130 KM RUNOFF FROM SUB-BASIN 72
 131 BA .2500
 132 LS 0 70
 133 UD .138
 134 KK RT-10
 135 KM ROUTE FLOW FROM SUB-BASIN 72 TO DESIGN POINT 70
 136 RK 3000 .035 .035 TRAP 10 4
 137 KK SB69
 138 KM RUNOFF FROM SUB-BASIN 69
 139 BA .2500
 140 LS 0 75
 141 UD .483
 142 KK DP69
 143 KM DESIGN POINT 69 COMBINE RUNOFF FROM SUB-BASIN 69 AND RT-10
 144 HC 2
 145 KK RT-12
 146 KM ROUTE FLOW FROM DESIGN POINT 69 TO DP 87
 147 RK 1400 .025 .035 TRAP 20 4
 148 KK SC3-15
 149 KM RUNOFF FROM SUB-BASIN SC3-15
 150 BA .0730
 151 LS 0 72.0
 152 UD .230
 153 KK DP87
 154 KM DESIGN POINT 87 COMBINE RUNOFF FROM SUB-BASIN SC3-15, RT-12 AND RT-11
 155 HC 3
 156 KK RT-13
 157 KM ROUTE FLOW FROM DESIGN POINT 87 TO DP 63
 158 RK 4400 .025 .035 TRAP 40 4
 159 KK SC3-12
 160 KM RUNOFF FROM SUB-BASIN SC3-12
 161 BA .2380
 162 LS 0 70.6
 163 UD .410
 164 KK DP63
 165 KM DESIGN POINT 63 COMBINE RUNOFF FROM SUB-BASIN SC3-12 AND RT-13
 166 HC 2

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

167 KK RT-17
 168 KM ROUTE FLOW FROM DESIGN POINT 63 TO DP 60
 169 RK 4390 .015 .035 TRAP 10 4
 170 KK SB68

171	KM	RUNOFF FROM SUB-BASIN 68							
172	BA	.2200							
173	LS	0	75						
174	UD	.390							
175	KK	RT-14							
176	KM	ROUTE FLOW FROM SUB-BASIN 68 TO DESIGN POINT 67							
177	RK	3300	.020	.035	TRAP	20		4	
178	KK	SC3-14							
179	KM	RUNOFF FROM SUB-BASIN SC3-14							
180	BA	.3150							
181	LS	0	70.0						
182	UD	.410							
183	KK	DP67							
184	KM	DESIGN POINT 67 COMBINE RUNOFF FROM SUB-BASIN SC3-14 AND RT-14							
185	HC	2							
186	KK	RT-15							
187	KM	ROUTE FLOW FROM DP 67 TO DESIGN POINT 64							
188	RK	2500	.035	.035	TRAP	20		6	
189	KK	SC3-13							
190	KM	RUNOFF FROM SUB-BASIN SC3-13							
191	BA	.2550							
192	LS	0	81.4						
193	UD	.390							
194	KK	DP64							
195	KM	DESIGN POINT 64 COMBINE RUNOFF FROM SUB-BASIN SC3-13 AND RT-15							
196	HC	2							
197	KK	RT-16							
198	KM	ROUTE FLOW FROM DP 64 TO DESIGN POINT 60							
199	RK	4010	.018	.035	TRAP	20		10	
200	KK	SC3-9							
201	KM	RUNOFF FROM SUB-BASIN SC3-9							
202	BA	.2360							
203	LS	0	75.5						
204	UD	.320							
205	KK	DP60							
206	KM	DESIGN POINT 60 COMBINE RUNOFF FROM SUB-BASIN SC3-9, RT-17 AND RT-16							
207	HC	3							

HEC-1 INPUT

1
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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

208	KK	RT-18							
209	KM	ROUTE FLOW FROM DP 60 TO DESIGN POINT 53							
210	RK	2690	.015	.035	TRAP	10		4	
211	KK	SC3-2							
212	KM	RUNOFF FROM SUB-BASIN SC3-2							
213	BA	.1920							
214	LS	0	76.1						
215	UD	.310							
216	KK	SC3-11							
217	KM	RUNOFF FROM SUB-BASIN SC3-11							
218	BA	.2750							
219	LS	0	80.8						
220	UD	.430							
221	KK	RT-19							
222	KM	ROUTE FLOW FROM SUB-BASIN SC3-11 TO DESIGN POINT 55							
223	RK	2600	.022	.035	TRAP	10		6	

224 KK SC3-10
 225 KM RUNOFF FROM SUB-BASIN SC3-10
 226 BA .1130
 227 LS 0 75.0
 228 UD .290

 229 KK DP55
 230 KM DESIGN POINT 55 COMBINE RUNOFF FROM SUB-BASIN SC3-10 AND RT-19
 231 HC 2

 232 KK RT-20
 233 KM ROUTE FLOW FROM DESIGN POINT 55 TO DESIGN POINT 56
 234 RK 3670 .012 .035 TRAP 10 10

 235 KK SC3-1
 236 KM RUNOFF FROM SUB-BASIN SC3-1
 237 BA .1290
 238 LS 0 80.0
 239 UD .320

 240 KK DP56
 241 KM DESIGN POINT 56 COMBINE RUNOFF FROM SUB-BASIN SC3-1 AND RT-20
 242 HC 2

 243 KK DB95
 244 KM ROUTE FLOW FROM DP 56 THROUGH DBPS STRUCTURE 95
 245 RS 1 ELEV 100
 246 SV 0 20 34 39 42 50
 247 SQ 0 10 20 30 40 43
 248 SE 100 104 107 108 109 110
 HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

 249 KK RT-21
 250 KM ROUTE FLOW FROM DETENTION BASIN DB95 TO DESIGN POINT 53
 251 RK 870 .023 .035 TRAP 10 10

 252 KK DP53
 253 KM DESIGN POINT 53 COMBINE RUNOFF FROM SUB-BASIN SC3-2, RT-21 AND RT-18
 254 HC 3

 255 KK DB94
 256 KM ROUTE FLOW FROM DP 53 THROUGH DBPS STRUCTURE 94
 257 RS 1 ELEV 100
 258 SV 0 30 40 60 80 120 130
 259 SQ 0 80 90 300 800 1750 2000
 260 SE 100 104 105 107 108 109 110

 261 KK RT-27
 262 KM ROUTE FLOW FROM DETETNION BASIN DB94 TO DESIGN POINT 50
 263 RK 1380 .022 .035 TRAP 10 10

 264 KK SC2-24
 265 KM RUNOFF FROM SUB-BASIN SC2-24
 266 BA .0660
 267 LS 0 90.7
 268 UD .190

 269 KK RT-29
 270 KM ROUTE FLOW FROM SUB-BASIN SC2-24 TO DESIGN POINT 50
 271 RK 330 .020 .013 CIRC 4

 272 KK SC2-27
 273 KM RUNOFF FROM SUB-BASIN SC2-27
 274 BA .0780
 275 LS 0 92.0
 276 UD .210

277	KK	DP50							
278	KM	DESIGN POINT 50 COMBINE RUNOFF FROM SC2-27, RT-27 AND RT-29							
279	HC	3							
280	KK	RT-31							
281	KM	ROUTE FLOW FROM DESIGN POINT 50 TO DESIGN POINT 49							
282	RK	940	.020	.035	TRAP	40		6	
283	KK	SC3-8							
284	KM	RUNOFF FROM SUB-BASIN SC3-8							
285	BA	.2420							
286	LS	0	67.0						
287	UD	.290							

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HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

288	KK	RT-25							
289	KM	ROUTE FLOW FROM SUB-BASIN SC3-8 TO DESIGN POINT 51							
290	RK	2690	.015	.035	TRAP	20		6	
291	KK	SC3-3							
292	KM	RUNOFF FROM SUB-BASIN SC3-3							
293	BA	.1270							
294	LS	0	88.5						
295	UD	.280							
296	KK	DP51							
297	KM	DESIGN POINT 51 COMBINE RUNOFF FROM SC3-3 AND RT-25							
298	HC	2							
299	KK	DB93							
300	KM	ROUTE FLOW FROM DP 51 THROUGH DBPS STRUCTURE 93							
301	RS	1	ELEV	100					
302	SV	0	10	20	30				
303	SQ	0	20	100	290				
304	SE	100	104	106	108				
305	KK	RT-30							
306	KM	ROUTE FLOW FROM DETENTION BASIN DB93 TO DESIGN POINT 49							
307	RK	1590	.019	.035	TRAP	20		10	
308	KK	DP49							
309	KM	DESIGN POINT 49 COMBINE RUNOFF FROM RT-31 AND RT-30							
310	HC	2							
311	KK	RT-32							
312	KM	ROUTE FLOW FROM DESIGN POINT 49 TO DESIGN POINT 48							
313	RK	4930	.017	.035	TRAP	40		6	
314	KK	SC3-6							
315	KM	RUNOFF FROM SUB-BASIN SC3-6							
316	BA	.2420							
317	LS	0	70.0						
318	UD	.310							
319	KK	RT-23							
320	KM	ROUTE FLOW FROM SUB-BASIN SC3-6 TO DESIGN POINT 54							
321	RK	1830	.023	.035	TRAP	10		10	
322	KK	SC3-5							
323	KM	RUNOFF FROM SUB-BASIN SC3-5							
324	BA	.1150							
325	LS	0	88.7						
326	UD	.280							

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HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
327	KK RT-24
328	KM ROUTE FLOW FROM SUB-BASIN SC3-5 TO DESIGN POINT 54
329	RK 1760 .041 .035 TRAP 10 4
330	KK SC3-7
331	KM RUNOFF FROM SUB-BASIN SC3-7
332	BA .5380
333	LS 0 81.5
334	UD .530
335	KK RT-22
336	KM ROUTE FLOW FROM SUB-BASIN SC3-7 TO DESIGN POINT 54
337	RK 2860 .021 .035 TRAP 30 6
338	KK SC3-4
339	KM RUNOFF FROM SUB-BASIN SC3-4
340	BA .3210
341	LS 0 81.9
342	UD .330
343	KK DP54
344	KM DESIGN POINT 54 COMBINE RUNOFF FROM SUB-BASIN SC3-4, RT-22, RT-23 AND RT-24
345	HC 4
346	KK DB92
347	KM ROUTE FLOW FROM DP 54 THROUGH DBPS STRUCTURE 92
348	RS 1 ELEV 100
349	SV 0 40 60 90 110 120 130
350	SQ 0 40 60 82 90 91 95
351	SE 100 104 105 106 108 109 110
352	KK RT-26
353	KM ROUTE FLOW FROM DETENTION BASIN 92 TO DESIGN POINT 47
354	RK 4680 .028 .035 TRAP 40 10
355	KK SC2-26
356	KM RUNOFF FROM SUB-BASIN SC2-26
357	BA .1860
358	LS 0 74.4
359	UD .320
360	KK RT-34
361	KM ROUTE FLOW FROM SUB-BASIN SC2-26 TO DESIGN POINT 47
362	RK 1620 .014 .035 TRAP 4 4
363	KK SC2-25
364	KM RUNOFF FROM SUB-BASIN SC2-25
365	BA .4440
366	LS 0 73.6
367	UD .440

HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
368	KK DP47
369	KM DESIGN POINT 47 COMBINE RUNOFF FROM SUB-BASIN SC3-26 AND RT-26 AND RT-34
370	HC 3
371	KK RT-33
372	KM ROUTE FLOW FROM DESIGN POINT 47 TO DESIGN POINT 48
373	RK 510 .020 .035 TRAP 6 4
374	KK SC2-28
375	KM RUNOFF FROM SUB-BASIN SC2-28
376	BA .2740
377	LS 0 67.1

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378      UD      .510
379      KK      SC2-21
380      KM      RUNOFF FROM SUB-BASIN SC2-21
381      BA      .1930
382      LS      0      79.0
383      UD      .390
384      KK      DP48
385      KM      DESIGN POINT 48 COMBINE RUNOFF FROM SB'S SC2-21 AND SC2-28, RT-33 AND RT-32
386      HC      4
387      KK      RT-37
388      KM      ROUTE FLOW FROM DESIGN POINT 48 TO DESIGN POINT 42
389      RK      1080      .022      .035      TRAP      60      6
390      KK      SC2-23
391      KM      RUNOFF FROM SUB-BASIN SC2-23
392      BA      .1960
393      LS      0      70.0
394      UD      .330
395      KK      RT-35
396      KM      ROUTE FLOW FROM SUB-BASIN SC2-23 TO DESIGN POINT 46
397      RK      1180      .015      .013      CIRC      5
398      KK      SC2-22
399      KM      RUNOFF FROM SUB-BASIN SC2-22
400      BA      .0810
401      LS      0      79.0
402      UD      .220
403      KK      DP46
404      KM      DESIGN POINT 46 COMBINE RUNOFF FROM SUB-BASIN SC2-22 AND RT-35
405      HC      2
406      KK      RT-36
407      KM      ROUTE FLOW FROM DESIGN POINT 46 TO DESIGN POINT 42
408      RK      1790      .017      .013      TRAP      30      4
                                     HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
409      KK      DP42
410      KM      DESIGN POINT 42 COMBINE RUNOFF FROM RT- 37 AND RT-36
411      HC      2
412      KK      RT-38
413      KM      ROUTE FLOW FROM DESIGN POINT 42 TO DESIGN POINT 85
414      RK      1790      .013      .035      TRAP      60      6
415      KK      SC2-20
416      KM      RUNOFF FROM SUB-BASIN SC2-20
417      BA      .3240
418      LS      0      79.0
419      UD      .380
420      KK      RT-41
421      KM      ROUTE FLOW FROM SUB-BASIN SC2-20 TO DESIGN POINT 85
422      RK      1560      .020      .013      CIRC      6.5
423      KK      SC2-15
424      KM      RUNOFF FROM SUB-BASIN SC2-15
425      BA      .1810
426      LS      0      79.0
427      UD      .240
428      KK      RT-39
429      KM      ROUTE FLOW FROM SUB-BASIN SC2-15 TO DESIGN POINT 85

```

430	RK	990	.022	.013	CIRC	6.5
431	KK	SC2-13				
432	KM	RUNOFF FROM SUB-BASIN SC2-13				
433	BA	.1630				
434	LS	0	79.0			
435	UD	.280				
436	KK	RT-40				
437	KM	ROUTE FLOW FROM SUB-BASIN SC2-13 TO DESIGN POINT 85				
438	RK	300	.033	.013	CIRC	5.5
439	KK	DP85				
440	KM	DESIGN POINT 85 COMBINE RUNOFF FROM RT-41, RT-38, RT-39 AND RT-40				
441	HC	4				
442	KK	RT-42				
443	KM	ROUTE FLOW FROM DESIGN POINT 85 TO DESIGN POINT 37				
444	RK	2900	.016	.035	TRAP	60 6
445	KK	SC2-14				
446	KM	RUNOFF FROM SUB-BASIN SC2-14				
447	BA	.2650				
448	LS	0	76.2			
449	UD	.450				

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

450	KK	SC2-16				
451	KM	RUNOFF FROM SUB-BASIN SC2-16				
452	BA	.2120				
453	LS	0	76.2			
454	UD	.310				
455	KK	DP37				
456	KM	DESIGN POINT 37 COMBINE RUNOFF FROM RT-42, SUB-BASINS SC2-14 AND SC2-16				
457	HC	3				
458	KK	RT-43				
459	KM	ROUTE FLOW FROM DESIGN POINT 37 TO DESIGN POINT 36				
460	RK	3380	.016	.035	TRAP	60 4
461	KK	SC2-12				
462	KM	RUNOFF FROM SUB-BASIN SC2-12				
463	BA	.1790				
464	LS	0	70.6			
465	UD	.280				
466	KK	RT-46				
467	KM	ROUTE FLOW FROM SUB-BASIN SC2-12 TO DESIGN POINT 33				
468	RK	2280	.024	.035	TRAP	20 4
469	KK	SC2-8				
470	KM	RUNOFF FROM SUB-BASIN SC2-8				
471	BA	.3480				
472	LS	0	79.0			
473	UD	.440				
474	KK	DP33				
475	KM	DESIGN POINT 33 COMBINE RUNOFF FROM RT-46 AND SUB-BASIN SC2-8				
476	HC	2				
477	KK	RT-45				
478	KM	ROUTE FLOW FROM DESIGN POINT 33 TO DESIGN POINT 36				
479	RK	850	.021	.035	TRAP	20 4
480	KK	DP36				
481	KM	DESIGN POINT 36 COMBINE RUNOFF FROM RT-45 AND RT-43				

482 HC 2
 483 KK RT-44
 484 KM ROUTE FLOW FROM DESIGN POINT 36 TO DESIGN POINT 35
 485 RK 1740 .016 .035 TRAP 60 4
 486 KK SC2-11
 487 KM RUNOFF FROM SUB-BASIN SC2-11
 488 BA .2100
 489 LS 0 79.0
 490 UD .240

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

491 KK RT-52
 492 KM ROUTE FLOW FROM SUB-BASIN SC2-11 TO DESIGN POINT 39
 493 RK 2680 .008 .035 TRAP 30 4
 494 KK SC2-19
 495 KM RUNOFF FROM SUB-BASIN SC2-19
 496 BA .1820
 497 LS 0 86.0
 498 UD .300
 499 KK RT-51
 500 KM ROUTE FLOW FROM SUB-BASIN SC2-19 TO DESIGN POINT 39
 501 RK 3100 .022 .013 CIRC 6
 502 KK SC2-18
 503 KM RUNOFF FROM SUB-BASIN SC2-18
 504 BA .1980
 505 LS 0 88.0
 506 UD .290
 507 KK DP39
 508 KM DESIGN POINT 39 COMBINE RUNOFF FROM RT-52, RT-51 AND SUB-BASIN SC2-18
 509 HC 3
 510 KK RT-49
 511 KM ROUTE FLOW FROM DESIGN POINT 39 TO DESIGN POINT 34
 512 RK 1160 .024 .035 TRAP 20 4
 513 KK SC2-17
 514 KM RUNOFF FROM SUB-BASIN SC2-17
 515 BA .1600
 516 LS 0 79.0
 517 UD .320
 518 KK RT-50
 519 KM ROUTE FLOW FROM SUB-BASIN SC2-17 TO DESIGN POINT 34
 520 RK 1840 .025 .035 TRAP 20 4
 521 KK SC2-29
 522 KM RUNOFF FROM SUB-BASIN SC2-29
 523 BA .1510
 524 LS 0 88.0
 525 UD .210
 526 KK DP34
 527 KM DESIGN POINT 34 COMBINE RUNOFF FROM RT-49, RT-50 AND SUB-BASIN SC2-29
 528 HC 3
 529 KK RT-48
 530 KM ROUTE FLOW FROM DESIGN POINT 34 TO DESIGN POINT 32
 531 RK 1470 .020 .04 TRAP 20 4

HEC-1 INPUT

634	KM	ROUTE FLOW FROM DP 20 THROUGH SAND CREEK DETENTION BASIN DBSC2									
635	RS	1	ELEV	6496.5							
636	SV	0	6.75	29.8	58.8	92.2	129.4	168.6	209.8	255.0	302.2
637	SQ	0	10	30	100	400	800	2000	4300	5000	7000
638	SE	6496.5	6498	6500	6502	6504	6506	6508	6510	6512	6514

639	KK	RT-63									
640	KM	ROUTE FLOW FROM DETENTION BASIN SC2 TO DESIGN POINT 18									
641	RK	1290	.007	.035		TRAP	100				3

642	KK	SC1-10									
643	KM	RUNOFF FROM SUB-BASIN SC1-10									
644	BA	.2880									
645	LS	0	78.2								
646	UD	.390									

647	KK	DP18									
648	KM	DESIGN POINT 18 COMBINE RUNOFF FROM RT-63 AND SUB-BASIN SC1-10									
649	HC	2									

650	KK	RT-66									
651	KM	ROUTE FLOW FROM DESIGN POINT 18 TO DESIGN POINT 17									
652	RK	320	.007	.035		TRAP	100				3
						HEC-1 INPUT					

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

653	KK	SC1-13									
654	KM	RUNOFF FROM SUB-BASIN SC1-13									
655	BA	.1300									
656	LS	0	87.3								
657	UD	.270									

658	KK	RT-64									
659	KM	ROUTE FLOW FROM SUB-BASIN SC1-13 TO DESIGN POINT 19									
660	RK	1970	.026	.013		CIRC	4.5				

661	KK	SC1-14									
662	KM	RUNOFF FROM SUB-BASIN SC1-14									
663	BA	.1100									
664	LS	0	92.0								
665	UD	.280									

666	KK	DP19									
667	KM	DESIGN POINT 19 COMBINE RUNOFF FROM RT-64 AND SUB-BASIN SC1-14									
668	HC	2									

669	KK	RT-65									
670	KM	ROUTE FLOW FROM DESIGN POINT 19 TO DESIGN POINT 17									
671	RK	700	.022	.013		TRAP	10				.1

672	KK	DP17									
673	KM	DESIGN POINT 17 COMBINE RUNOFF FROM RT-65 AND RT-66									
674	HC	2									

675	KK	RT-67									
676	KM	ROUTE FLOW FROM DESIGN POINT 17 TO DESIGN POINT 13									
677	RK	910	.007	.035		TRAP	100				3

678	KK	SC1-9									
679	KM	RUNOFF FROM SUB-BASIN SC1-9									
680	BA	.2400									
681	LS	0	76.9								
682	UD	.240									

683	KK	DBSR29									
684	KM	ROUTE FLOW FROM SB SC1-9 THROUGH SPRINGS RANCH DETENTION BASIN DBSR29									
685	RS	1	ELEV	100							
686	SV	0	.03	0.47	1.20	2.60	6.70	8.8	10.7		

687	SQ	0	.2	7.8	22	40	73	97	113
688	SE	100	100.5	101.5	102.5	103.5	104.5	105.5	106.5
689	KK	RT-70							
690	KM	ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR29 TO DESIGN POINT 14							
691	RD	2750	.013	.013		CIRC	5.5		
						HEC-1 INPUT			

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LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
692	KK SC1-8
693	KM RUNOFF FROM SUB-BASIN SC1-8
694	BA .2100
695	LS 0 79.5
696	UD .320
697	KK RT-71
698	KM ROUTE FLOW FROM SUB-BASIN SC1-8 TO DESIGN POINT 14
699	RK 600 .010 .013 CIRC 5
700	KK DP14
701	KM DESIGN POINT 14 COMBINE RUNOFF FROM RT-71 AND RT-70
702	HC 2
703	KK RT-69
704	KM ROUTE FLOW FROM DESIGN POINT 14 TO DESIGN POINT 12
705	RK 1550 .015 .013 TRAP 7 .1
706	KK SC1-7
707	KM RUNOFF FROM SUB-BASIN SC1-7
708	BA .0960
709	LS 0 80.1
710	UD .230
711	KK DP12
712	KM DESIGN POINT 12 COMBINE RUNOFF FROM SUB-BASIN SC1-7 AND RT-69
713	HC 2
714	KK RT-68
715	KM ROUTE FLOW FROM DESIGN POINT 12 TO DESIGN POINT 13
716	RK 390 .025 .013 TRAP 7 .1
717	KK DP13
718	KM DESIGN POINT 13 COMBINE RUNOFF FROM RT-67 AND RT-68
719	HC 2
720	KK RT-72
721	KM ROUTE FLOW FROM DESIGN POINT 13 TO DESIGN POINT 10
722	RK 1150 .007 .04 TRAP 100 3
723	KK SC1-6
724	KM RUNOFF FROM SUB-BASIN SC1-6
725	BA .1590
726	LS 0 80.6
727	UD .220
728	KK DBSR15
729	KM ROUTE FLOW FROM SB SC1-6 THROUGH SPRINGS RANCH DETENTION BASIN DBSR15
730	RS 1 ELEV 100
731	SV 0 1.50 3.80 6.50 9.70 13.80
732	SQ 0 80 125 150 180 210
733	SE 100 102 104 106 108 110
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LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
734	KK RT-74


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787      KK   SC1-4
788      KM   RUNOFF FROM SUB-BASIN SC1-4
789      BA   .1010
790      LS   0      92.0
791      UD   .270

792      KK   RT-80
793      KM   ROUTE FLOW FROM SUB-BASIN SC1-4 TO DESIGN POINT 4
794      RK   720   .014   .013           CIRC      5

795      KK   SC1-1
796      KM   RUNOFF FROM SUB-BASIN SC1-1
797      BA   .1090
798      LS   0      79.0
799      UD   .280

800      KK   RT-79
801      KM   ROUTE FLOW FROM SUB-BASIN SC1-1 TO DESIGN POINT 4
802      RK   1115  .027   .013           CIRC      3

803      KK   SC1-3
804      KM   RUNOFF FROM SUB-BASIN SC1-3
805      BA   .3530
806      LS   0      65.0
807      UD   .460

808      KK   DP4
809      KM   DESIGN POINT 4 COMBINE RUNOFF FROM SB SC1-3, RT-80, RT-77, RT-78 AND RT-79
810      HC   5

811      KK   DBSC1
812      KM   ROUTE FLOW FROM DP 4 THROUGH SAND CREEK DETENTION BASIN DBSC1
813      KO   0
814      RS   1      ELEV    6393
815      SV   0      3.06   14.95   31.12   53.59   69.95   86.31   106.5   126.7   150.4
816      SV   174.1  200.4  226.7  254.9  283.2  313.4  343.6  375.2  406.8  439.5
                                     HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
817      SQ      0      3      5      189      954      1465      2055      2431      2755      3045
818      SQ    3310    3555    3785    4037    5518    7863    10797    14211    18042    22245
819      SE    6393    6394    6396    6398    6400    6401    6402    6403    6404    6405
820      SE    6406    6407    6408    6409    6410    6411    6412    6413    6414    6415
821      ZZ

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OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	RATIO 2
				.68	1.00
HYDROGRAPH AT					
+	SB82	.19	1 FLOW	84.	232.
			TIME	6.50	6.50
ROUTED TO					
+	RT-4	.19	1 FLOW	83.	220.
			TIME	6.58	6.58
HYDROGRAPH AT					
+	SB74	.18	1 FLOW	49.	143.
			TIME	6.67	6.67
2 COMBINED AT					
+	DP74	.37	1 FLOW	122.	345.
			TIME	6.58	6.58

ROUTED TO +	RT-3	.37	1	FLOW TIME	119. 6.67	331. 6.67
HYDROGRAPH AT +	SB80	.22	1	FLOW TIME	68. 6.67	190. 6.67
ROUTED TO +	RT-2	.22	1	FLOW TIME	67. 6.75	185. 6.75
HYDROGRAPH AT +	SB81	.39	1	FLOW TIME	100. 6.67	298. 6.67
ROUTED TO +	RT-1	.39	1	FLOW TIME	99. 6.83	296. 6.75
HYDROGRAPH AT +	SB73	.12	1	FLOW TIME	31. 6.75	90. 6.67
HYDROGRAPH AT +	SB75	.12	1	FLOW TIME	25. 6.83	72. 6.75
HYDROGRAPH AT +	SB76	.17	1	FLOW TIME	35. 6.83	105. 6.75
6 COMBINED AT +	DP75	1.39	1	FLOW TIME	364. 6.75	1022. 6.75
ROUTED TO +	RT-5	1.39	1	FLOW TIME	345. 6.75	1021. 6.75
HYDROGRAPH AT +	SB79	.29	1	FLOW TIME	111. 6.58	306. 6.58
ROUTED TO +	RT-7	.29	1	FLOW TIME	107. 6.67	281. 6.58
HYDROGRAPH AT +	SB78	.88	1	FLOW TIME	259. 6.67	738. 6.67
2 COMBINED AT +	DP78	1.17	1	FLOW TIME	367. 6.67	1020. 6.67
ROUTED TO +	RT-6	1.17	1	FLOW TIME	351. 6.75	993. 6.67
HYDROGRAPH AT +	SB77	.25	1	FLOW TIME	61. 6.75	177. 6.75
HYDROGRAPH AT +	SB88	.10	1	FLOW TIME	34. 6.58	97. 6.58
4 COMBINED AT +	DP77	2.91	1	FLOW TIME	780. 6.75	2207. 6.67

ROUTED TO						
+	RT-8	2.91	1	FLOW TIME	761. 6.83	2201. 6.75
HYDROGRAPH AT						
+	SB71	.36	1	FLOW TIME	127. 6.67	334. 6.67
2 COMBINED AT						
+	DP71	3.27	1	FLOW TIME	861. 6.83	2508. 6.75
ROUTED TO						
+	RT-9	3.27	1	FLOW TIME	842. 6.83	2445. 6.75
HYDROGRAPH AT						
+	SB70	.31	1	FLOW TIME	99. 6.75	256. 6.75
2 COMBINED AT						
+	DP70	3.58	1	FLOW TIME	931. 6.83	2701. 6.75
ROUTED TO						
+	RT-11	3.58	1	FLOW TIME	905. 6.83	2653. 6.75
HYDROGRAPH AT						
+	SB72	.25	1	FLOW TIME	135. 6.58	321. 6.50
ROUTED TO						
+	RT-10	.25	1	FLOW TIME	128. 6.58	321. 6.58
HYDROGRAPH AT						
+	SB69	.25	1	FLOW TIME	93. 6.83	216. 6.83
2 COMBINED AT						
+	DP69	.50	1	FLOW TIME	191. 6.67	453. 6.58
ROUTED TO						
+	RT-12	.50	1	FLOW TIME	187. 6.67	446. 6.67
HYDROGRAPH AT						
+	SC3-15	.07	1	FLOW TIME	35. 6.58	87. 6.58
3 COMBINED AT						
+	DP87	4.15	1	FLOW TIME	1068. 6.83	3104. 6.75
ROUTED TO						
+	RT-13	4.15	1	FLOW TIME	1048. 6.92	3020. 6.83
HYDROGRAPH AT						
+	SC3-12	.24	1	FLOW TIME	70. 6.83	183. 6.75
2 COMBINED AT						
+	DP63	4.39	1	FLOW TIME	1111. 6.92	3198. 6.83
ROUTED TO						
+	RT-17	4.39	1	FLOW	1083.	3152.

				TIME	7.00	6.83
HYDROGRAPH AT						
+	SB68	.22	1	FLOW	97.	222.
				TIME	6.75	6.75
ROUTED TO						
+	RT-14	.22	1	FLOW	95.	220.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC3-14	.31	1	FLOW	88.	234.
				TIME	6.83	6.75
2 COMBINED AT						
+	DP67	.53	1	FLOW	183.	448.
				TIME	6.83	6.83
ROUTED TO						
+	RT-15	.53	1	FLOW	178.	446.
				TIME	6.92	6.83
HYDROGRAPH AT						
+	SC3-13	.25	1	FLOW	174.	346.
				TIME	6.75	6.75
2 COMBINED AT						
+	DP64	.79	1	FLOW	338.	763.
				TIME	6.83	6.75
ROUTED TO						
+	RT-16	.79	1	FLOW	332.	758.
				TIME	6.92	6.83
HYDROGRAPH AT						
+	SC3-9	.24	1	FLOW	123.	279.
				TIME	6.67	6.67
3 COMBINED AT						
+	DP60	5.42	1	FLOW	1462.	4126.
				TIME	6.92	6.83
ROUTED TO						
+	RT-18	5.42	1	FLOW	1456.	4013.
				TIME	7.00	6.92
HYDROGRAPH AT						
+	SC3-2	.19	1	FLOW	108.	239.
				TIME	6.67	6.67
HYDROGRAPH AT						
+	SC3-11	.28	1	FLOW	167.	341.
				TIME	6.75	6.75
ROUTED TO						
+	RT-19	.28	1	FLOW	166.	336.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC3-10	.11	1	FLOW	61.	138.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP55	.39	1	FLOW	211.	448.
				TIME	6.75	6.75
ROUTED TO						
+	RT-20	.39	1	FLOW	209.	445.
				TIME	6.92	6.83
HYDROGRAPH AT						

+	SC3-1	.13	1	FLOW TIME	92. 6.67	187. 6.67
2 COMBINED AT						
+	DP56	.52	1	FLOW TIME	274. 6.83	586. 6.83
ROUTED TO						
+	DB95	.52	1	FLOW TIME	13. 11.42	41. 11.00
** PEAK STAGES IN FEET **						
	1	STAGE			105.02	109.39
		TIME			11.42	11.00
ROUTED TO						
+	RT-21	.52	1	FLOW TIME	13. 11.42	41. 11.00
3 COMBINED AT						
+	DP53	6.13	1	FLOW TIME	1500. 7.00	4163. 6.83
ROUTED TO						
+	DB94	6.13	1	FLOW TIME	440. 10.75	1908. 7.25
** PEAK STAGES IN FEET **						
	1	STAGE			107.28	109.63
		TIME			10.75	7.25
ROUTED TO						
+	RT-27	6.13	1	FLOW TIME	438. 10.75	1895. 7.25
HYDROGRAPH AT						
+	SC2-24	.07	1	FLOW TIME	105. 6.58	172. 6.50
ROUTED TO						
+	RT-29	.07	1	FLOW TIME	105. 6.58	172. 6.58
HYDROGRAPH AT						
+	SC2-27	.08	1	FLOW TIME	128. 6.58	207. 6.58
3 COMBINED AT						
+	DP50	6.27	1	FLOW TIME	442. 10.75	1922. 7.25
ROUTED TO						
+	RT-31	6.27	1	FLOW TIME	440. 10.83	1920. 7.33
HYDROGRAPH AT						
+	SC3-8	.24	1	FLOW TIME	66. 6.67	192. 6.67
ROUTED TO						
+	RT-25	.24	1	FLOW TIME	63. 6.75	190. 6.75
HYDROGRAPH AT						
+	SC3-3	.13	1	FLOW TIME	155. 6.67	270. 6.58
2 COMBINED AT						
+	DP51	.37	1	FLOW TIME	198. 6.67	437. 6.67

ROUTED TO						
+	DB93	.37	1	FLOW	25.	77.
				TIME	10.50	7.17
				** PEAK STAGES IN FEET **		
			1	STAGE	104.11	105.42
				TIME	10.50	7.17
ROUTED TO						
+	RT-30	.37	1	FLOW	25.	77.
				TIME	10.58	7.25
2 COMBINED AT						
+	DP49	6.64	1	FLOW	464.	1996.
				TIME	10.83	7.33
ROUTED TO						
+	RT-32	6.64	1	FLOW	463.	1988.
				TIME	10.92	7.33
HYDROGRAPH AT						
+	SC3-6	.24	1	FLOW	83.	219.
				TIME	6.67	6.67
ROUTED TO						
+	RT-23	.24	1	FLOW	82.	213.
				TIME	6.75	6.75
HYDROGRAPH AT						
+	SC3-5	.12	1	FLOW	142.	247.
				TIME	6.67	6.58
ROUTED TO						
+	RT-24	.12	1	FLOW	142.	243.
				TIME	6.67	6.67
HYDROGRAPH AT						
+	SC3-7	.54	1	FLOW	290.	589.
				TIME	6.83	6.83
ROUTED TO						
+	RT-22	.54	1	FLOW	290.	585.
				TIME	6.92	6.92
HYDROGRAPH AT						
+	SC3-4	.32	1	FLOW	252.	496.
				TIME	6.67	6.67
4 COMBINED AT						
+	DP54	1.22	1	FLOW	681.	1393.
				TIME	6.75	6.75
ROUTED TO						
+	DB92	1.22	1	FLOW	54.	89.
				TIME	10.92	10.92
				** PEAK STAGES IN FEET **		
			1	STAGE	104.72	107.66
				TIME	10.92	10.92
ROUTED TO						
+	RT-26	1.22	1	FLOW	54.	89.
				TIME	11.08	11.08
HYDROGRAPH AT						
+	SC2-26	.19	1	FLOW	89.	208.
				TIME	6.67	6.67
ROUTED TO						
+	RT-34	.19	1	FLOW	88.	202.
				TIME	6.75	6.75

HYDROGRAPH AT						
+	SC2-25	.44	1	FLOW TIME	159. 6.83	378. 6.83
3 COMBINED AT						
+	DP47	1.85	1	FLOW TIME	244. 6.75	584. 6.75
ROUTED TO						
+	RT-33	1.85	1	FLOW TIME	242. 6.83	579. 6.75
HYDROGRAPH AT						
+	SC2-28	.27	1	FLOW TIME	48. 6.92	142. 6.92
HYDROGRAPH AT						
+	SC2-21	.19	1	FLOW TIME	113. 6.75	236. 6.75
4 COMBINED AT						
+	DP48	8.95	1	FLOW TIME	616. 6.75	2318. 7.33
ROUTED TO						
+	RT-37	8.95	1	FLOW TIME	604. 6.75	2314. 7.33
HYDROGRAPH AT						
+	SC2-23	.20	1	FLOW TIME	65. 6.75	169. 6.67
ROUTED TO						
+	RT-35	.20	1	FLOW TIME	64. 6.75	166. 6.75
HYDROGRAPH AT						
+	SC2-22	.08	1	FLOW TIME	67. 6.58	137. 6.58
2 COMBINED AT						
+	DP46	.28	1	FLOW TIME	122. 6.67	287. 6.67
ROUTED TO						
+	RT-36	.28	1	FLOW TIME	119. 6.67	284. 6.67
2 COMBINED AT						
+	DP42	9.23	1	FLOW TIME	717. 6.75	2358. 7.33
ROUTED TO						
+	RT-38	9.23	1	FLOW TIME	700. 6.83	2346. 7.33
HYDROGRAPH AT						
+	SC2-20	.32	1	FLOW TIME	193. 6.75	401. 6.75
ROUTED TO						
+	RT-41	.32	1	FLOW TIME	192. 6.75	400. 6.75
HYDROGRAPH AT						
+	SC2-15	.18	1	FLOW TIME	141. 6.58	291. 6.58
ROUTED TO						
+	RT-39	.18	1	FLOW	138.	288.

				TIME	6.58	6.58
HYDROGRAPH AT						
+	SC2-13	.16	1	FLOW TIME	118. 6.67	241. 6.67
ROUTED TO						
+	RT-40	.16	1	FLOW TIME	118. 6.67	241. 6.67
4 COMBINED AT						
+	DP85	9.90	1	FLOW TIME	1092. 6.75	2491. 7.33
ROUTED TO						
+	RT-42	9.90	1	FLOW TIME	1068. 6.83	2486. 7.42
HYDROGRAPH AT						
+	SC2-14	.26	1	FLOW TIME	114. 6.83	254. 6.83
HYDROGRAPH AT						
+	SC2-16	.21	1	FLOW TIME	120. 6.67	265. 6.67
3 COMBINED AT						
+	DP37	10.37	1	FLOW TIME	1275. 6.83	2818. 6.75
ROUTED TO						
+	RT-43	10.37	1	FLOW TIME	1268. 6.83	2756. 6.83
HYDROGRAPH AT						
+	SC2-12	.18	1	FLOW TIME	71. 6.67	179. 6.67
ROUTED TO						
+	RT-46	.18	1	FLOW TIME	69. 6.75	171. 6.67
HYDROGRAPH AT						
+	SC2-8	.35	1	FLOW TIME	184. 6.83	390. 6.75
2 COMBINED AT						
+	DP33	.53	1	FLOW TIME	252. 6.75	559. 6.75
ROUTED TO						
+	RT-45	.53	1	FLOW TIME	247. 6.83	553. 6.75
2 COMBINED AT						
+	DP36	10.90	1	FLOW TIME	1515. 6.83	3286. 6.83
ROUTED TO						
+	RT-44	10.90	1	FLOW TIME	1475. 6.83	3282. 6.83
HYDROGRAPH AT						
+	SC2-11	.21	1	FLOW TIME	163. 6.58	338. 6.58
ROUTED TO						
+	RT-52	.21	1	FLOW TIME	160. 6.67	335. 6.67
HYDROGRAPH AT						

+	SC2-19	.18	1	FLOW TIME	191. 6.67	345. 6.67
ROUTED TO						
+	RT-51	.18	1	FLOW TIME	189. 6.67	343. 6.67
HYDROGRAPH AT						
+	SC2-18	.20	1	FLOW TIME	234. 6.67	405. 6.67
3 COMBINED AT						
+	DP39	.59	1	FLOW TIME	582. 6.67	1083. 6.67
ROUTED TO						
+	RT-49	.59	1	FLOW TIME	570. 6.67	1073. 6.67
HYDROGRAPH AT						
+	SC2-17	.16	1	FLOW TIME	107. 6.67	222. 6.67
ROUTED TO						
+	RT-50	.16	1	FLOW TIME	105. 6.75	214. 6.67
HYDROGRAPH AT						
+	SC2-29	.15	1	FLOW TIME	208. 6.58	358. 6.58
3 COMBINED AT						
+	DP34	.90	1	FLOW TIME	846. 6.67	1587. 6.67
ROUTED TO						
+	RT-48	.90	1	FLOW TIME	827. 6.67	1574. 6.67
HYDROGRAPH AT						
+	SC2-10	.33	1	FLOW TIME	185. 6.75	391. 6.75
2 COMBINED AT						
+	DP32	1.23	1	FLOW TIME	999. 6.67	1949. 6.67
ROUTED TO						
+	RT-47	1.23	1	FLOW TIME	980. 6.67	1930. 6.67
HYDROGRAPH AT						
+	SC2-9	.21	1	FLOW TIME	61. 6.92	150. 6.83
3 COMBINED AT						
+	DP35	12.34	1	FLOW TIME	2389. 6.83	5152. 6.75
ROUTED TO						
+	RT-53	12.34	1	FLOW TIME	2372. 6.83	5091. 6.75
HYDROGRAPH AT						
+	SC2-7	.14	1	FLOW TIME	96. 6.67	200. 6.67
2 COMBINED AT						
+	DP29	12.48	1	FLOW TIME	2447. 6.83	5277. 6.75

ROUTED TO						
+	RT-54	12.48	1	FLOW TIME	2408. 6.83	5188. 6.83
HYDROGRAPH AT						
+	SC2-6	.15	1	FLOW TIME	137. 6.58	269. 6.58
ROUTED TO						
+	RT-55	.15	1	FLOW TIME	134. 6.67	258. 6.58
HYDROGRAPH AT						
+	SC2-2	.17	1	FLOW TIME	135. 6.67	266. 6.67
3 COMBINED AT						
+	DP28	12.80	1	FLOW TIME	2579. 6.83	5605. 6.75
ROUTED TO						
+	RT-56	12.80	1	FLOW TIME	2544. 6.83	5522. 6.75
HYDROGRAPH AT						
+	SC2-5	.16	1	FLOW TIME	235. 6.58	386. 6.58
ROUTED TO						
+	RT-58	.16	1	FLOW TIME	231. 6.67	379. 6.58
HYDROGRAPH AT						
+	SC2-0	.15	1	FLOW TIME	67. 6.67	160. 6.67
ROUTED TO						
+	RT-57	.15	1	FLOW TIME	66. 6.75	156. 6.67
3 COMBINED AT						
+	DP21	13.11	1	FLOW TIME	2754. 6.83	5996. 6.75
ROUTED TO						
+	RT-59	13.11	1	FLOW TIME	2724. 6.83	5924. 6.75
HYDROGRAPH AT						
+	SC2-4	.28	1	FLOW TIME	393. 6.67	641. 6.67
ROUTED TO						
+	RT-61	.28	1	FLOW TIME	388. 6.67	637. 6.67
HYDROGRAPH AT						
+	SC2-3	.07	1	FLOW TIME	117. 6.58	188. 6.58
2 COMBINED AT						
+	DP22	.36	1	FLOW TIME	485. 6.67	793. 6.67
ROUTED TO						
+	RT-60	.36	1	FLOW TIME	483. 6.67	793. 6.67
HYDROGRAPH AT						
+	SC2-1	.19	1	FLOW TIME	140. 6.67	282. 6.67

3 COMBINED AT	DP20	13.65	1	FLOW TIME	3187. 6.83	6885. 6.75	<i>IN SCH 2</i>
ROUTED TO							
+ DBSC2	13.65	1	FLOW TIME	1038. 7.42	4257. 7.08		<i>OUT SCH 2</i>
** PEAK STAGES IN FEET **							
			1	STAGE TIME	6506.40 7.42	6509.96 7.08	
ROUTED TO							
+ RT-63	13.65	1	FLOW TIME	1032. 7.50	4225. 7.08		
HYDROGRAPH AT							
+ SC1-10	.29	1	FLOW TIME	160. 6.75	339. 6.75		
2 COMBINED AT							
+ DP18	13.94	1	FLOW TIME	1066. 7.42	4376. 7.08		
ROUTED TO							
+ RT-66	13.94	1	FLOW TIME	1065. 7.50	4366. 7.08		
HYDROGRAPH AT							
+ SC1-13	.13	1	FLOW TIME	152. 6.58	271. 6.58		
ROUTED TO							
+ RT-64	.13	1	FLOW TIME	151. 6.67	267. 6.58		
HYDROGRAPH AT							
+ SC1-14	.11	1	FLOW TIME	160. 6.58	263. 6.58		
2 COMBINED AT							
+ DP19	.24	1	FLOW TIME	308. 6.58	530. 6.58		
ROUTED TO							
+ RT-65	.24	1	FLOW TIME	308. 6.67	527. 6.58		
2 COMBINED AT							
+ DP17	14.18	1	FLOW TIME	1102. 7.50	4462. 7.08		
ROUTED TO							
+ RT-67	14.18	1	FLOW TIME	1102. 7.50	4434. 7.08		
HYDROGRAPH AT							
+ SC1-9	.24	1	FLOW TIME	162. 6.58	352. 6.58		
ROUTED TO							
+ DBSR29	.24	1	FLOW TIME	51. 6.92	103. 6.92		
** PEAK STAGES IN FEET **							
			1	STAGE TIME	103.85 6.92	105.90 6.92	
ROUTED TO							
+ RT-70	.24	1	FLOW	51.	103.		

				TIME	7.00	7.00
HYDROGRAPH AT						
+	SC1-8	.21	1	FLOW	145.	298.
				TIME	6.67	6.67
ROUTED TO						
+	RT-71	.21	1	FLOW	144.	297.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP14	.45	1	FLOW	183.	365.
				TIME	6.75	6.67
ROUTED TO						
+	RT-69	.45	1	FLOW	182.	363.
				TIME	6.75	6.75
HYDROGRAPH AT						
+	SC1-7	.10	1	FLOW	83.	166.
				TIME	6.58	6.58
2 COMBINED AT						
+	DP12	.55	1	FLOW	253.	511.
				TIME	6.67	6.67
ROUTED TO						
+	RT-68	.55	1	FLOW	252.	509.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP13	14.72	1	FLOW	1180.	4641.
				TIME	7.50	7.08
ROUTED TO						
+	RT-72	14.72	1	FLOW	1175.	4599.
				TIME	7.50	7.08
HYDROGRAPH AT						
+	SC1-6	.16	1	FLOW	145.	286.
				TIME	6.58	6.58
ROUTED TO						
+	DBSR15	.16	1	FLOW	90.	138.
				TIME	6.75	6.83
				** PEAK STAGES IN FEET **		
			1	STAGE	102.45	105.01
				TIME	6.75	6.83
ROUTED TO						
+	RT-74	.16	1	FLOW	89.	137.
				TIME	6.75	6.83
HYDROGRAPH AT						
+	SC1-5	.17	1	FLOW	118.	245.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP11	.33	1	FLOW	203.	374.
				TIME	6.67	6.67
ROUTED TO						
+	RT-73	.33	1	FLOW	202.	373.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP10	15.06	1	FLOW	1217.	4784.
				TIME	7.50	7.08
ROUTED TO						

+	RT-75	15.06	1	FLOW TIME	1215. 7.58	4743. 7.17
	HYDROGRAPH AT					
+	SC1-2	.14	1	FLOW TIME	91. 6.67	189. 6.67
	ROUTED TO					
+	RT-76	.14	1	FLOW TIME	90. 6.67	188. 6.67
	2 COMBINED AT					
+	DP9	15.19	1	FLOW TIME	1231. 7.58	4781. 7.17
	ROUTED TO					
+	RT-78	15.19	1	FLOW TIME	1226. 7.58	4770. 7.17
	HYDROGRAPH AT					
+	SC1-12	.25	1	FLOW TIME	203. 6.75	386. 6.67
	ROUTED TO					
+	RT-81	.25	1	FLOW TIME	202. 6.75	383. 6.75
	HYDROGRAPH AT					
+	SC1-11	.16	1	FLOW TIME	228. 6.58	376. 6.58
	2 COMBINED AT					
+	DP8	.41	1	FLOW TIME	411. 6.67	735. 6.67
	ROUTED TO					
+	RT-77	.41	1	FLOW TIME	401. 6.75	730. 6.75
	HYDROGRAPH AT					
+	SC1-4	.10	1	FLOW TIME	150. 6.58	247. 6.58
	ROUTED TO					
+	RT-80	.10	1	FLOW TIME	149. 6.58	245. 6.58
	HYDROGRAPH AT					
+	SC1-1	.11	1	FLOW TIME	79. 6.67	161. 6.67
	ROUTED TO					
+	RT-79	.11	1	FLOW TIME	78. 6.67	161. 6.67
	HYDROGRAPH AT					
+	SC1-3	.35	1	FLOW TIME	53. 6.83	175. 6.83
	5 COMBINED AT					
+	DP4	16.16	1	FLOW TIME	1553. 6.75	5205. 7.17
	ROUTED TO					
+	DBSC1	16.16	1	FLOW TIME	1240. 8.00	3544. 8.08

** PEAK STAGES IN FEET **

1	STAGE	6400.56	6406.96
	TIME	8.00	8.08

JR ENGINEERING'S SAND CREEK DATA

CROSS SECTION

RIVER: SAND CREEK
REACH: ONE RS: 300

INPUT

Description: UPSTREAM OF PROPERTY

Station Elevation Data		num= 10							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
428	6792	430	6790	436	6788	443	6786	454	6784
500	6784	548	6784	560	6786	600	6788	701	6790

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
428	.04	430	.035	560	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	430	560		230	205		.2	.4

CROSS SECTION

RIVER: SAND CREEK
REACH: ONE RS: 295

INPUT

Description: NEAR UPSTREAM END OF PROPERTY

Station Elevation Data		num= 14							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
422	6787	445	6786	467	6784	476	6782	482	6780
500	6779.8	518	6780	573	6782	580	6784	586	6784
590	6784	616	6784	646	6786	698	6788		

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
422	.04	467	.035	573	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	467	573		87	86		.2	.4
Left Levee		Station=	455	Elevation=	6786			

CROSS SECTION

RIVER: SAND CREEK
REACH: ONE RS: 294.5

INPUT

Description: FIS CD-4082 ROTATED

Station Elevation Data		num= 11							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
415	6786.4	425	6784	430	6785	460	6781	475	6780
500	6779	549	6778	606	6777	633	6780	660	6786
682	6787								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
415	.04	500	.035	633	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	500	633		60	68		.2	.4

CROSS SECTION

RIVER: SAND CREEK
REACH: ONE RS: 290

INPUT

Description:

Station Elevation Data		num= 18							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
403	6785	420	6784	435	6782	446	6781	456	6780
486	6778	500	6778.39	534	6778	542	6780	546	6781.48

Post Project As-builts.rep

FLOW DATA

Flow Title: Floodplain

Flow File : x:\2910000.all\2910404\HEC-RAS\post project with no lower levee geometry\Post Project As-builts.f01

Flow Data (cfs)

River	Reach	RS	PF#1	PF 2	PF 3	PF 4
PF 5						
SAND CREEK	ONE	305	1200	2100	2600	3900
600						
SAND CREEK	ONE	300	1200	2100	2600	3900
600						
SAND CREEK	ONE	294.5	1200	2100	2600	3900
600						
SAND CREEK	ONE	214	1200	2400	3100	4600
600						
SAND CREEK	ONE	179	1200	2600	3150	4800
600						
SAND CREEK	ONE	108	1200	2600	3250	5100
600						
SAND CREEK	ONE	84	1200	2600	3300	5100
600						
SAND CREEK	ONE	75	1250	2700	3450	5350
625						
SAND CREEK	ONE	38	1300	2800	3600	5700
650						

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
SAND CREEK	ONE	PF#1	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 2	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 3	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 4	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 5	Normal S = 0.01	Normal S = 0.0065

GEOMETRY DATA

Geometry Title: As-builts modified for no lower levee

Geometry File : x:\2910000.all\2910404\HEC-RAS\post project with no lower levee geometry\Post Project As-builts.g01

CROSS SECTION

RIVER: SAND CREEK

REACH: ONE RS: 305

INPUT

Description: FIS CE-4080 Upstream of Property

Station Elevation Data		num= 30		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
1000	6820	1040	6816	1080	6812	1115	6808	1160	6804
1190	6800	1200	6796	1210	6792	1230	6788	1231	6786.4
1242	6786.4	1245	6788	1270	6792	1350	6792	1390	6796
1435	6800	1465	6800.2	1500	6800	1540	6802	1600	6802
1680	6802	1720	6801.6	1760	6802	1790	6802.2	1840	6802
1890	6802	1960	6803	2020	6804	2145	6806	2370	6808

Manning's n Values		num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
1000	.06	1230	.03	1245	.06		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1230	1245		390	395	.1	.3

No ox Bow levee with levee

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
ONE	305	PF 3	PPFNL	2600.00	6786.40	6793.98	6793.98	6795.31	0.004839	12.35	472.02	16.71	0.80
ONE	305	PF 3	PPFP	2600.00	6786.40	6793.98	6793.98	6795.31	0.004839	12.35	472.02	16.71	0.80
ONE	300	PF 3	PPFNL	2600.00	6784.00	6786.70	6786.70	6787.90	0.012999	8.80	299.07	13.52	0.99
ONE	300	PF 3	PPFP	2600.00	6784.00	6786.70	6786.70	6787.90	0.012999	8.80	299.07	13.52	0.99
ONE	295	PF 3	PPFNL	2600.00	6779.80	6783.45	6783.45	6784.75	0.012437	9.19	285.15	108.56	0.98
ONE	295	PF 3	PPFP	2600.00	6779.80	6783.45	6783.45	6784.75	0.012437	9.19	285.15	108.56	0.98
ONE	294.5	PF 3	PPFNL	2600.00	6777.00	6783.22	6780.38	6783.37	0.000670	3.27	877.91	218.56	0.25
ONE	294.5	PF 3	PPFP	2600.00	6777.00	6783.22	6780.38	6783.37	0.000670	3.27	877.91	218.56	0.25
ONE	290	PF 3	PPFNL	2600.00	6778.00	6781.70	6781.70	6783.02	0.010553	9.34	291.47	189.44	0.93
ONE	290	PF 3	PPFP	2600.00	6778.00	6781.70	6781.70	6783.02	0.010553	9.34	291.47	189.44	0.93
ONE	280	PF 3	PPFNL	2600.00	6776.80	6780.86	6780.56	6781.81	0.009060	7.85	332.35	199.06	0.84
ONE	280	PF 3	PPFP	2600.00	6776.80	6780.86	6780.56	6781.81	0.009060	7.85	332.35	199.06	0.84
ONE	265	PF 3	PPFNL	2600.00	6774.82	6779.09	6779.09	6780.37	0.012841	9.06	286.92	216.36	0.99
ONE	265	PF 3	PPFP	2600.00	6774.82	6779.09	6779.09	6780.37	0.012841	9.06	286.92	216.36	0.99
ONE	250	PF 3	PPFNL	2600.00	6768.00	6778.02	6773.08	6778.21	0.000522	3.49	779.81	189.51	0.23
ONE	250	PF 3	PPFP	2600.00	6768.00	6778.02	6773.08	6778.21	0.000522	3.49	779.81	189.51	0.23
ONE	240	PF 3	PPFNL	2600.00	6766.00	6778.04	6772.48	6778.16	0.000280	2.84	992.99	155.31	0.18
ONE	240	PF 3	PPFP	2600.00	6766.00	6778.04	6772.48	6778.16	0.000280	2.84	992.99	155.31	0.18
ONE	230	PF 3	PPFNL	2600.00	6764.00	6778.01	6771.26	6778.14	0.000249	2.95	970.71	133.21	0.17
ONE	230	PF 3	PPFP	2600.00	6764.00	6778.01	6771.26	6778.14	0.000249	2.95	970.71	133.21	0.17
ONE	225	PF 3	PPFNL	2600.00	6770.00	6777.77	6773.92	6778.08	0.001382	4.82	616.10	104.52	0.30
ONE	225	PF 3	PPFP	2600.00	6770.00	6777.77	6773.92	6778.08	0.001382	4.82	616.10	104.52	0.30
ONE	222	PF 3	PPFNL	2600.00	6770.10	6777.01	6774.46	6777.89	0.000455	7.52	345.74	50.12	0.50
ONE	222	PF 3	PPFP	2600.00	6770.10	6777.01	6774.46	6777.89	0.000455	7.52	345.74	50.12	0.50
ONE	221.6	PF 3	PPFNL	2600.00	6770.00	6777.01	6774.36	6777.86	0.000434	7.41	351.04	50.12	0.49
ONE	221.6	PF 3	PPFP	2600.00	6770.00	6777.01	6774.36	6777.86	0.000434	7.41	351.04	50.12	0.49
ONE	221.5	PF 3	PPFNL	2600.00	6770.00	6775.26	6775.26	6777.46	0.001953	11.92	218.21	49.98	1.00
ONE	221.5	PF 3	PPFP	2600.00	6770.00	6775.26	6775.26	6777.46	0.001953	11.92	218.21	49.98	1.00
ONE	221	PF 3	PPFNL	2600.00	6770.00	6774.36	6774.36	6776.57	0.001887	11.91	218.25	50.07	1.01
ONE	221	PF 3	PPFP	2600.00	6770.00	6774.36	6774.36	6776.57	0.001887	11.91	218.25	50.07	1.01
ONE	215	PF 3	PPFNL	2600.00	6758.45	6765.19	6762.82	6766.11	0.000491	7.71	337.10	50.11	0.52
ONE	215	PF 3	PPFP	2600.00	6758.45	6765.19	6762.82	6766.11	0.000491	7.71	337.10	50.11	0.52
ONE	214	PF 3	PPFNL	3100.00	6758.40	6763.31	6763.31	6765.78	0.001852	12.62	245.71	50.08	1.00
ONE	214	PF 3	PPFP	3100.00	6758.40	6763.31	6763.31	6765.78	0.001852	12.62	245.71	50.08	1.00
ONE	210	PF 3	PPFNL	3100.00	6749.75	6756.84	6754.66	6758.03	0.000596	8.73	355.04	50.12	0.58
ONE	210	PF 3	PPFP	3100.00	6749.75	6756.84	6754.66	6758.03	0.000596	8.73	355.04	50.12	0.58
ONE	209	PF 3	PPFNL	3100.00	6749.66	6756.83	6754.58	6757.99	0.006893	8.63	359.28	50.16	0.57
ONE	209	PF 3	PPFP	3100.00	6749.66	6756.83	6754.58	6757.99	0.006893	8.63	359.28	50.16	0.57
ONE	208	PF 3	PPFNL	3100.00	6750.00	6756.89	6755.38	6757.60	0.006520	6.81	455.35	110.13	0.59
ONE	208	PF 3	PPFP	3100.00	6750.00	6756.89	6755.38	6757.60	0.006520	6.81	455.35	110.13	0.59
ONE	207	PF 3	PPFNL	3100.00	6750.00	6754.29	6754.29	6756.06	0.016794	11.29	303.75	88.72	0.98
ONE	207	PF 3	PPFP	3100.00	6750.00	6754.29	6754.29	6756.06	0.016794	11.29	303.75	88.72	0.98
ONE	206	PF 3	PPFNL	3100.00	6748.00	6753.52	6752.93	6754.88	0.012785	9.37	331.00	81.89	0.82
ONE	206	PF 3	PPFP	3100.00	6748.00	6753.52	6752.93	6754.88	0.012785	9.37	331.00	81.89	0.82
ONE	205	PF 3	PPFNL	3100.00	6748.00	6753.61	6752.36	6754.62	0.008100	8.06	384.43	84.04	0.66
ONE	205	PF 3	PPFP	3100.00	6748.00	6753.61	6752.36	6754.62	0.008100	8.06	384.43	84.04	0.66
ONE	204	PF 3	PPFNL	3100.00	6747.00	6753.58	6752.12	6754.44	0.007068	7.47	414.95	92.29	0.62
ONE	204	PF 3	PPFP	3100.00	6747.00	6753.58	6752.12	6754.44	0.007068	7.47	414.95	92.29	0.62
ONE	203	PF 3	PPFNL	3100.00	6747.00	6753.37	6752.20	6754.29	0.008460	7.68	403.61	99.26	0.67
ONE	203	PF 3	PPFP	3100.00	6747.00	6753.37	6752.20	6754.29	0.008460	7.68	403.61	99.26	0.67

DRAINAGE MAPS

Berm at East boundary

Project Description

Worksheet	Triangular Channel - 1
Flow Element	Triangular Channel
Method	Manning's Formula
Solve For	Channel Depth

Input Data

Mannings Coefficient	0.035
Slope	2.00 %
Left Side Slope	4.00 H : V
Right Side Slope	50.00 H : V
Discharge	20.00 cfs

Results

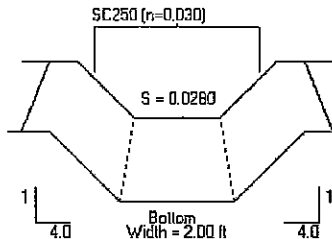
Depth	0.54 ft
Flow Area	8.0 ft ²
Wetted Perimeter	29.39 ft
Top Width	29.32 ft
Critical Depth	0.51 ft
Critical Slope	2.83 %
Velocity	2.51 ft/s
Velocity Head	0.10 ft
Specific Energy	0.64 ft
Froude Number	0.85
Flow Type	Subcriti cal

Swale A-A
SC250 Fabric Lined

North American Green - ECMD5 Version 4.3 8/2/2006 10:39 AM COMPUTED BY: Quentin Amijo
 PROJECT NAME: Quail Brush Creek PROJECT NO.: 0537.00
 FROM STATION/REACH: Swale B-B TO STATION/REACH: Swale B-B DRAINAGE AREA: DESIGN FREQUENCY: 100-year

HYDRAULIC RESULTS

Discharge (cfs)	Peak Flow Period (hrs)	Velocity (fps)	Area (sq.ft)	Hydraulic Radius(ft)	Normal Depth (ft)
80.0	2.0	7.17	11.16	0.80	1.44



LINER RESULTS

Not to Scale

Reach	Matting Type	Stability Analysis	Vegetation Characteristics				Permissible Shear Stress (psf)	Calculated Shear Stress (psf)	Safety Factor	Remarks
	Staple Pattern		Phase	Class	Type	Density				
Straight	SC250	Vegetation	2	C	Mix	50-75%	8.00	2.51	3.10	STABLE
	Staple E	Soil	Sandy Loam				2.500	0.533	4.69	STABLE

///

Back to Input Screen

KIOWA ENGINEERING HEC-1 ANALYSIS

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1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* JUN 1998
* VERSION 4.1
*
* RUN DATE 11NOV05 TIME 08:16:07
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID Sand Creek Detention Basin No. 2 Final design
2 ID FUTURE DEVELOPMENT CONDITION WITH SAND CREEK PER FINAL DESIGN
3 ID 10-YEAR AND 100-YEAR 24-HOUR STORM FN:SC2-FD.DAT
*DIAGRAM
4 IT 5 0 0 300
5 IO 5 0
6 JR PREC .68 1.0
7 KK SB82
8 KM RUNOFF FROM SUB-BASIN 82
9 BA .190
10 IN 30
11 PB 4.4
12 EC 0.0000 0.0025 0.0050 0.0075 0.0100 0.0150 0.0200 0.0250 0.0300 0.0500

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REVISED CHS
 STATION 23, 25+26


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90      KM  RUNOFF FROM SUB-BASIN 77
91      BA   .25
92      LS   0      67
93      UD   .336

94      KK   SB88
95      KM  RUNOFF FROM SUB-BASIN 88
96      BA   .1000
97      LS   0      67
98      UD   .204

99      KK   DP77
100     KM  DESIGN POINT 77 COMBINE RUNOFF FROM SUB-BASINS 77 AND 88 AND RT-6, AND RT-5
101     HC   4

102     KK   RT-8
103     KM  ROUTE FLOW FROM DESIGN POINT 77 TO DP 71
104     RK  3850   .025   .035           TRAP      20      10

105     KK   SB71
106     KM  RUNOFF FROM SUB-BASIN 71
107     BA   .36
108     LS   0      70
109     UD   .300

110     KK   DP71
111     KM  DESIGN POINT 71 COMBINE RUNOFF FROM SUB-BASIN 71 AND RT-8
112     HC   2

113     KK   RT-9
114     KM  ROUTE FLOW FROM DESIGN POINT 71 TO DP 70
115     RK  2500   .025   .035           TRAP      20      4

116     KK   SB70
117     KM  RUNOFF FROM SUB-BASIN 70
118     BA   .3100
119     LS   0      70
120     UD   .350

121     KK   DP70
122     KM  DESIGN POINT 70 COMBINE RUNOFF FROM SUB-BASIN 70 AND RT-9
123     HC   2

124     KK   RT-11
125     KM  ROUTE FLOW FROM DESIGN POINT 70 TO DP 87
126     RK  1200   .025   .035           TRAP      20      4
                                     HEC-1 INPUT

127     KK   SB72

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1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

128	KM	RUNOFF FROM SUB-BASIN 72				
129	BA	.2500				
130	LS	0	70			
131	UD	.138				
132	KK	RT-10				
133	KM	ROUTE FLOW FROM SUB-BASIN 72 TO DESIGN POINT 70				
134	RK	3000	.035	.035	TRAP 10	4
135	KK	SB69				
136	KM	RUNOFF FROM SUB-BASIN 69				
137	BA	.2500				
138	LS	0	75			
139	UD	.483				
140	KK	DP69				
141	KM	DESIGN POINT 69 COMBINE RUNOFF FROM SUB-BASIN 69 AND RT-10				
142	HC	2				
143	KK	RT-12				
144	KM	ROUTE FLOW FROM DESIGN POINT 69 TO DP 87				
145	RK	1400	.025	.035	TRAP 20	4
146	KK	SC3-15				
147	KM	RUNOFF FROM SUB-BASIN SC3-15				
148	BA	.0730				
149	LS	0	72.0			
150	UD	.230				
151	KK	DP87				
152	KM	DESIGN POINT 87 COMBINE RUNOFF FROM SUB-BASIN SC3-15, RT-12 AND RT-11				
153	HC	3				
154	KK	RT-13				
155	KM	ROUTE FLOW FROM DESIGN POINT 87 TO DP 63				
156	RK	4400	.025	.035	TRAP 40	4
157	KK	SC3-12				
158	KM	RUNOFF FROM SUB-BASIN SC3-12				
159	BA	.2380				
160	LS	0	70.6			
161	UD	.410				
162	KK	DP63				
163	KM	DESIGN POINT 63 COMBINE RUNOFF FROM SUB-BASIN SC3-12 AND RT-13				
164	HC	2				
165	KK	RT-17				
166	KM	ROUTE FLOW FROM DESIGN POINT 63 TO DP 60				
167	RK	4390	.015	.035	TRAP 10	4

HEC-1 INPUT

207 KM ROUTE FLOW FROM DP 60 TO DESIGN POINT 53
 208 RK 2690 .015 .035 TRAP 10 4
 HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

209 KK SC3-2
 210 KM RUNOFF FROM SUB-BASIN SC3-2
 211 BA .1920
 212 LS 0 76.1
 213 UD .310

214 KK SC3-11
 215 KM RUNOFF FROM SUB-BASIN SC3-11
 216 BA .2750
 217 LS 0 80.8
 218 UD .430

219 KK RT-19
 220 KM ROUTE FLOW FROM SUB-BASIN SC3-11 TO DESIGN POINT 55
 221 RK 2600 .022 .035 TRAP 10 6

222 KK SC3-10
 223 KM RUNOFF FROM SUB-BASIN SC3-10
 224 BA .1130
 225 LS 0 75.0
 226 UD .290

227 KK DP55
 228 KM DESIGN POINT 55 COMBINE RUNOFF FROM SUB-BASIN SC3-10 AND RT-19
 229 HC 2

230 KK RT-20
 231 KM ROUTE FLOW FROM DESIGN POINT 55 TO DESIGN POINT 56
 232 RK 3670 .012 .035 TRAP 10 10

233 KK SC3-1
 234 KM RUNOFF FROM SUB-BASIN SC3-1
 235 BA .1290
 236 LS 0 80.0
 237 UD .320

238 KK DP56
 239 KM DESIGN POINT 56 COMBINE RUNOFF FROM SUB-BASIN SC3-1 AND RT-20
 240 HC 2

241 KK DB95
 242 KM ROUTE FLOW FROM DP 56 THROUGH DBPS STRUCTURE 95
 243 RS 1 ELEV 100
 244 SV 0 20 34 39 42 50
 245 SQ 0 10 20 30 40 43

246 SE 100 104 107 108 109 110
 247 KK RT-21
 248 KM ROUTE FLOW FROM DETENTION BASIN DB95 TO DESIGN POINT 53
 249 RK 870 .023 .035 TRAP 10 10
 HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

250 KK DP53
 251 KM DESIGN POINT 53 COMBINE RUNOFF FROM SUB-BASIN SC3-2, RT-21 AND RT-18
 252 HC 3

253 KK DB94
 254 KM ROUTE FLOW FROM DP 53 THROUGH DBFS STRUCTURE 94
 255 RS 1 ELEV 100
 256 SV 0 30 40 60 80 120 130
 257 SQ 0 80 90 300 800 1750 2000
 258 SE 100 104 105 107 108 109 110

259 KK RT-27
 260 KM ROUTE FLOW FROM DETETNION BASIN DB94 TO DESIGN POINT 50
 261 RK 1380 .022 .035 TRAP 10 10

262 KK SC2-24
 263 KM RUNOFF FROM SUB-BASIN SC2-24
 264 BA .0660
 265 LS 0 90.7
 266 UD .190

267 KK RT-29
 268 KM ROUTE FLOW FROM SUB-BASIN SC2-24 TO DESIGN POINT 50
 269 RK 330 .020 .013 CIRC 4

270 KK SC2-27
 271 KM RUNOFF FROM SUB-BASIN SC2-27
 272 BA .0780
 273 LS 0 92.0
 274 UD .210

275 KK DP50
 276 KM DESIGN POINT 50 COMBINE RUNOFF FROM SC2-27, RT-27 AND RT-29
 277 HC 3

278 KK RT-31
 279 KM ROUTE FLOW FROM DESIGN POINT 50 TO DESIGN POINT 49
 280 RK 940 .020 .035 TRAP 40 6

281 KK SC3-8
 282 KM RUNOFF FROM SUB-BASIN SC3-8
 283 BA .2420

284 LS 0 67.0
 285 UD .290

 286 KK RT-25
 287 KM ROUTE FLOW FROM SUB-BASIN SC3-8 TO DESIGN POINT 51
 288 RK 2690 .015 .035 TRAP 20 6
 HEC-1 INPUT

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

289 KK SC3-3
 290 KM RUNOFF FROM SUB-BASIN SC3-3
 291 BA .1270
 292 LS 0 88.5
 293 UD .280

 294 KK DP51
 295 KM DESIGN POINT 51 COMBINE RUNOFF FROM SC3-3 AND RT-25
 296 HC 2

 297 KK DB93
 298 KM ROUTE FLOW FROM DP 51 THROUGH DBPS STRUCTURE 93
 299 RS 1 ELEV 100
 300 SV 0 10 20 30
 301 SQ 0 20 100 290
 302 SE 100 104 106 108

 303 KK RT-30
 304 KM ROUTE FLOW FROM DETENTION BASIN DB93 TO DESIGN POINT 49
 305 RK 1590 .019 .035 TRAP 20 10

 306 KK DP49
 307 KM DESIGN POINT 49 COMBINE RUNOFF FROM RT-31 AND RT-30
 308 HC 2

 309 KK RT-32
 310 KM ROUTE FLOW FROM DESIGN POINT 49 TO DESIGN POINT 48
 311 RK 4930 .017 .035 TRAP 40 6

 312 KK SC3-6
 313 KM RUNOFF FROM SUB-BASIN SC3-6
 314 BA .2420
 315 LS 0 70.0
 316 UD .310

 317 KK RT-23
 318 KM ROUTE FLOW FROM SUB-BASIN SC3-6 TO DESIGN POINT 54
 319 RK 1830 .023 .035 TRAP 10 10

 320 KK SC3-5
 321 KM RUNOFF FROM SUB-BASIN SC3-5

322 BA .1150
 323 LS 0 88.7
 324 UD .280

 325 KK RT-24
 326 KM ROUTE FLOW FROM SUB-BASIN SC3-5 TO DESIGN POINT 54
 327 RK 1760 .041 .035 TRAP 10 4

HEC-1 INPUT

PAGE 9

1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

328 KK SC3-7
 329 KM RUNOFF FROM SUB-BASIN SC3-7
 330 BA .5380
 331 LS 0 81.5
 332 UD .530

 333 KK RT-22
 334 KM ROUTE FLOW FROM SUB-BASIN SC3-7 TO DESIGN POINT 54
 335 RK 2860 .021 .035 TRAP 30 6

 336 KK SC3-4
 337 KM RUNOFF FROM SUB-BASIN SC3-4
 338 BA .3210
 339 LS 0 81.9
 340 UD .330

 341 KK DP54
 342 KM DESIGN POINT 54 COMBINE RUNOFF FROM SUB-BASIN SC3-4, RT-22, RT-23 AND RT-24
 343 HC 4

 344 KK DB92
 345 KM ROUTE FLOW FROM DP 54 THROUGH DBPS STRUCTURE 92
 346 RS 1 ELEV 100
 347 SV 0 40 60 90 110 120 130
 348 SQ 0 40 60 82 90 91 95
 349 SE 100 104 105 106 108 109 110

 350 KK RT-26
 351 KM ROUTE FLOW FROM DETENTION BASIN 92 TO DESIGN POINT 47
 352 RK 4680 .028 .035 TRAP 40 10

 353 KK SC2-26
 354 KM RUNOFF FROM SUB-BASIN SC2-26
 355 BA .1860
 356 LS 0 79
 357 UD .320

 358 KK RT-34
 359 KM ROUTE FLOW FROM SUB-BASIN SC2-26 TO DESIGN POINT 47
 360 RK 1620 .014 .035 TRAP 4 4

399 LS 0 79.0
 400 UD .220

 401 KK DP46
 402 KM DESIGN POINT 46 COMBINE RUNOFF FROM SUB-BASIN SC2-22 AND RT-35
 403 HC 2

 404 KK RT-36
 405 KM ROUTE FLOW FROM DESIGN POINT 46 TO DESIGN POINT 42
 406 RK 1790 .017 .013 TRAP 30 4

 407 KK DP42
 408 KM DESIGN POINT 42 COMBINE RUNOFF FROM RT- 37 AND RT-36
 409 HC 2

1

HEC-1 INPUT

PAGE 11

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10
410	KK	RT-38									
411	KM	ROUTE FLOW FROM DESIGN POINT 42 TO DESIGN POINT 85									
412	RK	1790	.013	.035	TRAP	60	6				
413	KK	SC2-20									
414	KM	RUNOFF FROM SUB-BASIN SC2-20									
415	BA	.3240									
416	LS	0	79.0								
417	UD	.380									
418	KK	RT-41									
419	KM	ROUTE FLOW FROM SUB-BASIN SC2-20 TO DESIGN POINT 85									
420	RK	1560	.020	.013	CIRC	6.5					
421	KK	SC2-15									
422	KM	RUNOFF FROM SUB-BASIN SC2-15									
423	BA	.1810									
424	LS	0	79.0								
425	UD	.240									
426	KK	RT-39									
427	KM	ROUTE FLOW FROM SUB-BASIN SC2-15 TO DESIGN POINT 85									
428	RK	990	.022	.013	CIRC	6.5					
429	KK	SC2-13									
430	KM	RUNOFF FROM SUB-BASIN SC2-13									
431	BA	.1630									
432	LS	0	79.0								
433	UD	.280									
434	KK	RT-40									
435	KM	ROUTE FLOW FROM SUB-BASIN SC2-13 TO DESIGN POINT 85									
436	RK	300	.033	.013	CIRC	5.5					

437 KK DP85
 438 KM DESIGN POINT 85 COMBINE RUNOFF FROM RT-41, RT-38, RT-39 AND RT-40
 439 HC 4

 440 KK RT-42
 441 KM ROUTE FLOW FROM DESIGN POINT 85 TO DESIGN POINT 37
 442 RK 2900 .016 .035 TRAP 60 6

 443 KK SC2-14
 444 KM RUNOFF FROM SUB-BASIN SC2-14
 445 BA .2650
 446 LS 0 76.2
 447 UD .450

 448 KK SC2-16
 449 KM RUNOFF FROM SUB-BASIN SC2-16
 450 BA .2120
 451 LS 0 76.2
 452 UD .310

HEC-1 INPUT

1

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
453	KK DP37
454	KM DESIGN POINT 37 COMBINE RUNOFF FROM RT-42, SUB-BASINS SC2-14 AND SC2-16
455	HC 3
456	KK RT-43
457	KM ROUTE FLOW FROM DESIGN POINT 37 TO DESIGN POINT 36
458	RK 3380 .016 .035 TRAP 60 4
459	KK SC2-12
460	KM RUNOFF FROM SUB-BASIN SC2-12
461	BA .1790
462	LS 0 70.6
463	UD .280
464	KK RT-46
465	KM ROUTE FLOW FROM SUB-BASIN SC2-12 TO DESIGN POINT 33
466	RK 2280 .024 .035 TRAP 20 4
467	KK SC2-8
468	KM RUNOFF FROM SUB-BASIN SC2-8
469	BA .3480
470	LS 0 79.0
471	UD .440
472	KK DP33
473	KM DESIGN POINT 33 COMBINE RUNOFF FROM RT-46 AND SUB-BASIN SC2-8
474	HC 2

475 KK RT-45
 476 KM ROUTE FLOW FROM DESIGN POINT 33 TO DESIGN POINT 36
 477 RK 850 .021 .035 TRAP 20 4

478 KK DP36
 479 KM DESIGN POINT 36 COMBINE RUNOFF FROM RT-45 AND RT-43
 480 HC 2

481 KK RT-44
 482 KM ROUTE FLOW FROM DESIGN POINT 36 TO DESIGN POINT 35
 483 RK 1740 .016 .035 TRAP 60 4

484 KK SC2-11
 485 KM RUNOFF FROM SUB-BASIN SC2-11
 486 BA .2100
 487 LS 0 79.0
 488 UD .240

489 KK RT-52
 490 KM ROUTE FLOW FROM SUB-BASIN SC2-11 TO DESIGN POINT 39
 491 RK 2680 .008 .035 TRAP 30 4

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HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

492 KK SC2-19
 493 KM RUNOFF FROM SUB-BASIN SC2-19
 494 BA .1820
 495 LS 0 86.0
 496 UD .300

497 KK RT-51
 498 KM ROUTE FLOW FROM SUB-BASIN SC2-19 TO DESIGN POINT 39
 499 RK 3100 .022 .013 CIRC 6

500 KK SC2-18
 501 KM RUNOFF FROM SUB-BASIN SC2-18
 502 BA .1980
 503 LS 0 88.0
 504 UD .290

505 KK DP39
 506 KM DESIGN POINT 39 COMBINE RUNOFF FROM RT-52, RT-51 AND SUB-BASIN SC2-18
 507 HC 3

508 KK RT-49
 509 KM ROUTE FLOW FROM DESIGN POINT 39 TO DESIGN POINT 34
 510 RK 1160 .024 .035 TRAP 20 4

511 KK SC2-17

512 KM RUNOFF FROM SUB-BASIN SC2-17
 513 BA .1600
 514 LS 0 79.0
 515 UD .320

 516 KK RT-50
 517 KM ROUTE FLOW FROM SUB-BASIN SC2-17 TO DESIGN POINT 34
 518 RK 1840 .025 .035 TRAP 20 4

 519 KK SC2-29
 520 KM RUNOFF FROM SUB-BASIN SC2-29
 521 BA .1510
 522 LS 0 88.0
 523 UD .210

 524 KK DP34
 525 KM DESIGN POINT 34 COMBINE RUNOFF FROM RT-49, RT-50 AND SUB-BASIN SC2-29
 526 HC 3

 527 KK RT-48
 528 KM ROUTE FLOW FROM DESIGN POINT 34 TO DESIGN POINT 32
 529 RK 1470 .020 .04 TRAP 20 4

 530 KK SC2-10
 531 KM RUNOFF FROM SUB-BASIN SC2-10
 532 BA .3290
 533 LS 0 78.4
 534 UD .390

1

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

 535 KK DP32
 536 KM DESIGN POINT 32 COMBINE RUNOFF FROM RT-48 AND SUB-BASIN SC2-10
 537 HC 2

 538 KK RT-47
 539 KM ROUTE FLOW FROM SUB-BASIN DESIGN POINT 32 TO DESIGN POINT 35
 540 RK 991 .028 .031 TRAP 20 4

 541 KK SC2-9
 542 KM RUNOFF FROM SUB-BASIN SC2-9
 543 BA .2060
 544 LS 0 73.0
 545 UD .520

 546 KK DP35
 547 KM DESIGN POINT 35 COMBINE RUNOFF FROM RT-47, RT-44 AND SUB-BASIN SC2-9
 548 HC 3

 549 KK RT-53


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550      KM  ROUTE FLOW FROM DESIGN POINT 35 TO DESIGN POINT 29
551      RK  1120   .020   .035           TRAP    60    6

552      KK  SC2-7
553      KM  RUNOFF FROM SUB-BASIN SC2-7
554      BA  .1440
555      LS  0     79.0
556      UD  .320

557      KK  DP29
558      KM  DESIGN POINT 29 COMBINE RUNOFF FROM RT-53 AND SUB-BASIN SC2-7
559      HC  2

560      KK  RT-54
561      KM  ROUTE FLOW FROM DESIGN POINT 29 TO DESIGN POINT 28
562      RK  1560   .011   .035           TRAP    60    4

563      KK  SC2-6
564      KM  RUNOFF FROM SUB-BASIN SC2-6
565      BA  .1500
566      LS  0     81.6
567      UD  .240

568      KK  RT-55
569      KM  ROUTE FLOW FROM SUB-BASIN SC2-6 TO DESIGN POINT 28
570      RD  950   .020   .035           TRAP    10    4

571      KK  SC2-2
572      KM  RUNOFF FROM SUB-BASIN SC2-2
573      BA  .1670
574      LS  0     80.8
575      UD  .280

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HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

576      KK  DP28
577      KM  DESIGN POINT 28 COMBINE RUNOFF FROM RT-54, RT-55 AND SUB-BASIN SC2-2
578      HC  3

579      KK  RT-56
580      KM  ROUTE FLOW FROM DESIGN POINT 28 TO DESIGN POINT 21
581      RK  1280   .017   .035           TRAP    60    4

582      KK  SC2-5
583      KM  RUNOFF FROM SUB-BASIN SC2-5
584      BA  .1610
585      LS  0     92.0
586      UD  .280

587      KK  RT-58

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588 KM ROUTE FLOW FROM SUB-BASIN SC2-5 TO DESIGN POINT 21
589 RK 1670 .027 .024 CIRC 5

590 KK SC2-0
591 KM RUNOFF FROM SUB-BASIN SC2-0
592 BA .1500
593 LS 0 73.4
594 UD .320

595 KK RT-57
596 KM ROUTE FLOW FROM SUB-BASIN SC2-0 TO DESIGN POINT 21
597 RK 800 .032 .035 TRAP 10 4

598 KK DP21
599 KM DESIGN POINT 21 COMBINE RUNOFF FROM RT-58, RT-56 AND RT- 57
600 HC 3

601 KK RT-59
602 KM ROUTE FLOW FROM DESIGN POINT 21 TO DESIGN POINT 20
603 RK 830 .01 .035 TRAP 80 4

604 KK SC2-4
605 KM RUNOFF FROM SUB-BASIN SC2-4
606 BA .2850
607 LS 0 92.0
608 UD .320

609 KK RT-61
610 KM ROUTE FLOW FROM SUB-BASIN SC2-4 TO DESIGN POINT 22
611 RK 1670 .019 .035 TRAP 20 3

612 KK SC2-3
613 KM RUNOFF FROM SUB-BASIN SC2-3
614 BA .0710
615 LS 0 92.0
616 UD .210

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

617 KK DP22
618 KM DESIGN POINT 22 COMBINE RUNOFF FROM RT-61 AND SC2-3
619 HC 2

620 KK RT-60
621 KM ROUTE FLOW FROM DESIGN POINT 22 TO DESIGN POINT 20
622 RK 1010 .018 .035 TRAP 20 3

623 KK SC2-1
624 KM RUNOFF FROM SUB-BASIN SC2-1
625 BA .1850

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626      LS      0      80.0
627      UD      .290

628      KK      DP20
629      KM      DESIGN POINT 20 COMBINE RUNOFF FROM RT-60, RT-59 AND SB SC2-1
630      HC      3

631      KK      DBSC2
632      KM      ROUTE FLOW FROM DP 20 THROUGH SAND CREEK DETENTION BASIN DBSC2
633      RS      1      ELEV 6496.5
634      SV      0      6.75      29.8      58.8      92.2      129.4      168.6      209.8      255.0      302.2
635      SQ      0      10      30      100      400      800      2000      4300      5000      7000
636      SE      6496.5      6498      6500      6502      6504      6506      6508      6510      6512      6514

637      KK      RT-63
638      KM      ROUTE FLOW FROM DETENTION BASIN SC2 TO DESIGN POINT 18
639      RK      1290      .007      .035      TRAP      100      3

640      KK      SC1-10
641      KM      RUNOFF FROM SUB-BASIN SC1-10
642      BA      .2880
643      LS      0      78.2
644      UD      .390

645      KK      DP18
646      KM      DESIGN POINT 18 COMBINE RUNOFF FROM RT-63 AND SUB-BASIN SC1-10
647      HC      2

648      KK      RT-66
649      KM      ROUTE FLOW FROM DESIGN POINT 18 TO DESIGN POINT 17
650      RK      320      .007      .035      TRAP      100      3

651      KK      SC1-13
652      KM      RUNOFF FROM SUB-BASIN SC1-13
653      BA      .1300
654      LS      0      87.3
655      UD      .270

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HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

656      KK      RT-64
657      KM      ROUTE FLOW FROM SUB-BASIN SC1-13 TO DESIGN POINT 19
658      RK      1970      .026      .013      CIRC      4.5

659      KK      SC1-14
660      KM      RUNOFF FROM SUB-BASIN SC1-14
661      BA      .1100
662      LS      0      92.0
663      UD      .280

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664      KK      DP19
665      KM      DESIGN POINT 19 COMBINE RUNOFF FROM RT-64 AND SUB-BASIN SC1-14
666      HC          2

667      KK      RT-65
668      KM      ROUTE FLOW FROM DESIGN POINT 19 TO DESIGN POINT 17
669      RK      700      .022      .013      TRAP      10      .1

670      KK      DP17
671      KM      DESIGN POINT 17 COMBINE RUNOFF FROM RT-65 AND RT-66
672      HC          2

673      KK      RT-67
674      KM      ROUTE FLOW FROM DESIGN POINT 17 TO DESIGN POINT 13
675      RK      910      .007      .035      TRAP      100      3

676      KK      SC1-9
677      KM      RUNOFF FROM SUB-BASIN SC1-9
678      BA      .2400
679      LS      0      76.9
680      UD      .240

681      KK      DBSR29
682      KM      ROUTE FLOW FROM SB SC1-9 THROUGH SPRINGS RANCH DETENTION BASIN DBSR29
683      RS      1      ELEV      100
684      SV      0      .03      0.47      1.20      2.60      6.70      8.8      10.7
685      SQ      0      .2      7.8      22      40      73      97      113
686      SE      100      100.5      101.5      102.5      103.5      104.5      105.5      106.5

687      KK      RT-70
688      KM      ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR29 TO DESIGN POINT 14
689      RD      2750      .013      .013      CIRC      5.5

690      KK      SC1-8
691      KM      RUNOFF FROM SUB-BASIN SC1-8
692      BA      .2100
693      LS      0      79.5
694      UD      .320

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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695      KK      RT-71
696      KM      ROUTE FLOW FROM SUB-BASIN SC1-8 TO DESIGN POINT 14
697      RK      600      .010      .013      CIRC      5

698      KK      DP14
699      KM      DESIGN POINT 14 COMBINE RUNOFF FROM RT-71 AND RT-70
700      HC          2

701      KK      RT-69

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702      KM  ROUTE FLOW FROM DESIGN POINT 14 TO DESIGN POINT 12
703      RK  1550  .015  .013          TRAP      7      .1

704      KK  SC1-7
705      KM  RUNOFF FROM SUB-BASIN SC1-7
706      BA  .0960
707      LS  0      80.1
708      UD  .230

709      KK  DP12
710      KM  DESIGN POINT 12 COMBINE RUNOFF FROM SUB-BASIN SC1-7 AND RT-69
711      HC  2

712      KK  RT-68
713      KM  ROUTE FLOW FROM DESIGN POINT 12 TO DESIGN POINT 13
714      RK  390   .025  .013          TRAP      7      .1

715      KK  DP13
716      KM  DESIGN POINT 13 COMBINE RUNOFF FROM RT-67 AND RT-68
717      HC  2

718      KK  RT-72
719      KM  ROUTE FLOW FROM DESIGN POINT 13 TO DESIGN POINT 10
720      RK  1150  .007  .04          TRAP     100     3

721      KK  SC1-6
722      KM  RUNOFF FROM SUB-BASIN SC1-6
723      BA  .1590
724      LS  0      80.6
725      UD  .220

726      KK  DBSR15
727      KM  ROUTE FLOW FROM SB SC1-6 THROUGH SPRINGS RANCH DETENTION BASIN DBSR15
728      RS  1      ELEV    100
729      SV  0      1.50    3.80    6.50    9.70    13.80
730      SQ  0      80      125    150    180     210
731      SE  100    102     104    106    108     110

732      KK  RT-74
733      KM  ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR15 TO DESIGN POINT 11
734      RK  2080  .021  .013          CIRC     3.5

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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735      KK  SC1-5
736      KM  RUNOFF FROM SUB-BASIN SC1-5
737      BA  .1730
738      LS  0      79.0
739      UD  .310

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740	KK	DP11							
741	KM	DESIGN POINT 11 COMBINE RUNOFF FROM SUB-BASIN SC1-5 AND RT-74							
742	HC	2							
743	KK	RT-73							
744	KM	ROUTE FLOW FROM DESIGN POINT 11 TO DESIGN POINT 10							
745	RK	580 .024 .013 CIRC 3.5							
746	KK	DP10							
747	KM	DESIGN POINT 10 COMBINE RUNOFF FROM RT-72 AND RT-73							
748	HC	2							
749	KK	RT-75							
750	KM	ROUTE FLOW FROM DESIGN POINT 10 TO DESIGN POINT 9							
751	RK	2270 .007 .035 TRAP 100 3							
752	KK	SC1-2							
753	KM	RUNOFF FROM SUB-BASIN SC1-2							
754	BA	.1360							
755	LS	0 79.0							
756	UD	.320							
757	KK	RT-76							
758	KM	ROUTE FLOW FROM SUB-BASIN SC1-2 TO DESIGN POINT 9							
759	RD	500 .039 .013 CIRC 3							
760	KK	DP9							
761	KM	DESIGN POINT 9 COMBINE RUNOFF FROM RT-75 AND RT-76							
762	HC	2							
763	KK	RT-78							
764	KM	ROUTE FLOW FROM DESIGN POINT 9 TO DESIGN POINT 4							
765	RK	1530 .007 .035 TRAP 100 3							
766	KK	SC1-12							
767	KM	RUNOFF FROM SUB-BASIN SC1-12							
768	BA	.2480							
769	LS	0 84.0							
770	UD	.370							
771	KK	RT-81							
772	KM	ROUTE FLOW FROM SUB-BASIN SC1-12 TO DESIGN POINT 8							
773	RK	1400 .008 .035 TRAP 30 4							

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

774	KK	SC1-11							
775	KM	RUNOFF FROM SUB-BASIN SC1-11							
776	BA	.1600							
777	LS	0 92.0							

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
 819 ZZ

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 * *
 809 KK * DBSC1 *
 * *

811 KO OUTPUT CONTROL VARIABLES
 IPRNT 5 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	RATIO 2
				.68	1.00
HYDROGRAPH AT					
+ SB82	.19	1	FLOW	84.	232.
			TIME	6.50	6.50
ROUTED TO					
+ RT-4	.19	1	FLOW	83.	220.
			TIME	6.58	6.58
HYDROGRAPH AT					
+ SB74	.18	1	FLOW	49.	143.
			TIME	6.67	6.67
2 COMBINED AT					
+ DP74	.37	1	FLOW	122.	345.
			TIME	6.58	6.58
ROUTED TO					
+ RT-3	.37	1	FLOW	119.	331.
			TIME	6.67	6.67
HYDROGRAPH AT					

+	SB80	.22	1	FLOW TIME	68. 6.67	190. 6.67
ROUTED TO						
+	RT-2	.22	1	FLOW TIME	67. 6.75	185. 6.75
HYDROGRAPH AT						
+	SB81	.39	1	FLOW TIME	100. 6.67	298. 6.67
ROUTED TO						
+	RT-1	.39	1	FLOW TIME	99. 6.83	296. 6.75
HYDROGRAPH AT						
+	SB73	.12	1	FLOW TIME	31. 6.75	90. 6.67
HYDROGRAPH AT						
+	SB75	.12	1	FLOW TIME	25. 6.83	72. 6.75
HYDROGRAPH AT						
+	SB76	.17	1	FLOW TIME	35. 6.83	105. 6.75
6 COMBINED AT						
+	DP75	1.39	1	FLOW TIME	364. 6.75	1022. 6.75
ROUTED TO						
+	RT-5	1.39	1	FLOW TIME	345. 6.75	1021. 6.75
HYDROGRAPH AT						
+	SB79	.29	1	FLOW TIME	111. 6.58	306. 6.58
ROUTED TO						
+	RT-7	.29	1	FLOW TIME	107. 6.67	281. 6.58
HYDROGRAPH AT						
+	SB78	.88	1	FLOW TIME	259. 6.67	738. 6.67
2 COMBINED AT						
+	DP78	1.17	1	FLOW TIME	367. 6.67	1020. 6.67
ROUTED TO						

+	RT-6	1.17	1	FLOW TIME	351. 6.75	993. 6.67
HYDROGRAPH AT						
+	SB77	.25	1	FLOW TIME	61. 6.75	177. 6.75
HYDROGRAPH AT						
+	SB88	.10	1	FLOW TIME	34. 6.58	97. 6.58
4 COMBINED AT						
+	DP77	2.91	1	FLOW TIME	780. 6.75	2207. 6.67
ROUTED TO						
+	RT-8	2.91	1	FLOW TIME	761. 6.83	2201. 6.75
HYDROGRAPH AT						
+	SB71	.36	1	FLOW TIME	127. 6.67	334. 6.67
2 COMBINED AT						
+	DP71	3.27	1	FLOW TIME	861. 6.83	2508. 6.75
ROUTED TO						
+	RT-9	3.27	1	FLOW TIME	842. 6.83	2445. 6.75
HYDROGRAPH AT						
+	SB70	.31	1	FLOW TIME	99. 6.75	256. 6.75
2 COMBINED AT						
+	DP70	3.58	1	FLOW TIME	931. 6.83	2701. 6.75
ROUTED TO						
+	RT-11	3.58	1	FLOW TIME	905. 6.83	2653. 6.75
HYDROGRAPH AT						
+	SB72	.25	1	FLOW TIME	135. 6.58	321. 6.50
ROUTED TO						
+	RT-10	.25	1	FLOW TIME	128. 6.58	321. 6.58
HYDROGRAPH AT						

+	SB69	.25	1	FLOW TIME	93. 6.83	216. 6.83
2 COMBINED AT						
+	DP69	.50	1	FLOW TIME	191. 6.67	453. 6.58
ROUTED TO						
+	RT-12	.50	1	FLOW TIME	187. 6.67	446. 6.67
HYDROGRAPH AT						
+	SC3-15	.07	1	FLOW TIME	35. 6.58	87. 6.58
3 COMBINED AT						
+	DP87	4.15	1	FLOW TIME	1068. 6.83	3104. 6.75
ROUTED TO						
+	RT-13	4.15	1	FLOW TIME	1048. 6.92	3020. 6.83
HYDROGRAPH AT						
+	SC3-12	.24	1	FLOW TIME	70. 6.83	183. 6.75
2 COMBINED AT						
+	DP63	4.39	1	FLOW TIME	1111. 6.92	3198. 6.83
ROUTED TO						
+	RT-17	4.39	1	FLOW TIME	1083. 7.00	3152. 6.83
HYDROGRAPH AT						
+	SB68	.22	1	FLOW TIME	97. 6.75	222. 6.75
ROUTED TO						
+	RT-14	.22	1	FLOW TIME	95. 6.83	220. 6.83
HYDROGRAPH AT						
+	SC3-14	.31	1	FLOW TIME	88. 6.83	234. 6.75
2 COMBINED AT						
+	DP67	.53	1	FLOW TIME	183. 6.83	448. 6.83
ROUTED TO						

+	RT-15	.53	1	FLOW TIME	178. 6.92	446. 6.83
HYDROGRAPH AT						
+	SC3-13	.25	1	FLOW TIME	174. 6.75	346. 6.75
2 COMBINED AT						
+	DP64	.79	1	FLOW TIME	338. 6.83	763. 6.75
ROUTED TO						
+	RT-16	.79	1	FLOW TIME	332. 6.92	758. 6.83
HYDROGRAPH AT						
+	SC3-9	.24	1	FLOW TIME	123. 6.67	279. 6.67
3 COMBINED AT						
+	DP60	5.42	1	FLOW TIME	1462. 6.92	4126. 6.83
ROUTED TO						
+	RT-18	5.42	1	FLOW TIME	1456. 7.00	4013. 6.92
HYDROGRAPH AT						
+	SC3-2	.19	1	FLOW TIME	108. 6.67	239. 6.67
HYDROGRAPH AT						
+	SC3-11	.28	1	FLOW TIME	167. 6.75	341. 6.75
ROUTED TO						
+	RT-19	.28	1	FLOW TIME	166. 6.83	336. 6.83
HYDROGRAPH AT						
+	SC3-10	.11	1	FLOW TIME	61. 6.67	138. 6.67
2 COMBINED AT						
+	DP55	.39	1	FLOW TIME	211. 6.75	448. 6.75
ROUTED TO						
+	RT-20	.39	1	FLOW TIME	209. 6.92	445. 6.83
HYDROGRAPH AT						

+	SC3-1	.13	1	FLOW TIME	92. 6.67	187. 6.67
2 COMBINED AT						
+	DP56	.52	1	FLOW TIME	274. 6.83	586. 6.83
ROUTED TO						
+	DB95	.52	1	FLOW TIME	13. 11.42	41. 11.00
** PEAK STAGES IN FEET **						
			1	STAGE TIME	105.02 11.42	109.39 11.00
ROUTED TO						
+	RT-21	.52	1	FLOW TIME	13. 11.42	41. 11.00
3 COMBINED AT						
+	DP53	6.13	1	FLOW TIME	1500. 7.00	4163. 6.83
ROUTED TO						
+	DB94	6.13	1	FLOW TIME	440. 10.75	1908. 7.25
** PEAK STAGES IN FEET **						
			1	STAGE TIME	107.28 10.75	109.63 7.25
ROUTED TO						
+	RT-27	6.13	1	FLOW TIME	438. 10.75	1895. 7.25
HYDROGRAPH AT						
+	SC2-24	.07	1	FLOW TIME	105. 6.58	172. 6.50
ROUTED TO						
+	RT-29	.07	1	FLOW TIME	105. 6.58	172. 6.58
HYDROGRAPH AT						
+	SC2-27	.08	1	FLOW TIME	128. 6.58	207. 6.58
3 COMBINED AT						
+	DP50	6.27	1	FLOW TIME	442. 10.75	1922. 7.25
ROUTED TO						

+	RT-31	6.27	1	FLOW TIME	440. 10.83	1920. 7.33
HYDROGRAPH AT						
+	SC3-8	.24	1	FLOW TIME	66. 6.67	192. 6.67
ROUTED TO						
+	RT-25	.24	1	FLOW TIME	63. 6.75	190. 6.75
HYDROGRAPH AT						
+	SC3-3	.13	1	FLOW TIME	155. 6.67	270. 6.58
2 COMBINED AT						
+	DP51	.37	1	FLOW TIME	198. 6.67	437. 6.67
ROUTED TO						
+	DB93	.37	1	FLOW TIME	25. 10.50	77. 7.17
** PEAK STAGES IN FEET **						
1	STAGE				104.11	105.42
	TIME				10.50	7.17
ROUTED TO						
+	RT-30	.37	1	FLOW TIME	25. 10.58	77. 7.25
2 COMBINED AT						
+	DP49	6.64	1	FLOW TIME	464. 10.83	1996. 7.33
ROUTED TO						
+	RT-32	6.64	1	FLOW TIME	463. 10.92	1988. 7.33
HYDROGRAPH AT						
+	SC3-6	.24	1	FLOW TIME	83. 6.67	219. 6.67
ROUTED TO						
+	RT-23	.24	1	FLOW TIME	82. 6.75	213. 6.75
HYDROGRAPH AT						
+	SC3-5	.12	1	FLOW TIME	142. 6.67	247. 6.58
ROUTED TO						

+	RT-24	.12	1	FLOW TIME	142. 6.67	243. 6.67
HYDROGRAPH AT						
+	SC3-7	.54	1	FLOW TIME	290. 6.83	589. 6.83
ROUTED TO						
+	RT-22	.54	1	FLOW TIME	290. 6.92	585. 6.92
HYDROGRAPH AT						
+	SC3-4	.32	1	FLOW TIME	252. 6.67	496. 6.67
4 COMBINED AT						
+	DP54	1.22	1	FLOW TIME	681. 6.75	1393. 6.75
ROUTED TO						
+	DB92	1.22	1	FLOW TIME	54. 10.92	89. 10.92
** PEAK STAGES IN FEET **						
1	STAGE				104.72	107.66
	TIME				10.92	10.92
ROUTED TO						
+	RT-26	1.22	1	FLOW TIME	54. 11.08	89. 11.08
HYDROGRAPH AT						
+	SC2-26	.19	1	FLOW TIME	124. 6.67	259. 6.67
ROUTED TO						
+	RT-34	.19	1	FLOW TIME	121. 6.75	249. 6.67
HYDROGRAPH AT						
+	SC2-25	.44	1	FLOW TIME	235. 6.83	497. 6.75
3 COMBINED AT						
+	DP47	1.85	1	FLOW TIME	355. 6.75	749. 6.75
ROUTED TO						
+	RT-33	1.85	1	FLOW TIME	352. 6.75	745. 6.75
HYDROGRAPH AT						

+	SC2-28	.27	1	FLOW TIME	48. 6.92	142. 6.92
HYDROGRAPH AT						
+	SC2-21	.19	1	FLOW TIME	113. 6.75	236. 6.75
4 COMBINED AT						
+	DP48	8.95	1	FLOW TIME	728. 6.75	2353. 7.33
ROUTED TO						
+	RT-37	8.95	1	FLOW TIME	715. 6.75	2350. 7.33
HYDROGRAPH AT						
+	SC2-23	.20	1	FLOW TIME	128. 6.67	267. 6.67
ROUTED TO						
+	RT-35	.20	1	FLOW TIME	125. 6.67	264. 6.67
HYDROGRAPH AT						
+	SC2-22	.08	1	FLOW TIME	67. 6.58	137. 6.58
2 COMBINED AT						
+	DP46	.28	1	FLOW TIME	187. 6.67	386. 6.67
ROUTED TO						
+	RT-36	.28	1	FLOW TIME	183. 6.67	383. 6.67
2 COMBINED AT						
+	DP42	9.23	1	FLOW TIME	889. 6.75	2407. 7.33
ROUTED TO						
+	RT-38	9.23	1	FLOW TIME	863. 6.83	2399. 7.33
HYDROGRAPH AT						
+	SC2-20	.32	1	FLOW TIME	193. 6.75	401. 6.75
ROUTED TO						
+	RT-41	.32	1	FLOW TIME	192. 6.75	400. 6.75
HYDROGRAPH AT						

+	SC2-15	.18	1	FLOW TIME	141. 6.58	291. 6.58
ROUTED TO						
+	RT-39	.18	1	FLOW TIME	138. 6.58	288. 6.58
HYDROGRAPH AT						
+	SC2-13	.16	1	FLOW TIME	118. 6.67	241. 6.67
ROUTED TO						
+	RT-40	.16	1	FLOW TIME	118. 6.67	241. 6.67
4 COMBINED AT						
+	DP85	9.90	1	FLOW TIME	1265. 6.75	2615. 6.75
ROUTED TO						
+	RT-42	9.90	1	FLOW TIME	1235. 6.83	2586. 6.75
HYDROGRAPH AT						
+	SC2-14	.26	1	FLOW TIME	114. 6.83	254. 6.83
HYDROGRAPH AT						
+	SC2-16	.21	1	FLOW TIME	120. 6.67	265. 6.67
3 COMBINED AT						
+	DP37	10.37	1	FLOW TIME	1442. 6.83	3082. 6.75
ROUTED TO						
+	RT-43	10.37	1	FLOW TIME	1438. 6.83	3007. 6.83
HYDROGRAPH AT						
+	SC2-12	.18	1	FLOW TIME	71. 6.67	179. 6.67
ROUTED TO						
+	RT-46	.18	1	FLOW TIME	69. 6.75	171. 6.67
HYDROGRAPH AT						
+	SC2-8	.35	1	FLOW TIME	184. 6.83	390. 6.75

2 COMBINED AT

+	DP33	.53	1	FLOW TIME	252. 6.75	559. 6.75
ROUTED TO						
+	RT-45	.53	1	FLOW TIME	247. 6.83	553. 6.75
2 COMBINED AT						
+	DP36	10.90	1	FLOW TIME	1685. 6.83	3537. 6.83
ROUTED TO						
+	RT-44	10.90	1	FLOW TIME	1647. 6.83	3536. 6.83
HYDROGRAPH AT						
+	SC2-11	.21	1	FLOW TIME	163. 6.58	338. 6.58
ROUTED TO						
+	RT-52	.21	1	FLOW TIME	160. 6.67	335. 6.67
HYDROGRAPH AT						
+	SC2-19	.18	1	FLOW TIME	191. 6.67	345. 6.67
ROUTED TO						
+	RT-51	.18	1	FLOW TIME	189. 6.67	343. 6.67
HYDROGRAPH AT						
+	SC2-18	.20	1	FLOW TIME	234. 6.67	405. 6.67
3 COMBINED AT						
+	DP39	.59	1	FLOW TIME	582. 6.67	1083. 6.67
ROUTED TO						
+	RT-49	.59	1	FLOW TIME	570. 6.67	1073. 6.67
HYDROGRAPH AT						
+	SC2-17	.16	1	FLOW TIME	107. 6.67	222. 6.67
ROUTED TO						
+	RT-50	.16	1	FLOW TIME	105. 6.75	214. 6.67
HYDROGRAPH AT						

+	SC2-29	.15	1	FLOW TIME	208. 6.58	358. 6.58
3 COMBINED AT						
+	DP34	.90	1	FLOW TIME	846. 6.67	1587. 6.67
ROUTED TO						
+	RT-48	.90	1	FLOW TIME	827. 6.67	1574. 6.67
HYDROGRAPH AT						
+	SC2-10	.33	1	FLOW TIME	185. 6.75	391. 6.75
2 COMBINED AT						
+	DP32	1.23	1	FLOW TIME	999. 6.67	1949. 6.67
ROUTED TO						
+	RT-47	1.23	1	FLOW TIME	980. 6.67	1930. 6.67
HYDROGRAPH AT						
+	SC2-9	.21	1	FLOW TIME	61. 6.92	150. 6.83
3 COMBINED AT						
+	DP35	12.34	1	FLOW TIME	2561. 6.83	5419. 6.75
ROUTED TO						
+	RT-53	12.34	1	FLOW TIME	2545. 6.83	5359. 6.75
HYDROGRAPH AT						
+	SC2-7	.14	1	FLOW TIME	96. 6.67	200. 6.67
2 COMBINED AT						
+	DP29	12.48	1	FLOW TIME	2621. 6.83	5544. 6.75
ROUTED TO						
+	RT-54	12.48	1	FLOW TIME	2582. 6.83	5445. 6.83
HYDROGRAPH AT						
+	SC2-6	.15	1	FLOW TIME	137. 6.58	269. 6.58

ROUTED TO

+	RT-55	.15	1	FLOW TIME	134. 6.67	258. 6.58
HYDROGRAPH AT						
+	SC2-2	.17	1	FLOW TIME	135. 6.67	266. 6.67
3 COMBINED AT						
+	DP28	12.80	1	FLOW TIME	2754. 6.83	5874. 6.75
ROUTED TO						
+	RT-56	12.80	1	FLOW TIME	2717. 6.83	5789. 6.75
HYDROGRAPH AT						
+	SC2-5	.16	1	FLOW TIME	235. 6.58	386. 6.58
ROUTED TO						
+	RT-58	.16	1	FLOW TIME	231. 6.67	379. 6.58
HYDROGRAPH AT						
+	SC2-0	.15	1	FLOW TIME	67. 6.67	160. 6.67
ROUTED TO						
+	RT-57	.15	1	FLOW TIME	66. 6.75	156. 6.67
3 COMBINED AT						
+	DP21	13.11	1	FLOW TIME	2928. 6.83	6263. 6.75
ROUTED TO						
+	RT-59	13.11	1	FLOW TIME	2896. 6.83	6190. 6.75
HYDROGRAPH AT						
+	SC2-4	.28	1	FLOW TIME	393. 6.67	641. 6.67
ROUTED TO						
+	RT-61	.28	1	FLOW TIME	388. 6.67	637. 6.67
HYDROGRAPH AT						
+	SC2-3	.07	1	FLOW TIME	117. 6.58	188. 6.58
2 COMBINED AT						

+ DP22 .36 1 FLOW 485. 793.
TIME 6.67 6.67

ROUTED TO

+ RT-60 .36 1 FLOW 483. 793.
TIME 6.67 6.67

HYDROGRAPH AT

+ SC2-1 .19 1 FLOW 140. 282.
TIME 6.67 6.67

3 COMBINED AT

+ DP20 13.65 1 FLOW 3359. 7151.
TIME 6.83 6.75

IN SC#2

ROUTED TO

+ DBSC2 13.65 1 FLOW 1153. 4357.
TIME 7.42 7.08

OUT SC#2

** PEAK STAGES IN FEET **

1 STAGE 6506.59 6510.16
TIME 7.42 7.08

ROUTED TO

+ RT-63 13.65 1 FLOW 1150. 4348.
TIME 7.42 7.08

HYDROGRAPH AT

+ SC1-10 .29 1 FLOW 160. 339.
TIME 6.75 6.75

2 COMBINED AT

+ DP18 13.94 1 FLOW 1185. 4499.
TIME 7.42 7.08

ROUTED TO

+ RT-66 13.94 1 FLOW 1183. 4493.
TIME 7.42 7.08

HYDROGRAPH AT

+ SC1-13 .13 1 FLOW 152. 271.
TIME 6.58 6.58

ROUTED TO

+ RT-64 .13 1 FLOW 151. 267.
TIME 6.67 6.58

HYDROGRAPH AT

+ SC1-14 .11 1 FLOW 160. 263.
TIME 6.58 6.58

2 COMBINED AT

+	DP19	.24	1	FLOW TIME	308. 6.58	530. 6.58
ROUTED TO						
+	RT-65	.24	1	FLOW TIME	308. 6.67	527. 6.58
2 COMBINED AT						
+	DP17	14.18	1	FLOW TIME	1219. 7.42	4589. 7.08
ROUTED TO						
+	RT-67	14.18	1	FLOW TIME	1213. 7.50	4575. 7.08
HYDROGRAPH AT						
+	SC1-9	.24	1	FLOW TIME	162. 6.58	352. 6.58
ROUTED TO						
+	DBSR29	.24	1	FLOW TIME	51. 6.92	103. 6.92
** PEAK STAGES IN FEET **						
1	STAGE	103.85	105.90			
	TIME	6.92	6.92			
ROUTED TO						
+	RT-70	.24	1	FLOW TIME	51. 7.00	103. 7.00
HYDROGRAPH AT						
+	SC1-8	.21	1	FLOW TIME	145. 6.67	298. 6.67
ROUTED TO						
+	RT-71	.21	1	FLOW TIME	144. 6.67	297. 6.67
2 COMBINED AT						
+	DP14	.45	1	FLOW TIME	183. 6.75	365. 6.67
ROUTED TO						
+	RT-69	.45	1	FLOW TIME	182. 6.75	363. 6.75
HYDROGRAPH AT						
+	SC1-7	.10	1	FLOW TIME	83. 6.58	166. 6.58
2 COMBINED AT						

Preliminary Design
Hydrologic

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1*****
*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
CORPS OF ENGINEERS *
* JUN 1998 *
ENGINEERING CENTER *
* VERSION 4.1 *
SECOND STREET *
*
CALIFORNIA 95616 *
* RUN DATE 06AUG04 TIME 13:04:30 *
756-1104 *
*
*****
*****

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SC# 2 w/
DBPS + SR# 15
+ SR# 24

U.S. ARMY
HYDROLOGIC
609
DAVIS,
(916)

PER SC# 2
PRELIMINARY DESIGN

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,

DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

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PAGE 1

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Sand Creek Detention Basin No. 2 Preliminary design									
2	ID	Future development condition with Sand Creek No. 1									
3	ID	DETENTION BASIN MODELED PER DBPS WITH SPRINGS RANCH DETENTION									
4	ID	BASINS 15 AND 29									
5	ID	10-year and 100 Year, 24 hr Type IIA Storm fn: SC1-FD.dat									
	*DIAGRAM										
6	IT	5	0	0	300						
7	IO	5	0								
8	JR	PREC	.68	1.0							
9	KK	SB82									
10	KM	RUNOFF FROM SUB-BASIN 82									
11	BA	.190									
12	IN	30									
13	PB	4.4									
14	PC	0.0000	0.0025	0.0050	0.0075	0.0100	0.0150	0.0200	0.0250	0.0300	0.0500
15	PC	0.0600	0.1000	0.1000	0.7000	0.7000	0.7500	0.7800	0.7980	0.8200	0.8300
16	PC	0.9150	0.9210	0.9270	0.9330	0.9400	0.9450	0.9500	0.9525	0.9550	0.9600
17	PC	0.9650	0.9700	0.9750	0.9800	0.9830	0.9850	0.9880	0.9900	0.9930	0.9950

123 KK DP70
 124 KM DESIGN POINT 70 COMBINE RUNOFF FROM SUB-BASIN 70 AND RT-9
 125 HC 2

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

126 KK RT-11
 127 KM ROUTE FLOW FROM DESIGN POINT 70 TO DP 87
 128 RK 1200 .025 .035 TRAP 20 4
 129 KK SB72
 130 KM RUNOFF FROM SUB-BASIN 72
 131 BA .2500
 132 LS 0 70
 133 UD .138
 134 KK RT-10
 135 KM ROUTE FLOW FROM SUB-BASIN 72 TO DESIGN POINT 70
 136 RK 3000 .035 .035 TRAP 10 4
 137 KK SB69
 138 KM RUNOFF FROM SUB-BASIN 69
 139 BA .2500
 140 LS 0 75
 141 UD .483
 142 KK DP69
 143 KM DESIGN POINT 69 COMBINE RUNOFF FROM SUB-BASIN 69 AND RT-10
 144 HC 2
 145 KK RT-12
 146 KM ROUTE FLOW FROM DESIGN POINT 69 TO DP 87
 147 RK 1400 .025 .035 TRAP 20 4
 148 KK SC3-15
 149 KM RUNOFF FROM SUB-BASIN SC3-15
 150 BA .0730
 151 LS 0 72.0
 152 UD .230
 153 KK DP87
 154 KM DESIGN POINT 87 COMBINE RUNOFF FROM SUB-BASIN SC3-15, RT-12 AND RT-11
 155 HC 3
 156 KK RT-13
 157 KM ROUTE FLOW FROM DESIGN POINT 87 TO DP 63
 158 RK 4400 .025 .035 TRAP 40 4
 159 KK SC3-12
 160 KM RUNOFF FROM SUB-BASIN SC3-12
 161 BA .2380
 162 LS 0 70.6
 163 UD .410
 164 KK DP63
 165 KM DESIGN POINT 63 COMBINE RUNOFF FROM SUB-BASIN SC3-12 AND RT-13
 166 HC 2

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

167 KK RT-17
 168 KM ROUTE FLOW FROM DESIGN POINT 63 TO DP 60
 169 RK 4390 .015 .035 TRAP 10 4
 170 KK SB68

171	KM	RUNOFF FROM SUB-BASIN 68							
172	BA	.2200							
173	LS	0	75						
174	UD	.390							
175	KK	RT-14							
176	KM	ROUTE FLOW FROM SUB-BASIN 68 TO DESIGN POINT 67							
177	RK	3300	.020	.035	TRAP	20		4	
178	KK	SC3-14							
179	KM	RUNOFF FROM SUB-BASIN SC3-14							
180	BA	.3150							
181	LS	0	70.0						
182	UD	.410							
183	KK	DP67							
184	KM	DESIGN POINT 67 COMBINE RUNOFF FROM SUB-BASIN SC3-14 AND RT-14							
185	HC	2							
186	KK	RT-15							
187	KM	ROUTE FLOW FROM DP 67 TO DESIGN POINT 64							
188	RK	2500	.035	.035	TRAP	20		6	
189	KK	SC3-13							
190	KM	RUNOFF FROM SUB-BASIN SC3-13							
191	BA	.2550							
192	LS	0	81.4						
193	UD	.390							
194	KK	DP64							
195	KM	DESIGN POINT 64 COMBINE RUNOFF FROM SUB-BASIN SC3-13 AND RT-15							
196	HC	2							
197	KK	RT-16							
198	KM	ROUTE FLOW FROM DP 64 TO DESIGN POINT 60							
199	RK	4010	.018	.035	TRAP	20		10	
200	KK	SC3-9							
201	KM	RUNOFF FROM SUB-BASIN SC3-9							
202	BA	.2360							
203	LS	0	75.5						
204	UD	.320							
205	KK	DP60							
206	KM	DESIGN POINT 60 COMBINE RUNOFF FROM SUB-BASIN SC3-9, RT-17 AND RT-16							
207	HC	3							

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

208	KK	RT-18							
209	KM	ROUTE FLOW FROM DP 60 TO DESIGN POINT 53							
210	RK	2690	.015	.035	TRAP	10		4	
211	KK	SC3-2							
212	KM	RUNOFF FROM SUB-BASIN SC3-2							
213	BA	.1920							
214	LS	0	76.1						
215	UD	.310							
216	KK	SC3-11							
217	KM	RUNOFF FROM SUB-BASIN SC3-11							
218	BA	.2750							
219	LS	0	80.8						
220	UD	.430							
221	KK	RT-19							
222	KM	ROUTE FLOW FROM SUB-BASIN SC3-11 TO DESIGN POINT 55							
223	RK	2600	.022	.035	TRAP	10		6	

224 KK SC3-10
 225 KM RUNOFF FROM SUB-BASIN SC3-10
 226 BA .1130
 227 LS 0 75.0
 228 UD .290

 229 KK DP55
 230 KM DESIGN POINT 55 COMBINE RUNOFF FROM SUB-BASIN SC3-10 AND RT-19
 231 HC 2

 232 KK RT-20
 233 KM ROUTE FLOW FROM DESIGN POINT 55 TO DESIGN POINT 56
 234 RK 3670 .012 .035 TRAP 10 10

 235 KK SC3-1
 236 KM RUNOFF FROM SUB-BASIN SC3-1
 237 BA .1290
 238 LS 0 80.0
 239 UD .320

 240 KK DP56
 241 KM DESIGN POINT 56 COMBINE RUNOFF FROM SUB-BASIN SC3-1 AND RT-20
 242 HC 2

 243 KK DB95
 244 KM ROUTE FLOW FROM DP 56 THROUGH DBPS STRUCTURE 95
 245 RS 1 ELEV 100
 246 SV 0 20 34 39 42 50
 247 SQ 0 10 20 30 40 43
 248 SE 100 104 107 108 109 110
 HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

249 KK RT-21
 250 KM ROUTE FLOW FROM DETENTION BASIN DB95 TO DESIGN POINT 53
 251 RK 870 .023 .035 TRAP 10 10

 252 KK DP53
 253 KM DESIGN POINT 53 COMBINE RUNOFF FROM SUB-BASIN SC3-2, RT-21 AND RT-18
 254 HC 3

 255 KK DB94
 256 KM ROUTE FLOW FROM DP 53 THROUGH DBPS STRUCTURE 94
 257 RS 1 ELEV 100
 258 SV 0 30 40 60 80 120 130
 259 SQ 0 80 90 300 800 1750 2000
 260 SE 100 104 105 107 108 109 110

 261 KK RT-27
 262 KM ROUTE FLOW FROM DETETNION BASIN DB94 TO DESIGN POINT 50
 263 RK 1380 .022 .035 TRAP 10 10

 264 KK SC2-24
 265 KM RUNOFF FROM SUB-BASIN SC2-24
 266 BA .0660
 267 LS 0 90.7
 268 UD .190

 269 KK RT-29
 270 KM ROUTE FLOW FROM SUB-BASIN SC2-24 TO DESIGN POINT 50
 271 RK 330 .020 .013 CIRC 4

 272 KK SC2-27
 273 KM RUNOFF FROM SUB-BASIN SC2-27
 274 BA .0780
 275 LS 0 92.0
 276 UD .210

277	KK	DP50							
278	KM	DESIGN POINT 50 COMBINE RUNOFF FROM SC2-27, RT-27 AND RT-29							
279	HC	3							
280	KK	RT-31							
281	KM	ROUTE FLOW FROM DESIGN POINT 50 TO DESIGN POINT 49							
282	RK	940	.020	.035	TRAP	40		6	
283	KK	SC3-8							
284	KM	RUNOFF FROM SUB-BASIN SC3-8							
285	BA	.2420							
286	LS	0	67.0						
287	UD	.290							

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HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

288	KK	RT-25							
289	KM	ROUTE FLOW FROM SUB-BASIN SC3-8 TO DESIGN POINT 51							
290	RK	2690	.015	.035	TRAP	20		6	
291	KK	SC3-3							
292	KM	RUNOFF FROM SUB-BASIN SC3-3							
293	BA	.1270							
294	LS	0	88.5						
295	UD	.280							
296	KK	DP51							
297	KM	DESIGN POINT 51 COMBINE RUNOFF FROM SC3-3 AND RT-25							
298	HC	2							
299	KK	DB93							
300	KM	ROUTE FLOW FROM DP 51 THROUGH DBPS STRUCTURE 93							
301	RS	1	ELEV	100					
302	SV	0	10	20	30				
303	SQ	0	20	100	290				
304	SE	100	104	106	108				
305	KK	RT-30							
306	KM	ROUTE FLOW FROM DETENTION BASIN DB93 TO DESIGN POINT 49							
307	RK	1590	.019	.035	TRAP	20		10	
308	KK	DP49							
309	KM	DESIGN POINT 49 COMBINE RUNOFF FROM RT-31 AND RT-30							
310	HC	2							
311	KK	RT-32							
312	KM	ROUTE FLOW FROM DESIGN POINT 49 TO DESIGN POINT 48							
313	RK	4930	.017	.035	TRAP	40		6	
314	KK	SC3-6							
315	KM	RUNOFF FROM SUB-BASIN SC3-6							
316	BA	.2420							
317	LS	0	70.0						
318	UD	.310							
319	KK	RT-23							
320	KM	ROUTE FLOW FROM SUB-BASIN SC3-6 TO DESIGN POINT 54							
321	RK	1830	.023	.035	TRAP	10		10	
322	KK	SC3-5							
323	KM	RUNOFF FROM SUB-BASIN SC3-5							
324	BA	.1150							
325	LS	0	88.7						
326	UD	.280							

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HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
327	KK RT-24
328	KM ROUTE FLOW FROM SUB-BASIN SC3-5 TO DESIGN POINT 54
329	RK 1760 .041 .035 TRAP 10 4
330	KK SC3-7
331	KM RUNOFF FROM SUB-BASIN SC3-7
332	BA .5380
333	LS 0 81.5
334	UD .530
335	KK RT-22
336	KM ROUTE FLOW FROM SUB-BASIN SC3-7 TO DESIGN POINT 54
337	RK 2860 .021 .035 TRAP 30 6
338	KK SC3-4
339	KM RUNOFF FROM SUB-BASIN SC3-4
340	BA .3210
341	LS 0 81.9
342	UD .330
343	KK DP54
344	KM DESIGN POINT 54 COMBINE RUNOFF FROM SUB-BASIN SC3-4, RT-22, RT-23 AND RT-24
345	HC 4
346	KK DB92
347	KM ROUTE FLOW FROM DP 54 THROUGH DBPS STRUCTURE 92
348	RS 1 ELEV 100
349	SV 0 40 60 90 110 120 130
350	SQ 0 40 60 82 90 91 95
351	SE 100 104 105 106 108 109 110
352	KK RT-26
353	KM ROUTE FLOW FROM DETENTION BASIN 92 TO DESIGN POINT 47
354	RK 4680 .028 .035 TRAP 40 10
355	KK SC2-26
356	KM RUNOFF FROM SUB-BASIN SC2-26
357	BA .1860
358	LS 0 74.4
359	UD .320
360	KK RT-34
361	KM ROUTE FLOW FROM SUB-BASIN SC2-26 TO DESIGN POINT 47
362	RK 1620 .014 .035 TRAP 4 4
363	KK SC2-25
364	KM RUNOFF FROM SUB-BASIN SC2-25
365	BA .4440
366	LS 0 73.6
367	UD .440

HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
368	KK DP47
369	KM DESIGN POINT 47 COMBINE RUNOFF FROM SUB-BASIN SC3-26 AND RT-26 AND RT-34
370	HC 3
371	KK RT-33
372	KM ROUTE FLOW FROM DESIGN POINT 47 TO DESIGN POINT 48
373	RK 510 .020 .035 TRAP 6 4
374	KK SC2-28
375	KM RUNOFF FROM SUB-BASIN SC2-28
376	BA .2740
377	LS 0 67.1

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378      UD      .510
379      KK      SC2-21
380      KM      RUNOFF FROM SUB-BASIN SC2-21
381      BA      .1930
382      LS      0      79.0
383      UD      .390
384      KK      DP48
385      KM      DESIGN POINT 48 COMBINE RUNOFF FROM SB'S SC2-21 AND SC2-28, RT-33 AND RT-32
386      HC      4
387      KK      RT-37
388      KM      ROUTE FLOW FROM DESIGN POINT 48 TO DESIGN POINT 42
389      RK      1080      .022      .035      TRAP      60      6
390      KK      SC2-23
391      KM      RUNOFF FROM SUB-BASIN SC2-23
392      BA      .1960
393      LS      0      70.0
394      UD      .330
395      KK      RT-35
396      KM      ROUTE FLOW FROM SUB-BASIN SC2-23 TO DESIGN POINT 46
397      RK      1180      .015      .013      CIRC      5
398      KK      SC2-22
399      KM      RUNOFF FROM SUB-BASIN SC2-22
400      BA      .0810
401      LS      0      79.0
402      UD      .220
403      KK      DP46
404      KM      DESIGN POINT 46 COMBINE RUNOFF FROM SUB-BASIN SC2-22 AND RT-35
405      HC      2
406      KK      RT-36
407      KM      ROUTE FLOW FROM DESIGN POINT 46 TO DESIGN POINT 42
408      RK      1790      .017      .013      TRAP      30      4
                                     HEC-1 INPUT

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
409      KK      DP42
410      KM      DESIGN POINT 42 COMBINE RUNOFF FROM RT- 37 AND RT-36
411      HC      2
412      KK      RT-38
413      KM      ROUTE FLOW FROM DESIGN POINT 42 TO DESIGN POINT 85
414      RK      1790      .013      .035      TRAP      60      6
415      KK      SC2-20
416      KM      RUNOFF FROM SUB-BASIN SC2-20
417      BA      .3240
418      LS      0      79.0
419      UD      .380
420      KK      RT-41
421      KM      ROUTE FLOW FROM SUB-BASIN SC2-20 TO DESIGN POINT 85
422      RK      1560      .020      .013      CIRC      6.5
423      KK      SC2-15
424      KM      RUNOFF FROM SUB-BASIN SC2-15
425      BA      .1810
426      LS      0      79.0
427      UD      .240
428      KK      RT-39
429      KM      ROUTE FLOW FROM SUB-BASIN SC2-15 TO DESIGN POINT 85

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430	RK	990	.022	.013	CIRC	6.5		
431	KK	SC2-13						
432	KM	RUNOFF FROM SUB-BASIN SC2-13						
433	BA	.1630						
434	LS	0	79.0					
435	UD	.280						
436	KK	RT-40						
437	KM	ROUTE FLOW FROM SUB-BASIN SC2-13 TO DESIGN POINT 85						
438	RK	300	.033	.013	CIRC	5.5		
439	KK	DP85						
440	KM	DESIGN POINT 85 COMBINE RUNOFF FROM RT-41, RT-38, RT-39 AND RT-40						
441	HC	4						
442	KK	RT-42						
443	KM	ROUTE FLOW FROM DESIGN POINT 85 TO DESIGN POINT 37						
444	RK	2900	.016	.035	TRAP	60	6	
445	KK	SC2-14						
446	KM	RUNOFF FROM SUB-BASIN SC2-14						
447	BA	.2650						
448	LS	0	76.2					
449	UD	.450						

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

450	KK	SC2-16						
451	KM	RUNOFF FROM SUB-BASIN SC2-16						
452	BA	.2120						
453	LS	0	76.2					
454	UD	.310						
455	KK	DP37						
456	KM	DESIGN POINT 37 COMBINE RUNOFF FROM RT-42, SUB-BASINS SC2-14 AND SC2-16						
457	HC	3						
458	KK	RT-43						
459	KM	ROUTE FLOW FROM DESIGN POINT 37 TO DESIGN POINT 36						
460	RK	3380	.016	.035	TRAP	60	4	
461	KK	SC2-12						
462	KM	RUNOFF FROM SUB-BASIN SC2-12						
463	BA	.1790						
464	LS	0	70.6					
465	UD	.280						
466	KK	RT-46						
467	KM	ROUTE FLOW FROM SUB-BASIN SC2-12 TO DESIGN POINT 33						
468	RK	2280	.024	.035	TRAP	20	4	
469	KK	SC2-8						
470	KM	RUNOFF FROM SUB-BASIN SC2-8						
471	BA	.3480						
472	LS	0	79.0					
473	UD	.440						
474	KK	DP33						
475	KM	DESIGN POINT 33 COMBINE RUNOFF FROM RT-46 AND SUB-BASIN SC2-8						
476	HC	2						
477	KK	RT-45						
478	KM	ROUTE FLOW FROM DESIGN POINT 33 TO DESIGN POINT 36						
479	RK	850	.021	.035	TRAP	20	4	
480	KK	DP36						
481	KM	DESIGN POINT 36 COMBINE RUNOFF FROM RT-45 AND RT-43						

482 HC 2
 483 KK RT-44
 484 KM ROUTE FLOW FROM DESIGN POINT 36 TO DESIGN POINT 35
 485 RK 1740 .016 .035 TRAP 60 4
 486 KK SC2-11
 487 KM RUNOFF FROM SUB-BASIN SC2-11
 488 BA .2100
 489 LS 0 79.0
 490 UD .240

HEC-1 INPUT

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

491 KK RT-52
 492 KM ROUTE FLOW FROM SUB-BASIN SC2-11 TO DESIGN POINT 39
 493 RK 2680 .008 .035 TRAP 30 4
 494 KK SC2-19
 495 KM RUNOFF FROM SUB-BASIN SC2-19
 496 BA .1820
 497 LS 0 86.0
 498 UD .300
 499 KK RT-51
 500 KM ROUTE FLOW FROM SUB-BASIN SC2-19 TO DESIGN POINT 39
 501 RK 3100 .022 .013 CIRC 6
 502 KK SC2-18
 503 KM RUNOFF FROM SUB-BASIN SC2-18
 504 BA .1980
 505 LS 0 88.0
 506 UD .290
 507 KK DP39
 508 KM DESIGN POINT 39 COMBINE RUNOFF FROM RT-52, RT-51 AND SUB-BASIN SC2-18
 509 HC 3
 510 KK RT-49
 511 KM ROUTE FLOW FROM DESIGN POINT 39 TO DESIGN POINT 34
 512 RK 1160 .024 .035 TRAP 20 4
 513 KK SC2-17
 514 KM RUNOFF FROM SUB-BASIN SC2-17
 515 BA .1600
 516 LS 0 79.0
 517 UD .320
 518 KK RT-50
 519 KM ROUTE FLOW FROM SUB-BASIN SC2-17 TO DESIGN POINT 34
 520 RK 1840 .025 .035 TRAP 20 4
 521 KK SC2-29
 522 KM RUNOFF FROM SUB-BASIN SC2-29
 523 BA .1510
 524 LS 0 88.0
 525 UD .210
 526 KK DP34
 527 KM DESIGN POINT 34 COMBINE RUNOFF FROM RT-49, RT-50 AND SUB-BASIN SC2-29
 528 HC 3
 529 KK RT-48
 530 KM ROUTE FLOW FROM DESIGN POINT 34 TO DESIGN POINT 32
 531 RK 1470 .020 .04 TRAP 20 4

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634	KM	ROUTE FLOW FROM DP 20 THROUGH SAND CREEK DETENTION BASIN DBSC2									
635	RS	1	ELEV	6496.5							
636	SV	0	6.75	29.8	58.8	92.2	129.4	168.6	209.8	255.0	302.2
637	SQ	0	10	30	100	400	800	2000	4300	5000	7000
638	SE	6496.5	6498	6500	6502	6504	6506	6508	6510	6512	6514

639	KK	RT-63									
640	KM	ROUTE FLOW FROM DETENTION BASIN SC2 TO DESIGN POINT 18									
641	RK	1290	.007	.035		TRAP	100				3

642	KK	SC1-10									
643	KM	RUNOFF FROM SUB-BASIN SC1-10									
644	BA	.2880									
645	LS	0	78.2								
646	UD	.390									

647	KK	DP18									
648	KM	DESIGN POINT 18 COMBINE RUNOFF FROM RT-63 AND SUB-BASIN SC1-10									
649	HC	2									

650	KK	RT-66									
651	KM	ROUTE FLOW FROM DESIGN POINT 18 TO DESIGN POINT 17									
652	RK	320	.007	.035		TRAP	100				3
						HEC-1 INPUT					

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LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

653	KK	SC1-13									
654	KM	RUNOFF FROM SUB-BASIN SC1-13									
655	BA	.1300									
656	LS	0	87.3								
657	UD	.270									

658	KK	RT-64									
659	KM	ROUTE FLOW FROM SUB-BASIN SC1-13 TO DESIGN POINT 19									
660	RK	1970	.026	.013		CIRC	4.5				

661	KK	SC1-14									
662	KM	RUNOFF FROM SUB-BASIN SC1-14									
663	BA	.1100									
664	LS	0	92.0								
665	UD	.280									

666	KK	DP19									
667	KM	DESIGN POINT 19 COMBINE RUNOFF FROM RT-64 AND SUB-BASIN SC1-14									
668	HC	2									

669	KK	RT-65									
670	KM	ROUTE FLOW FROM DESIGN POINT 19 TO DESIGN POINT 17									
671	RK	700	.022	.013		TRAP	10				.1

672	KK	DP17									
673	KM	DESIGN POINT 17 COMBINE RUNOFF FROM RT-65 AND RT-66									
674	HC	2									

675	KK	RT-67									
676	KM	ROUTE FLOW FROM DESIGN POINT 17 TO DESIGN POINT 13									
677	RK	910	.007	.035		TRAP	100				3

678	KK	SC1-9									
679	KM	RUNOFF FROM SUB-BASIN SC1-9									
680	BA	.2400									
681	LS	0	76.9								
682	UD	.240									

683	KK	DBSR29									
684	KM	ROUTE FLOW FROM SB SC1-9 THROUGH SPRINGS RANCH DETENTION BASIN DBSR29									
685	RS	1	ELEV	100							
686	SV	0	.03	0.47	1.20	2.60	6.70	8.8	10.7		

687	SQ	0	.2	7.8	22	40	73	97	113
688	SE	100	100.5	101.5	102.5	103.5	104.5	105.5	106.5
689	KK	RT-70							
690	KM	ROUTE FLOW FROM SPRINGS RANCH DETENTION BASIN DBSR29 TO DESIGN POINT 14							
691	RD	2750	.013	.013		CIRC	5.5		
						HEC-1 INPUT			

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LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
692	KK SC1-8
693	KM RUNOFF FROM SUB-BASIN SC1-8
694	BA .2100
695	LS 0 79.5
696	UD .320
697	KK RT-71
698	KM ROUTE FLOW FROM SUB-BASIN SC1-8 TO DESIGN POINT 14
699	RK 600 .010 .013 CIRC 5
700	KK DP14
701	KM DESIGN POINT 14 COMBINE RUNOFF FROM RT-71 AND RT-70
702	HC 2
703	KK RT-69
704	KM ROUTE FLOW FROM DESIGN POINT 14 TO DESIGN POINT 12
705	RK 1550 .015 .013 TRAP 7 .1
706	KK SC1-7
707	KM RUNOFF FROM SUB-BASIN SC1-7
708	BA .0960
709	LS 0 80.1
710	UD .230
711	KK DP12
712	KM DESIGN POINT 12 COMBINE RUNOFF FROM SUB-BASIN SC1-7 AND RT-69
713	HC 2
714	KK RT-68
715	KM ROUTE FLOW FROM DESIGN POINT 12 TO DESIGN POINT 13
716	RK 390 .025 .013 TRAP 7 .1
717	KK DP13
718	KM DESIGN POINT 13 COMBINE RUNOFF FROM RT-67 AND RT-68
719	HC 2
720	KK RT-72
721	KM ROUTE FLOW FROM DESIGN POINT 13 TO DESIGN POINT 10
722	RK 1150 .007 .04 TRAP 100 3
723	KK SC1-6
724	KM RUNOFF FROM SUB-BASIN SC1-6
725	BA .1590
726	LS 0 80.6
727	UD .220
728	KK DBSR15
729	KM ROUTE FLOW FROM SB SC1-6 THROUGH SPRINGS RANCH DETENTION BASIN DBSR15
730	RS 1 ELEV 100
731	SV 0 1.50 3.80 6.50 9.70 13.80
732	SQ 0 80 125 150 180 210
733	SE 100 102 104 106 108 110
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LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
734	KK RT-74

787 KK SC1-4
 788 KM RUNOFF FROM SUB-BASIN SC1-4
 789 BA .1010
 790 LS 0 92.0
 791 UD .270

 792 KK RT-80
 793 KM ROUTE FLOW FROM SUB-BASIN SC1-4 TO DESIGN POINT 4
 794 RK 720 .014 .013 CIRC 5

 795 KK SC1-1
 796 KM RUNOFF FROM SUB-BASIN SC1-1
 797 BA .1090
 798 LS 0 79.0
 799 UD .280

 800 KK RT-79
 801 KM ROUTE FLOW FROM SUB-BASIN SC1-1 TO DESIGN POINT 4
 802 RK 1115 .027 .013 CIRC 3

 803 KK SC1-3
 804 KM RUNOFF FROM SUB-BASIN SC1-3
 805 BA .3530
 806 LS 0 65.0
 807 UD .460

 808 KK DP4
 809 KM DESIGN POINT 4 COMBINE RUNOFF FROM SB SC1-3, RT-80, RT-77, RT-78 AND RT-79
 810 HC 5

 811 KK DBSC1
 812 KM ROUTE FLOW FROM DP 4 THROUGH SAND CREEK DETENTION BASIN DBSC1
 813 KO 0
 814 RS 1 ELEV 6393
 815 SV 0 3.06 14.95 31.12 53.59 69.95 86.31 106.5 126.7 150.4
 816 SV 174.1 200.4 226.7 254.9 283.2 313.4 343.6 375.2 406.8 439.5
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LINE	ID	1	2	3	4	5	6	7	8	9	10
817	SQ	0	3	5	189	954	1465	2055	2431	2755	3045
818	SQ	3310	3555	3785	4037	5518	7863	10797	14211	18042	22245
819	SE	6393	6394	6396	6398	6400	6401	6402	6403	6404	6405
820	SE	6406	6407	6408	6409	6410	6411	6412	6413	6414	6415
821	ZZ										

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OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	RATIO 2
				.68	1.00
HYDROGRAPH AT					
+	SB82	.19	1 FLOW	84.	232.
			TIME	6.50	6.50
ROUTED TO					
+	RT-4	.19	1 FLOW	83.	220.
			TIME	6.58	6.58
HYDROGRAPH AT					
+	SB74	.18	1 FLOW	49.	143.
			TIME	6.67	6.67
2 COMBINED AT					
+	DP74	.37	1 FLOW	122.	345.
			TIME	6.58	6.58

ROUTED TO +	RT-3	.37	1	FLOW TIME	119. 6.67	331. 6.67
HYDROGRAPH AT +	SB80	.22	1	FLOW TIME	68. 6.67	190. 6.67
ROUTED TO +	RT-2	.22	1	FLOW TIME	67. 6.75	185. 6.75
HYDROGRAPH AT +	SB81	.39	1	FLOW TIME	100. 6.67	298. 6.67
ROUTED TO +	RT-1	.39	1	FLOW TIME	99. 6.83	296. 6.75
HYDROGRAPH AT +	SB73	.12	1	FLOW TIME	31. 6.75	90. 6.67
HYDROGRAPH AT +	SB75	.12	1	FLOW TIME	25. 6.83	72. 6.75
HYDROGRAPH AT +	SB76	.17	1	FLOW TIME	35. 6.83	105. 6.75
6 COMBINED AT +	DP75	1.39	1	FLOW TIME	364. 6.75	1022. 6.75
ROUTED TO +	RT-5	1.39	1	FLOW TIME	345. 6.75	1021. 6.75
HYDROGRAPH AT +	SB79	.29	1	FLOW TIME	111. 6.58	306. 6.58
ROUTED TO +	RT-7	.29	1	FLOW TIME	107. 6.67	281. 6.58
HYDROGRAPH AT +	SB78	.88	1	FLOW TIME	259. 6.67	738. 6.67
2 COMBINED AT +	DP78	1.17	1	FLOW TIME	367. 6.67	1020. 6.67
ROUTED TO +	RT-6	1.17	1	FLOW TIME	351. 6.75	993. 6.67
HYDROGRAPH AT +	SB77	.25	1	FLOW TIME	61. 6.75	177. 6.75
HYDROGRAPH AT +	SB88	.10	1	FLOW TIME	34. 6.58	97. 6.58
4 COMBINED AT +	DP77	2.91	1	FLOW TIME	780. 6.75	2207. 6.67

ROUTED TO						
+	RT-8	2.91	1	FLOW TIME	761. 6.83	2201. 6.75
HYDROGRAPH AT						
+	SB71	.36	1	FLOW TIME	127. 6.67	334. 6.67
2 COMBINED AT						
+	DP71	3.27	1	FLOW TIME	861. 6.83	2508. 6.75
ROUTED TO						
+	RT-9	3.27	1	FLOW TIME	842. 6.83	2445. 6.75
HYDROGRAPH AT						
+	SB70	.31	1	FLOW TIME	99. 6.75	256. 6.75
2 COMBINED AT						
+	DP70	3.58	1	FLOW TIME	931. 6.83	2701. 6.75
ROUTED TO						
+	RT-11	3.58	1	FLOW TIME	905. 6.83	2653. 6.75
HYDROGRAPH AT						
+	SB72	.25	1	FLOW TIME	135. 6.58	321. 6.50
ROUTED TO						
+	RT-10	.25	1	FLOW TIME	128. 6.58	321. 6.58
HYDROGRAPH AT						
+	SB69	.25	1	FLOW TIME	93. 6.83	216. 6.83
2 COMBINED AT						
+	DP69	.50	1	FLOW TIME	191. 6.67	453. 6.58
ROUTED TO						
+	RT-12	.50	1	FLOW TIME	187. 6.67	446. 6.67
HYDROGRAPH AT						
+	SC3-15	.07	1	FLOW TIME	35. 6.58	87. 6.58
3 COMBINED AT						
+	DP87	4.15	1	FLOW TIME	1068. 6.83	3104. 6.75
ROUTED TO						
+	RT-13	4.15	1	FLOW TIME	1048. 6.92	3020. 6.83
HYDROGRAPH AT						
+	SC3-12	.24	1	FLOW TIME	70. 6.83	183. 6.75
2 COMBINED AT						
+	DP63	4.39	1	FLOW TIME	1111. 6.92	3198. 6.83
ROUTED TO						
+	RT-17	4.39	1	FLOW	1083.	3152.

				TIME	7.00	6.83
HYDROGRAPH AT						
+	SB68	.22	1	FLOW	97.	222.
				TIME	6.75	6.75
ROUTED TO						
+	RT-14	.22	1	FLOW	95.	220.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC3-14	.31	1	FLOW	88.	234.
				TIME	6.83	6.75
2 COMBINED AT						
+	DP67	.53	1	FLOW	183.	448.
				TIME	6.83	6.83
ROUTED TO						
+	RT-15	.53	1	FLOW	178.	446.
				TIME	6.92	6.83
HYDROGRAPH AT						
+	SC3-13	.25	1	FLOW	174.	346.
				TIME	6.75	6.75
2 COMBINED AT						
+	DP64	.79	1	FLOW	338.	763.
				TIME	6.83	6.75
ROUTED TO						
+	RT-16	.79	1	FLOW	332.	758.
				TIME	6.92	6.83
HYDROGRAPH AT						
+	SC3-9	.24	1	FLOW	123.	279.
				TIME	6.67	6.67
3 COMBINED AT						
+	DP60	5.42	1	FLOW	1462.	4126.
				TIME	6.92	6.83
ROUTED TO						
+	RT-18	5.42	1	FLOW	1456.	4013.
				TIME	7.00	6.92
HYDROGRAPH AT						
+	SC3-2	.19	1	FLOW	108.	239.
				TIME	6.67	6.67
HYDROGRAPH AT						
+	SC3-11	.28	1	FLOW	167.	341.
				TIME	6.75	6.75
ROUTED TO						
+	RT-19	.28	1	FLOW	166.	336.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC3-10	.11	1	FLOW	61.	138.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP55	.39	1	FLOW	211.	448.
				TIME	6.75	6.75
ROUTED TO						
+	RT-20	.39	1	FLOW	209.	445.
				TIME	6.92	6.83
HYDROGRAPH AT						

+	SC3-1	.13	1	FLOW TIME	92. 6.67	187. 6.67
2 COMBINED AT						
+	DP56	.52	1	FLOW TIME	274. 6.83	586. 6.83
ROUTED TO						
+	DB95	.52	1	FLOW TIME	13. 11.42	41. 11.00
** PEAK STAGES IN FEET **						
	1	STAGE			105.02	109.39
		TIME			11.42	11.00
ROUTED TO						
+	RT-21	.52	1	FLOW TIME	13. 11.42	41. 11.00
3 COMBINED AT						
+	DP53	6.13	1	FLOW TIME	1500. 7.00	4163. 6.83
ROUTED TO						
+	DB94	6.13	1	FLOW TIME	440. 10.75	1908. 7.25
** PEAK STAGES IN FEET **						
	1	STAGE			107.28	109.63
		TIME			10.75	7.25
ROUTED TO						
+	RT-27	6.13	1	FLOW TIME	438. 10.75	1895. 7.25
HYDROGRAPH AT						
+	SC2-24	.07	1	FLOW TIME	105. 6.58	172. 6.50
ROUTED TO						
+	RT-29	.07	1	FLOW TIME	105. 6.58	172. 6.58
HYDROGRAPH AT						
+	SC2-27	.08	1	FLOW TIME	128. 6.58	207. 6.58
3 COMBINED AT						
+	DP50	6.27	1	FLOW TIME	442. 10.75	1922. 7.25
ROUTED TO						
+	RT-31	6.27	1	FLOW TIME	440. 10.83	1920. 7.33
HYDROGRAPH AT						
+	SC3-8	.24	1	FLOW TIME	66. 6.67	192. 6.67
ROUTED TO						
+	RT-25	.24	1	FLOW TIME	63. 6.75	190. 6.75
HYDROGRAPH AT						
+	SC3-3	.13	1	FLOW TIME	155. 6.67	270. 6.58
2 COMBINED AT						
+	DP51	.37	1	FLOW TIME	198. 6.67	437. 6.67

ROUTED TO						
+	DB93	.37	1	FLOW	25.	77.
				TIME	10.50	7.17
				** PEAK STAGES IN FEET **		
			1	STAGE	104.11	105.42
				TIME	10.50	7.17
ROUTED TO						
+	RT-30	.37	1	FLOW	25.	77.
				TIME	10.58	7.25
2 COMBINED AT						
+	DP49	6.64	1	FLOW	464.	1996.
				TIME	10.83	7.33
ROUTED TO						
+	RT-32	6.64	1	FLOW	463.	1988.
				TIME	10.92	7.33
HYDROGRAPH AT						
+	SC3-6	.24	1	FLOW	83.	219.
				TIME	6.67	6.67
ROUTED TO						
+	RT-23	.24	1	FLOW	82.	213.
				TIME	6.75	6.75
HYDROGRAPH AT						
+	SC3-5	.12	1	FLOW	142.	247.
				TIME	6.67	6.58
ROUTED TO						
+	RT-24	.12	1	FLOW	142.	243.
				TIME	6.67	6.67
HYDROGRAPH AT						
+	SC3-7	.54	1	FLOW	290.	589.
				TIME	6.83	6.83
ROUTED TO						
+	RT-22	.54	1	FLOW	290.	585.
				TIME	6.92	6.92
HYDROGRAPH AT						
+	SC3-4	.32	1	FLOW	252.	496.
				TIME	6.67	6.67
4 COMBINED AT						
+	DP54	1.22	1	FLOW	681.	1393.
				TIME	6.75	6.75
ROUTED TO						
+	DB92	1.22	1	FLOW	54.	89.
				TIME	10.92	10.92
				** PEAK STAGES IN FEET **		
			1	STAGE	104.72	107.66
				TIME	10.92	10.92
ROUTED TO						
+	RT-26	1.22	1	FLOW	54.	89.
				TIME	11.08	11.08
HYDROGRAPH AT						
+	SC2-26	.19	1	FLOW	89.	208.
				TIME	6.67	6.67
ROUTED TO						
+	RT-34	.19	1	FLOW	88.	202.
				TIME	6.75	6.75

HYDROGRAPH AT						
+	SC2-25	.44	1	FLOW TIME	159. 6.83	378. 6.83
3 COMBINED AT						
+	DP47	1.85	1	FLOW TIME	244. 6.75	584. 6.75
ROUTED TO						
+	RT-33	1.85	1	FLOW TIME	242. 6.83	579. 6.75
HYDROGRAPH AT						
+	SC2-28	.27	1	FLOW TIME	48. 6.92	142. 6.92
HYDROGRAPH AT						
+	SC2-21	.19	1	FLOW TIME	113. 6.75	236. 6.75
4 COMBINED AT						
+	DP48	8.95	1	FLOW TIME	616. 6.75	2318. 7.33
ROUTED TO						
+	RT-37	8.95	1	FLOW TIME	604. 6.75	2314. 7.33
HYDROGRAPH AT						
+	SC2-23	.20	1	FLOW TIME	65. 6.75	169. 6.67
ROUTED TO						
+	RT-35	.20	1	FLOW TIME	64. 6.75	166. 6.75
HYDROGRAPH AT						
+	SC2-22	.08	1	FLOW TIME	67. 6.58	137. 6.58
2 COMBINED AT						
+	DP46	.28	1	FLOW TIME	122. 6.67	287. 6.67
ROUTED TO						
+	RT-36	.28	1	FLOW TIME	119. 6.67	284. 6.67
2 COMBINED AT						
+	DP42	9.23	1	FLOW TIME	717. 6.75	2358. 7.33
ROUTED TO						
+	RT-38	9.23	1	FLOW TIME	700. 6.83	2346. 7.33
HYDROGRAPH AT						
+	SC2-20	.32	1	FLOW TIME	193. 6.75	401. 6.75
ROUTED TO						
+	RT-41	.32	1	FLOW TIME	192. 6.75	400. 6.75
HYDROGRAPH AT						
+	SC2-15	.18	1	FLOW TIME	141. 6.58	291. 6.58
ROUTED TO						
+	RT-39	.18	1	FLOW	138.	288.

				TIME	6.58	6.58
HYDROGRAPH AT						
+	SC2-13	.16	1	FLOW	118.	241.
				TIME	6.67	6.67
ROUTED TO						
+	RT-40	.16	1	FLOW	118.	241.
				TIME	6.67	6.67
4 COMBINED AT						
+	DP85	9.90	1	FLOW	1092.	2491.
				TIME	6.75	7.33
ROUTED TO						
+	RT-42	9.90	1	FLOW	1068.	2486.
				TIME	6.83	7.42
HYDROGRAPH AT						
+	SC2-14	.26	1	FLOW	114.	254.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC2-16	.21	1	FLOW	120.	265.
				TIME	6.67	6.67
3 COMBINED AT						
+	DP37	10.37	1	FLOW	1275.	2818.
				TIME	6.83	6.75
ROUTED TO						
+	RT-43	10.37	1	FLOW	1268.	2756.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC2-12	.18	1	FLOW	71.	179.
				TIME	6.67	6.67
ROUTED TO						
+	RT-46	.18	1	FLOW	69.	171.
				TIME	6.75	6.67
HYDROGRAPH AT						
+	SC2-8	.35	1	FLOW	184.	390.
				TIME	6.83	6.75
2 COMBINED AT						
+	DP33	.53	1	FLOW	252.	559.
				TIME	6.75	6.75
ROUTED TO						
+	RT-45	.53	1	FLOW	247.	553.
				TIME	6.83	6.75
2 COMBINED AT						
+	DP36	10.90	1	FLOW	1515.	3286.
				TIME	6.83	6.83
ROUTED TO						
+	RT-44	10.90	1	FLOW	1475.	3282.
				TIME	6.83	6.83
HYDROGRAPH AT						
+	SC2-11	.21	1	FLOW	163.	338.
				TIME	6.58	6.58
ROUTED TO						
+	RT-52	.21	1	FLOW	160.	335.
				TIME	6.67	6.67
HYDROGRAPH AT						

+	SC2-19	.18	1	FLOW TIME	191. 6.67	345. 6.67
ROUTED TO						
+	RT-51	.18	1	FLOW TIME	189. 6.67	343. 6.67
HYDROGRAPH AT						
+	SC2-18	.20	1	FLOW TIME	234. 6.67	405. 6.67
3 COMBINED AT						
+	DP39	.59	1	FLOW TIME	582. 6.67	1083. 6.67
ROUTED TO						
+	RT-49	.59	1	FLOW TIME	570. 6.67	1073. 6.67
HYDROGRAPH AT						
+	SC2-17	.16	1	FLOW TIME	107. 6.67	222. 6.67
ROUTED TO						
+	RT-50	.16	1	FLOW TIME	105. 6.75	214. 6.67
HYDROGRAPH AT						
+	SC2-29	.15	1	FLOW TIME	208. 6.58	358. 6.58
3 COMBINED AT						
+	DP34	.90	1	FLOW TIME	846. 6.67	1587. 6.67
ROUTED TO						
+	RT-48	.90	1	FLOW TIME	827. 6.67	1574. 6.67
HYDROGRAPH AT						
+	SC2-10	.33	1	FLOW TIME	185. 6.75	391. 6.75
2 COMBINED AT						
+	DP32	1.23	1	FLOW TIME	999. 6.67	1949. 6.67
ROUTED TO						
+	RT-47	1.23	1	FLOW TIME	980. 6.67	1930. 6.67
HYDROGRAPH AT						
+	SC2-9	.21	1	FLOW TIME	61. 6.92	150. 6.83
3 COMBINED AT						
+	DP35	12.34	1	FLOW TIME	2389. 6.83	5152. 6.75
ROUTED TO						
+	RT-53	12.34	1	FLOW TIME	2372. 6.83	5091. 6.75
HYDROGRAPH AT						
+	SC2-7	.14	1	FLOW TIME	96. 6.67	200. 6.67
2 COMBINED AT						
+	DP29	12.48	1	FLOW TIME	2447. 6.83	5277. 6.75

ROUTED TO						
+	RT-54	12.48	1	FLOW TIME	2408. 6.83	5188. 6.83
HYDROGRAPH AT						
+	SC2-6	.15	1	FLOW TIME	137. 6.58	269. 6.58
ROUTED TO						
+	RT-55	.15	1	FLOW TIME	134. 6.67	258. 6.58
HYDROGRAPH AT						
+	SC2-2	.17	1	FLOW TIME	135. 6.67	266. 6.67
3 COMBINED AT						
+	DP28	12.80	1	FLOW TIME	2579. 6.83	5605. 6.75
ROUTED TO						
+	RT-56	12.80	1	FLOW TIME	2544. 6.83	5522. 6.75
HYDROGRAPH AT						
+	SC2-5	.16	1	FLOW TIME	235. 6.58	386. 6.58
ROUTED TO						
+	RT-58	.16	1	FLOW TIME	231. 6.67	379. 6.58
HYDROGRAPH AT						
+	SC2-0	.15	1	FLOW TIME	67. 6.67	160. 6.67
ROUTED TO						
+	RT-57	.15	1	FLOW TIME	66. 6.75	156. 6.67
3 COMBINED AT						
+	DP21	13.11	1	FLOW TIME	2754. 6.83	5996. 6.75
ROUTED TO						
+	RT-59	13.11	1	FLOW TIME	2724. 6.83	5924. 6.75
HYDROGRAPH AT						
+	SC2-4	.28	1	FLOW TIME	393. 6.67	641. 6.67
ROUTED TO						
+	RT-61	.28	1	FLOW TIME	388. 6.67	637. 6.67
HYDROGRAPH AT						
+	SC2-3	.07	1	FLOW TIME	117. 6.58	188. 6.58
2 COMBINED AT						
+	DP22	.36	1	FLOW TIME	485. 6.67	793. 6.67
ROUTED TO						
+	RT-60	.36	1	FLOW TIME	483. 6.67	793. 6.67
HYDROGRAPH AT						
+	SC2-1	.19	1	FLOW TIME	140. 6.67	282. 6.67

3 COMBINED AT	DP20	13.65	1	FLOW TIME	3187. 6.83	6885. 6.75	<i>IN SCH 2</i>
ROUTED TO							
+ DBSC2	13.65	1	FLOW TIME	1038. 7.42	4257. 7.08		<i>OUT SCH 2</i>
** PEAK STAGES IN FEET **							
			1	STAGE TIME	6506.40 7.42	6509.96 7.08	
ROUTED TO							
+ RT-63	13.65	1	FLOW TIME	1032. 7.50	4225. 7.08		
HYDROGRAPH AT							
+ SC1-10	.29	1	FLOW TIME	160. 6.75	339. 6.75		
2 COMBINED AT							
+ DP18	13.94	1	FLOW TIME	1066. 7.42	4376. 7.08		
ROUTED TO							
+ RT-66	13.94	1	FLOW TIME	1065. 7.50	4366. 7.08		
HYDROGRAPH AT							
+ SC1-13	.13	1	FLOW TIME	152. 6.58	271. 6.58		
ROUTED TO							
+ RT-64	.13	1	FLOW TIME	151. 6.67	267. 6.58		
HYDROGRAPH AT							
+ SC1-14	.11	1	FLOW TIME	160. 6.58	263. 6.58		
2 COMBINED AT							
+ DP19	.24	1	FLOW TIME	308. 6.58	530. 6.58		
ROUTED TO							
+ RT-65	.24	1	FLOW TIME	308. 6.67	527. 6.58		
2 COMBINED AT							
+ DP17	14.18	1	FLOW TIME	1102. 7.50	4462. 7.08		
ROUTED TO							
+ RT-67	14.18	1	FLOW TIME	1102. 7.50	4434. 7.08		
HYDROGRAPH AT							
+ SC1-9	.24	1	FLOW TIME	162. 6.58	352. 6.58		
ROUTED TO							
+ DBSR29	.24	1	FLOW TIME	51. 6.92	103. 6.92		
** PEAK STAGES IN FEET **							
			1	STAGE TIME	103.85 6.92	105.90 6.92	
ROUTED TO							
+ RT-70	.24	1	FLOW	51.	103.		

				TIME	7.00	7.00
HYDROGRAPH AT						
+	SC1-8	.21	1	FLOW	145.	298.
				TIME	6.67	6.67
ROUTED TO						
+	RT-71	.21	1	FLOW	144.	297.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP14	.45	1	FLOW	183.	365.
				TIME	6.75	6.67
ROUTED TO						
+	RT-69	.45	1	FLOW	182.	363.
				TIME	6.75	6.75
HYDROGRAPH AT						
+	SC1-7	.10	1	FLOW	83.	166.
				TIME	6.58	6.58
2 COMBINED AT						
+	DP12	.55	1	FLOW	253.	511.
				TIME	6.67	6.67
ROUTED TO						
+	RT-68	.55	1	FLOW	252.	509.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP13	14.72	1	FLOW	1180.	4641.
				TIME	7.50	7.08
ROUTED TO						
+	RT-72	14.72	1	FLOW	1175.	4599.
				TIME	7.50	7.08
HYDROGRAPH AT						
+	SC1-6	.16	1	FLOW	145.	286.
				TIME	6.58	6.58
ROUTED TO						
+	DBSR15	.16	1	FLOW	90.	138.
				TIME	6.75	6.83
				** PEAK STAGES IN FEET **		
			1	STAGE	102.45	105.01
				TIME	6.75	6.83
ROUTED TO						
+	RT-74	.16	1	FLOW	89.	137.
				TIME	6.75	6.83
HYDROGRAPH AT						
+	SC1-5	.17	1	FLOW	118.	245.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP11	.33	1	FLOW	203.	374.
				TIME	6.67	6.67
ROUTED TO						
+	RT-73	.33	1	FLOW	202.	373.
				TIME	6.67	6.67
2 COMBINED AT						
+	DP10	15.06	1	FLOW	1217.	4784.
				TIME	7.50	7.08
ROUTED TO						

+	RT-75	15.06	1	FLOW TIME	1215. 7.58	4743. 7.17
	HYDROGRAPH AT					
+	SC1-2	.14	1	FLOW TIME	91. 6.67	189. 6.67
	ROUTED TO					
+	RT-76	.14	1	FLOW TIME	90. 6.67	188. 6.67
	2 COMBINED AT					
+	DP9	15.19	1	FLOW TIME	1231. 7.58	4781. 7.17
	ROUTED TO					
+	RT-78	15.19	1	FLOW TIME	1226. 7.58	4770. 7.17
	HYDROGRAPH AT					
+	SC1-12	.25	1	FLOW TIME	203. 6.75	386. 6.67
	ROUTED TO					
+	RT-81	.25	1	FLOW TIME	202. 6.75	383. 6.75
	HYDROGRAPH AT					
+	SC1-11	.16	1	FLOW TIME	228. 6.58	376. 6.58
	2 COMBINED AT					
+	DP8	.41	1	FLOW TIME	411. 6.67	735. 6.67
	ROUTED TO					
+	RT-77	.41	1	FLOW TIME	401. 6.75	730. 6.75
	HYDROGRAPH AT					
+	SC1-4	.10	1	FLOW TIME	150. 6.58	247. 6.58
	ROUTED TO					
+	RT-80	.10	1	FLOW TIME	149. 6.58	245. 6.58
	HYDROGRAPH AT					
+	SC1-1	.11	1	FLOW TIME	79. 6.67	161. 6.67
	ROUTED TO					
+	RT-79	.11	1	FLOW TIME	78. 6.67	161. 6.67
	HYDROGRAPH AT					
+	SC1-3	.35	1	FLOW TIME	53. 6.83	175. 6.83
	5 COMBINED AT					
+	DP4	16.16	1	FLOW TIME	1553. 6.75	5205. 7.17
	ROUTED TO					
+	DBSC1	16.16	1	FLOW TIME	1240. 8.00	3544. 8.08

** PEAK STAGES IN FEET **

1	STAGE	6400.56	6406.96
	TIME	8.00	8.08

JR ENGINEERING'S SAND CREEK DATA

CROSS SECTION

RIVER: SAND CREEK
 REACH: ONE RS: 300

INPUT

Description: UPSTREAM OF PROPERTY

Station Elevation Data		num= 10							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
428	6792	430	6790	436	6788	443	6786	454	6784
500	6784	548	6784	560	6786	600	6788	701	6790

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
428	.04	430	.035	560	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	430	560		230	205		.2	.4

CROSS SECTION

RIVER: SAND CREEK
 REACH: ONE RS: 295

INPUT

Description: NEAR UPSTREAM END OF PROPERTY

Station Elevation Data		num= 14							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
422	6787	445	6786	467	6784	476	6782	482	6780
500	6779.8	518	6780	573	6782	580	6784	586	6784
590	6784	616	6784	646	6786	698	6788		

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
422	.04	467	.035	573	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	467	573		87	86		.2	.4
Left Levee		Station=	455	Elevation=	6786			

CROSS SECTION

RIVER: SAND CREEK
 REACH: ONE RS: 294.5

INPUT

Description: FIS CD-4082 ROTATED

Station Elevation Data		num= 11							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
415	6786.4	425	6784	430	6785	460	6781	475	6780
500	6779	549	6778	606	6777	633	6780	660	6786
682	6787								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
415	.04	500	.035	633	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	500	633		60	68		.2	.4

CROSS SECTION

RIVER: SAND CREEK
 REACH: ONE RS: 290

INPUT

Description:

Station Elevation Data		num= 18							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
403	6785	420	6784	435	6782	446	6781	456	6780
486	6778	500	6778.39	534	6778	542	6780	546	6781.48

Post Project As-builts.rep

FLOW DATA

Flow Title: Floodplain

Flow File : x:\2910000.all\2910404\HEC-RAS\post project with no lower levee geometry\Post Project As-builts.f01

Flow Data (cfs)

River	Reach	RS	PF#1	PF 2	PF 3	PF 4
PF 5						
SAND CREEK	ONE	305	1200	2100	2600	3900
600						
SAND CREEK	ONE	300	1200	2100	2600	3900
600						
SAND CREEK	ONE	294.5	1200	2100	2600	3900
600						
SAND CREEK	ONE	214	1200	2400	3100	4600
600						
SAND CREEK	ONE	179	1200	2600	3150	4800
600						
SAND CREEK	ONE	108	1200	2600	3250	5100
600						
SAND CREEK	ONE	84	1200	2600	3300	5100
600						
SAND CREEK	ONE	75	1250	2700	3450	5350
625						
SAND CREEK	ONE	38	1300	2800	3600	5700
650						

Boundary Conditions

River	Reach	Profile	Upstream	Downstream
SAND CREEK	ONE	PF#1	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 2	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 3	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 4	Normal S = 0.01	Normal S = 0.0065
SAND CREEK	ONE	PF 5	Normal S = 0.01	Normal S = 0.0065

GEOMETRY DATA

Geometry Title: As-builts modified for no lower levee

Geometry File : x:\2910000.all\2910404\HEC-RAS\post project with no lower levee geometry\Post Project As-builts.g01

CROSS SECTION

RIVER: SAND CREEK

REACH: ONE RS: 305

INPUT

Description: FIS CE-4080 Upstream of Property

Station Elevation Data		num= 30		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
1000	6820	1040	6816	1080	6812	1115	6808	1160	6804
1190	6800	1200	6796	1210	6792	1230	6788	1231	6786.4
1242	6786.4	1245	6788	1270	6792	1350	6792	1390	6796
1435	6800	1465	6800.2	1500	6800	1540	6802	1600	6802
1680	6802	1720	6801.6	1760	6802	1790	6802.2	1840	6802
1890	6802	1960	6803	2020	6804	2145	6806	2370	6808

Manning's n Values		num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
1000	.06	1230	.03	1245	.06		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1230	1245		390	395	.1	.3

No ox Bow levee with levee

Reach	River Sta	Profile	Plan	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
ONE	305	PF 3	PPFNL	2600.00	6786.40	6793.98	6793.98	6795.31	0.004839	12.35	472.02	16.71	0.80
ONE	305	PF 3	PPFP	2600.00	6786.40	6793.98	6793.98	6795.31	0.004839	12.35	472.02	16.71	0.80
ONE	300	PF 3	PPFNL	2600.00	6784.00	6786.70	6786.70	6787.90	0.012999	8.80	299.07	13.52	0.99
ONE	300	PF 3	PPFP	2600.00	6784.00	6786.70	6786.70	6787.90	0.012999	8.80	299.07	13.52	0.99
ONE	295	PF 3	PPFNL	2600.00	6779.80	6783.45	6783.45	6784.75	0.012437	9.19	285.15	108.56	0.98
ONE	295	PF 3	PPFP	2600.00	6779.80	6783.45	6783.45	6784.75	0.012437	9.19	285.15	108.56	0.98
ONE	294.5	PF 3	PPFNL	2600.00	6777.00	6783.22	6780.38	6783.37	0.000670	3.27	877.91	218.56	0.25
ONE	294.5	PF 3	PPFP	2600.00	6777.00	6783.22	6780.38	6783.37	0.000670	3.27	877.91	218.56	0.25
ONE	290	PF 3	PPFNL	2600.00	6778.00	6781.70	6781.70	6783.02	0.010553	9.34	291.47	189.44	0.93
ONE	290	PF 3	PPFP	2600.00	6778.00	6781.70	6781.70	6783.02	0.010553	9.34	291.47	189.44	0.93
ONE	280	PF 3	PPFNL	2600.00	6776.80	6780.86	6780.56	6781.81	0.009060	7.85	332.35	199.06	0.84
ONE	280	PF 3	PPFP	2600.00	6776.80	6780.86	6780.56	6781.81	0.009060	7.85	332.35	199.06	0.84
ONE	265	PF 3	PPFNL	2600.00	6774.82	6779.09	6779.09	6780.37	0.012841	9.06	286.92	216.36	0.99
ONE	265	PF 3	PPFP	2600.00	6774.82	6779.09	6779.09	6780.37	0.012841	9.06	286.92	216.36	0.99
ONE	250	PF 3	PPFNL	2600.00	6768.00	6778.02	6773.08	6778.21	0.000522	3.49	779.81	189.51	0.23
ONE	250	PF 3	PPFP	2600.00	6768.00	6778.02	6773.08	6778.21	0.000522	3.49	779.81	189.51	0.23
ONE	240	PF 3	PPFNL	2600.00	6766.00	6778.04	6772.48	6778.16	0.000280	2.84	992.99	155.31	0.18
ONE	240	PF 3	PPFP	2600.00	6766.00	6778.04	6772.48	6778.16	0.000280	2.84	992.99	155.31	0.18
ONE	230	PF 3	PPFNL	2600.00	6764.00	6778.01	6771.26	6778.14	0.000249	2.95	970.71	133.21	0.17
ONE	230	PF 3	PPFP	2600.00	6764.00	6778.01	6771.26	6778.14	0.000249	2.95	970.71	133.21	0.17
ONE	225	PF 3	PPFNL	2600.00	6770.00	6777.77	6773.92	6778.08	0.001382	4.82	616.10	104.52	0.30
ONE	225	PF 3	PPFP	2600.00	6770.00	6777.77	6773.92	6778.08	0.001382	4.82	616.10	104.52	0.30
ONE	222	PF 3	PPFNL	2600.00	6770.10	6777.01	6774.46	6777.89	0.000455	7.52	345.74	50.12	0.50
ONE	222	PF 3	PPFP	2600.00	6770.10	6777.01	6774.46	6777.89	0.000455	7.52	345.74	50.12	0.50
ONE	221.6	PF 3	PPFNL	2600.00	6770.00	6777.01	6774.36	6777.86	0.000434	7.41	351.04	50.12	0.49
ONE	221.6	PF 3	PPFP	2600.00	6770.00	6777.01	6774.36	6777.86	0.000434	7.41	351.04	50.12	0.49
ONE	221.5	PF 3	PPFNL	2600.00	6770.00	6775.26	6775.26	6777.46	0.001953	11.92	218.21	49.98	1.00
ONE	221.5	PF 3	PPFP	2600.00	6770.00	6775.26	6775.26	6777.46	0.001953	11.92	218.21	49.98	1.00
ONE	221	PF 3	PPFNL	2600.00	6770.00	6774.36	6774.36	6776.57	0.001887	11.91	218.25	50.07	1.01
ONE	221	PF 3	PPFP	2600.00	6770.00	6774.36	6774.36	6776.57	0.001887	11.91	218.25	50.07	1.01
ONE	215	PF 3	PPFNL	2600.00	6758.45	6765.19	6762.82	6766.11	0.000491	7.71	337.10	50.11	0.52
ONE	215	PF 3	PPFP	2600.00	6758.45	6765.19	6762.82	6766.11	0.000491	7.71	337.10	50.11	0.52
ONE	214	PF 3	PPFNL	3100.00	6758.40	6763.31	6763.31	6765.78	0.001852	12.62	245.71	50.08	1.00
ONE	214	PF 3	PPFP	3100.00	6758.40	6763.31	6763.31	6765.78	0.001852	12.62	245.71	50.08	1.00
ONE	210	PF 3	PPFNL	3100.00	6749.75	6756.84	6754.66	6758.03	0.000596	8.73	355.04	50.12	0.58
ONE	210	PF 3	PPFP	3100.00	6749.75	6756.84	6754.66	6758.03	0.000596	8.73	355.04	50.12	0.58
ONE	209	PF 3	PPFNL	3100.00	6749.66	6756.83	6754.58	6757.99	0.006893	8.63	359.28	50.16	0.57
ONE	209	PF 3	PPFP	3100.00	6749.66	6756.83	6754.58	6757.99	0.006893	8.63	359.28	50.16	0.57
ONE	208	PF 3	PPFNL	3100.00	6750.00	6756.89	6755.38	6757.60	0.006520	6.81	455.35	110.13	0.59
ONE	208	PF 3	PPFP	3100.00	6750.00	6756.89	6755.38	6757.60	0.006520	6.81	455.35	110.13	0.59
ONE	207	PF 3	PPFNL	3100.00	6750.00	6754.29	6754.29	6756.06	0.016794	11.29	303.75	88.72	0.98
ONE	207	PF 3	PPFP	3100.00	6750.00	6754.29	6754.29	6756.06	0.016794	11.29	303.75	88.72	0.98
ONE	206	PF 3	PPFNL	3100.00	6748.00	6753.52	6752.93	6754.88	0.012785	9.37	331.00	81.89	0.82
ONE	206	PF 3	PPFP	3100.00	6748.00	6753.52	6752.93	6754.88	0.012785	9.37	331.00	81.89	0.82
ONE	205	PF 3	PPFNL	3100.00	6748.00	6753.61	6752.36	6754.62	0.008100	8.06	384.43	84.04	0.66
ONE	205	PF 3	PPFP	3100.00	6748.00	6753.61	6752.36	6754.62	0.008100	8.06	384.43	84.04	0.66
ONE	204	PF 3	PPFNL	3100.00	6747.00	6753.58	6752.12	6754.44	0.007068	7.47	414.95	92.29	0.62
ONE	204	PF 3	PPFP	3100.00	6747.00	6753.58	6752.12	6754.44	0.007068	7.47	414.95	92.29	0.62
ONE	203	PF 3	PPFNL	3100.00	6747.00	6753.37	6752.20	6754.29	0.008460	7.68	403.61	99.26	0.67
ONE	203	PF 3	PPFP	3100.00	6747.00	6753.37	6752.20	6754.29	0.008460	7.68	403.61	99.26	0.67

DRAINAGE MAPS